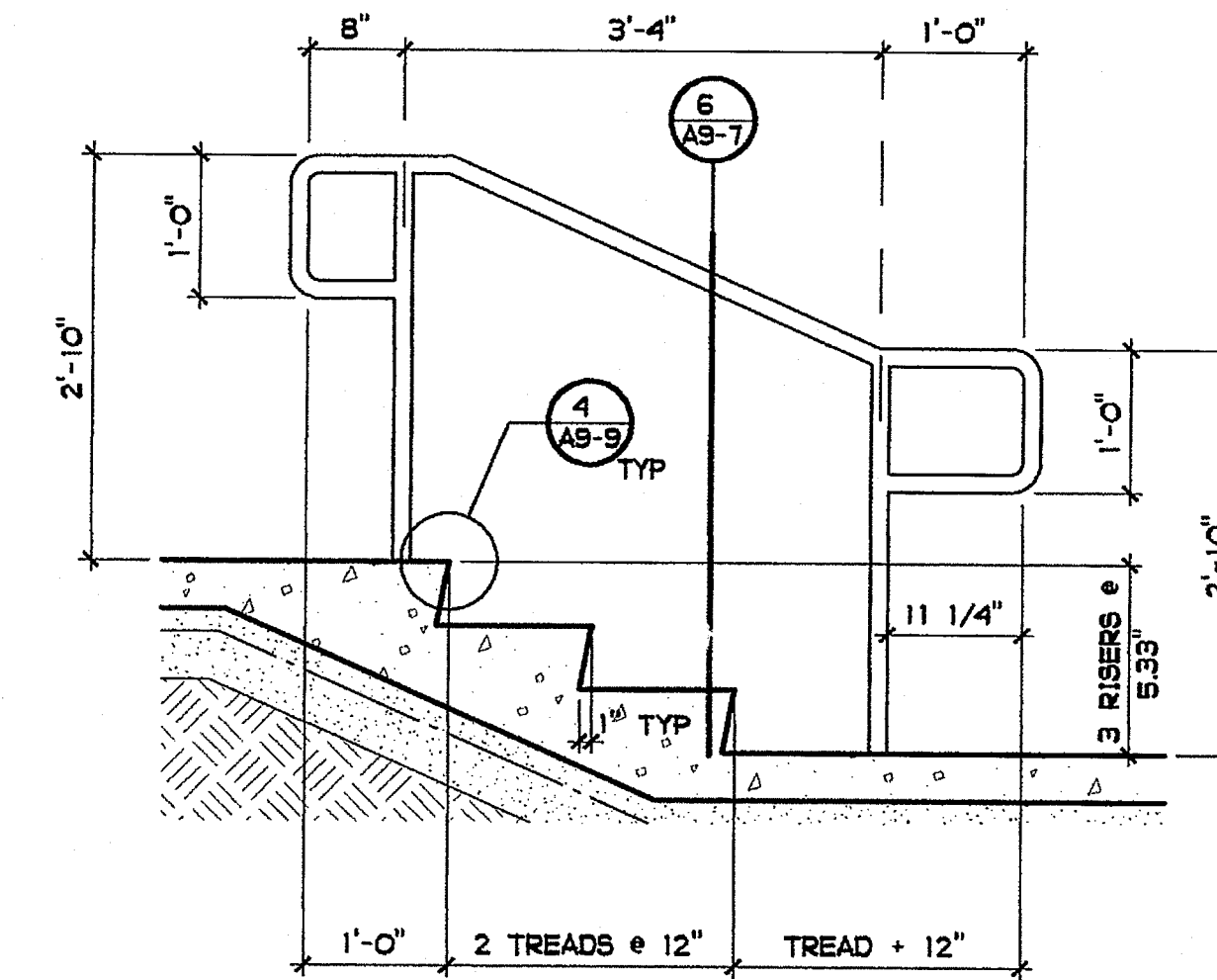
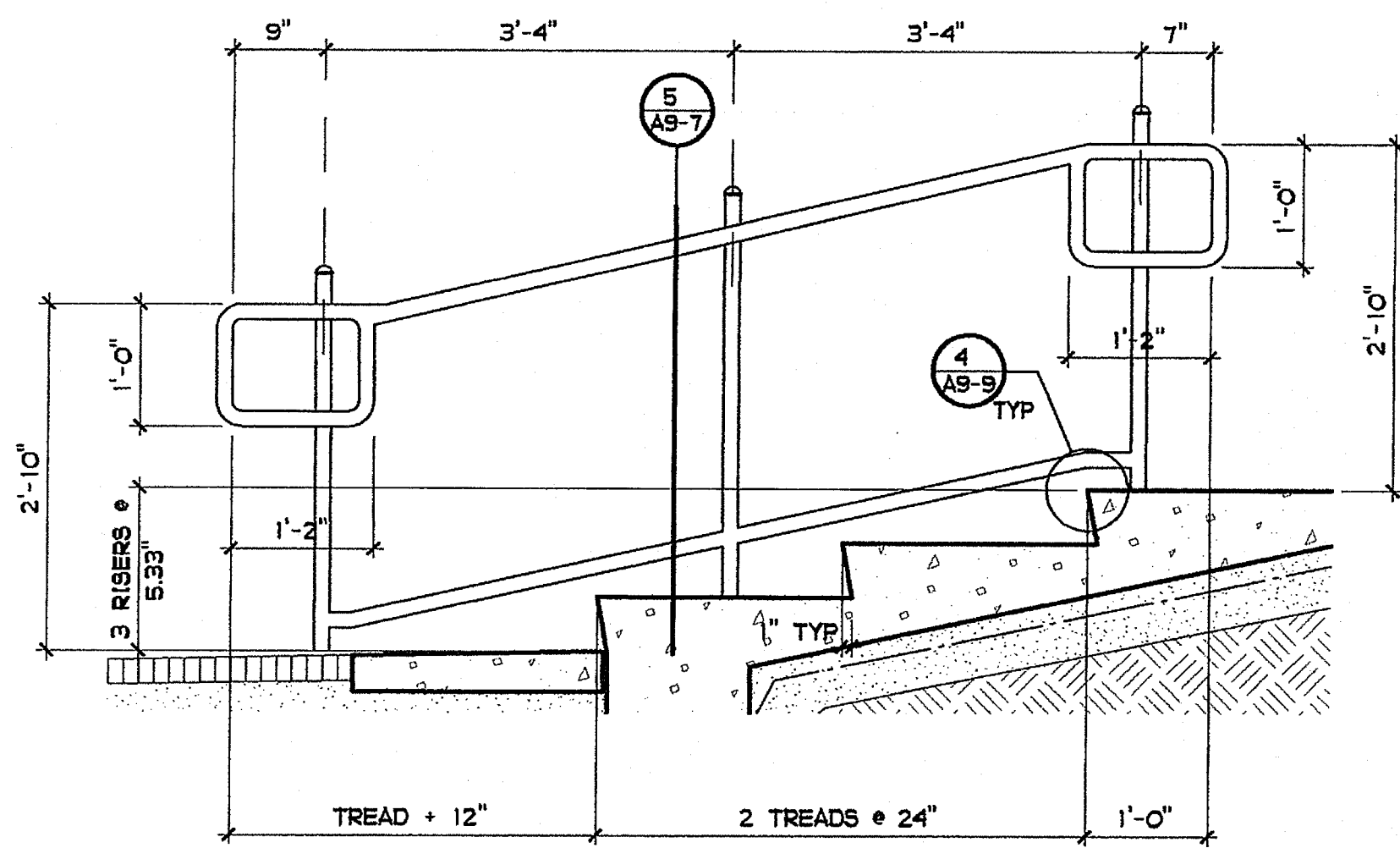


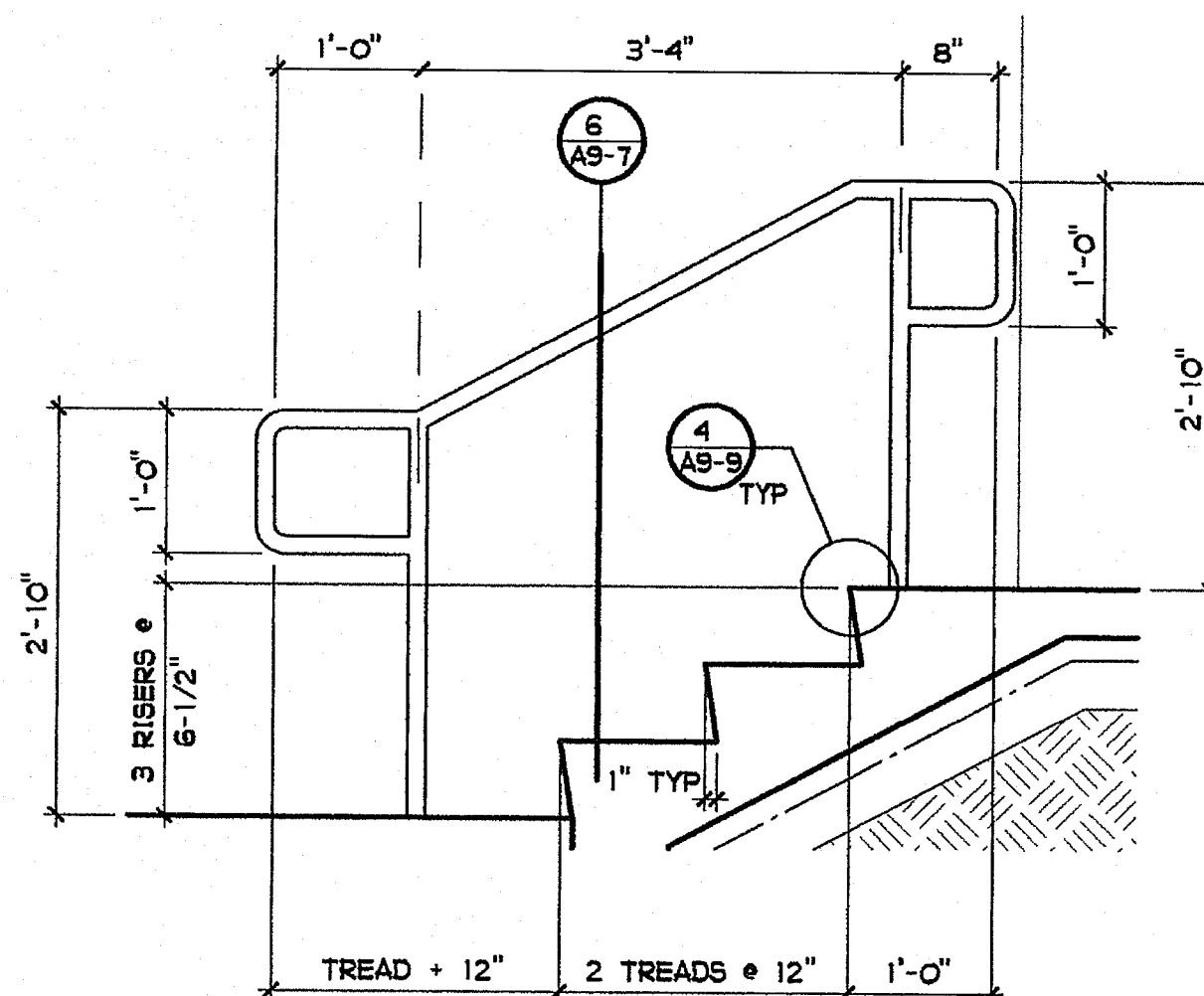
1 CONCRETE RAMP SECTION
3/4" = 1'-0"



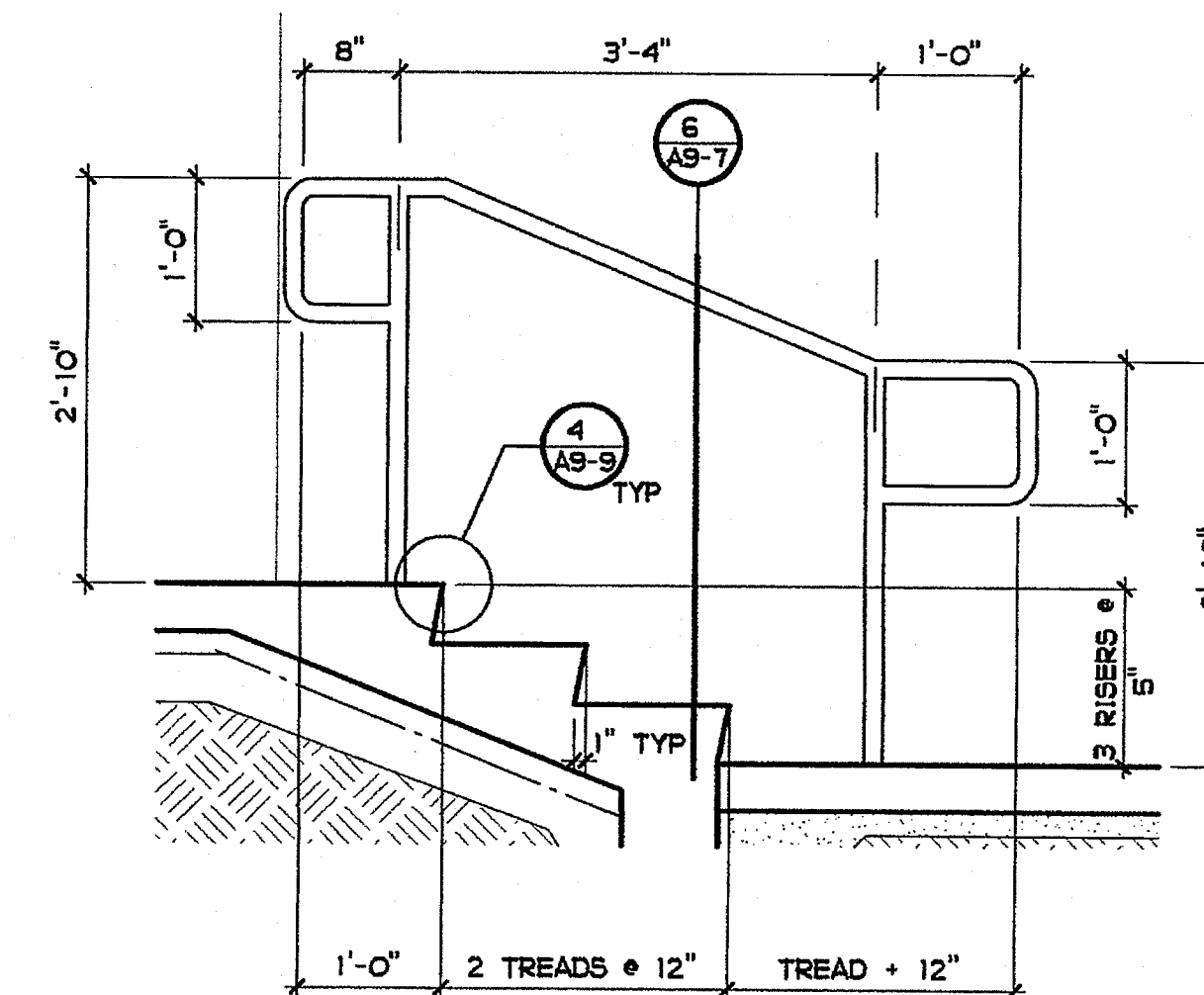
2 CONCRETE STEPS SECTION
3/4" = 1'-0"



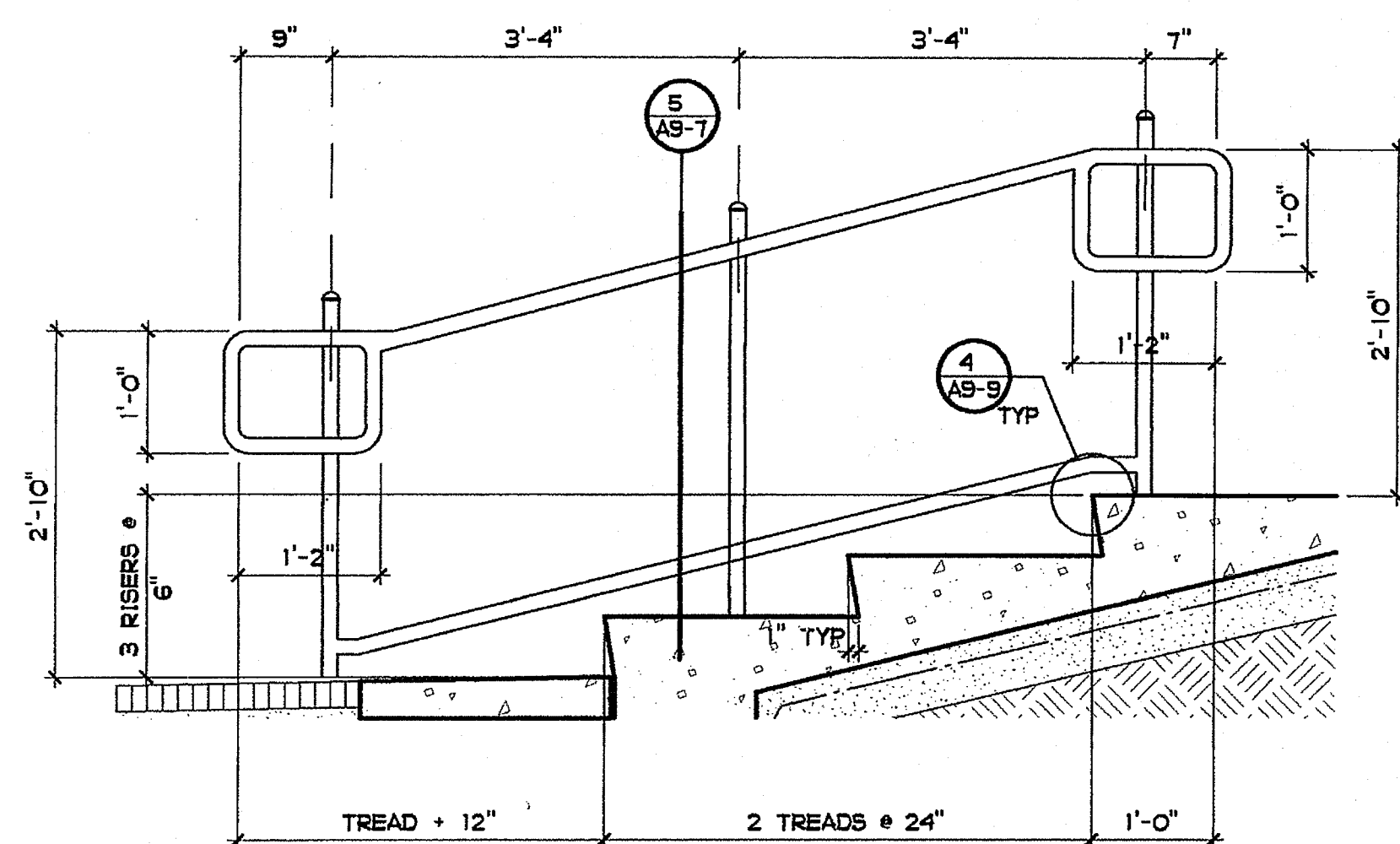
3 CONCRETE STEPS SECTION
3/4" = 1'-0"



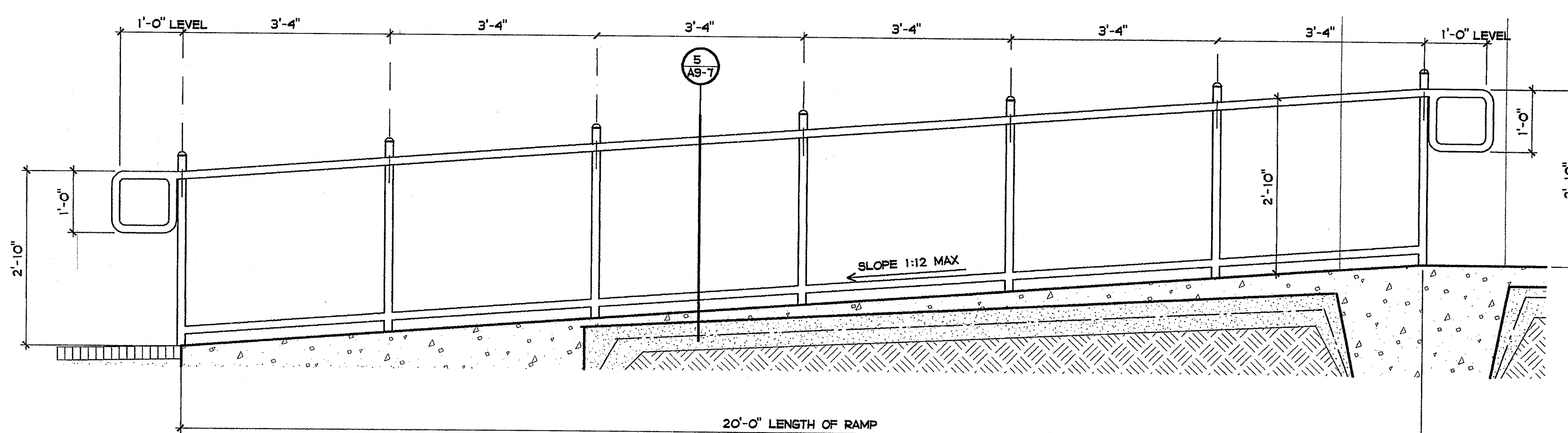
4 CONCRETE STEPS SECTION
3/4" = 1'-0"



5 CONCRETE STEPS SECTION
3/4" = 1'-0"



6 CONCRETE STEPS SECTION
3/4" = 1'-0"



7 CONCRETE RAMP SECTION
3/4" = 1'-0"

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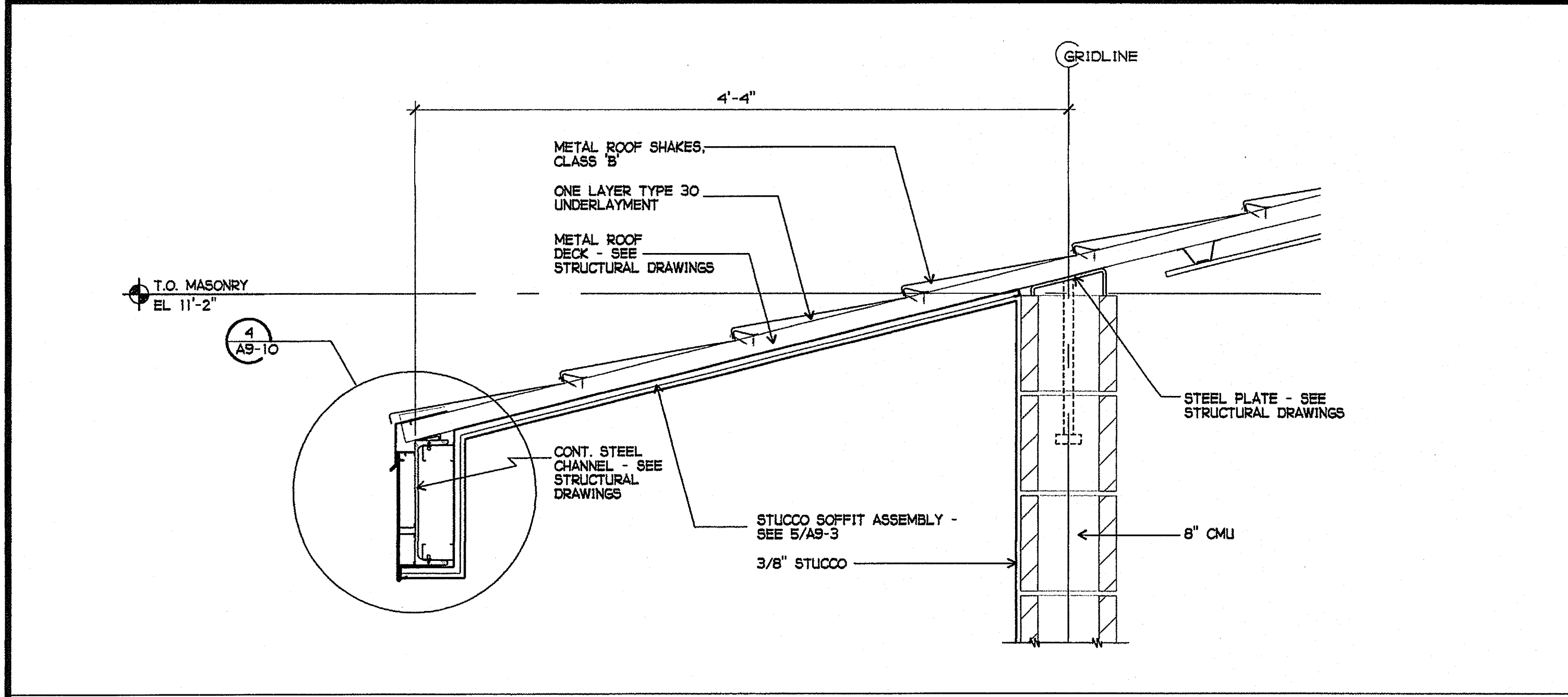
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DATE: MAR 28 2005

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JOHN SCOTT GROTH
C-26609
4/30/2007 RENEWAL
STATE OF CALIFORNIA

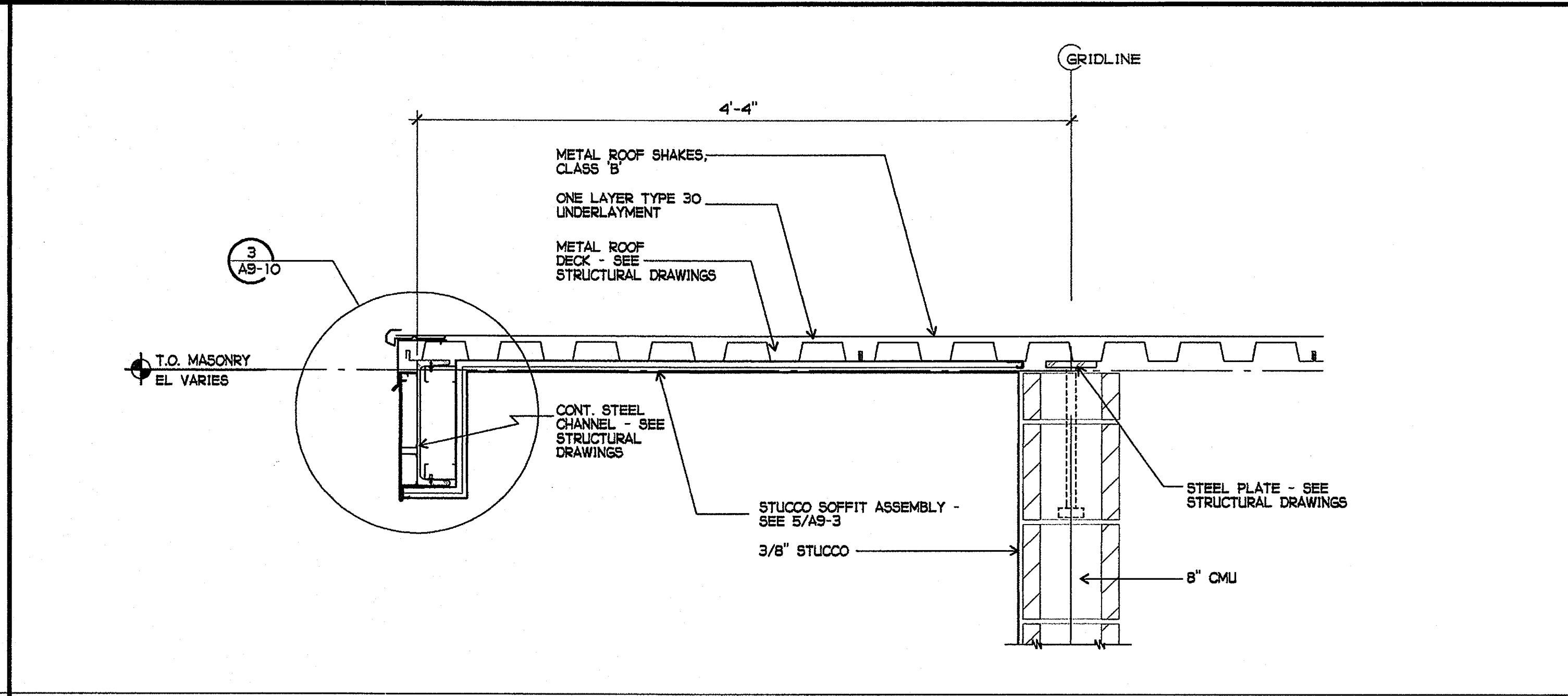
BUILDING N
STEPS AND RAMP
SECTIONS

AN4-16

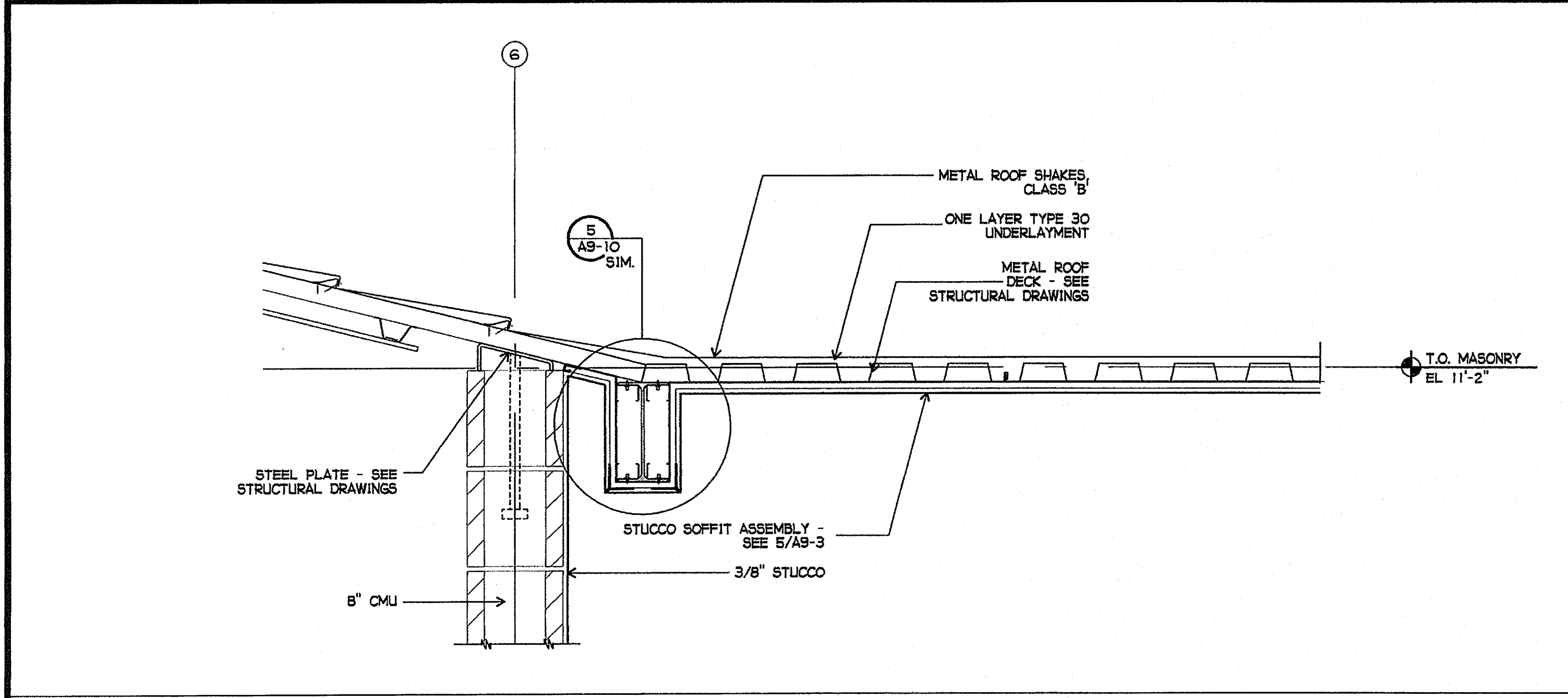
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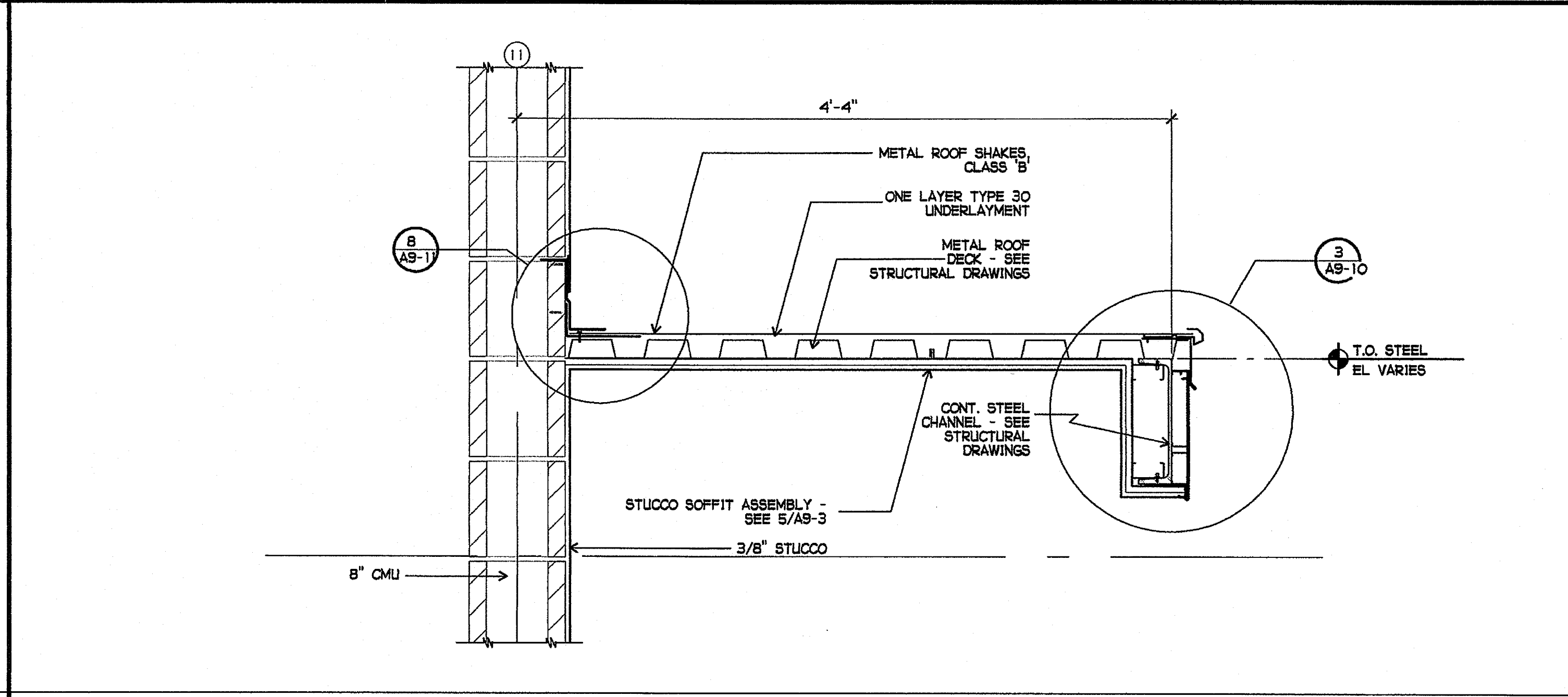
1 BUILDING SOFFIT AT EAVE
1-1/2" = 1'-0"



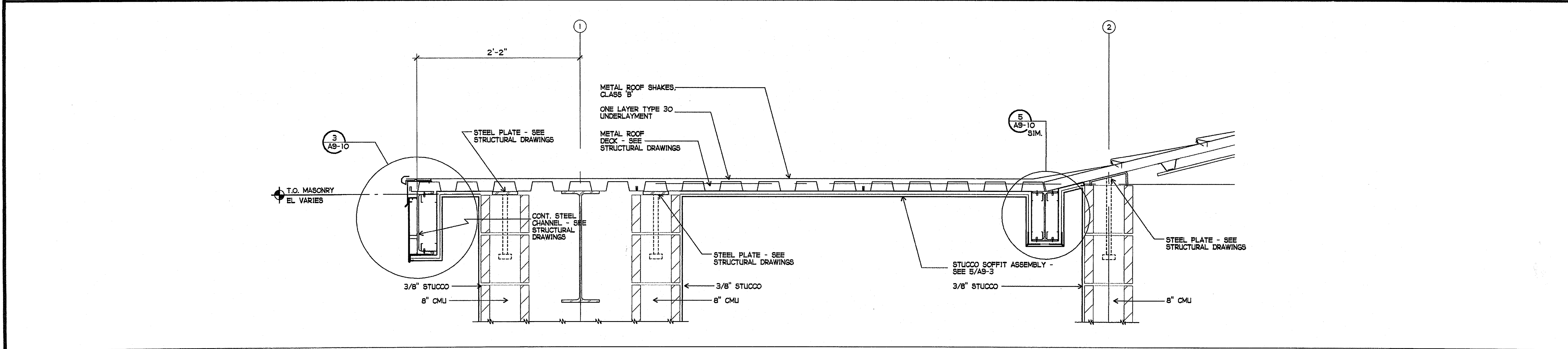
2 BUILDING SOFFIT AT RAKE
1-1/2" = 1'-0"



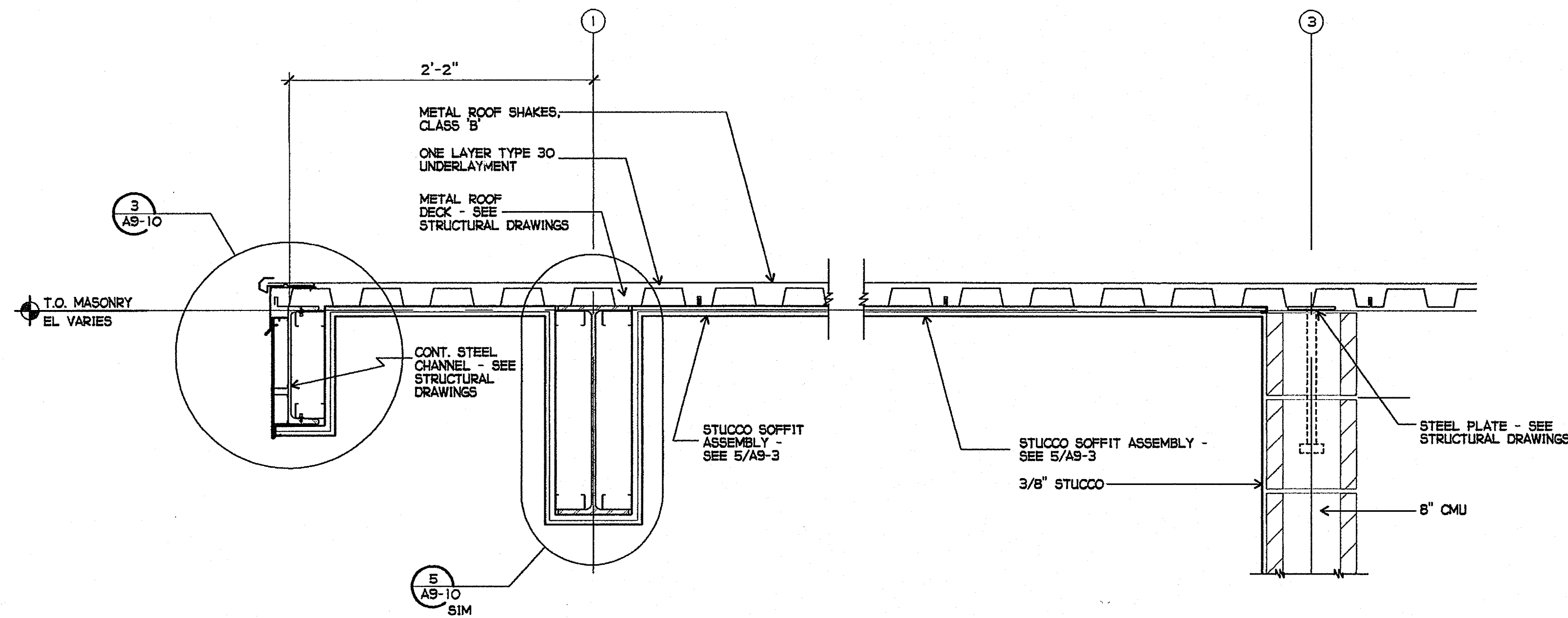
3 BUILDING SOFFIT
1-1/2" = 1'-0"



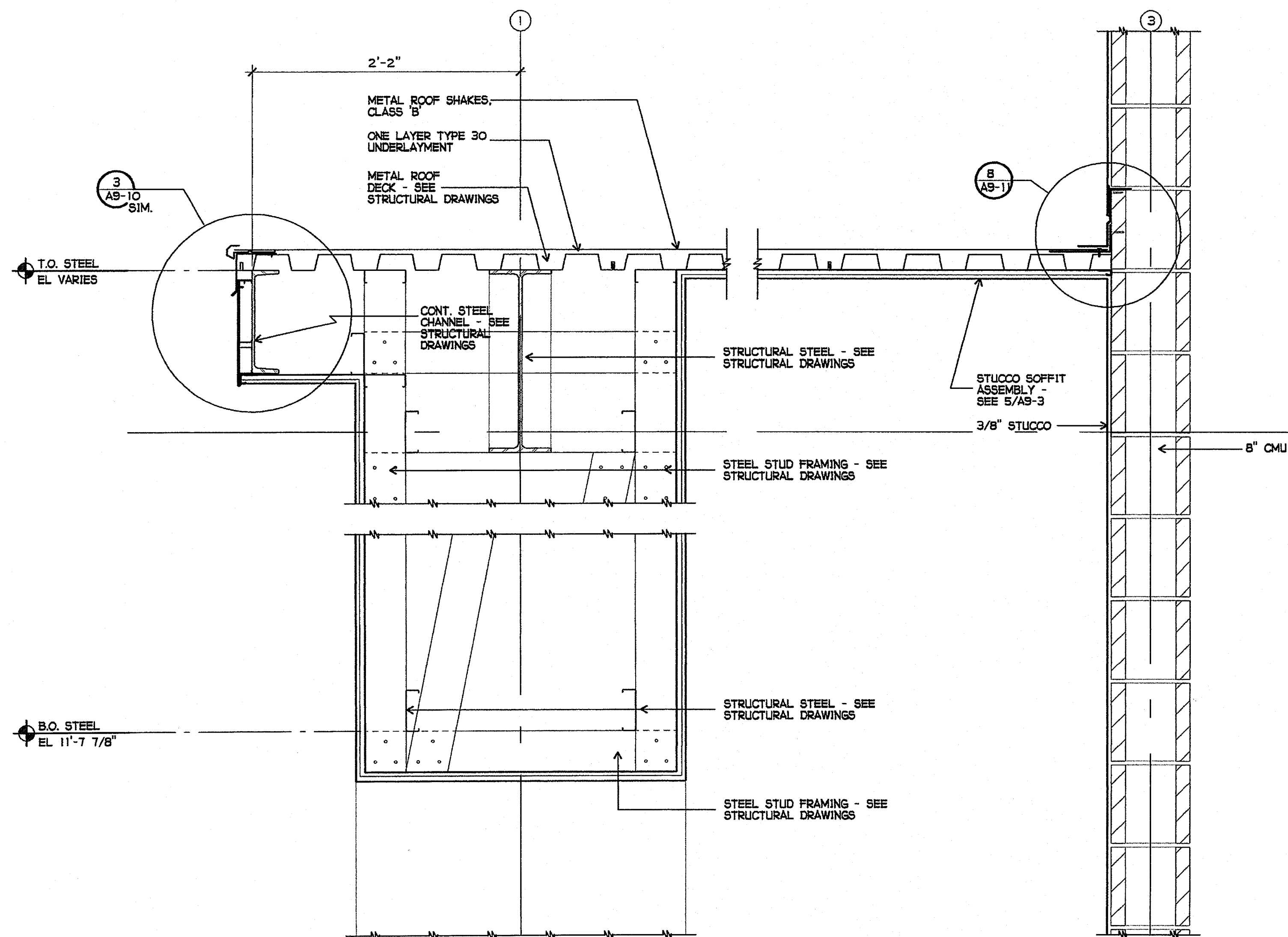
4 BUILDING SOFFIT AT RAKE/PARAPET
1-1/2" = 1'-0"



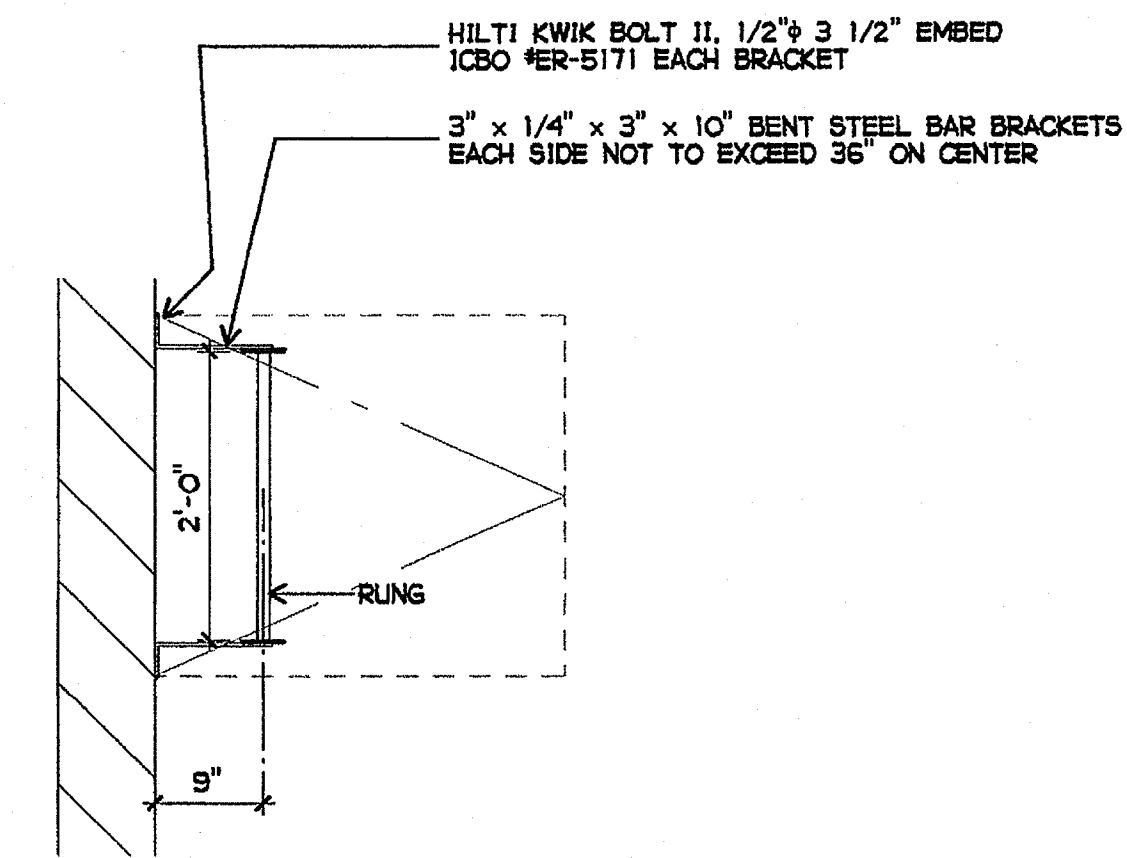
5 BUILDING SOFFIT AT LANDING COLUMN
1-1/2" = 1'-0"



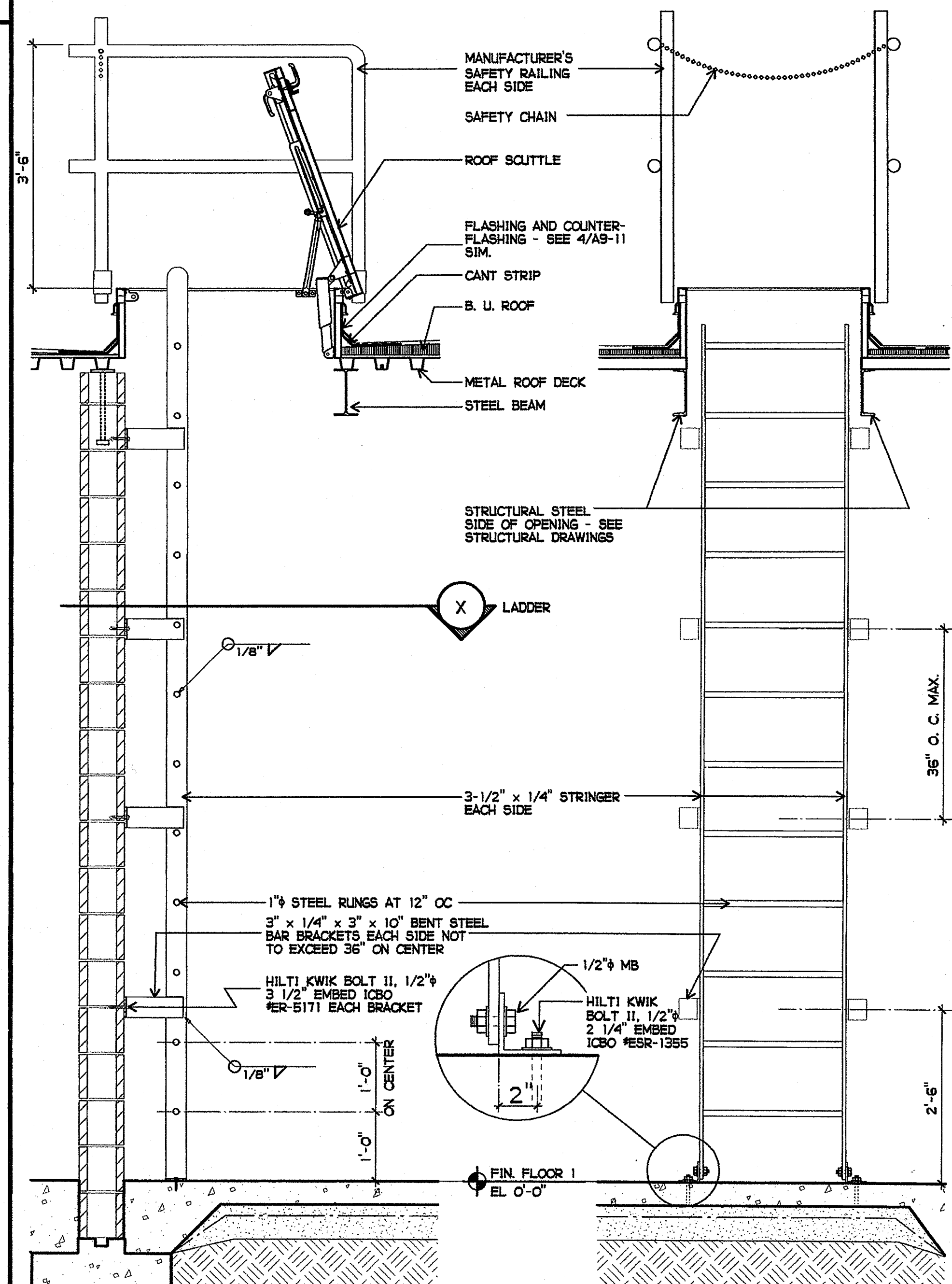
1 BUILDING SOFFIT AT LANDING
1-1/2" = 1'-0"



2 BUILDING SOFFIT AT LANDING
1-1/2" = 1'-0"



X LADDER
3/4" = 1'-0"



3 ROOF HATCH AND LADDER
3/4" = 1'-0"

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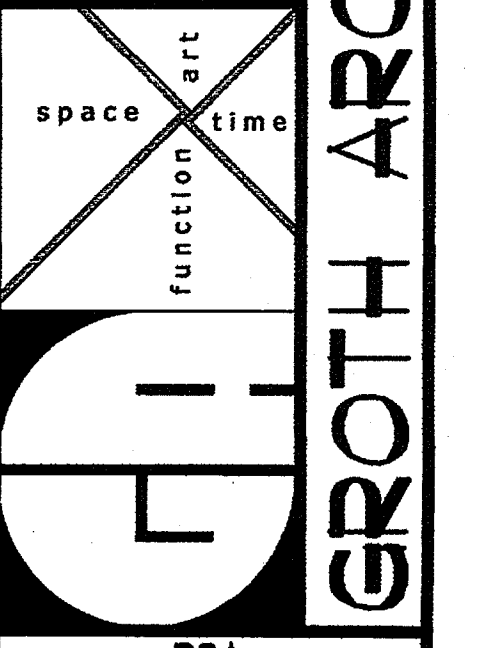
BUILDING SOFFIT,
ROOF HATCH AND
LADDER SECTIONS

AN4-18

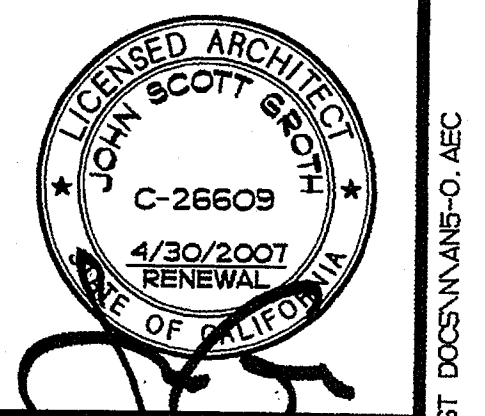
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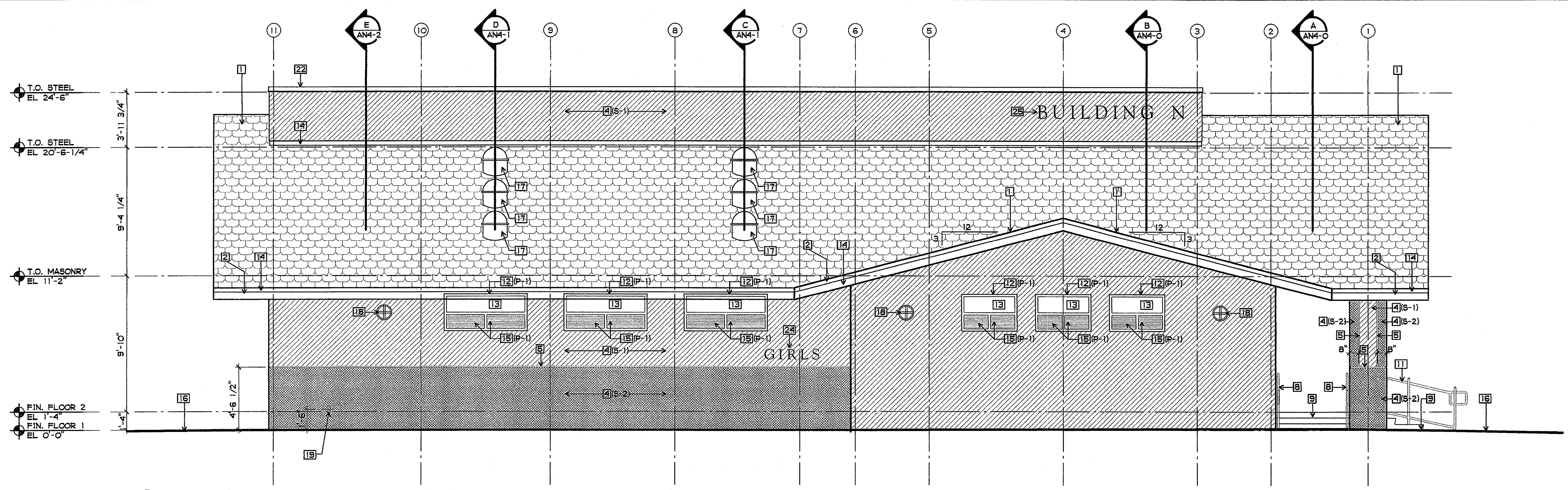
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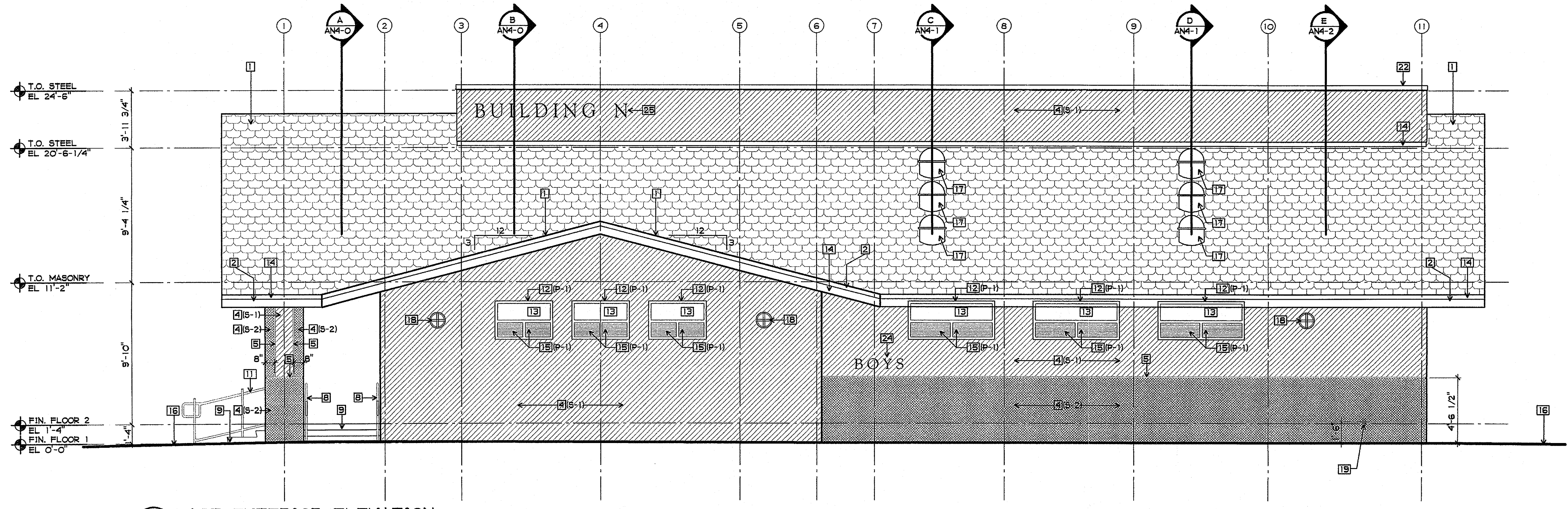
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SHEET TITLE
BUILDING N
EXTERIOR
ELEVATIONS
AN5-O



1 WEST EXTERIOR ELEVATION
1/4" = 1'-0"



2 EAST EXTERIOR ELEVATION
1/4" = 1'-0"

SHEET NOTES

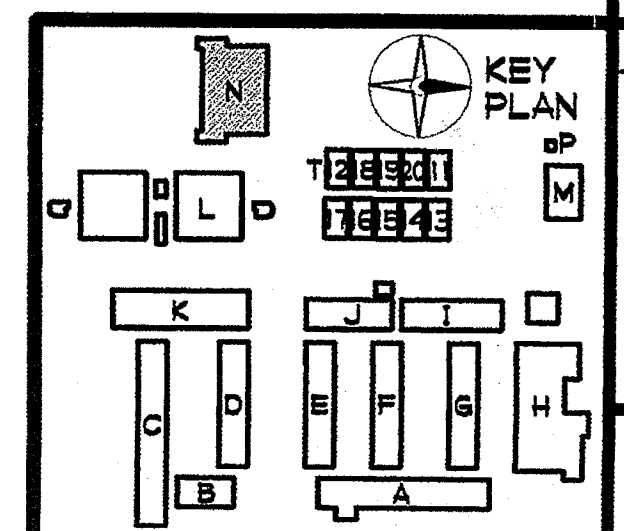
- 1 METAL ROOF SHAKES, CLASS 'B'
- 2 PREFINISHED METAL FASCIA
- 3 NOT USED
- 4 EXTERIOR STUCCO FINISH
- 5 1" STUCCO SCREED - SEE 10/A9-3
- 6 NOT USED
- 7 NOT USED
- 8 HANDRAIL - SEE 6/A9-7
- 9 TYPICAL STAIR STEP - SEE 4/A9-9

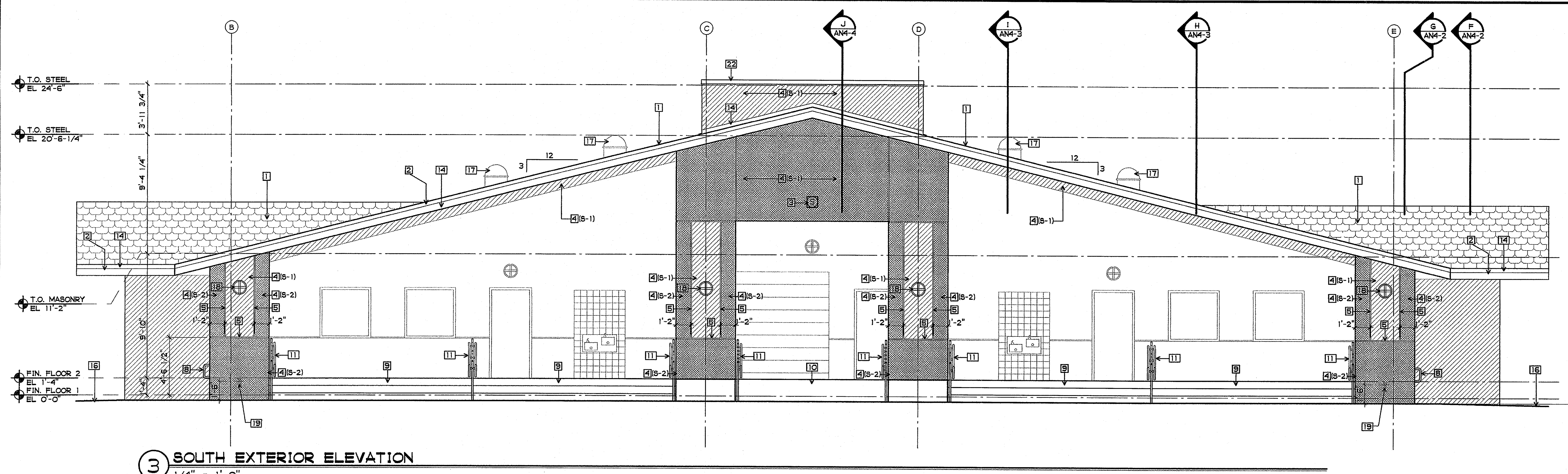
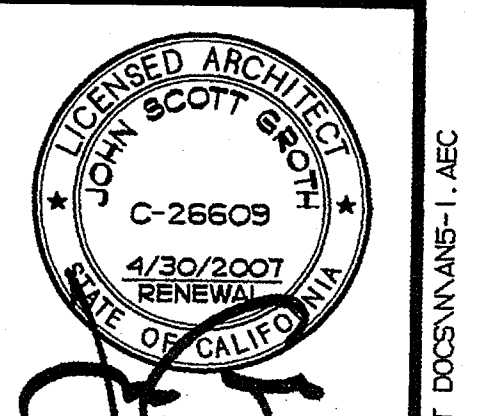
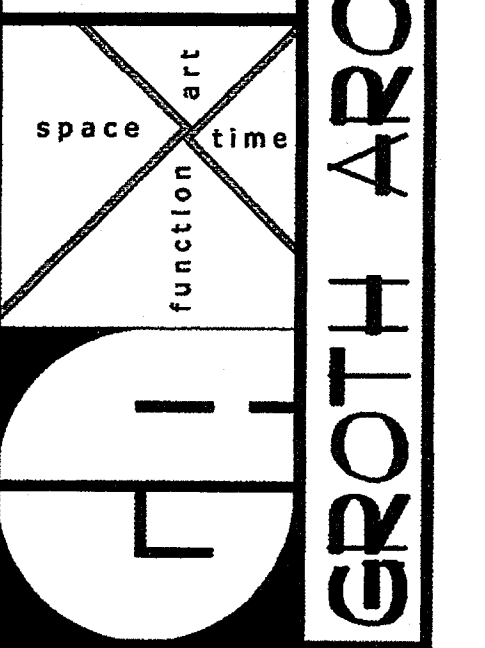
- 10 NOT USED
- 11 HANDRAIL - SEE 7/A9-7
- 12 METAL FRAME - SEMI-GLOSS PAINT
- 13 TRANSLUCENT GLAZING
- 14 FLASHING
- 15 LOLLERS - SEMI-GLOSS PAINT
- 16 ASPHALTIC CONCRETE
- 17 21" TUBULAR SKYLIGHT - SEE 1/A9-10
- 18 LIGHT FIXTURE - SEE ELECTRICAL DRAWINGS

- 19 WP POWER OUTLET - SEE ELECTRICAL DRAWINGS
- 20 NOT USED
- 21 NOT USED
- 22 PREFINISHED METAL PARAPET CAP
- 23 NOT USED
- 24 9" HIGH FLAT CUT ALUMINUM LETTERS
- 25 12" HIGH FLAT CUT ALUMINUM LETTERS

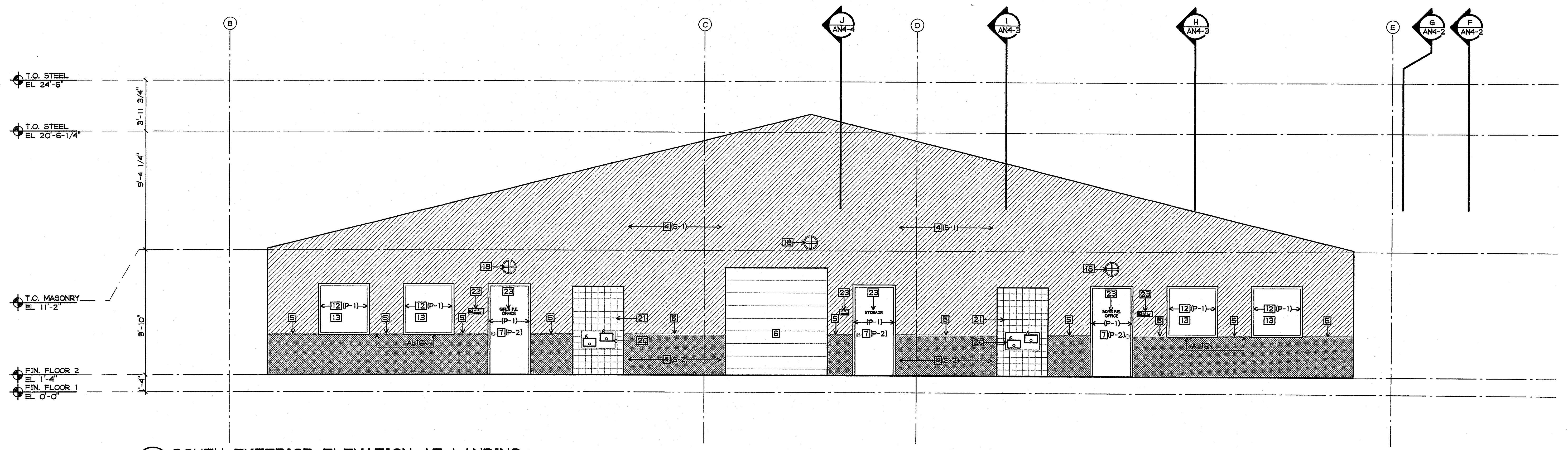
FINISH PALETTE

- (5-1) EXTERIOR CEMENTITIOUS COATING (STUCCO), COLOR #1
- (5-2) EXTERIOR CEMENTITIOUS COATING (STUCCO), COLOR #2
- (P-1) PAINT, COLOR #1
- (P-2) PAINT, COLOR #2





3 SOUTH EXTERIOR ELEVATION
 1/4" = 1'-0"



4 SOUTH EXTERIOR ELEVATION AT LANDING
 1/4" = 1'-0"

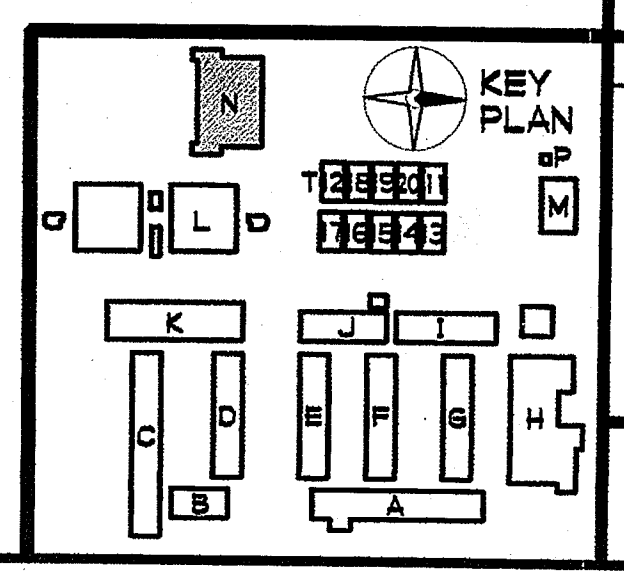
SHEET NOTES

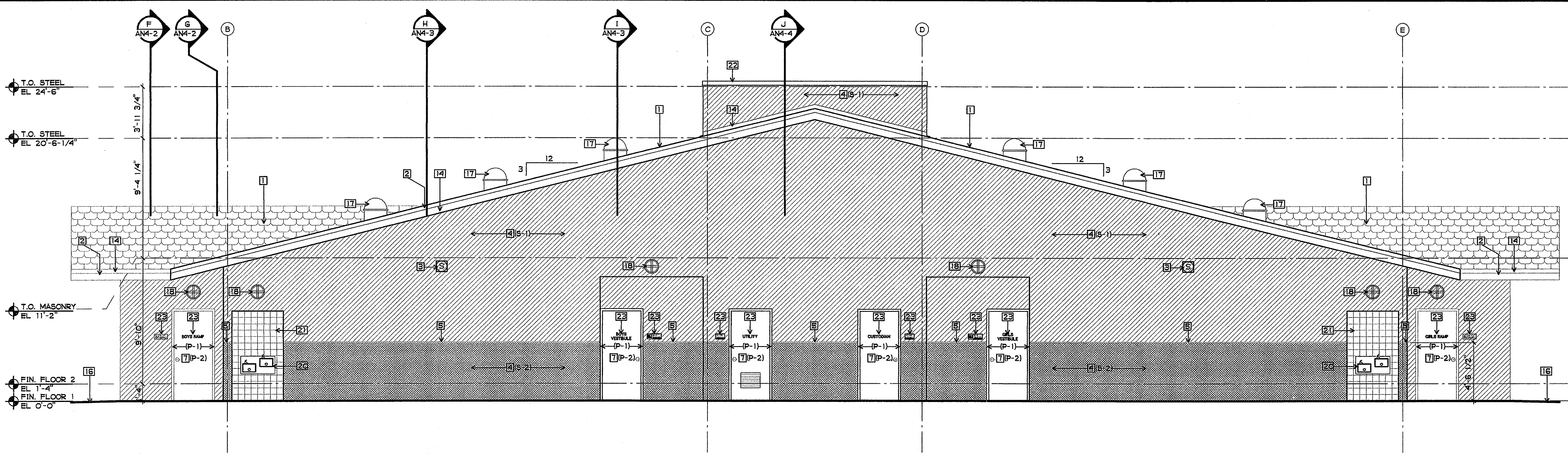
- 1 METAL ROOF SHAKES, CLASS 'B'
- 2 PREFINISHED METAL FASCIA
- 3 SPEAKER - SEE ELECTRICAL DRAWINGS
- 4 EXTERIOR STUCCO FINISH
- 5 1" STUCCO SCREED - SEE 10/A9-3
- 6 ROLL-UP DOOR
- 7 DOOR AND FRAME - SEMI-GLOSS PAINT
- 8 HANDRAIL - SEE 6/A9-6
- 9 TYPICAL STAIR STEP - SEE 4/A9-9
- 10 ACCESSIBLE RAMP
- 11 HANDRAIL - SEE 6/A9-7
- 12 METAL FRAME - SEMI-GLOSS PAINT
- 13 REFLECTIVE GLAZING
- 14 FLASHING
- 15 NOT USED
- 16 ASPHALTIC CONCRETE
- 17 21" TUBULAR SKYLIGHT - SEE 1/A9-10
- 18 LIGHT FIXTURE - SEE ELECTRICAL DRAWINGS

- 19 WP POWER OUTLET - SEE ELECTRICAL DRAWINGS
- 20 HI-LO ACCESSIBLE DRINKING FOUNTAIN - SEE 1/A9-7
- 21 CERAMIC TILE APRON - PATTERN PER 6/ANS-2
- 22 PREFINISHED METAL PARAPET CAP
- 23 DOOR SIGNAGE - SEE DOOR SCHEDULE

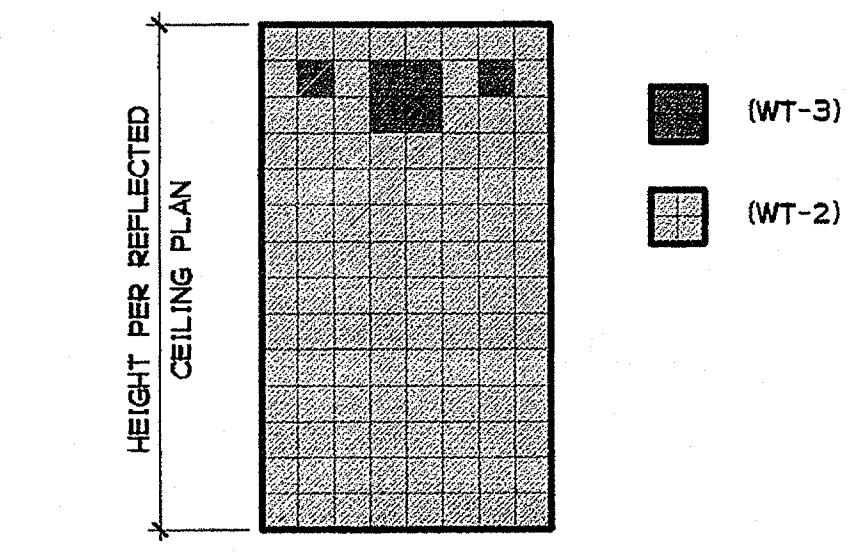
FINISH PALETTE

- (S-1) EXTERIOR CEMENTITIOUS COATING (STUCCO), COLOR #1
- (S-2) EXTERIOR CEMENTITIOUS COATING (STUCCO), COLOR #2
- (P-1) PAINT, COLOR #1
- (P-2) PAINT, COLOR #2





5 NORTH EXTERIOR ELEVATION
1/4" = 1'-0"



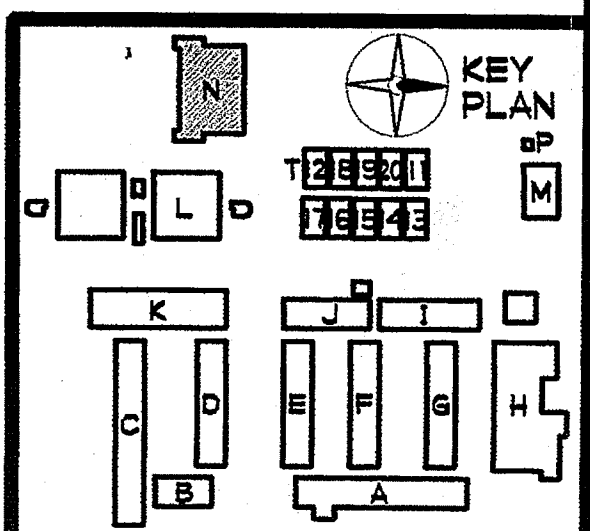
6 CERAMIC WALL TILE PATTERN
3/8" = 1'-0"

SHEET NOTES

- | | | |
|-------------------------------------|--|--|
| 1 METAL ROOF SHAKES, CLASS 'b' | 10 NOT USED | 19 NOT USED |
| 2 PREFINISHED METAL FASCIA | 11 NOT USED | 20 HI-LO ACCESSIBLE DRINKING FOUNTAIN - SEE 2/A9-7 |
| 3 SPEAKER - SEE ELECTRICAL DRAWINGS | 12 NOT USED | 21 CERAMIC TILE APRON - PATTERN PER 6/AN5-2 |
| 4 EXTERIOR STUCCO FINISH | 13 NOT USED | 22 PREFINISHED METAL PARAPET CAP |
| 5 1" STUCCO SCREED - SEE 10/A9-3 | 14 FLASHING | 23 DOOR SIGNAGE - SEE DOOR SCHEDULE |
| 6 NOT USED | 15 NOT USED | |
| 7 DOOR AND FRAME - SEMI-GLOSS PAINT | 16 ASPHALTIC CONCRETE | |
| 8 NOT USED | 17 21" TUBULAR SKYLIGHT - SEE 1/A9-10 | |
| 9 NOT USED | 18 LIGHT FIXTURE - SEE ELECTRICAL DRAWINGS | |

FINISH PALETTE

- (S-1) EXTERIOR CEMENTITIOUS COATING (STUCCO), COLOR #1
- (S-2) EXTERIOR CEMENTITIOUS COATING (STUCCO), COLOR #2
- (P-1) PAINT, COLOR #1
- (P-2) PAINT, COLOR #2



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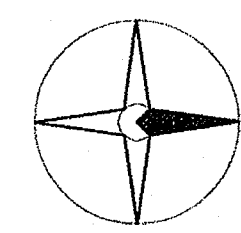
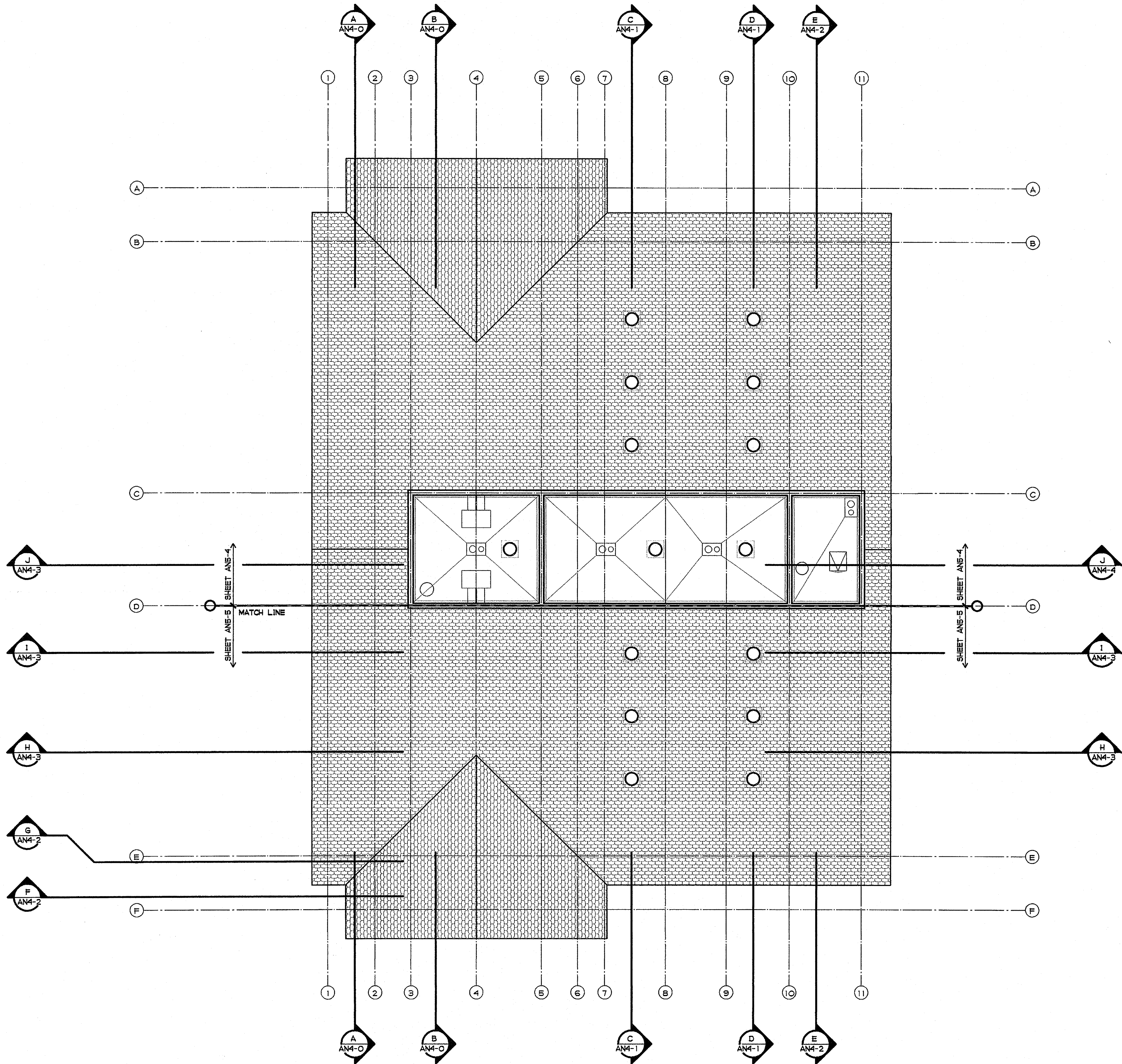
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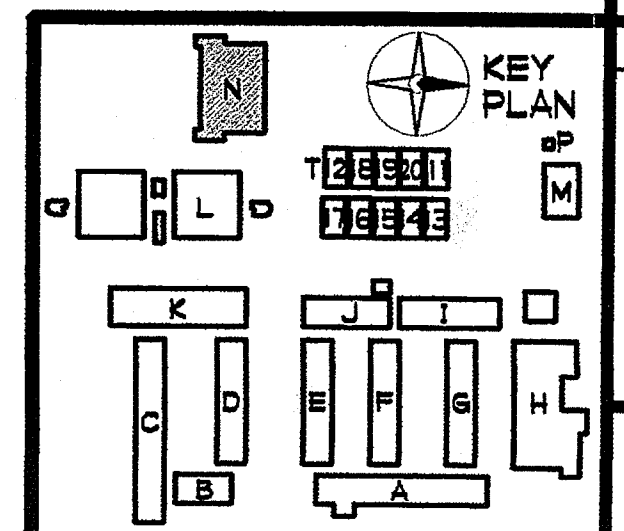
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 4/30/2007
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SHEET TITLE
 BUILDING N
 EXTERIOR
 ELEVATIONS
AN5-2



BUILDING N ROOF PLAN
1/8" = 1'-0"



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4/30/2007
RENEWAL
STATE OF CALIFORNIA

SHEET TITLE
BUILDING N
ROOF PLAN

AN5-3

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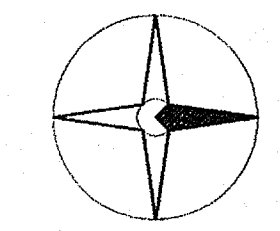
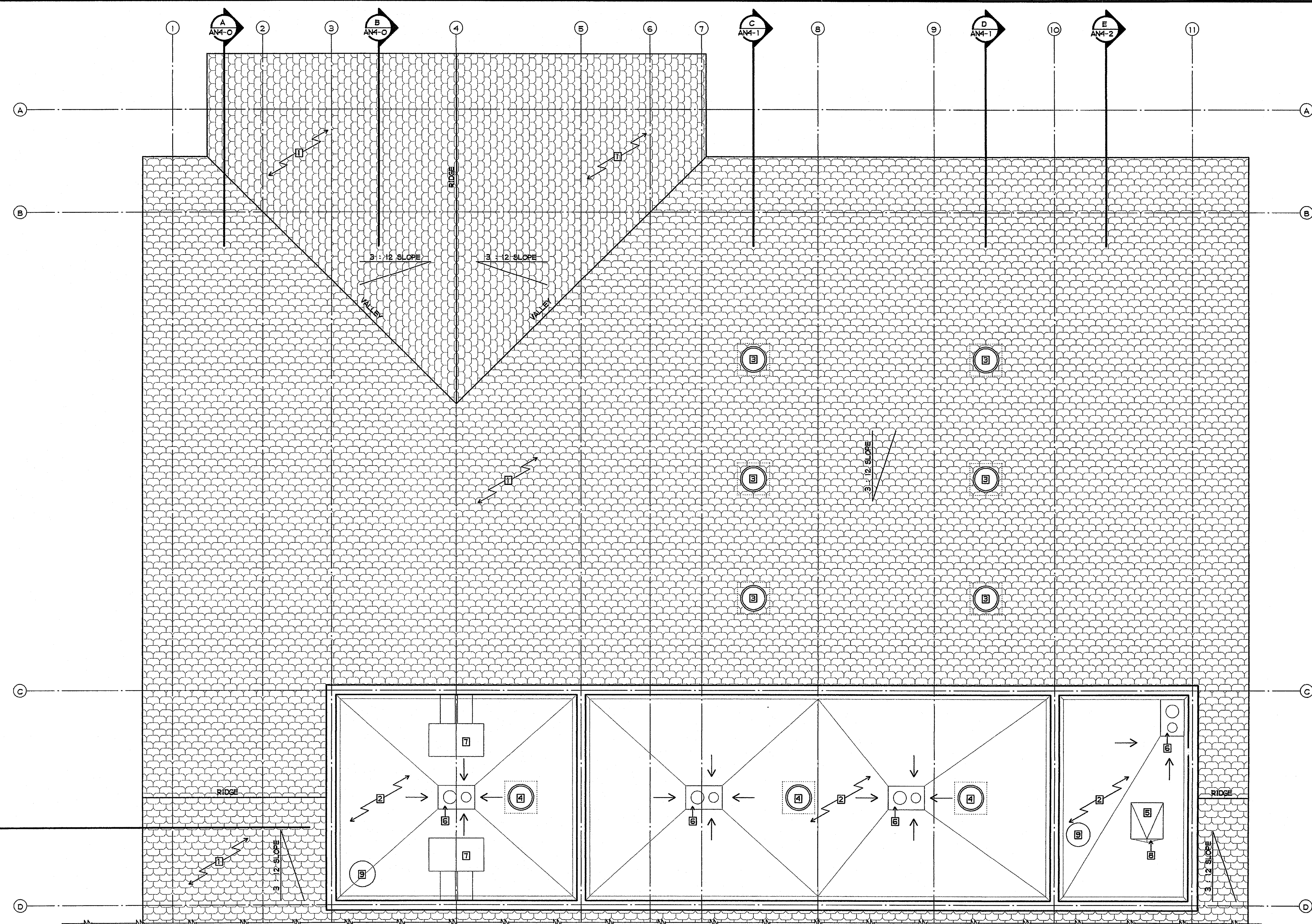
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SHEET TITLE
 BUILDING N
 ENLARGED
 ROOF PLAN

AN5-4

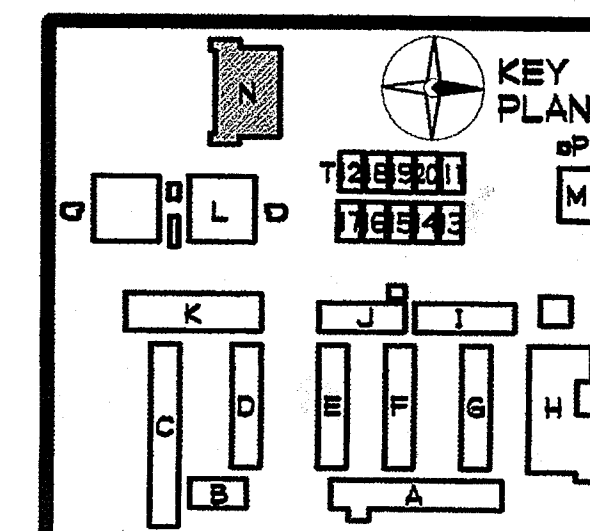


BUILDING N ENLARGED PARTIAL ROOF PLAN

1/4" = 1'-0"

SHEET NOTES

- 1 METAL SHAKE ROOFING, CLASS 'B'
- 2 BUILT-UP ROOF
- 3 21" TUBULAR SKYLIGHT CEILING DIFFUSER AT METAL SHAKE ROOFING - SEE 1/A9-10
- 4 21" TUBULAR SKYLIGHT CEILING DIFFUSER AT BUILT-UP ROOF - SEE 2/A9-10
- 5 ROOF SCUTTLE - SEE 3/AN4-18
- 6 ROOF AND OVERFLOW DRAIN - SEE 1/A9-13
- 7 MECHANICAL UNIT - SEE MECHANICAL DRAWINGS
- 8 CRICKET
- 9 EXHAUST FAN - SEE MECHANICAL DRAWINGS

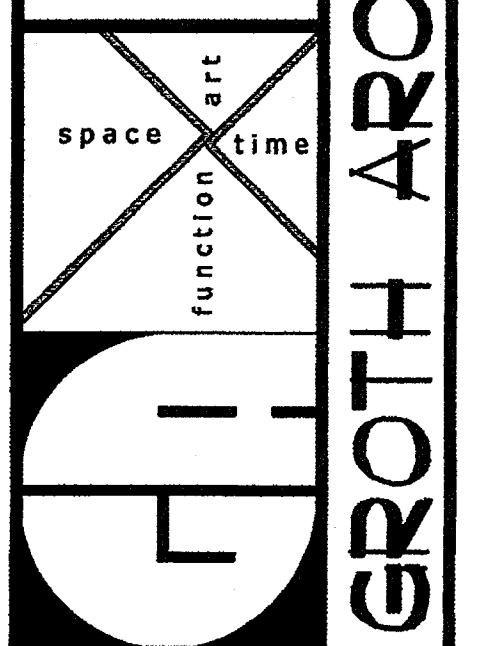


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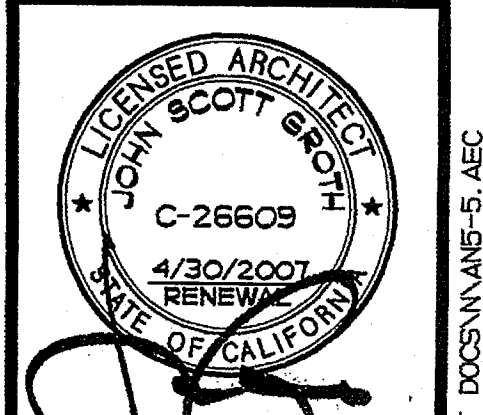
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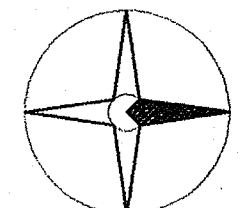
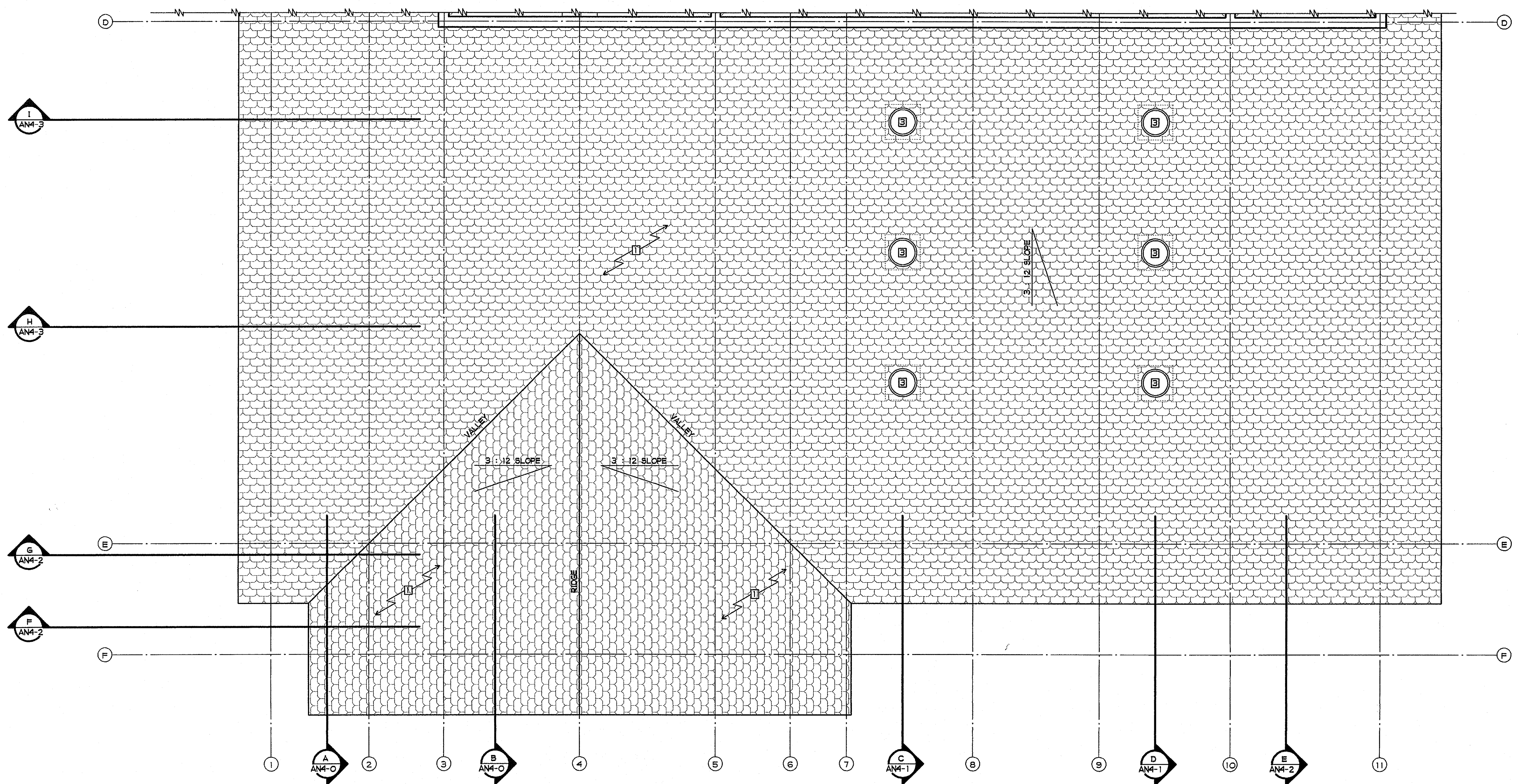
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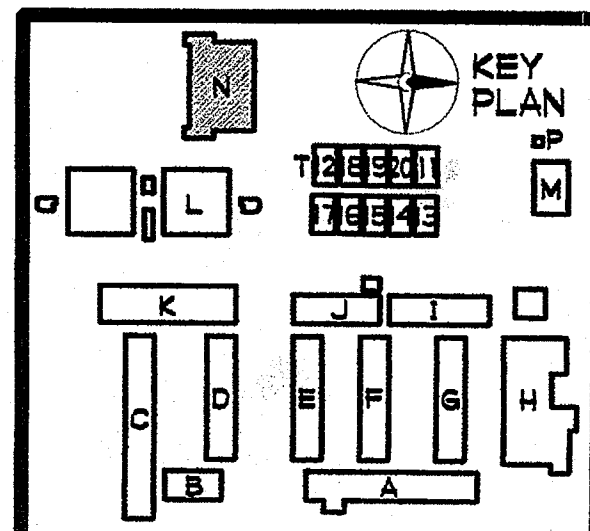


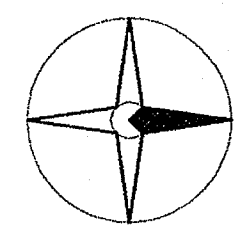
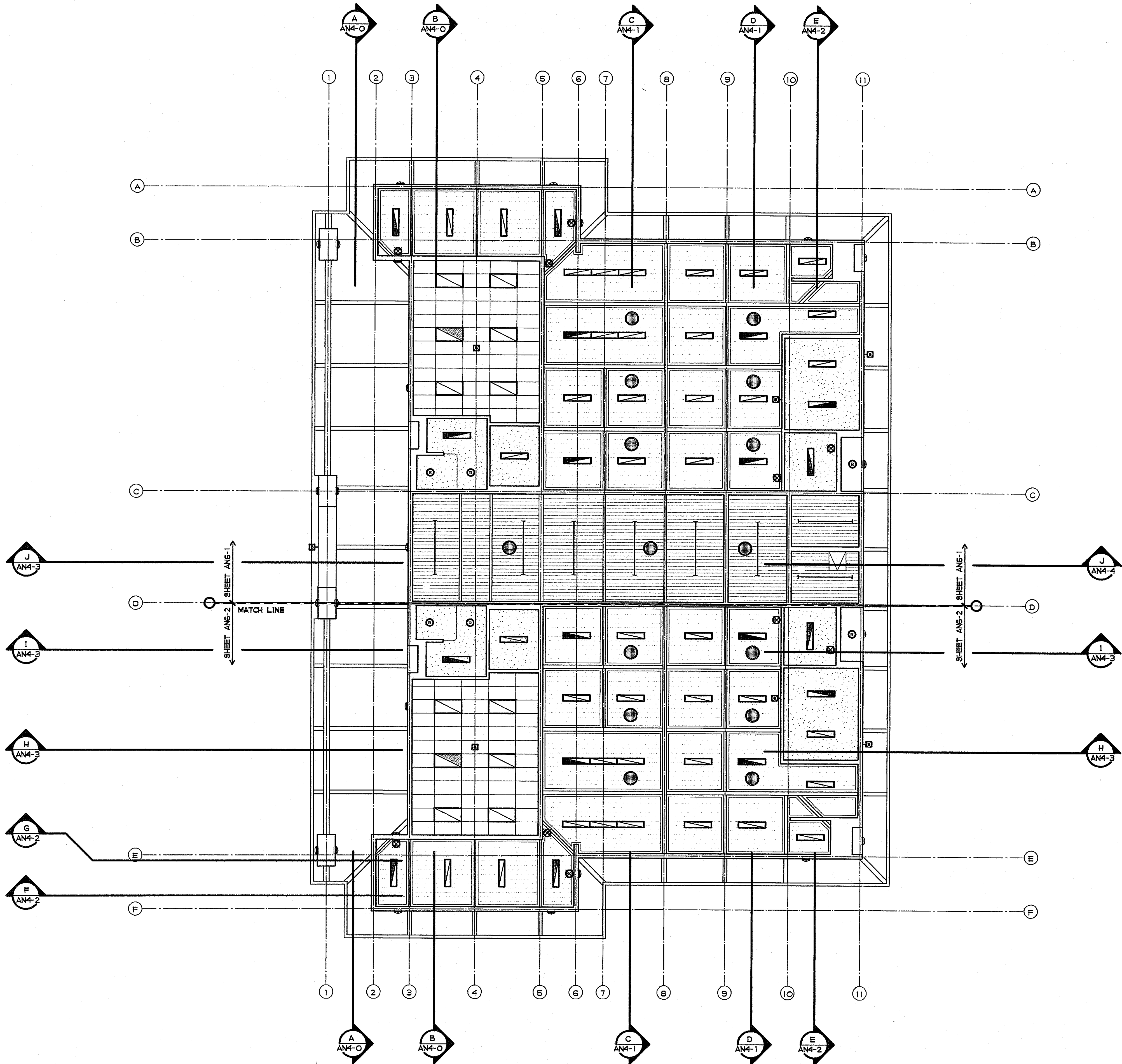
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ROOF PLAN
AN5-5



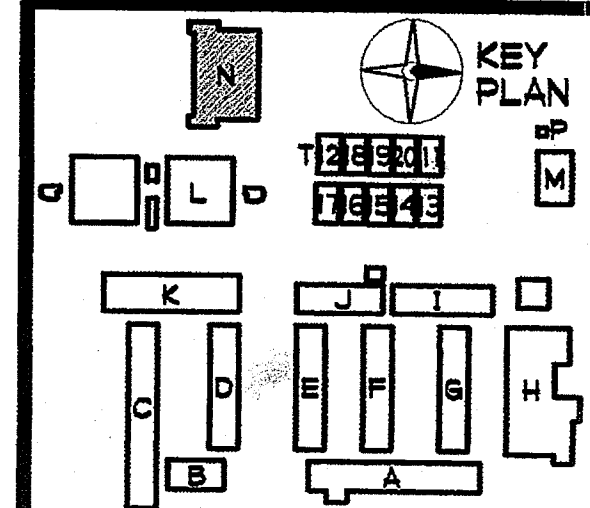
BUILDING N ENLARGED PARTIAL ROOF PLAN
1/4" = 1'-0"

- SHEET NOTES**
- 1 METAL SHAKE ROOFING, CLASS 'B'
 - 2 NOT USED
 - 3 2 1/2" TUBULAR SKYLIGHT CEILING DIFFUSER AT METAL SHAKE ROOFING - SEE 1/A9-10





BUILDING N REFLECTED CEILING PLAN
 1/8" = 1'-0"



PLOTTED 3/18/2005 11:03 AM

DLD

PHONE 760-754-8191
FAX 760-754-8291

OLSD NO.
758-000

PROJECT NOS.
025

P. T. N.
73569-9

DATE

REVISIONS

JEFFERSON MS NEW CONSTRUCTION
 823 ACACIA STREET
 OCEANSIDE, CA 92054
 OCEANSIDE UNIFIED S.D.

space time
function art

GROTH ARCHITECTS, INC.

3355 MISSION AVE. SUITE 234
 OCEANSIDE, CALIFORNIA 92054

DSA

IDENTIFICATION STAMP
 DIV. OF THE STATE ARCHITECT
 OFFICE OF REGULATION SERVICES

4-106494

AC. PLS. SS. 2

DATE MAR 28 2005

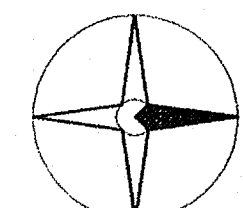
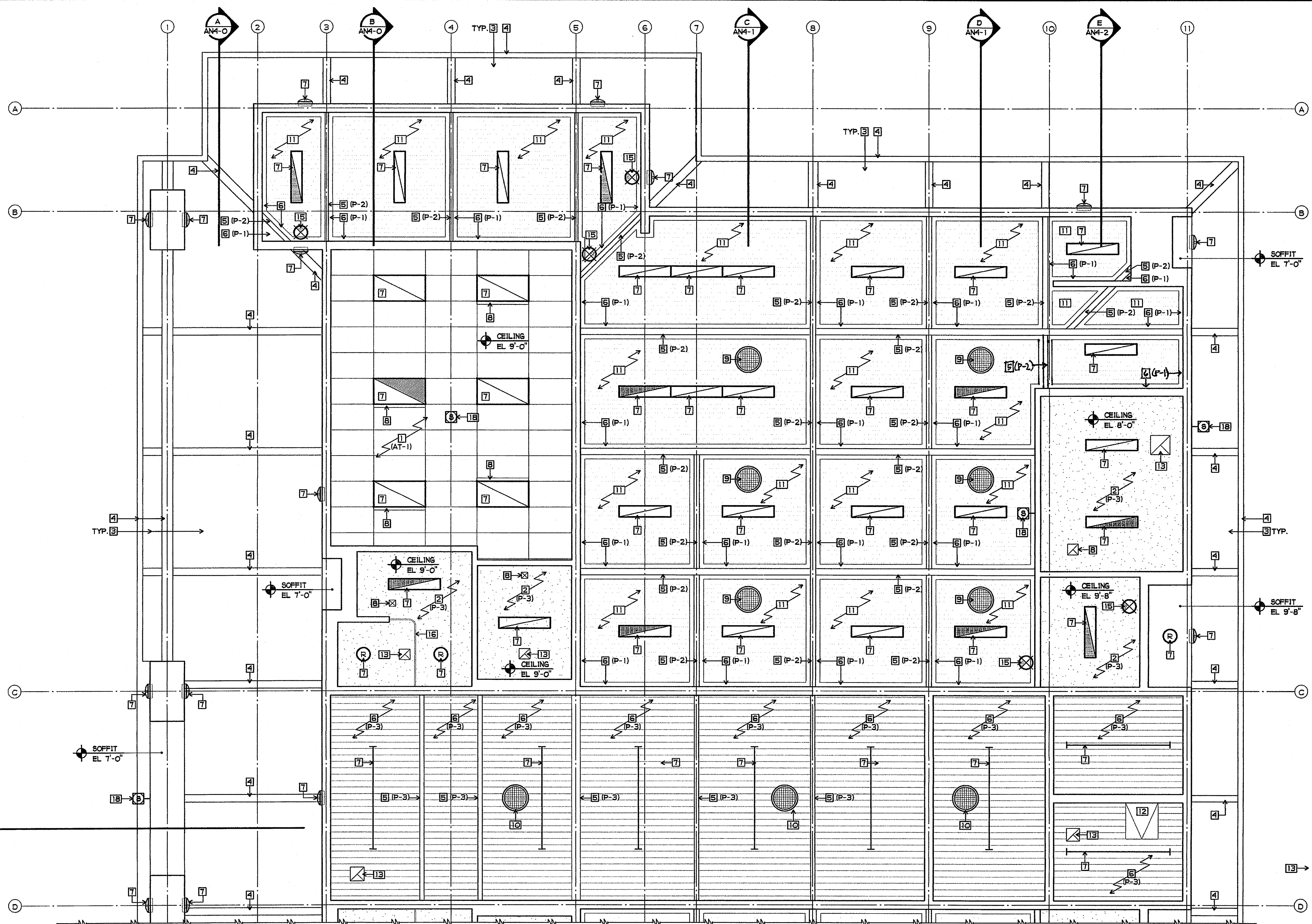
LICENSED ARCHITECT
 JOHN SCOTT GROTH
 C-26609
 4/30/2007 RENEWAL
 STATE OF CALIFORNIA

SHEET TITLE

BUILDING N
 REFLECTED
 CEILING
 PLAN

AN6-0

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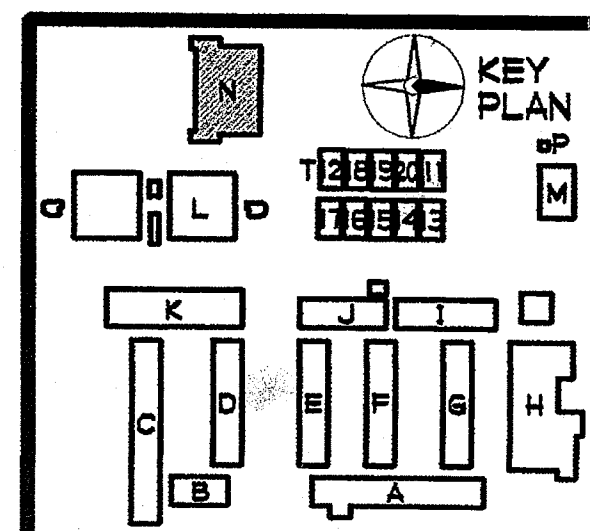


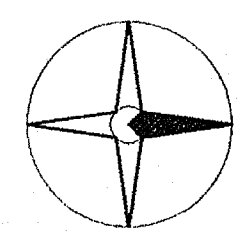
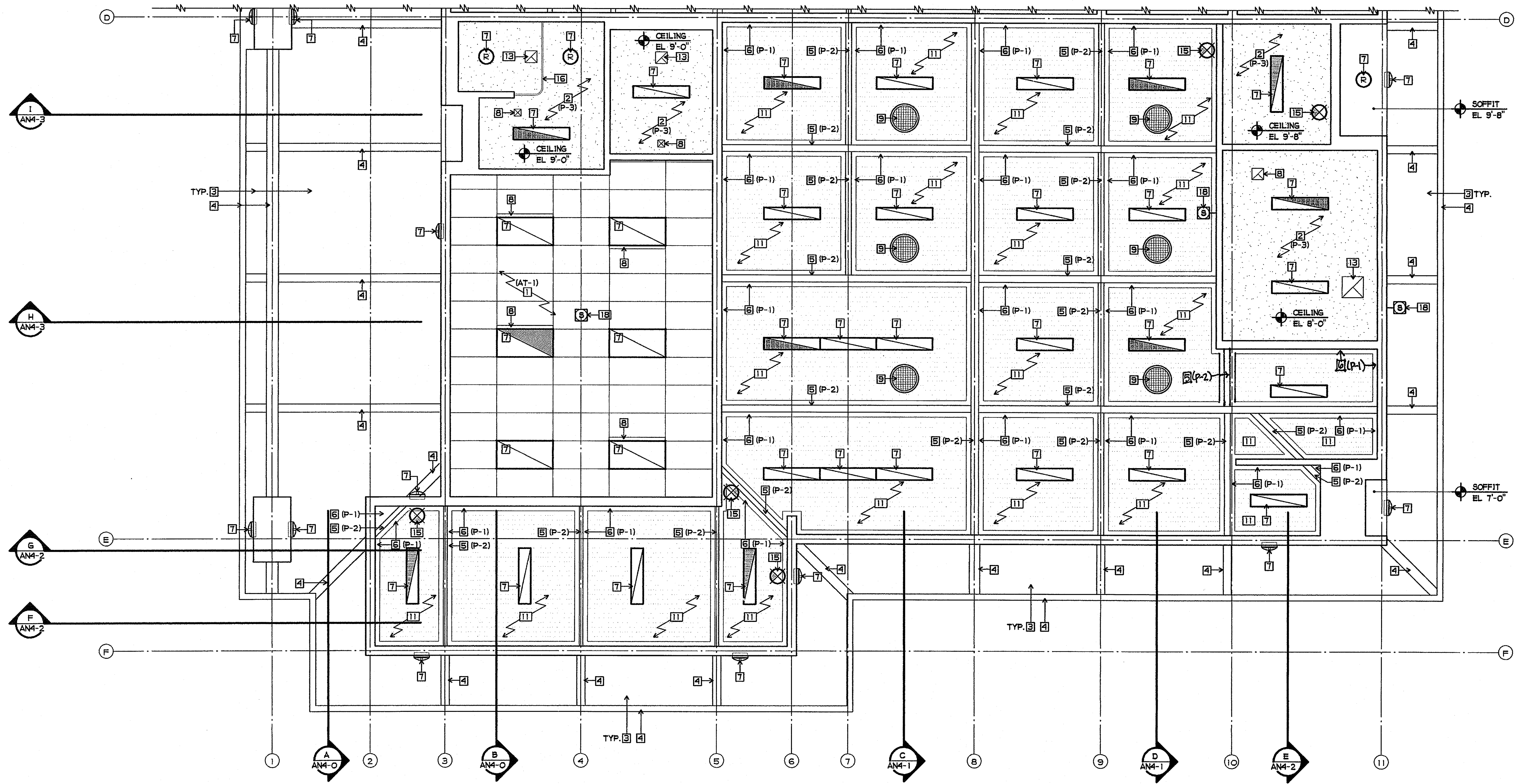
BUILDING N ENLARGED PARTIAL REFLECTED CEILING PLAN

1/4" = 1'-0"

SHEET NOTES

- 1 ACOUSTICAL CEILING
- 2 GYP. BOARD CEILING - SEMI-GLOSS PAINT
- 3 STUCCO SOFFIT - SEE 5/A9-3
- 4 WRAPPED STRUCTURAL STEEL WITH STUCCO FINISH - SEE 3, 4, 5 & 6/A9-10
- 5 EXPOSED STRUCTURAL STEEL - SEMI-GLOSS PAINT
- 6 EXPOSED METAL DECKING - SEMI-GLOSS PAINT
- 7 LIGHT FIXTURE - SEE ELECTRICAL DRAWINGS
- 8 AIR DIFFUSER/GRILLE - SEE MECHANICAL DRAWINGS
- 9 21" TUBULAR SKYLIGHT CEILING DIFFUSER AT METAL SHAKE ROOFING - SEE 1/A9-10
- 10 21" TUBULAR SKYLIGHT CEILING DIFFUSER AT BUILT-UP ROOF - SEE 2/A9-10
- 11 PERFORATED ALUMINUM PANELS MOUNTED ON 1-1/2" HAT CHANNELS. STOP PANELS 4" FROM EDGE OF STRUCTURAL STEEL OR WALL - SEE 7/A9-14
- 12 ROOF SCUTTLE - SEE 3/A9-18
- 13 EXHAUST FAN REGISTER - SEE MECHANICAL DRAWINGS
- 14 SPEAKER - SEE ELECTRICAL DRAWINGS
- 15 ILLUMINATED EXIT SIGN - SEE ELECTRICAL DRAWINGS
- 16 CUBICLE CURTAIN TRACK - SEE 4/A9-4
- 17 J-BOX W/ COVER
- 18 SPEAKER - SEE ELECTRICAL DRAWINGS

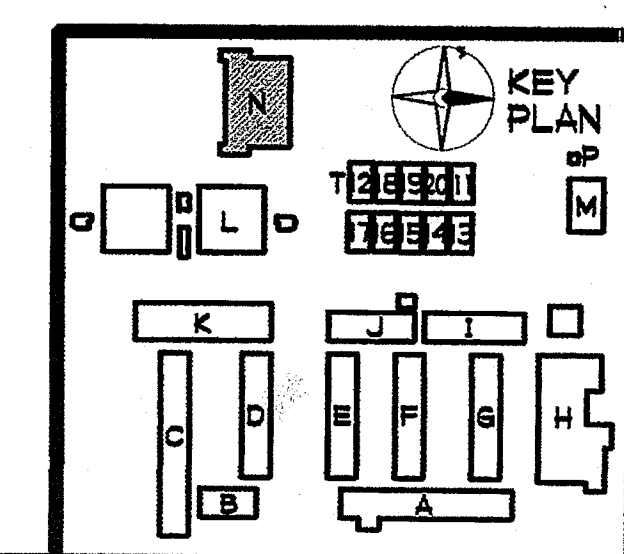


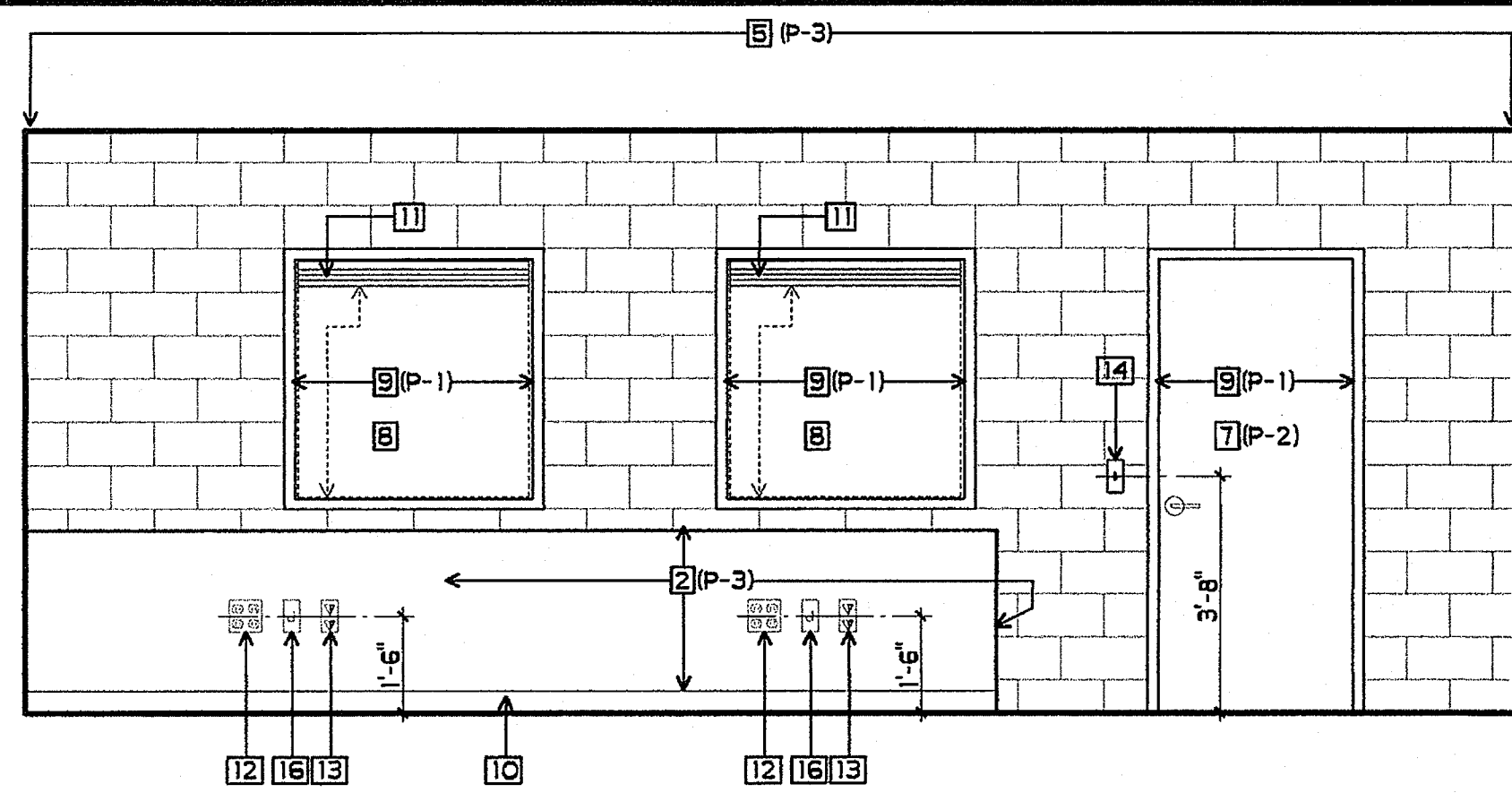


BUILDING N ENLARGED PARTIAL REFLECTED CEILING PLAN
 1/4" = 1'-0"

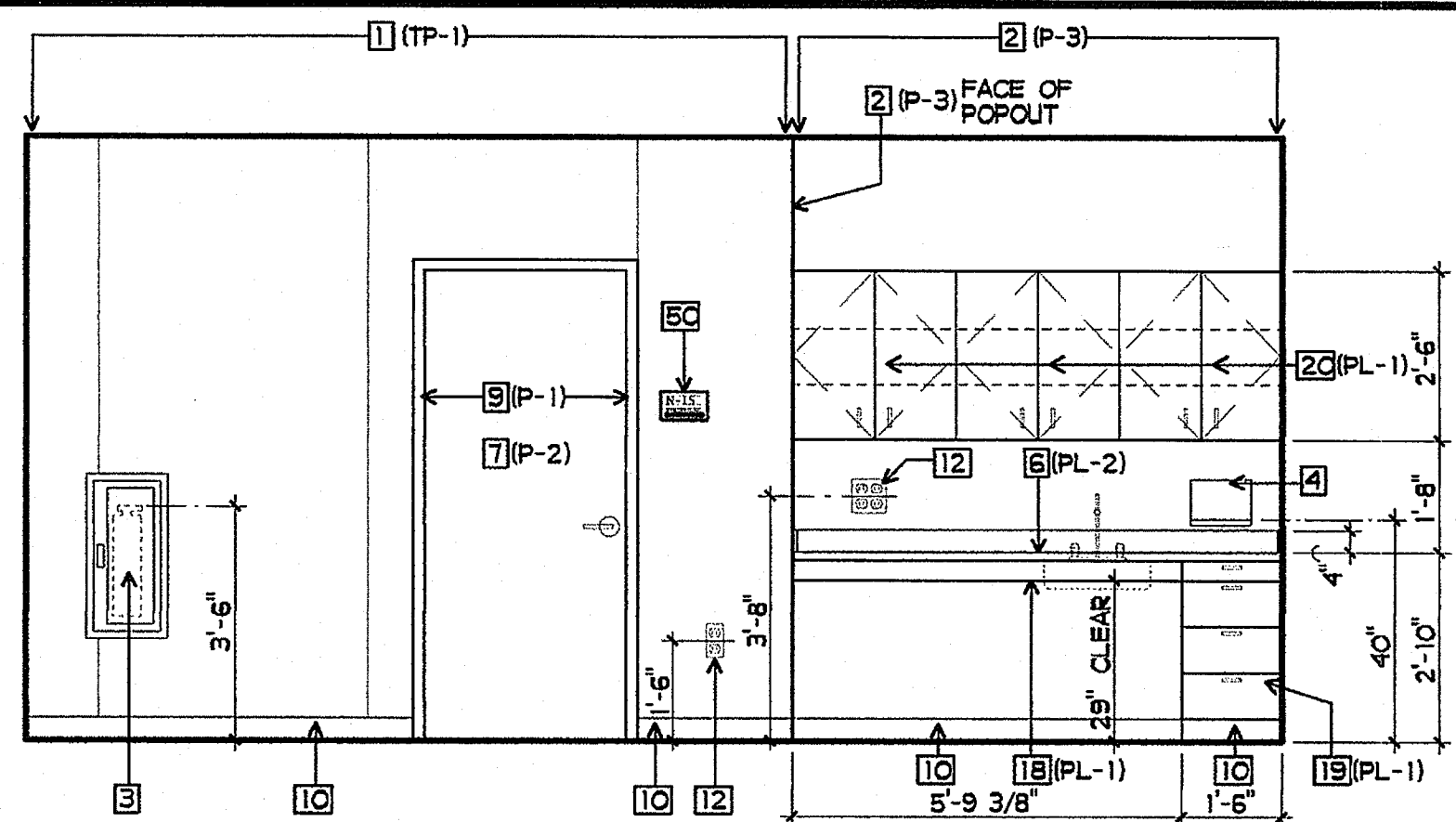
SHEET NOTES

- | | | |
|--|---|---|
| <p> 1 ACoustical CEILING
 2 GYP. BOARD CEILING - SEMI-GLOSS PAINT
 3 STUCCO SOFFIT - SEE 5/A9-3
 4 WRAPPED STRUCTURAL STEEL WITH STUCCO FINISH - SEE 3, 4, 5 & 6/A9-10
 5 EXPOSED STRUCTURAL STEEL - SEMI-GLOSS PAINT
 6 EXPOSED METAL DECKING - SEMI-GLOSS PAINT
 7 LIGHT FIXTURE - SEE ELECTRICAL DRAWINGS
 8 AIR DIFFUSER/GRILLE - SEE MECHANICAL DRAWINGS
 9 21" TUBULAR SKYLIGHT CEILING DIFFUSER AT METAL SHAKE ROOFING - SEE 1/A9-10 </p> | <p> 10 21" TUBULAR SKYLIGHT CEILING DIFFUSER AT BUILT-UP ROOF - SEE 2/A9-10
 11 PERFORATED ALUMINUM PANELS MOUNTED ON 1-1/2" HAT CHANNELS. STOP PANELS 4" FROM EDGE OF STRUCTURAL STEEL OR WALL - SEE 7/A9-14
 12 ROOF SCUTTLE - SEE 3/AN4-18
 13 EXHAUST FAN REGISTER - SEE MECHANICAL DRAWINGS
 14 SPEAKER - SEE ELECTRICAL DRAWINGS
 15 ILLUMINATED EXIT SIGN - SEE ELECTRICAL DRAWINGS
 16 CUBICLE CURTAIN TRACK - SEE 4/A9-4
 17 J-BOX W/ COVER </p> | <p> 18 SPEAKER - SEE ELECTRICAL DRAWINGS </p> |
|--|---|---|

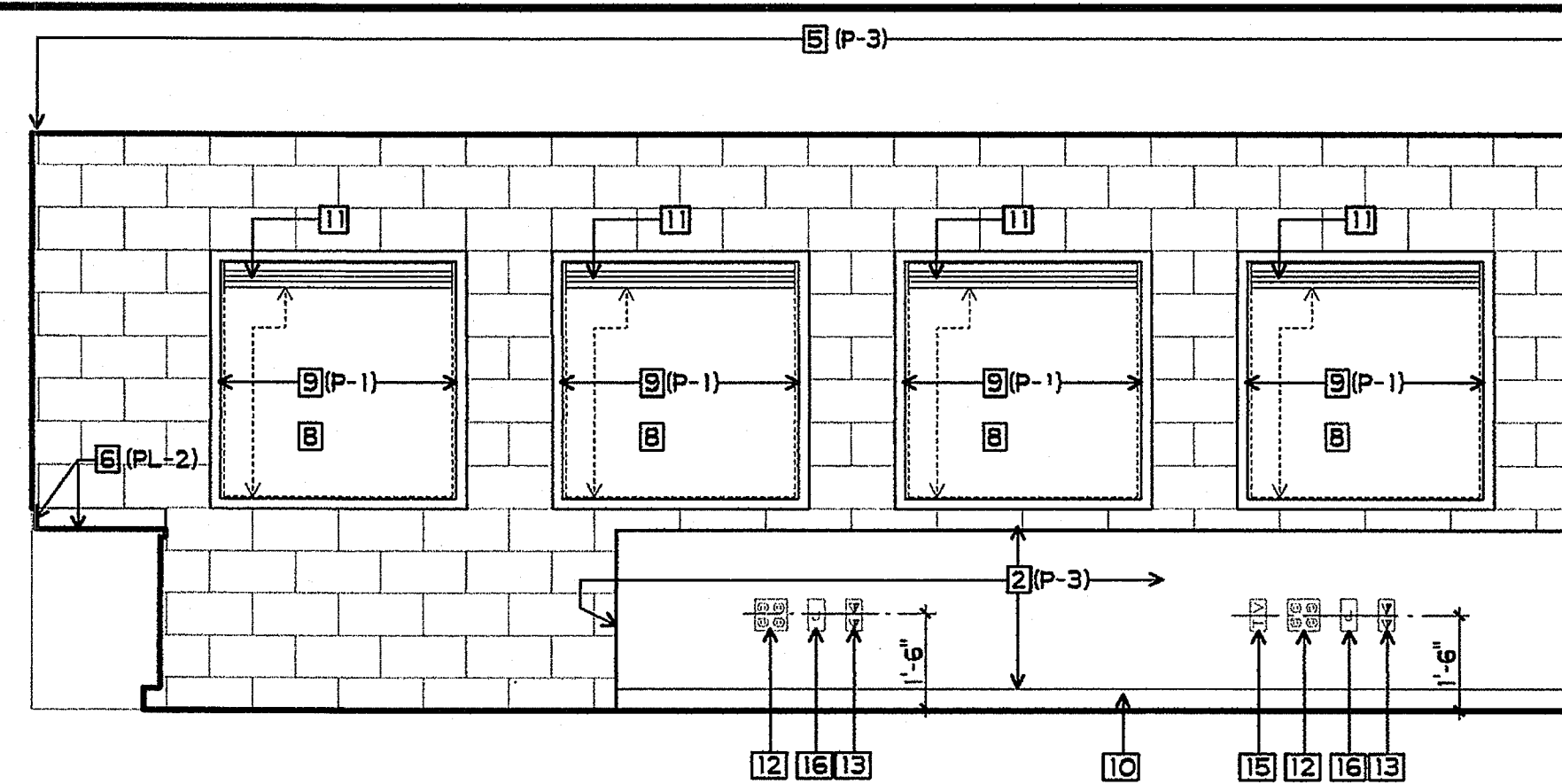




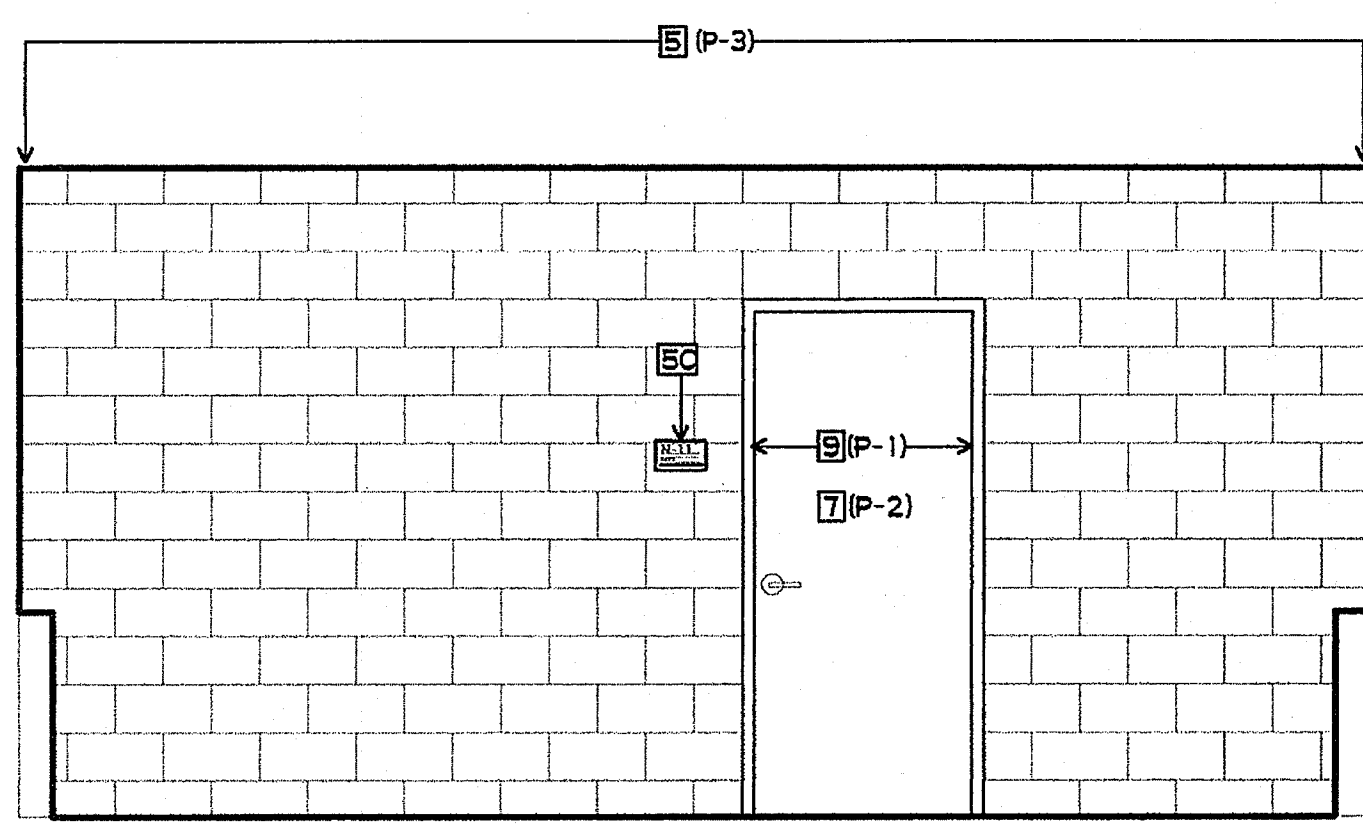
1 BOYS P.E. OFFICE
3/8" = 1'-0"



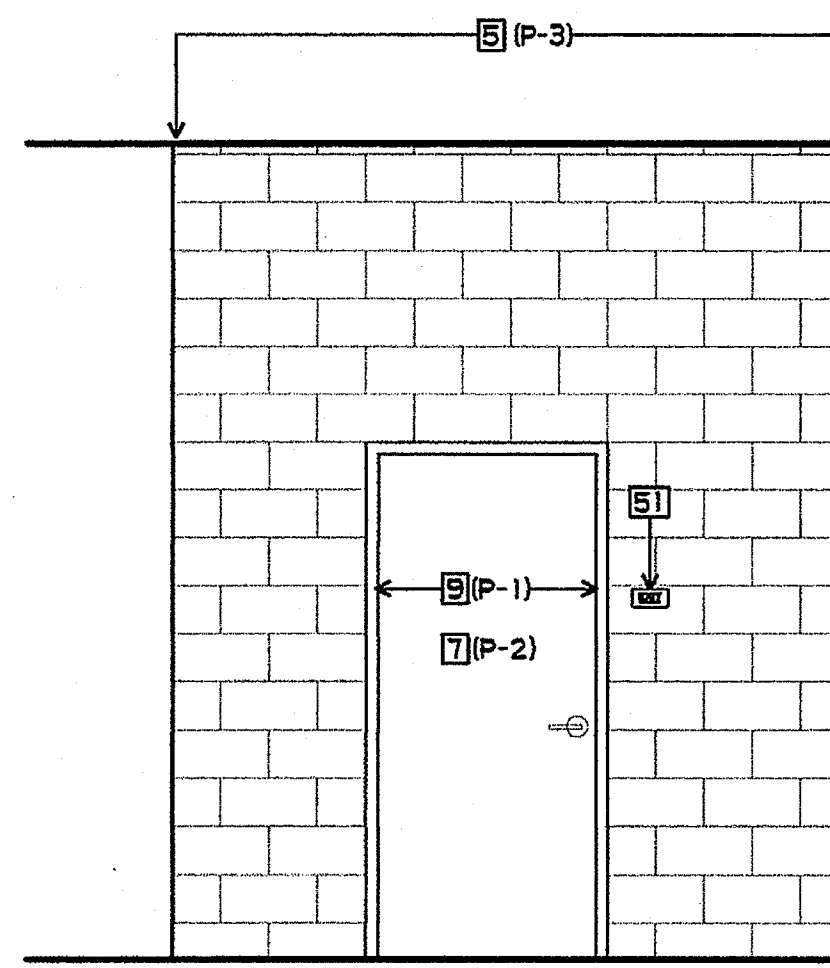
2 BOYS P.E. OFFICE
3/8" = 1'-0"



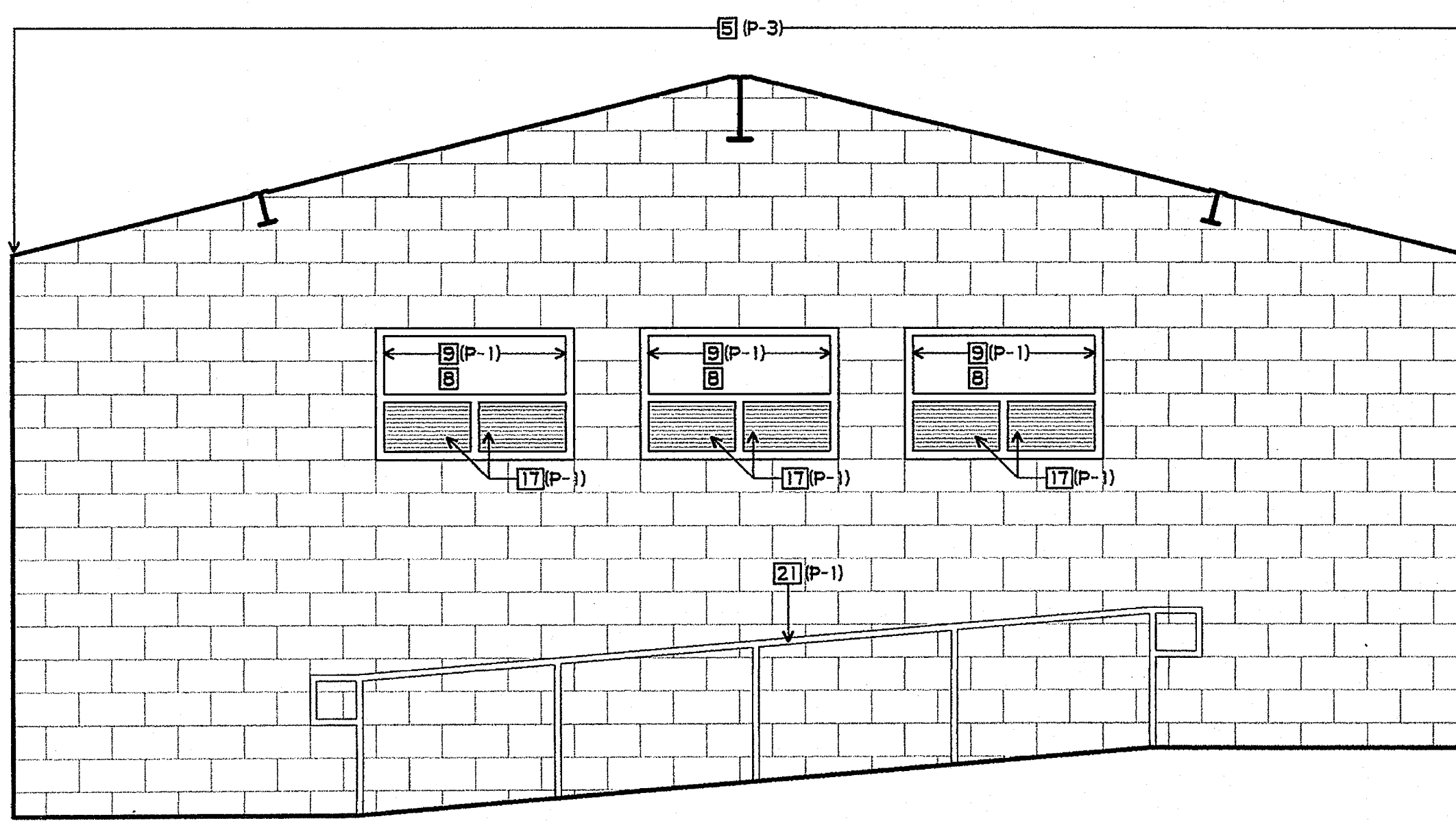
3 BOYS P.E. OFFICE
3/8" = 1'-0"



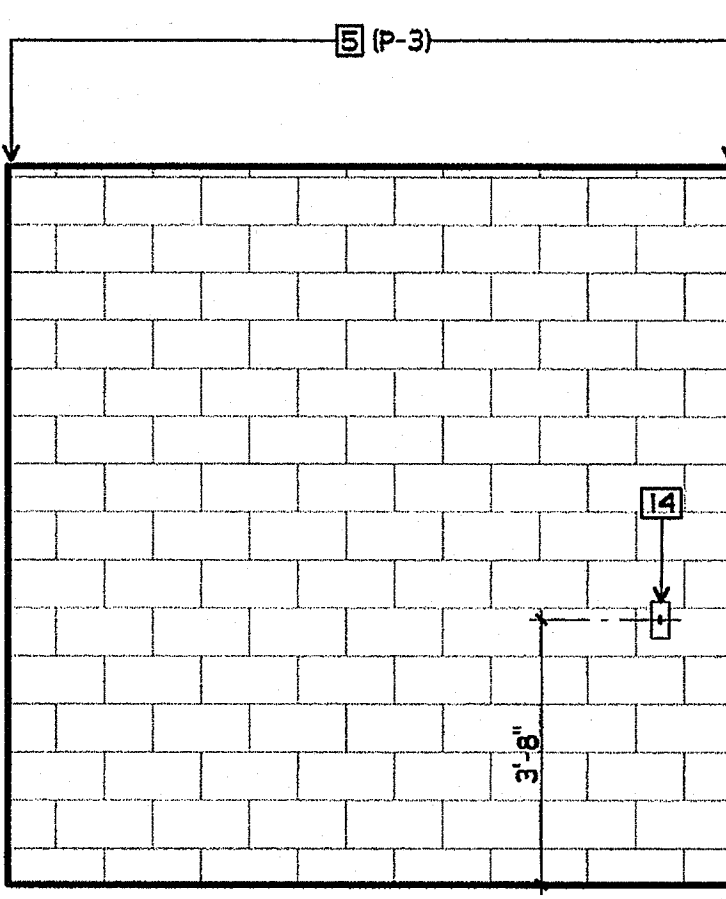
4 BOYS P.E. OFFICE
3/8" = 1'-0"



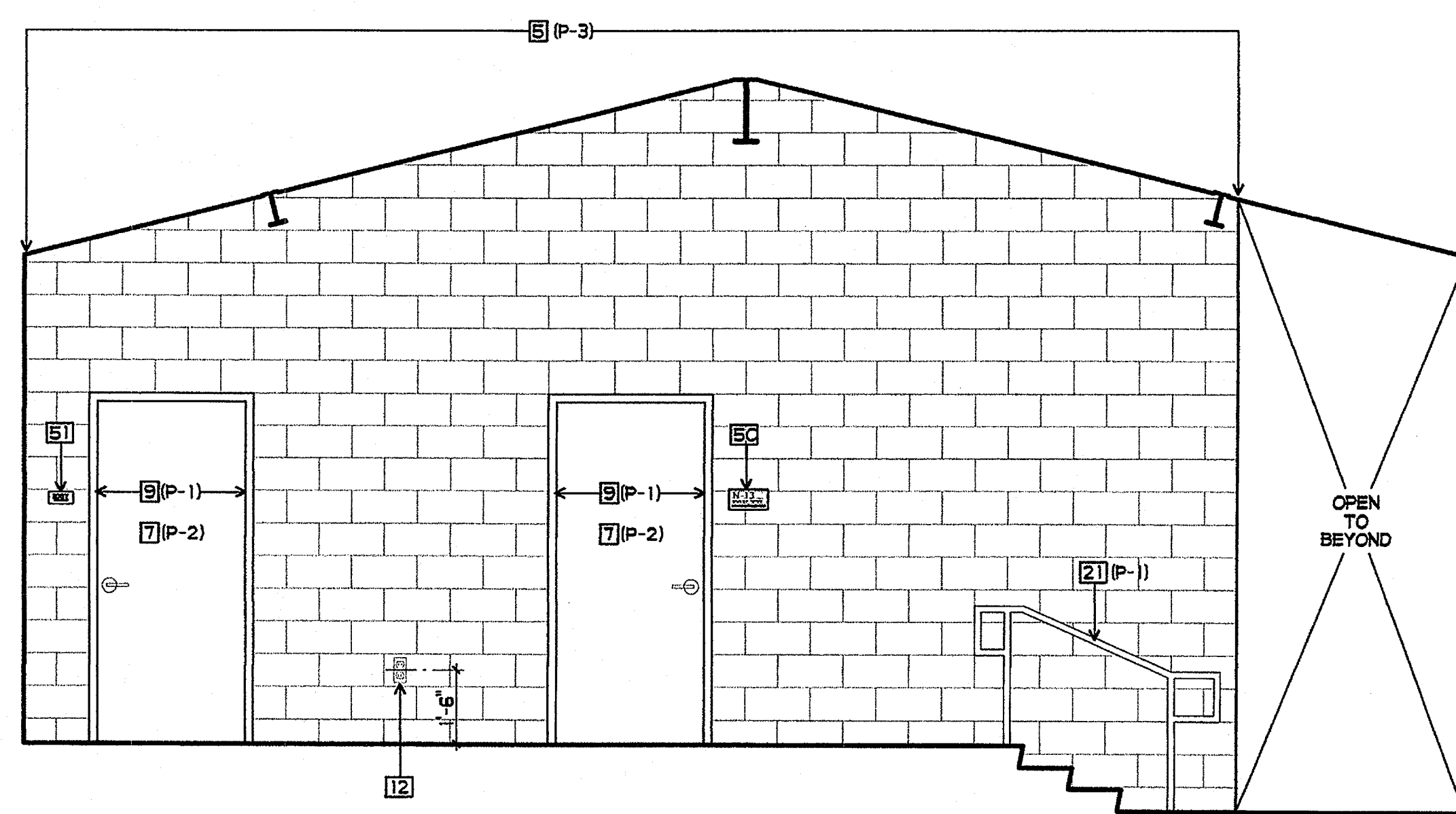
5 RAMP
3/8" = 1'-0"



6 RAMP
3/8" = 1'-0"



7 RAMP
3/8" = 1'-0"



8 RAMP
3/8" = 1'-0"

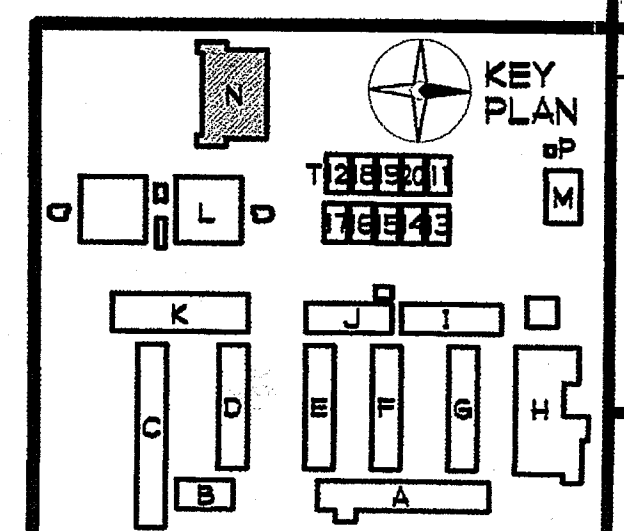
SHEET NOTES

- 1 TACKABLE WALL PANEL
- 2 5/8" GBX - ORANGE PEAL TEXT. - SEMI-GLOSS PAINT
- 3 FIRE EXTINGUISHER AND CABINET - SEE 3/A9-4
- 4 PAPER TOWEL DISPENSER
- 5 CMU - SEMI-GLOSS PAINT
- 6 PLASTIC LAMINATED BACKSPLASH AND/OR COUNTERTOP
- 7 DOOR - SEMI-GLOSS PAINT
- 8 VISION PANEL
- 9 METAL FRAME - SEMI-GLOSS PAINT
- 10 RESILIENT BASE
- 11 WINDOW SHADES
- 12 POWER OUTLET
- 13 DATA OUTLET
- 14 LIGHT CONTROLS
- 15 TV OUTLET
- 16 J-BOX W/ COVER

- 17 LOUVERS
- 18 ACCESSIBLE SINK BASE - SEE 11/A10-0
- 19 BASE CABINET W/ DRAWERS - SEE 13/A10-0
- 20 WALL CABINET, 12" DEEP - SEE 9/A10-1
- 21 RAILING - SEE 1/AN4-16 AND 2/AN4-16
- 22 DOOR SIGNAGE - SEE DOOR SCHEDULE
- 23 EXIT SIGN - SEE SIGNAGE PLAN AND 7/AS-8

FINISH PALETTE

- (TP-2) TACKABLE WALL PANEL, FABRIC COLOR #2
- (PL-1) PLASTIC LAMINATE, COLOR #2
- (PL-2) PLASTIC LAMINATE, COLOR #3
- (P-1) PAINT, COLOR #1
- (P-2) PAINT, COLOR #2
- (P-3) PAINT, COLOR #3

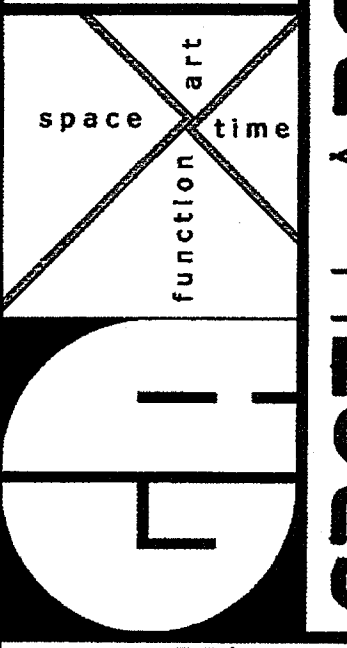


SHEET TITLE
BUILDING N
INTERIOR
ELEVATIONS

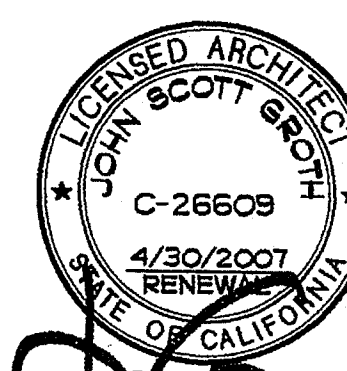
AN7-0

PLOTTED 3/18/2005 1:09 PM
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PROJECT NOS. 025
P. T. N. 73569-9
DATE

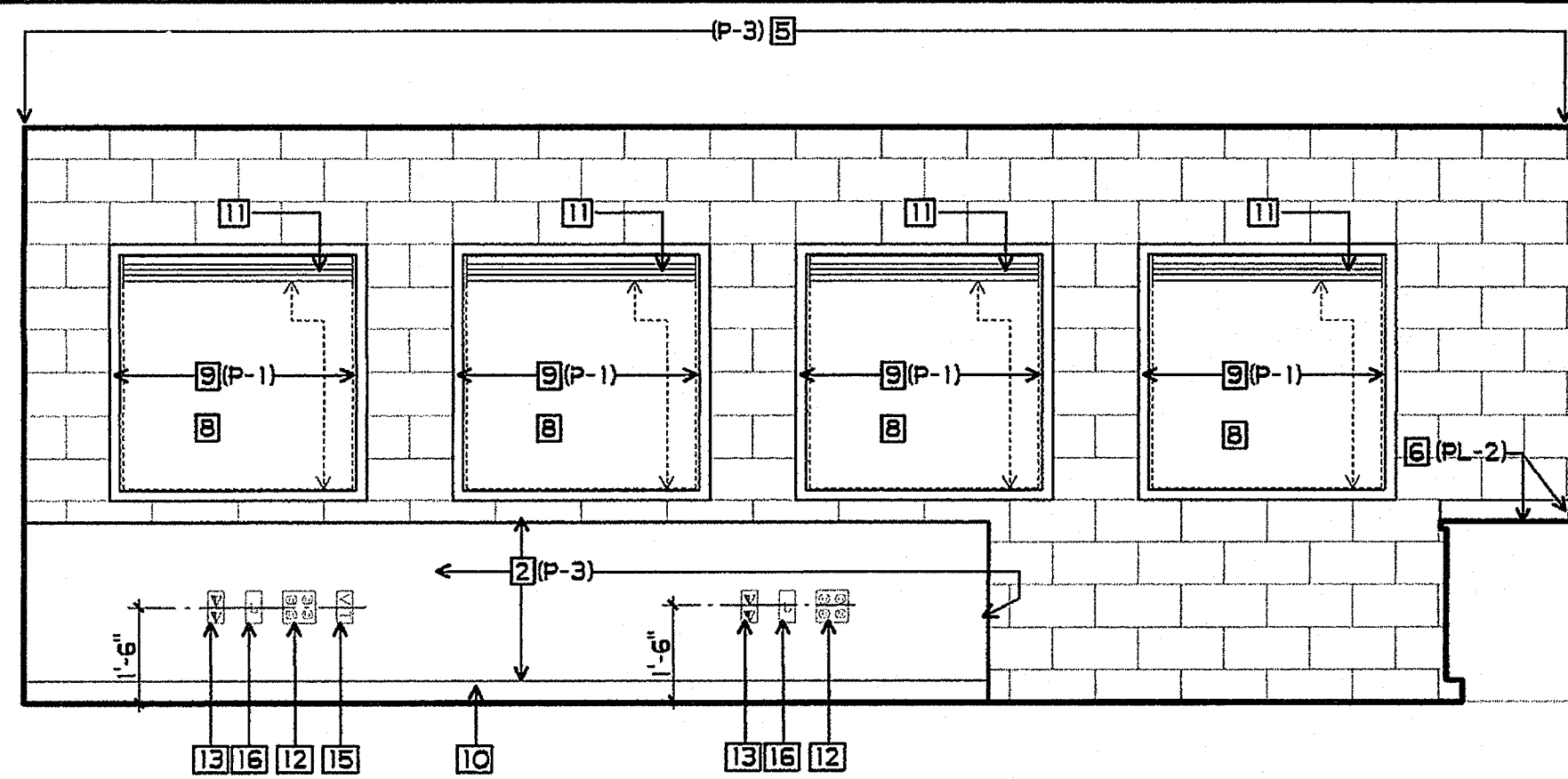
JEFFERSON MS NEW CONSTRUCTION
823 ACACIA STREET
OCEANSIDE, CA 92054
OCEANSIDE UNIFIED S.D.



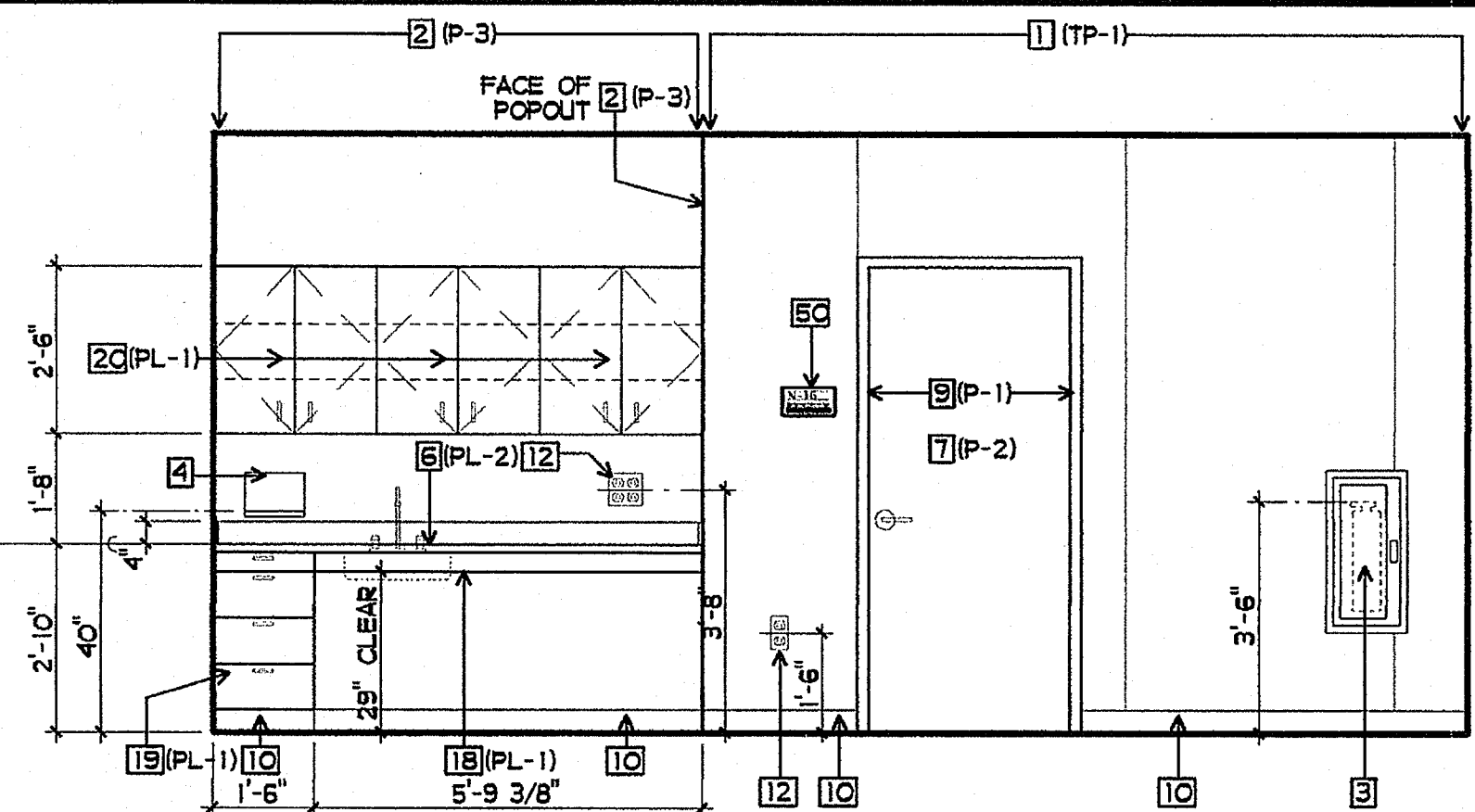
DSA
IDENTIFICATION STAMP
DIV. OF THE STATE ARCHITECT
OFFICE OF REGULATION SERVICES
4-106494
AC. PLS. SS.
DATE MAR 28 2005



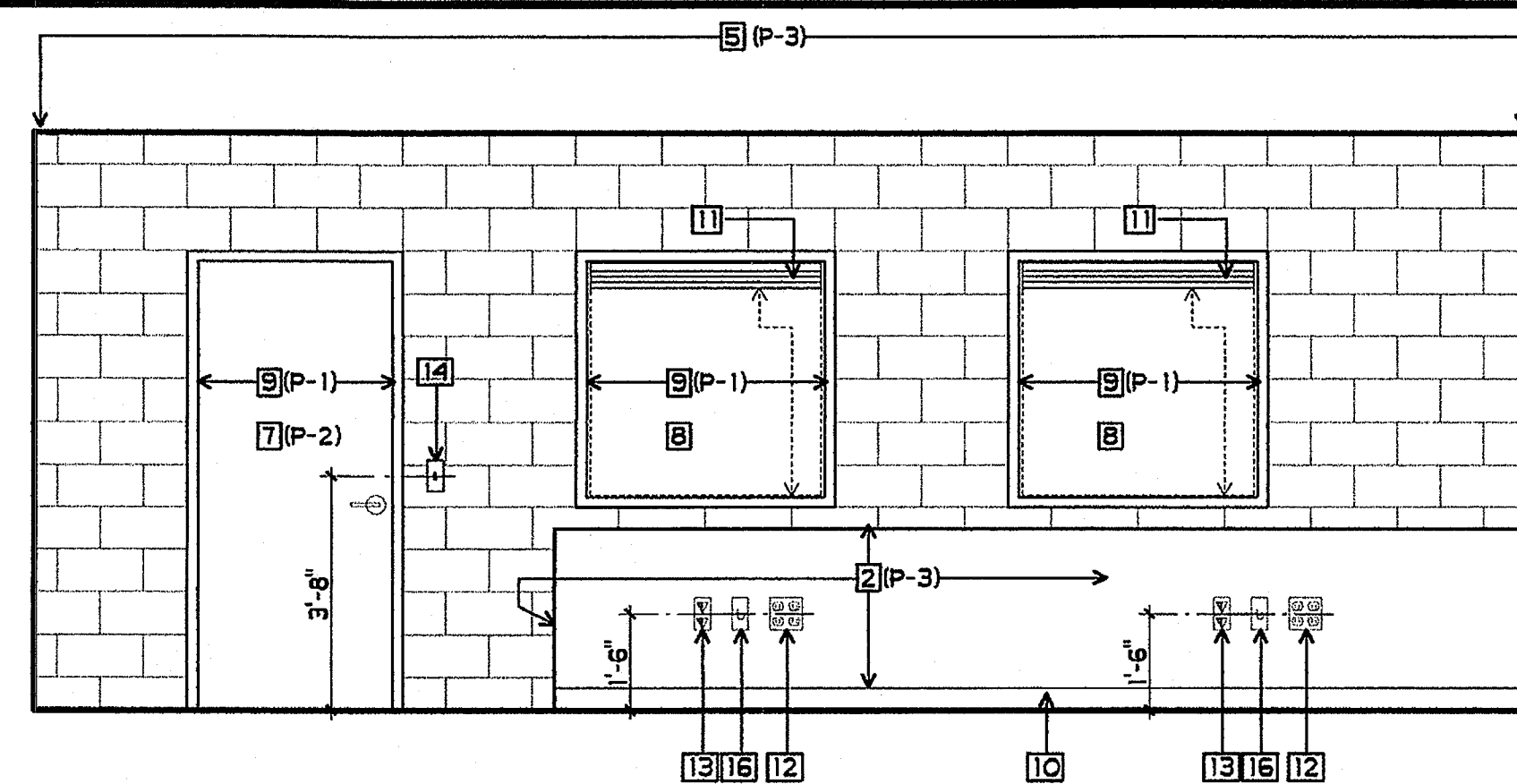
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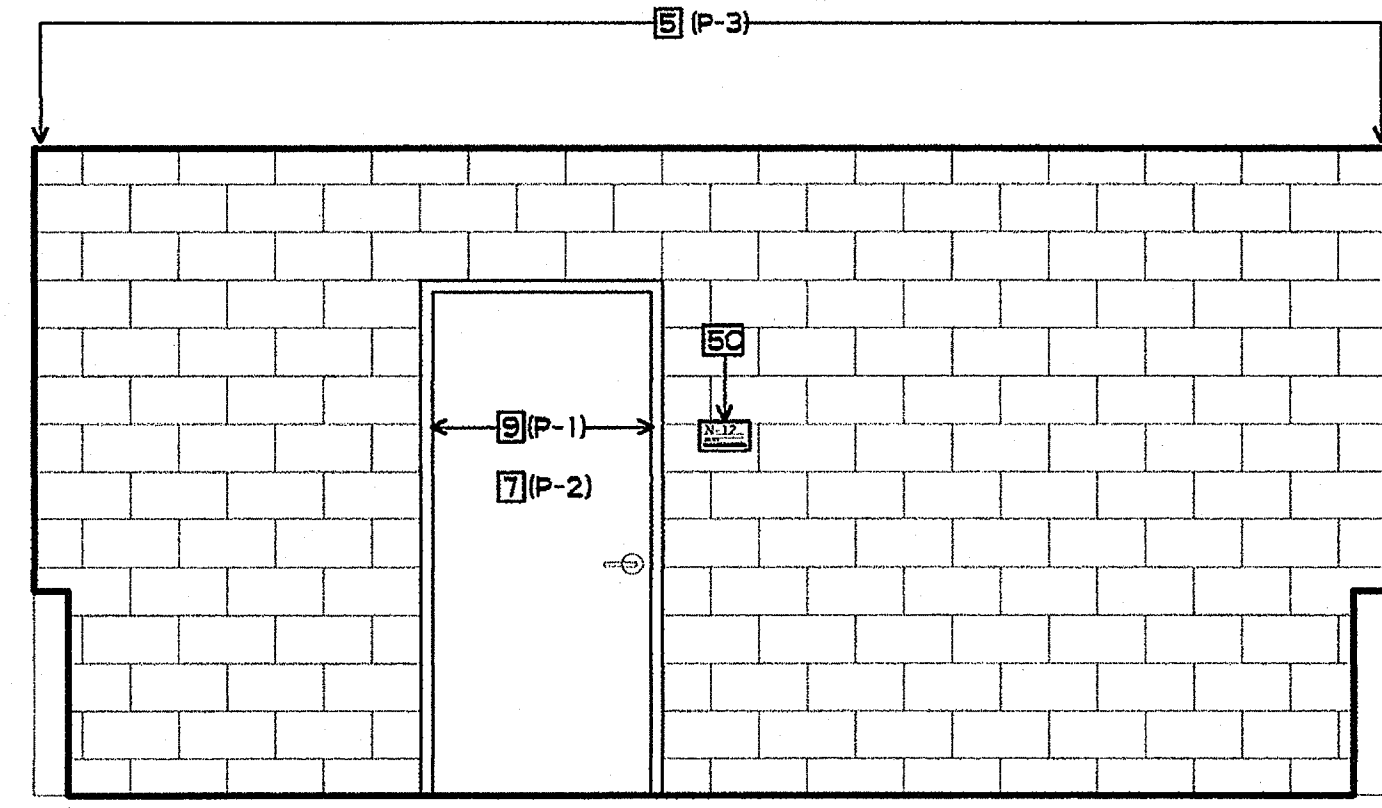
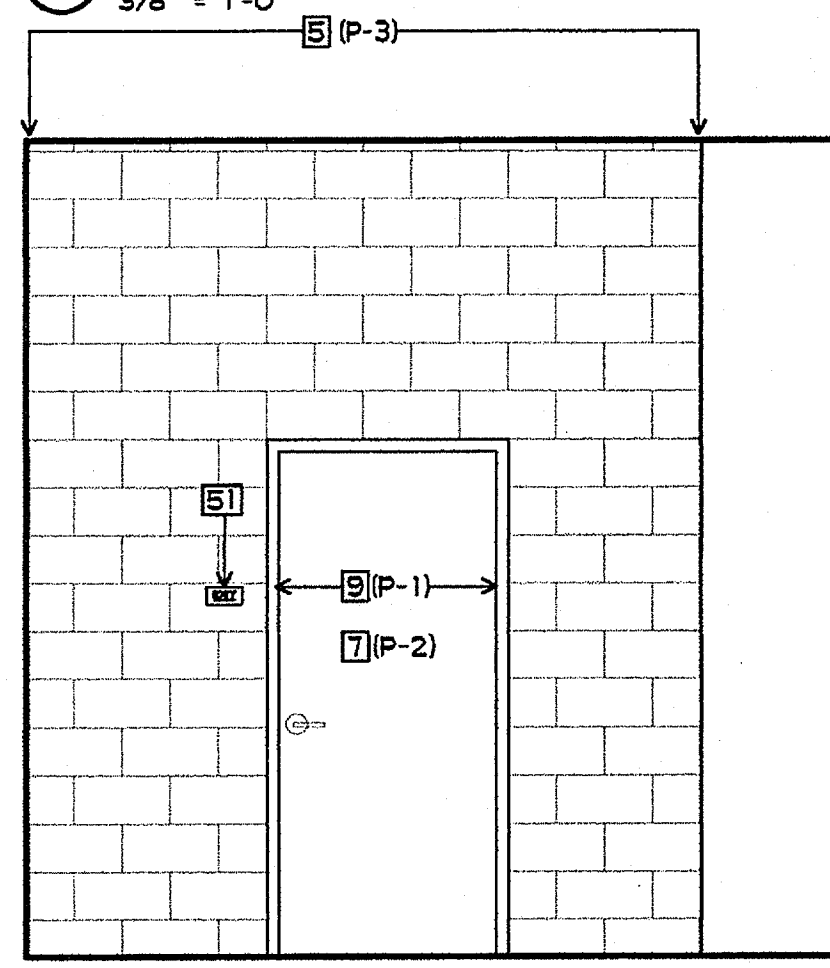
9 GIRLS P.E. OFFICE
3/8" = 1'-0"



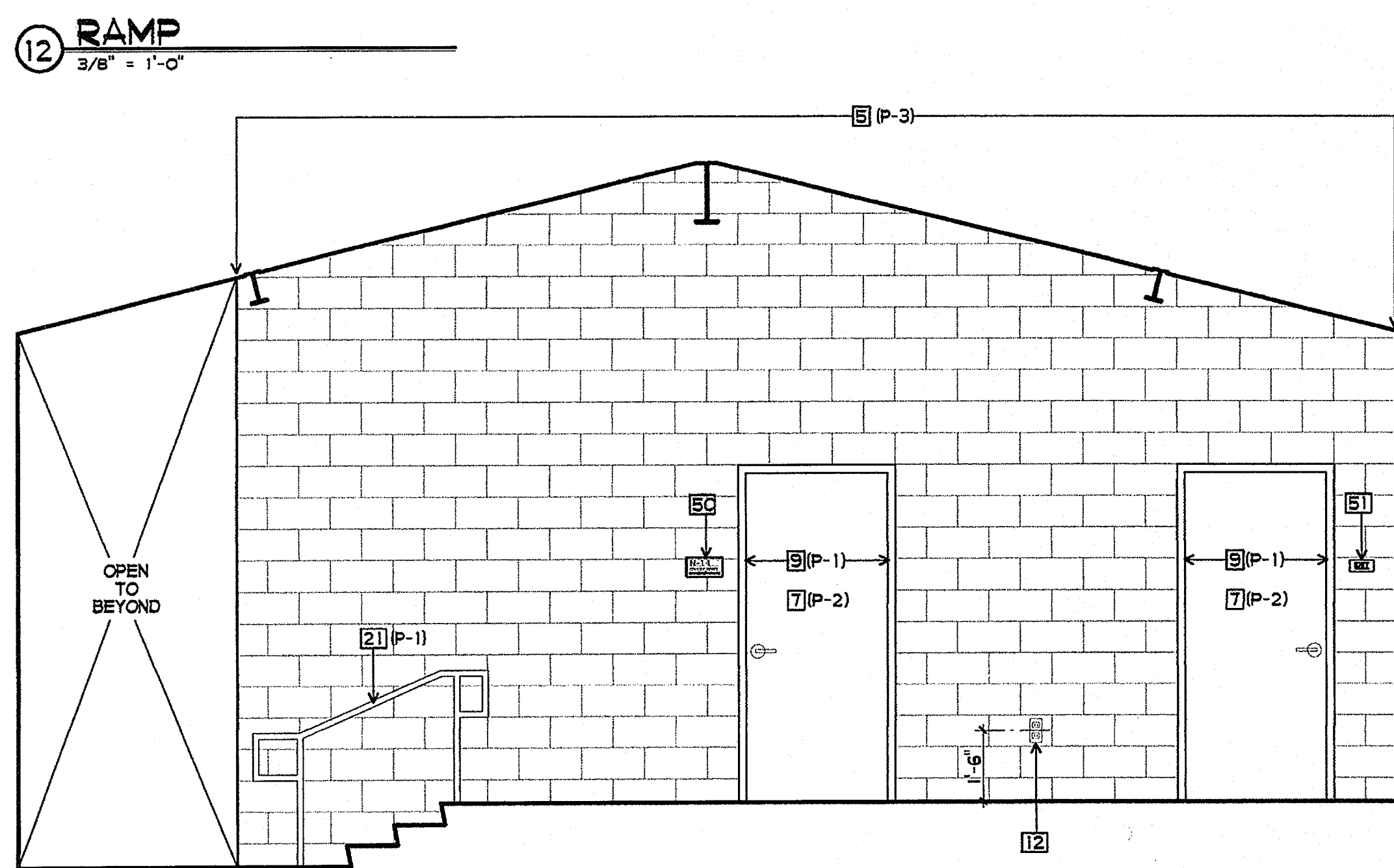
10 GIRLS P.E. OFFICE
3/8" = 1'-0"



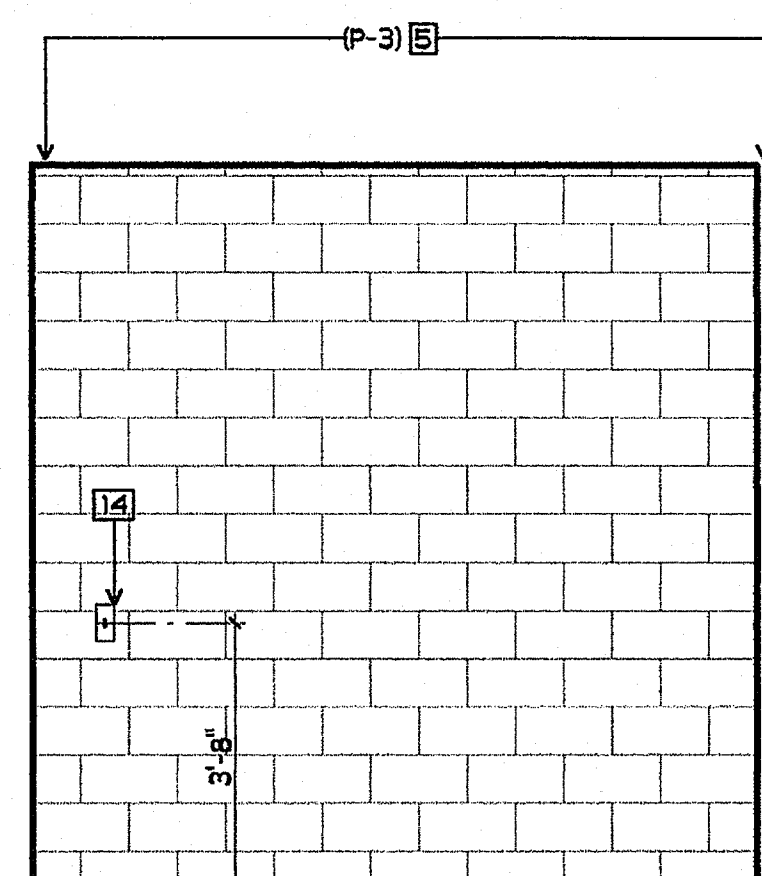
11 GIRLS P.E. OFFICE
3/8" = 1'-0"



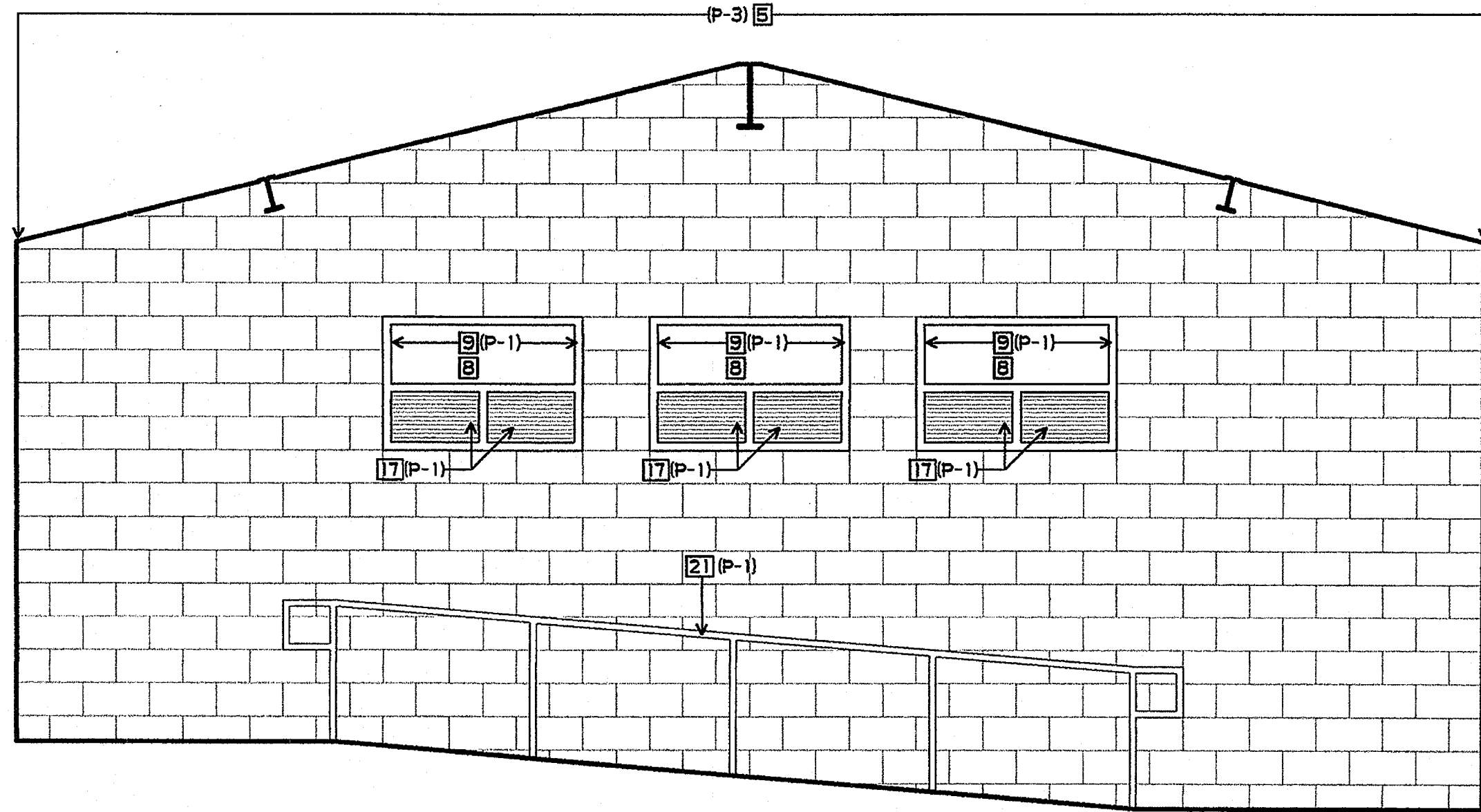
13 GIRLS P.E. OFFICE
3/8" = 1'-0"



14 RAMP
3/8" = 1'-0"



15 RAMP
3/8" = 1'-0"



16 RAMP
3/8" = 1'-0"

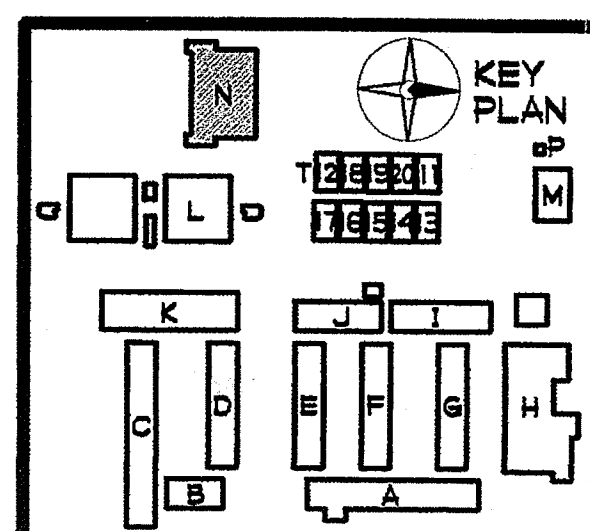
SHEET NOTES

- 1 TACKABLE WALL PANEL
- 2 5/8" GBX - ORANGE PEAL TEXT. - SEMI-GLOSS PAINT
- 3 FIRE EXTINGUISHER AND CABINET - SEE 3/A9-4
- 4 PAPER TOWEL DISPENSER
- 5 CMLU - SEMI-GLOSS PAINT
- 6 PLASTIC LAMINATED BACKSPLASH AND/OR COUNTERTOP
- 7 DOOR - SEMI-GLOSS PAINT
- 8 VISION PANEL
- 9 METAL FRAME - SEMI-GLOSS PAINT
- 10 RESILIENT BASE
- 11 WINDOW SHADES
- 12 POWER OUTLET
- 13 DATA OUTLET
- 14 LIGHT CONTROLS
- 15 TV OUTLET
- 16 J-BOX W/ COVER

- 17 LOUVERS
- 18 ACCESSIBLE SINK BASE - SEE 11/A10-0
- 19 BASE CABINET W/ DRAWERS - SEE 13/A10-0
- 20 WALL CABINET, 12" DEEP - SEE 9/A10-1
- 21 RAILING - SEE 1/AM-16 AND 2/AM-16
- 50 DOOR SIGNAGE - SEE DOOR SCHEDULE
- 51 EXIT SIGN - SEE SIGNAGE PLAN AND 7/A9-8

FINISH PALETTE

- (TP-2) TACKABLE WALL PANEL, FABRIC COLOR #2
- (PL-1) PLASTIC LAMINATE, COLOR #2
- (PL-2) PLASTIC LAMINATE, COLOR #3
- (P-1) PAINT, COLOR #1
- (P-2) PAINT, COLOR #2
- (P-3) PAINT, COLOR #3



PLOTTED 3/18/2005 12:58 PM

GROTH ARCHITECTS, INC.
 823 ACACIA STREET
 OCEANSIDE, CA 92054
 OCEANSIDE UNIFIED S.D.

CLUSD NO.
758-000

PROJECT NOS.
025

P. T. N.
73569-9

DATE

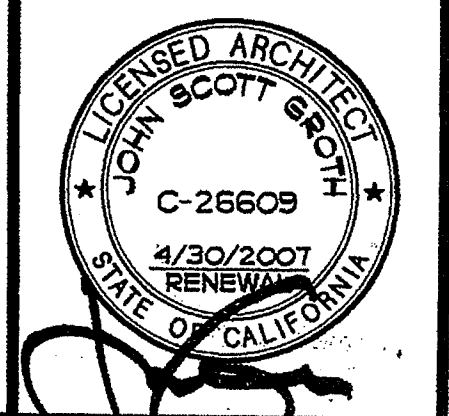
REVISIONS

JEFFERSON MS NEW CONSTRUCTION
 823 ACACIA STREET
 OCEANSIDE, CA 92054
 OCEANSIDE UNIFIED S.D.

space art
function time

GROTH ARCHITECTS, INC.
 3355 MISSION AVE. SUITE 234
 OCEANSIDE, CALIFORNIA 92054
 PHONE 760-754-8191
 FAX 760-754-8291

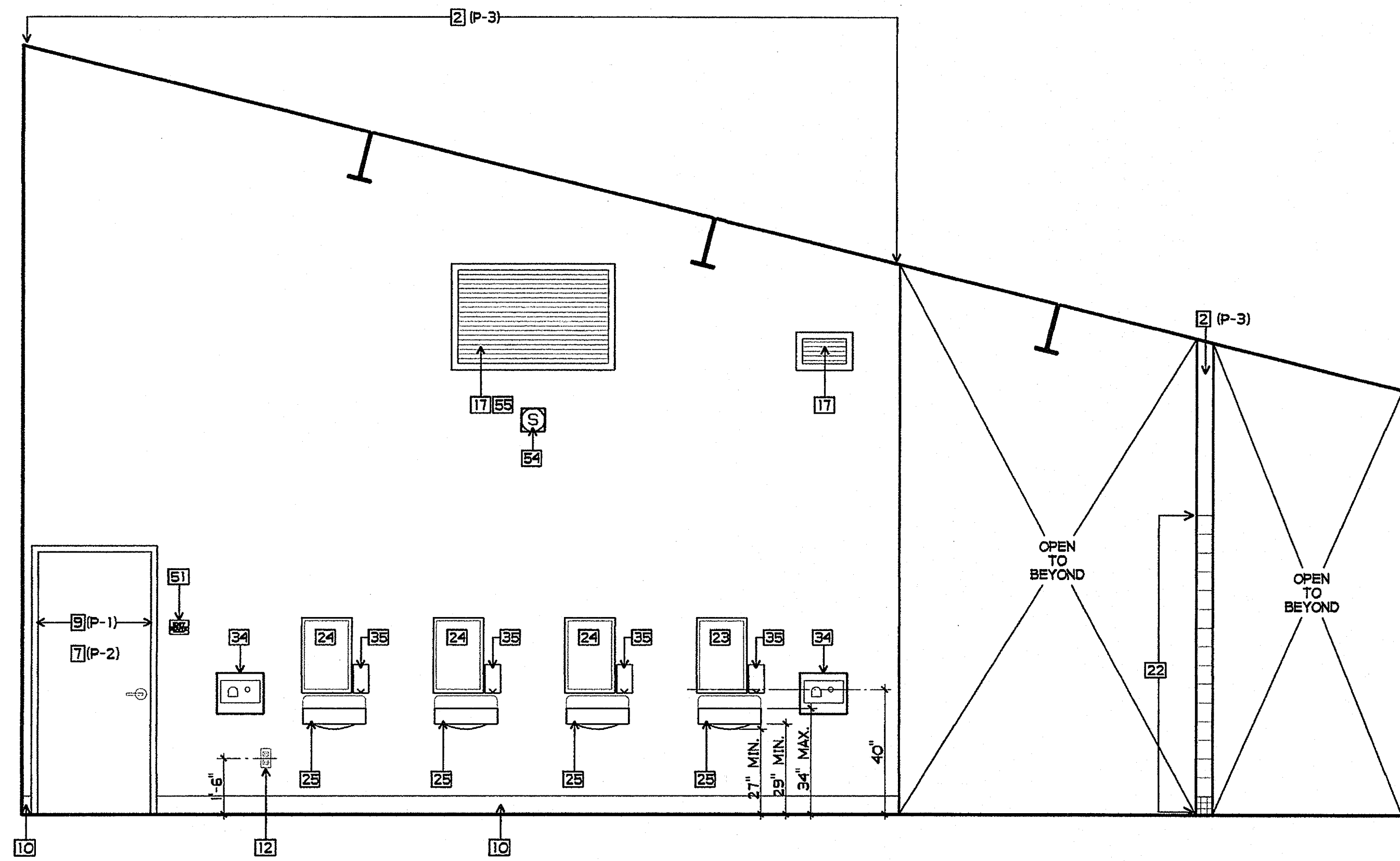
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 DIV. OF THE STATE ARCHITECT
 OFFICE OF REGULATION SERVICES
 4-106494
 AC: [Signature] FL: [Signature] SS: [Signature]
 DATE: MAR 28 2005



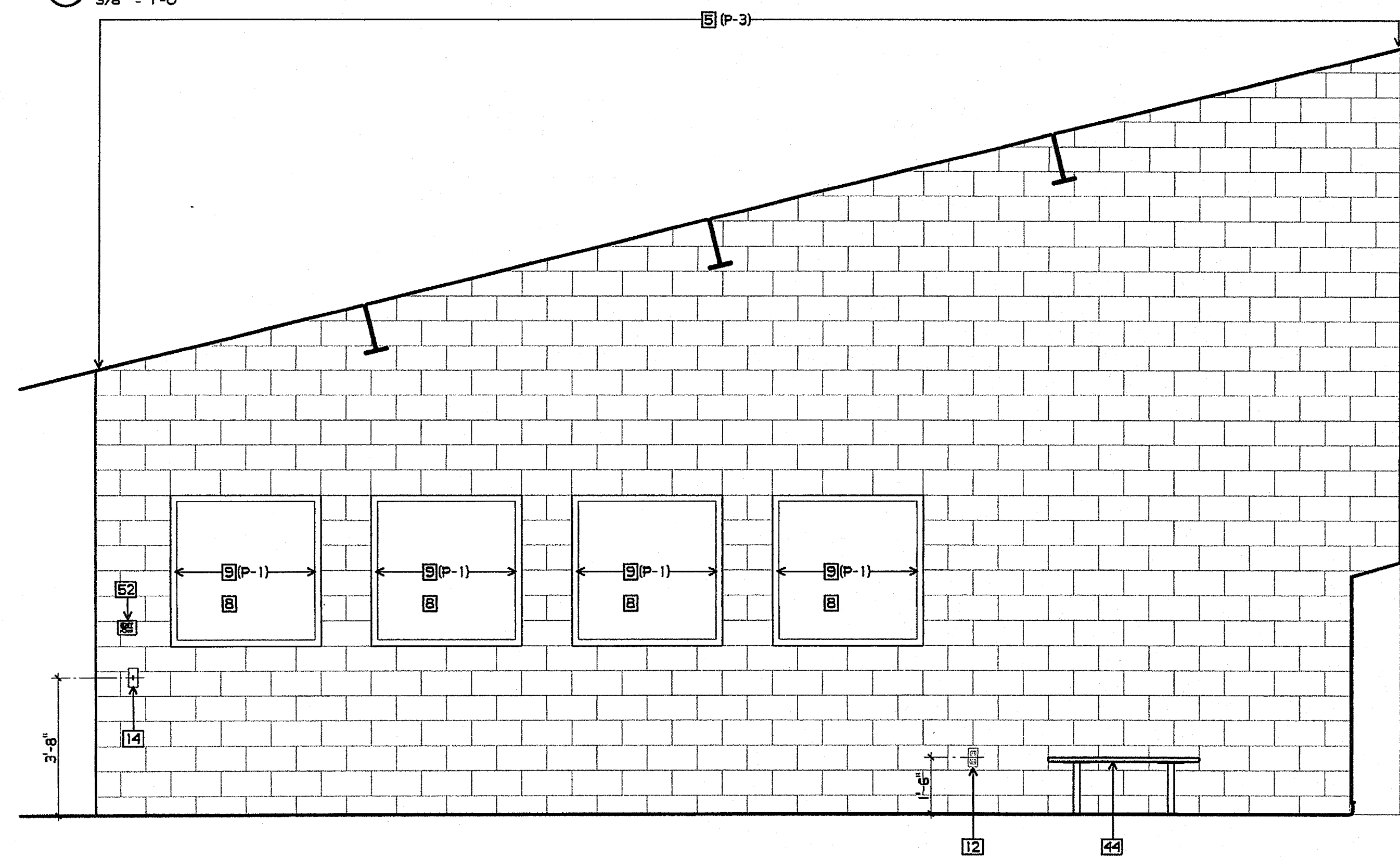
SHEET TITLE

BUILDING N
 INTERIOR
 ELEVATIONS

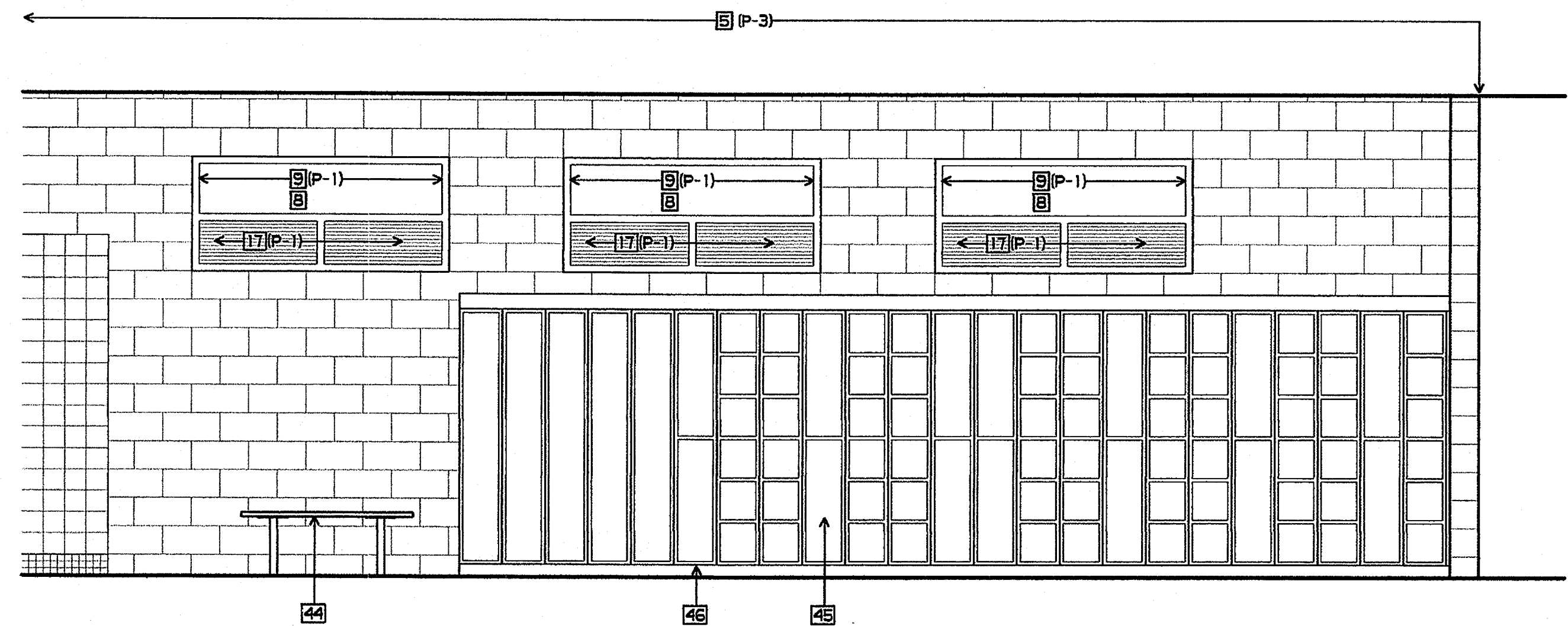
AN7-1



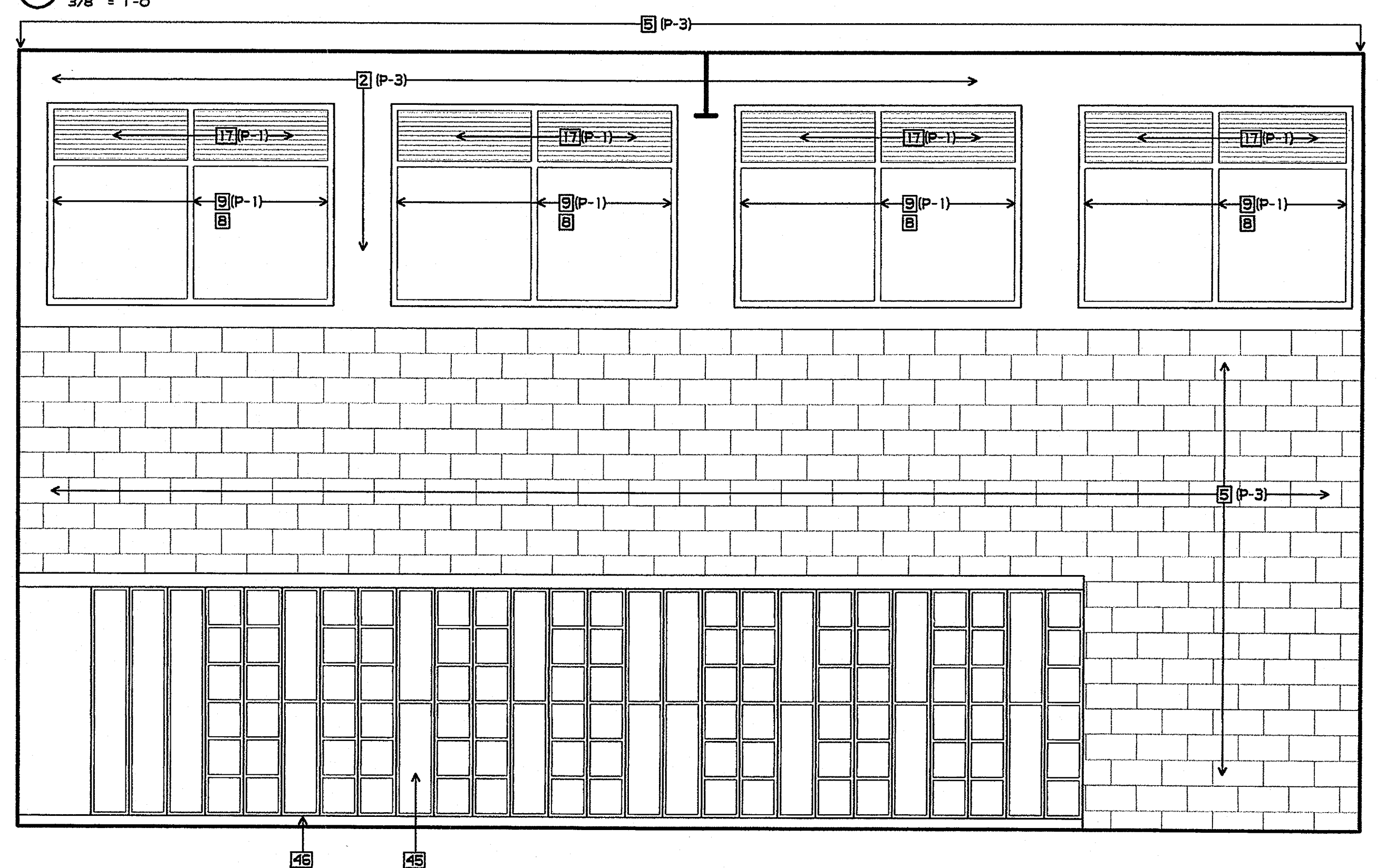
17 BOYS DRESSING ROOM
3/8" = 1'-0"



19 BOYS DRESSING ROOM
3/8" = 1'-0"



18 BOYS DRESSING ROOM
3/8" = 1'-0"



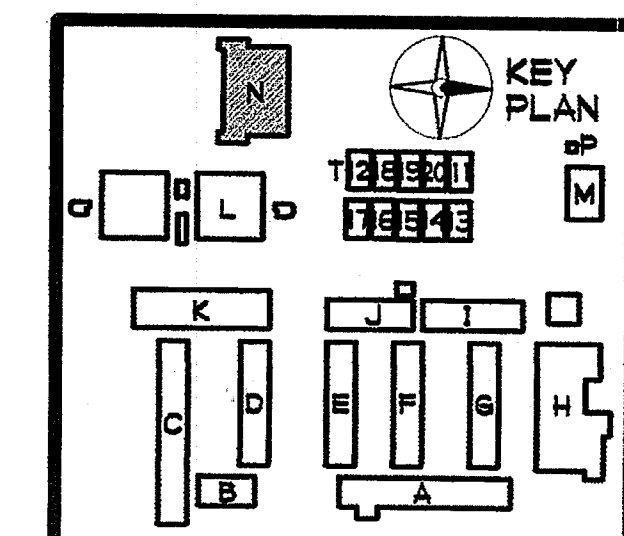
20 BOYS DRESSING ROOM
3/8" = 1'-0"

SHEET NOTES

- | | | |
|---|--|--|
| 2 5/8" GEX - ORANGE PEAL TEXT. - SEMI-GLOSS PAINT | 17 LOUVERS - SEE MECHANICAL DRAWINGS | 24 BENCH |
| 5 CMU - SEMI-GLOSS PAINT | 22 CERAMIC TILE - PATTERN PER 29/AN7-4 | 45 LOCKERS |
| 7 DOOR - SEMI-GLOSS PAINT | 23 ACCESSIBLE MIRROR 16" X 24" | 46 CONCRETE CURB |
| 8 VISION PANEL | 24 MIRROR 16" X 24" | 51 EXIT SIGN - SEE SIGNAGE PLAN AND 7/A9-8 |
| 9 METAL FRAME - SEMI-GLOSS PAINT | 25 ACCESSIBLE LAVATORY | 52 DIRECTIONAL EXIT SIGN - SEE SIGNAGE PLAN AND 4/A9-8 |
| 10 RESILIENT BASE | 34 RECESSED HAND DRYER | 54 SPEAKER - SEE ELECTRICAL DRAWINGS |
| 12 POWER OUTLET | 35 SOAP DISPENSER - C TO BE 1-1/2" FROM EDGE OF LAVATORY | 55 WALL OPENING FRAMING - SEE STRUCTURAL DRAWINGS |

FINISH PALETTE

- (P-1) PAINT, COLOR #1
(P-2) PAINT, COLOR #2
(P-3) PAINT, COLOR #3



PLOTTED 3/18/2005 12:55 PM

GROTH ARCHITECTS, INC.
823 ACACIA STREET
OCEANSIDE, CA 92054
PHONE 760-754-8191
FAX 760-754-8291

PROJECT NO. 025
DATE

REVISIONS

JEFFERSON MS NEW CONSTRUCTION
3355 MISSION AVE. SUITE 234
OCEANSIDE, CALIFORNIA 92054

GROTH ARCHITECTS, INC. OCEANSIDE UNIFIED S.D.

DSA
IDENTIFICATION STAMP
DIV. OF THE STATE ARCHITECT
OFFICE OF REGULATION SERVICES
4-106494
AC: PLS FL: SS: Z
DATE MAR 28 2005

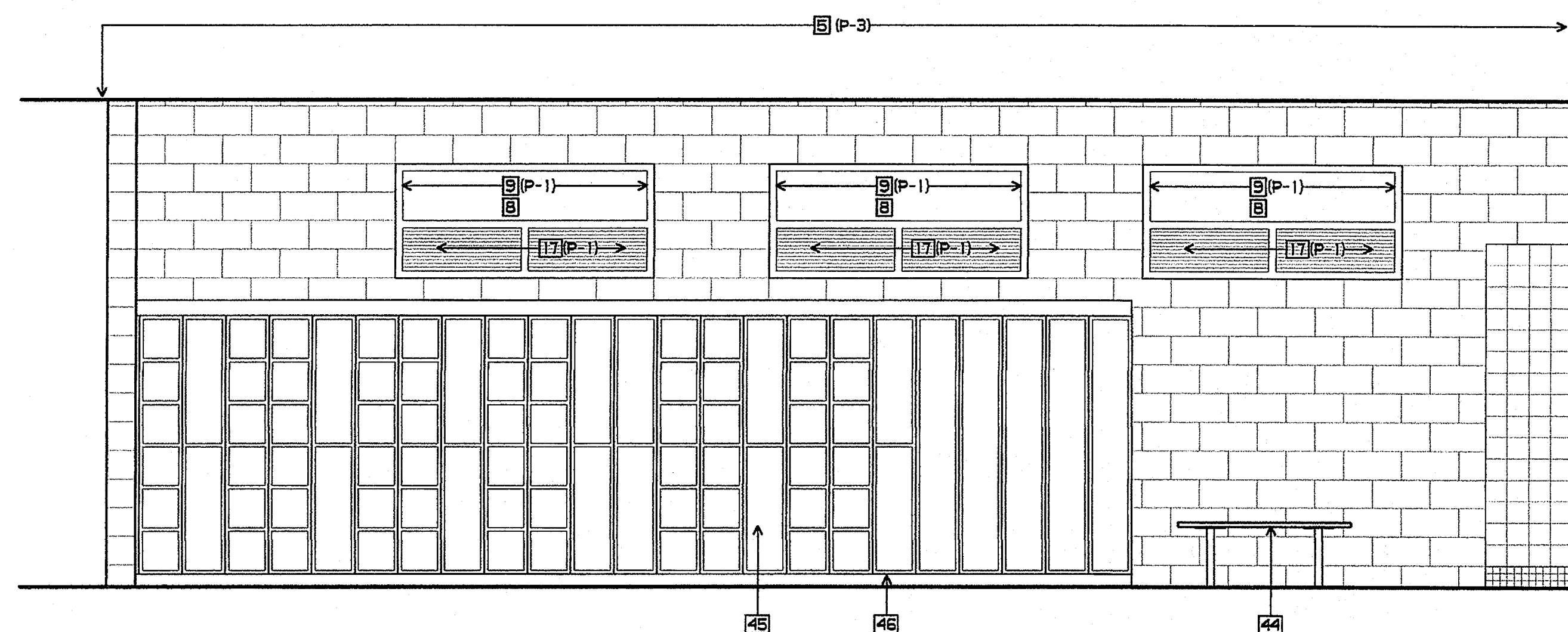
LICENSED ARCHITECT
JOHN SCOTT GROTH
C-26609
4/30/2007
RENEWAL

SHEET TITLE

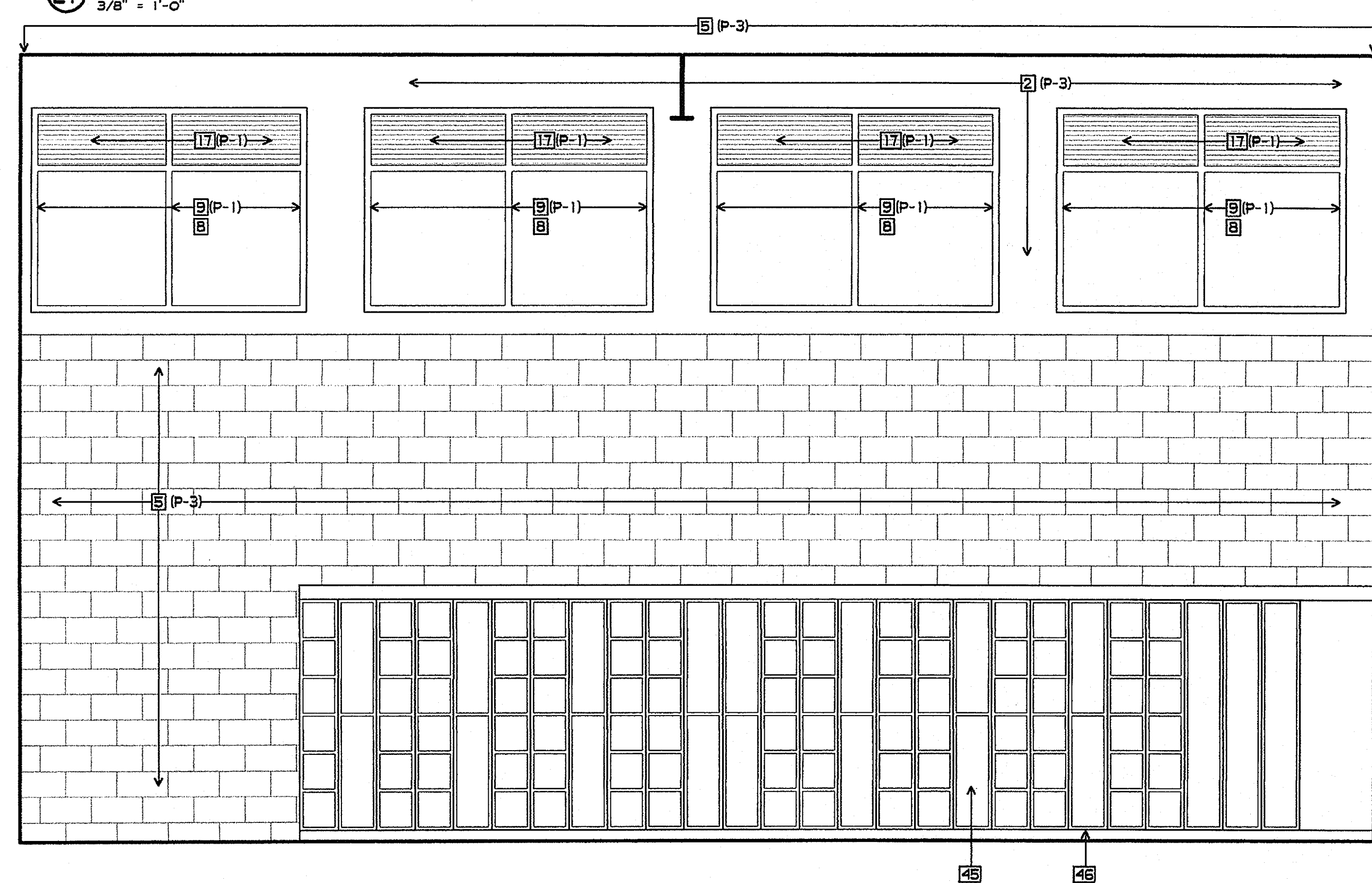
BUILDING N
INTERIOR
ELEVATIONS

AN7-2

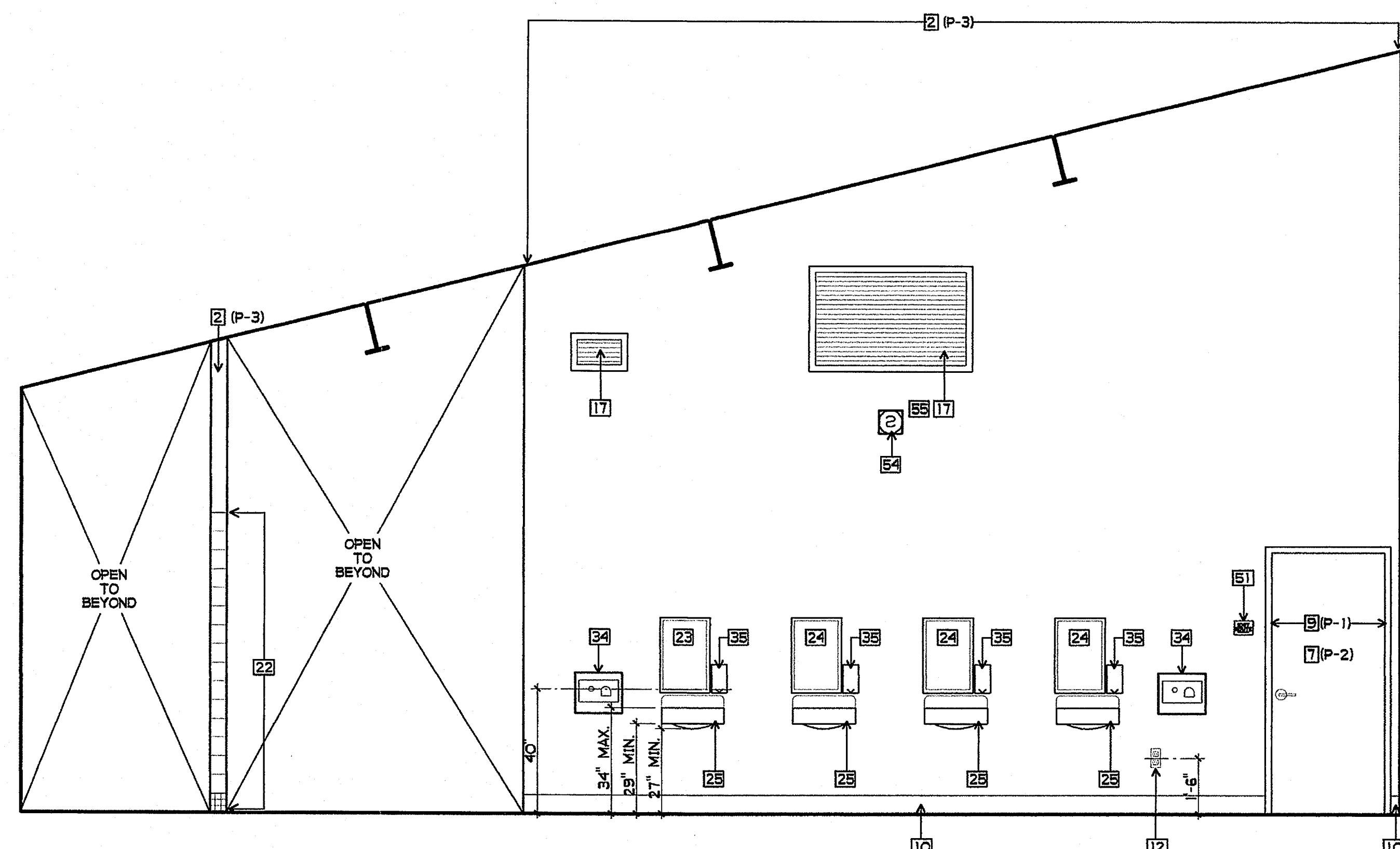
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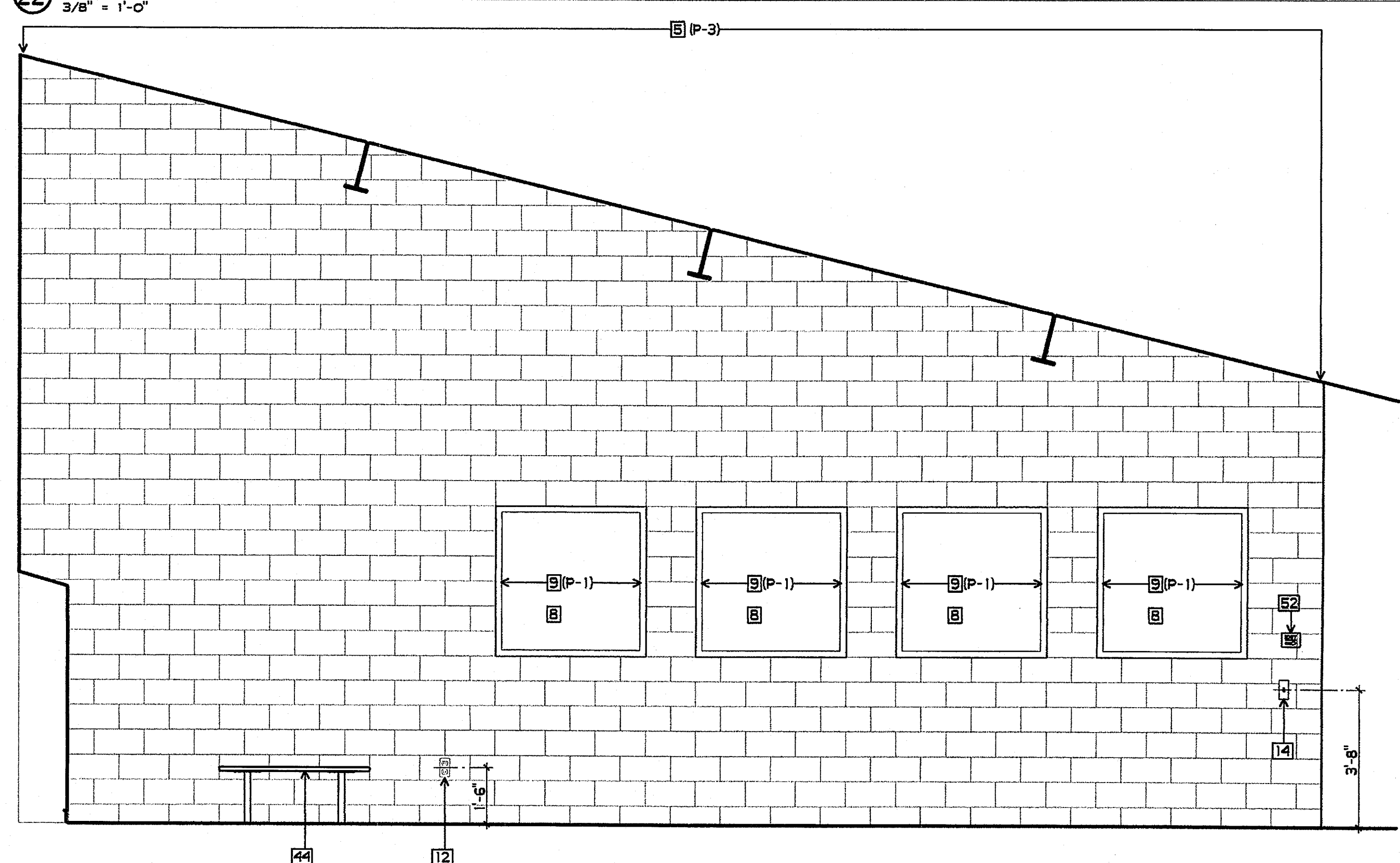
21 GIRLS DRESSING ROOM
 3/8" = 1'-0"



23 GIRLS DRESSING ROOM
 3/8" = 1'-0"



22 GIRLS DRESSING ROOM
 3/8" = 1'-0"



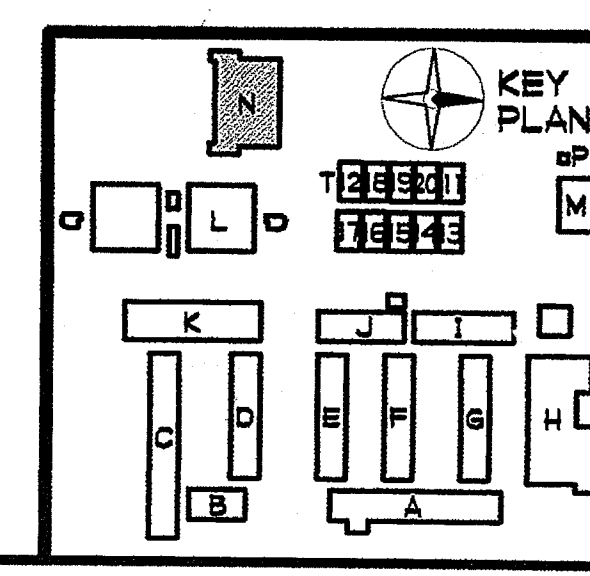
24 GIRLS DRESSING ROOM
 3/8" = 1'-0"

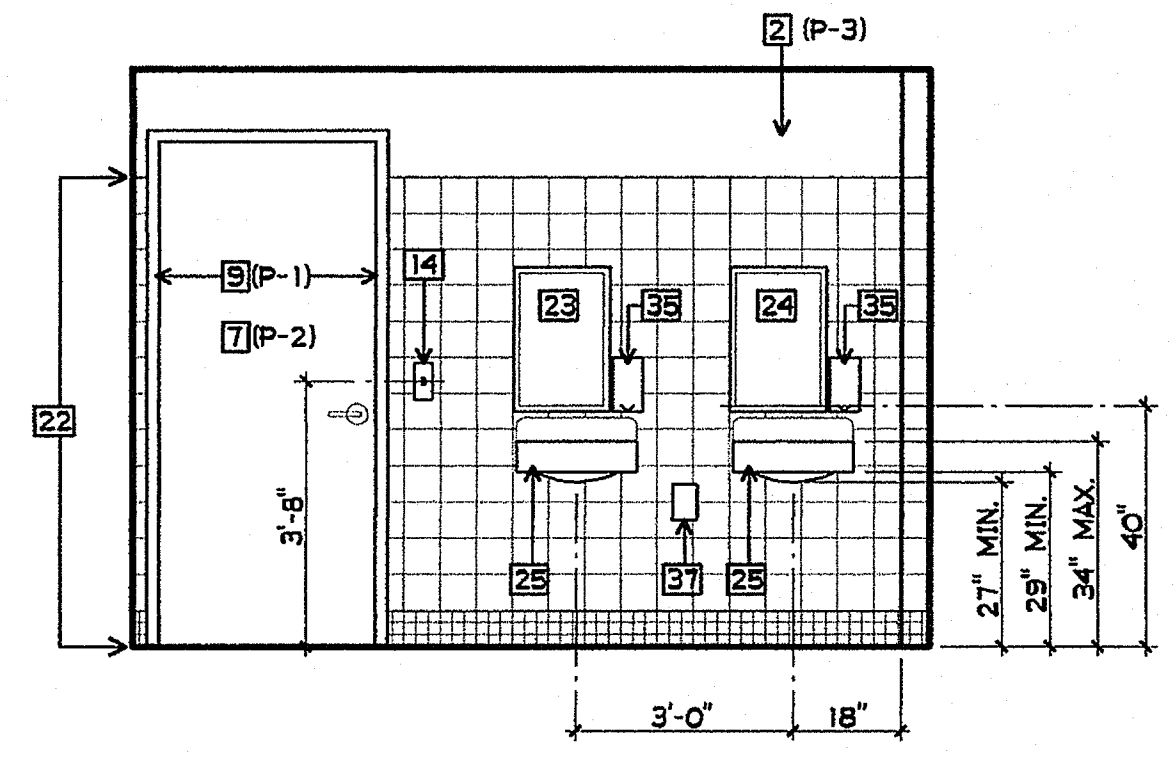
SHEET NOTES

- | | | |
|---|---|--|
| 2 5/8" GBX - ORANGE PEAL TEXT. - SEMI-GLOSS PAINT | 17 LOUVERS - SEE MECHANICAL DRAWINGS | 44 BENCH |
| 5 CMU - SEMI-GLOSS PAINT | 22 CERAMIC TILE - PATTERN PER 29/AN7-4 | 45 LOCKERS |
| 7 DOOR - SEMI-GLOSS PAINT | 23 ACCESSIBLE MIRROR 16" X 24" | 46 CONCRETE CURB |
| 8 VISION PANEL | 24 MIRROR 16" X 24" | 51 EXIT SIGN - SEE SIGNAGE PLAN AND 7/A9-8 |
| 9 METAL FRAME - SEMI-GLOSS PAINT | 25 ACCESSIBLE LAVATORY | 52 DIRECTIONAL EXIT SIGN - SEE SIGNAGE PLAN AND 4/A9-8 |
| 10 RESILIENT BASE | 34 RECESSED HAND DRYER | 54 SPEAKER - SEE ELECTRICAL PLANS |
| 12 POWER OUTLET | 35 SOAP DISPENSER - 4" TO BE 1-1/2" FROM EDGE OF LAVATORY | 55 WALL OPENING FRAMING - SEE STRUCTURAL DRAWINGS |
| 14 LIGHT CONTROLS | | |

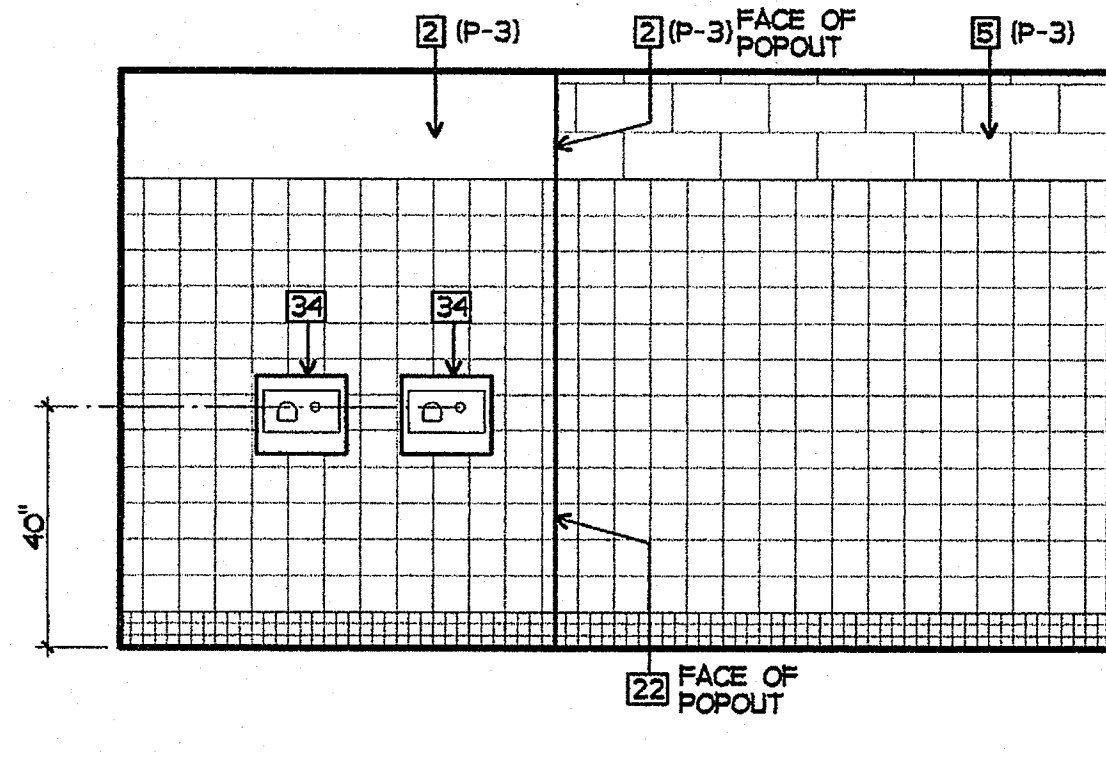
FINISH PALETTE

- (P-1) PAINT, COLOR #1
 (P-2) PAINT, COLOR #2
 (P-3) PAINT, COLOR #3

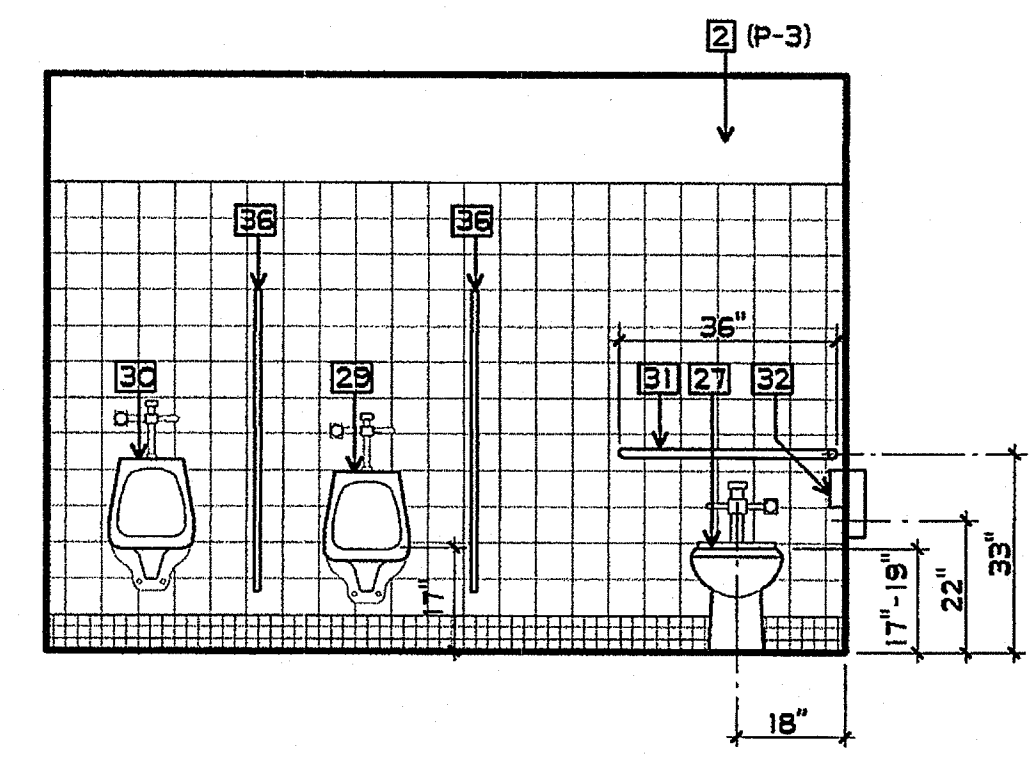




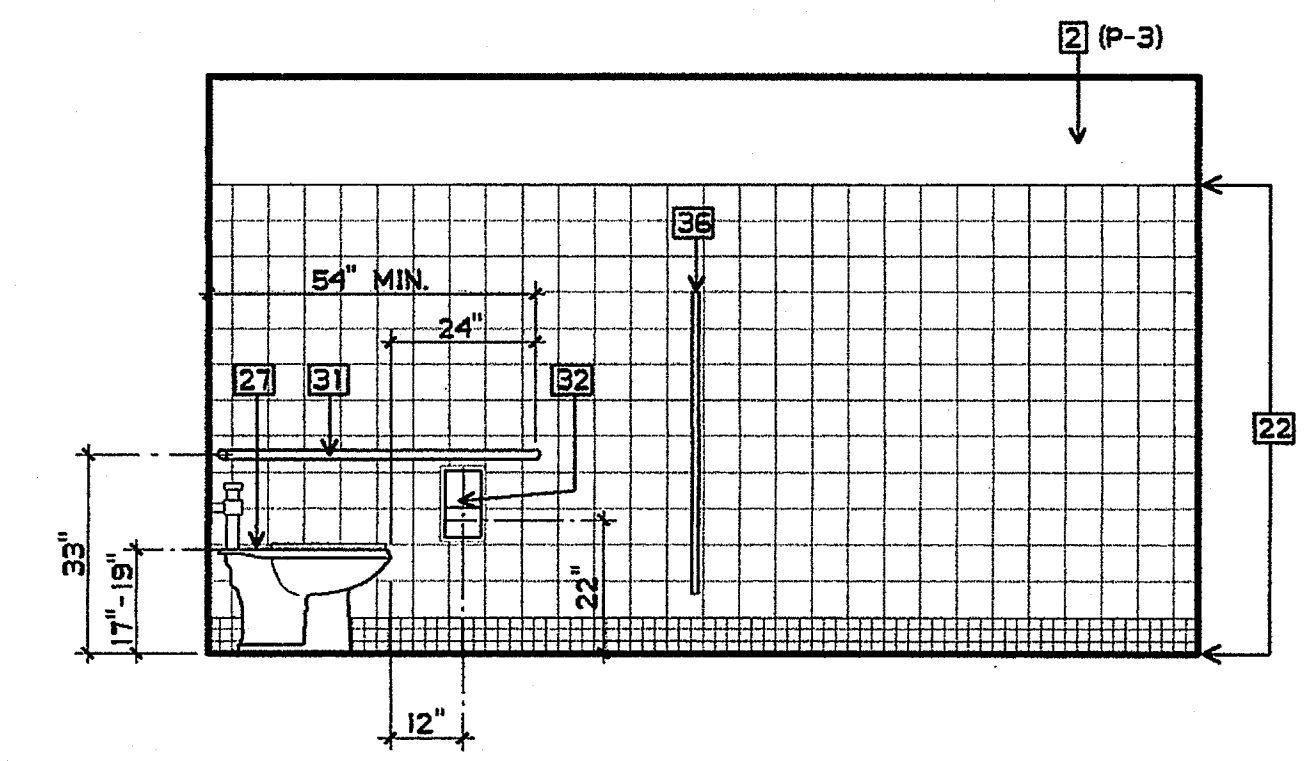
25 BOYS
3/8" = 1'-0"



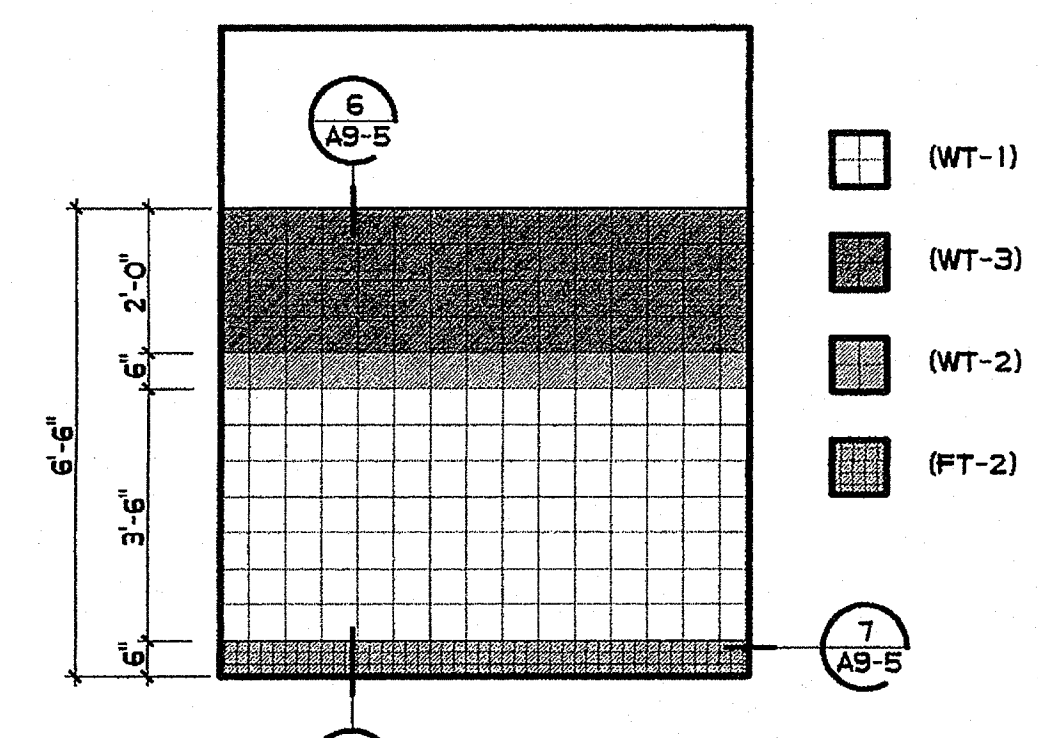
26 BOYS
3/8" = 1'-0"



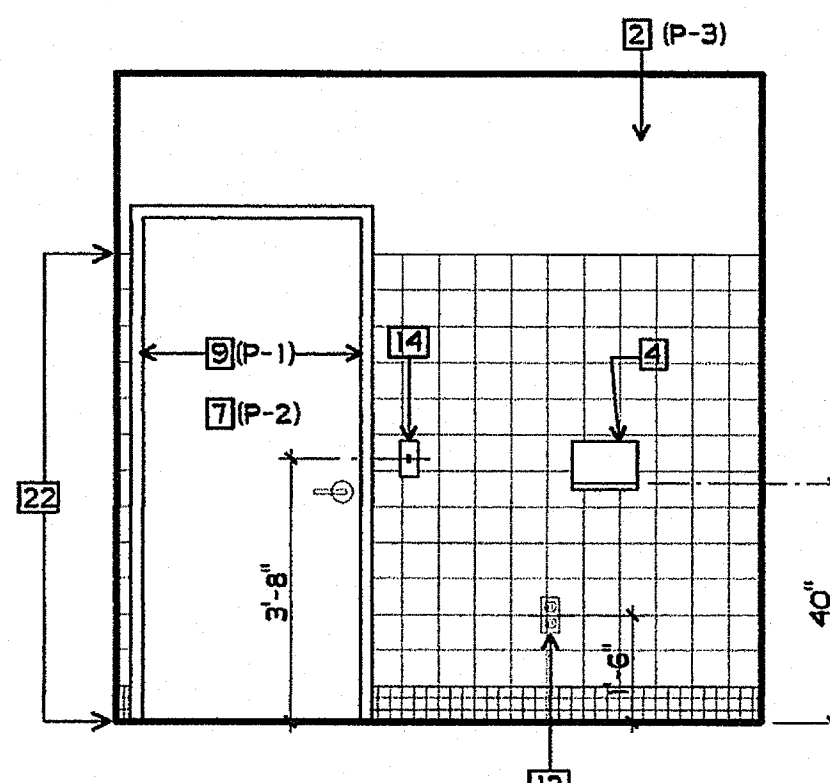
27 BOYS
3/8" = 1'-0"



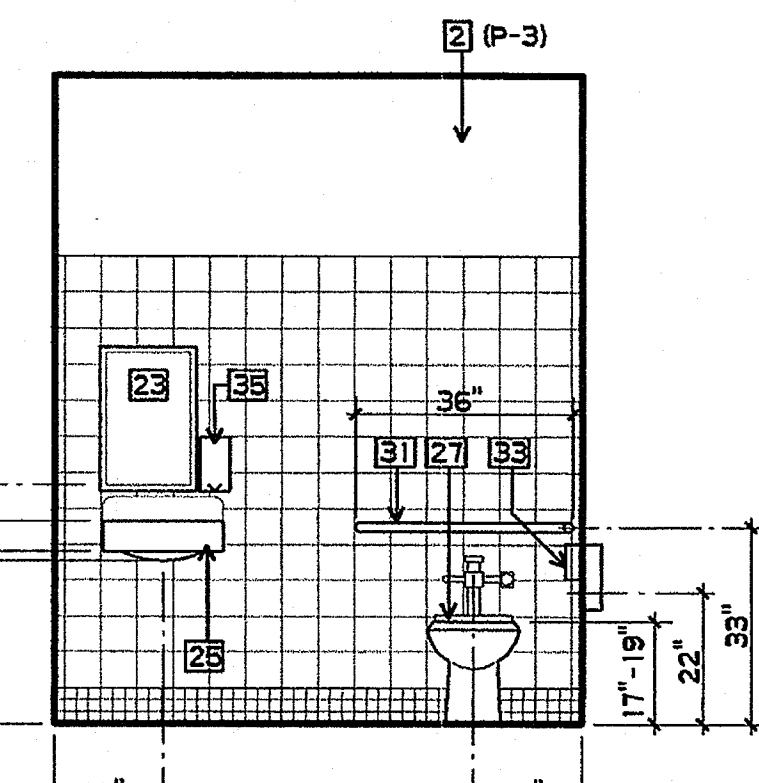
28 BOYS
3/8" = 1'-0"



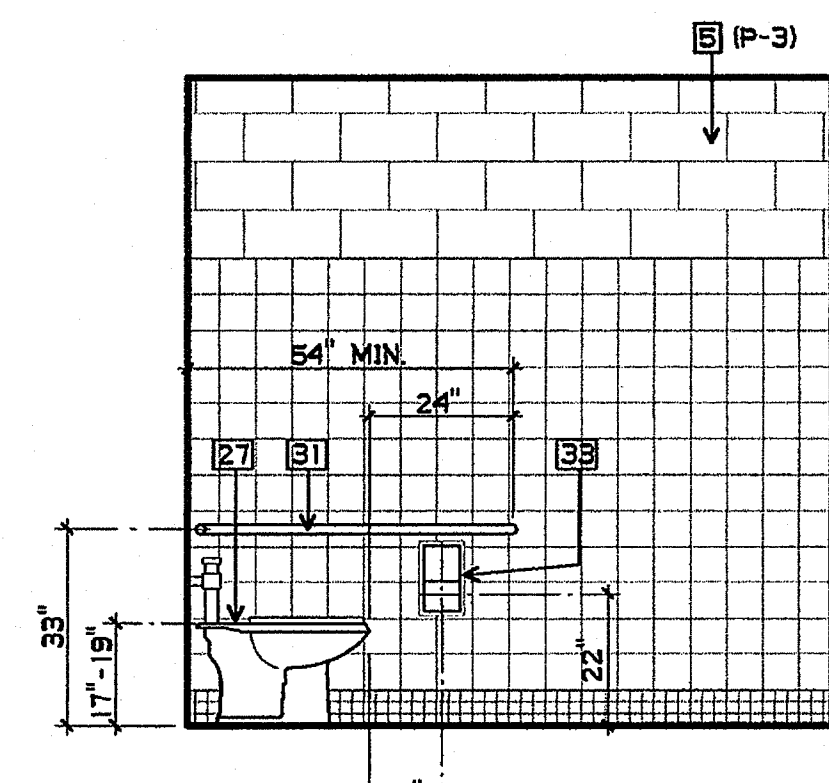
29 CERAMIC WALL TILE PATTERN AT TOILET
3/8" = 1'-0"



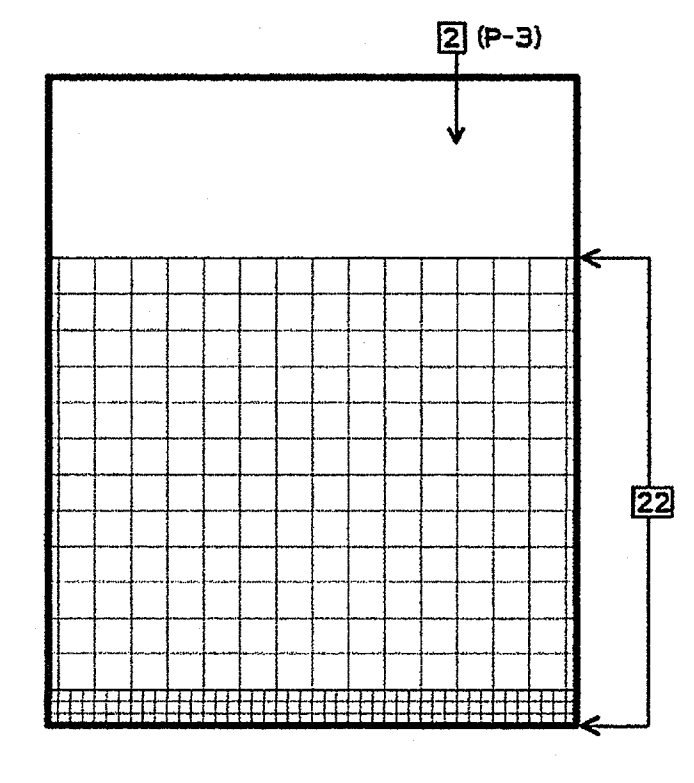
30 MEN
3/8" = 1'-0"



31 MEN
3/8" = 1'-0"



32 MEN
3/8" = 1'-0"



33 MEN
3/8" = 1'-0"

TOILET ROOM ACCESSIBILITY NOTES

1. WATER CLOSETS: THE HEIGHT OF ACCESSIBLE WATER CLOSETS SHALL BE A MIN. OF 17 INCHES AND A MAXIMUM OF 19 INCHES MEASURED TO THE TOP OF THE TOILET SEAT FROM THE FLOOR. CONTROLS SHALL BE OPERABLE WITH ONE HAND, AND SHALL NOT REQUIRE TIGHT GRASPING, PINCHING, OR TWISTING OF WRIST. CONTROLS FOR FLUSH VALVES SHALL BE MOUNTED ON WIDE SIDE OF TOILET AREAS. THE FORCE REQUIRED TO ACTIVATE SHALL BE NO GREATER THAN 5 POUNDS FORCE POUNDS FORCE.
2. GRAB BARS AT WATER CLOSETS: GRAB BARS SHALL BE LOCATED IN DESIGNATED ACCESSIBLE STALLS MOUNTED 33" O.C. ABOVE THE FLOOR, ONE SIDE TO BE 42" LONG AND SHALL EXTEND MIN. 24" BEYOND THE FRONT OF WATER CLOSET, THE REAR BAR SHALL BE 36" LONG CENTERED BEHIND WATER CLOSET. BARS SHALL BE 1-1/4" TO 1-1/2" OUTSIDE DIAMETER AND MOUNTED WITH 1-1/2" INCH CLEARANCE TO THE WALL. BAR, FASTENERS, AND MOUNTING SHALL BE ABLE TO SUPPORT 250#S PER FOOT IN BENDING, SHEAR, AND TENSION. BARS SHALL NOT ROTATE WITHIN FITTINGS.
3. URINALS: WHERE URINALS ARE PROVIDED, AT LEAST ONE WITH THE RIM PROJECTING A MIN. OF 14 INCHES FROM THE WALL AND A MAX. OF 17 INCHES ABOVE THE FLOOR SHALL BE PROVIDED. FLUSH CONTROLS SHALL BE HAND-OPERATED AND SHALL BE MOUNTED WITHIN 44" OF THE FLOOR. MAX. FORCE TO OPERATE CONTROLS SHALL BE 5 POUNDS FORCE.
4. TOILET TISSUE DISPENSER: DISPENSERS ARE TO PROJECT NO MORE THAN 3" FROM SURFACE OF WALL, AND NO CLOSER THAN 1 1/2" TO THE TANGENT POINT OF THE GRAB BAR. DISPENSER MUST PROVIDE FOR CONTINUOUS PAPER FLOW.
5. LAVATORIES: ALL ACCESSIBLE LAVATORIES SHALL BE MOUNTED WITH RIM OR COUNTER SURFACE NO HIGHER THAN 34" ABOVE THE FLOOR. PROVIDE CLEAR SPACE BELOW LAVATORIES 29" HIGH AT APRON, 27" MIN. KNEE CLEARANCE BY 30" WIDE BY 8" DEEP, AND 9" HIGH BY 30" WIDE BY 17" DEEP AT FLOOR FOR TOE SPACE. HOT WATER / DRAIN PIPES SHALL BE INSULATED OR OTHERWISE COVERED. THERE SHALL BE NO SHARP OR ABRASIVE SURFACES UNDER LAVATORIES. THE FAUCET CONTROLS AND OPERATING MECHANISMS (OPERABLE W/ ONE HAND) SHALL NOT REQUIRE TIGHT GRASPING, PINCHING, OR TWISTING OF THE WRIST AND AN OPERATING FORCE NOT EXCEEDING 5 POUNDS FORCE. SELF-CLOSING VALVES PERMITTED IF THE FAUCET REMAINS OPEN FOR AT LEAST 10 SECONDS.
6. TOILET ROOM ACCESSORIES: THE BOTTOM EDGE OF THE REFLECTED SURFACE SHALL BE WITHIN 40" OF THE FLOOR. TOILET TISSUE PAPER DISPENSERS SHALL BE MOUNTED WITHIN 12" FROM THE FRONT EDGE OF THE WATER CLOSET. FOR TOWEL, SANITARY NAPKIN, WASTE RECEPTACLE, COIN SLOTS, ETC. OR OTHER SIMILAR TOILET ROOM ACCESSORIES, AT LEAST ONE SHALL BE MOUNTED SO THAT ALL OPERATING ARE WITHIN 40" OF THE FLOOR.
7. STANDARD COMPARTMENT DOORS TO ACCESSIBLE WATER CLOSET COMPARTMENTS SHALL BE EQUIPPED WITH A SELF CLOSING DEVICE AND PROVIDE MIN 32" CLEARANCE WHEN LOCATED AT THE END OF THE COMPARTMENT AND MIN. 34" WHEN LOCATED AT THE SIDE. PROVIDE A MIN. OF 9" CLEARANCE UNDERNEATH COMPARTMENT DOORS.
8. ON ALL DOORWAYS LEADING TO SANITARY FACILITIES, PROVIDE IDENTIFICATION SYMBOLS (12" DIAMETER CIRCLE FOR WOMEN, 12" EQUILATERAL TRIANGLE FOR MEN) SIGNS SHALL BE 1/4" THICK WITH 1/32" RAISED CHARACTERS AND SHALL INCLUDE GRADE 2 BRAILLE IDENTIFICATION. SIGN SHALL BE MOUNTED CENTERED 60" ABOVE FLOOR. SIGN SHALL BE PLAIN WHITE CHARACTERS W/ ROYAL BLUE BACKGROUND. SEE SECTION 10426. SIGNS SHALL BE OF A CONTRASTING COLOR TO DOOR.

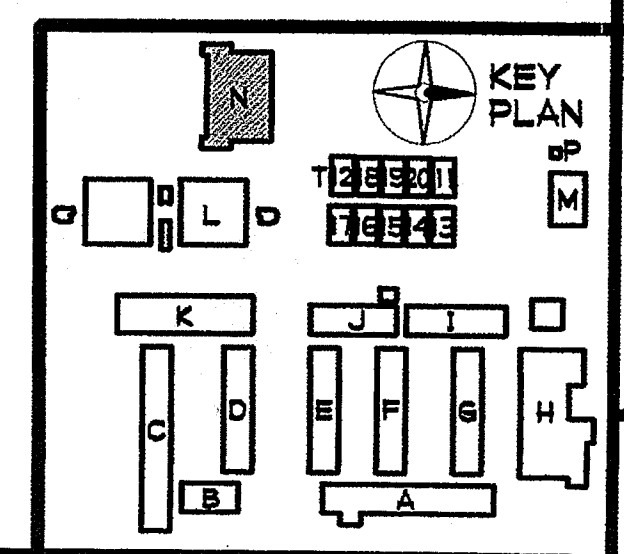
SHEET NOTES

- 2 5/8" GBX - ORANGE PEAL TEXT. - SEMI-GLOSS PAINT
- 4 PAPER TOWEL DISPENSER
- 5 CMJ - SEMI-GLOSS PAINT
- 7 DOOR - SEMI-GLOSS PAINT
- 9 METAL FRAME - SEMI-GLOSS PAINT
- 12 POWER OUTLET
- 14 LIGHT CONTROLS
- 22 CERAMIC TILE - PATTERN PER 29/AN7-4
- 23 ACCESSIBLE MIRROR 16" X 24"
- 24 MIRROR 16" X 24"
- 25 ACCESSIBLE LAVATORY
- 27 ACCESSIBLE WATER CLOSET
- 28 ACCESSIBLE URINAL
- 30 URINAL
- 31 GRAB BARS - SEE 11/A9-5 AND 12/A9-5

- 32 ACCESSIBLE TOILET TISSUE DISPENSER AT TYP. INTERIOR PARTITION - SEE 9/A9-5
- 33 ACCESSIBLE TOILET TISSUE DISPENSER AT CMJ WALL - SEE 10/A9-5
- 34 RECESSED HAND DRYER
- 35 SOAP DISPENSER - 1/2" TO BE 1-1/2" FROM EDGE OF LAVATORY
- 36 PHENOLIC TOILET PARTITIONS
- 37 HOSE BIBB WITH COVER

FINISH PALETTE

- (WT-1) CERAMIC WALL TILE, COLOR #1
- (WT-2) CERAMIC WALL TILE, COLOR #2
- (WT-3) CERAMIC WALL TILE, COLOR #3
- (FT-2) CERAMIC FLOOR TILE, COLOR #2
- (P-1) PAINT, COLOR #1
- (P-2) PAINT, COLOR #2
- (P-3) PAINT, COLOR #3



GROTH ARCHITECTS, INC.
823 ACACIA STREET
OCEANSIDE, CA 92054
OCEANSIDE UNIFIED S.D.

JEFFERSON MS NEW CONSTRUCTION

CLSD NO. 758-000
PROJECT NOS. 025
P. T. N. 73569-9
DATE
REVISIONS

3355 MISSION AVE. SUITE 234
OCEANSIDE, CALIFORNIA 92054

PHONE 760-754-8191
FAX 760-754-8291

DSA
IDENTIFICATION STAMP
DIV. OF THE STATE ARCHITECT
OFFICE OF REGULATION SERVICES
4-106494
AC [initials] FLS [initials] SS [initials]
DATE MAR 28 2005

REGISTERED ARCHITECT
JOHN SCOTT BERRY
C-26609
4/30/2007 RENEWAL
STATE OF CALIFORNIA

BUILDING N
INTERIOR
ELEVATIONS

AN7-4

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GROTH ARCHITECTS, INC.
 823 ACACIA STREET
 OCEANSIDE, CA 92054
 OCEANSIDE UNIFIED S.D.

CLUSD NO.
758-000

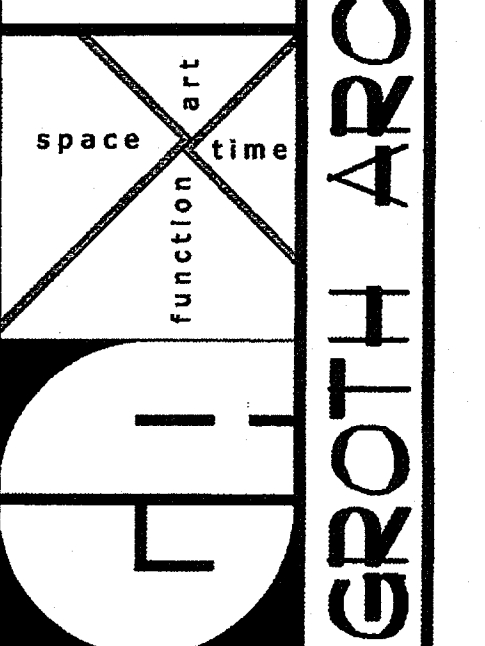
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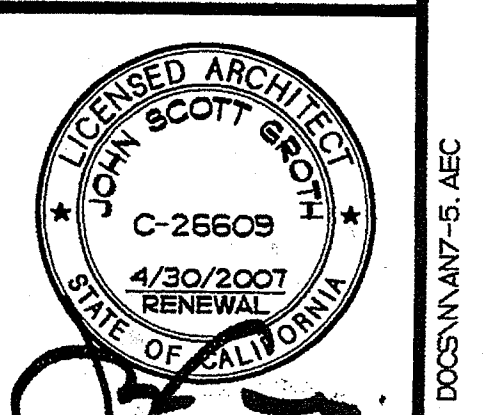
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REVISIONS

JEFFERSON MS NEW CONSTRUCTION
 823 ACACIA STREET
 OCEANSIDE, CA 92054
 OCEANSIDE UNIFIED S.D.



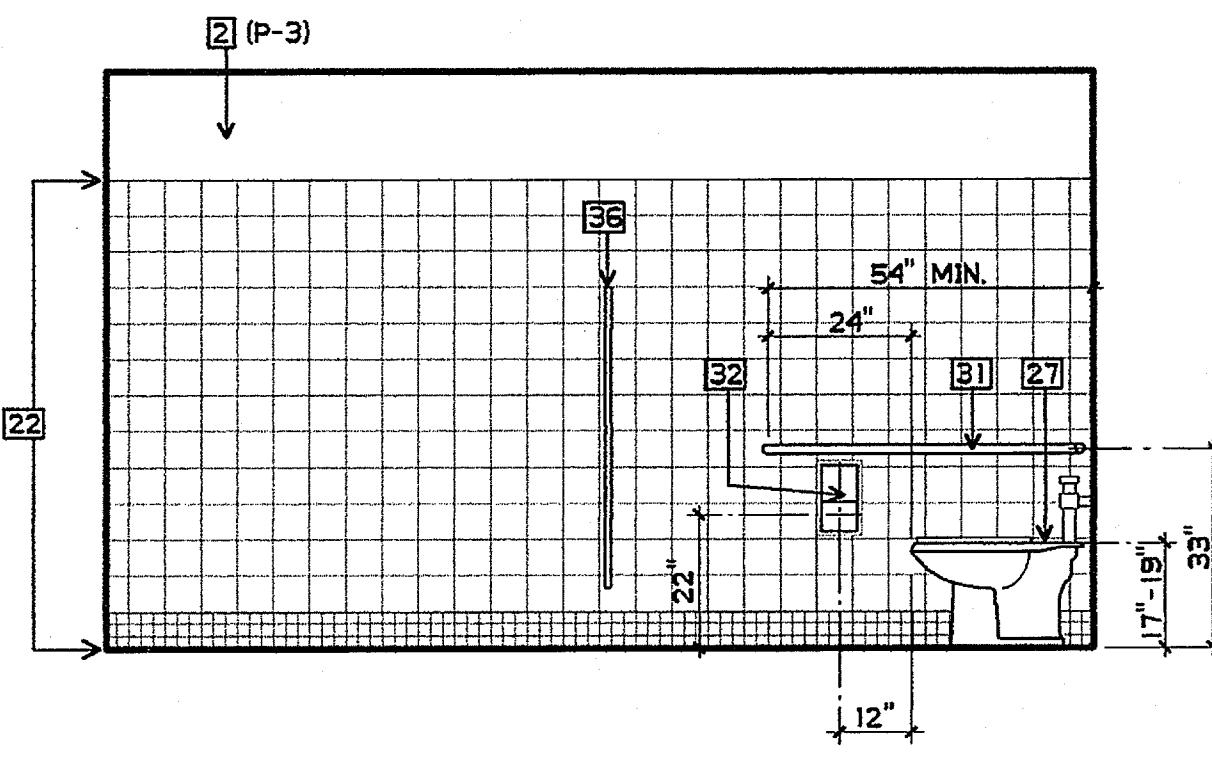
DSA
 IDENTIFICATION STAMP
 DIV. OF THE STATE ARCHITECT
 OFFICE OF REGULATION SERVICES
 4-106494
 AC. *AS* FLS. *AS*
 DATE: **MAR 28 2005**



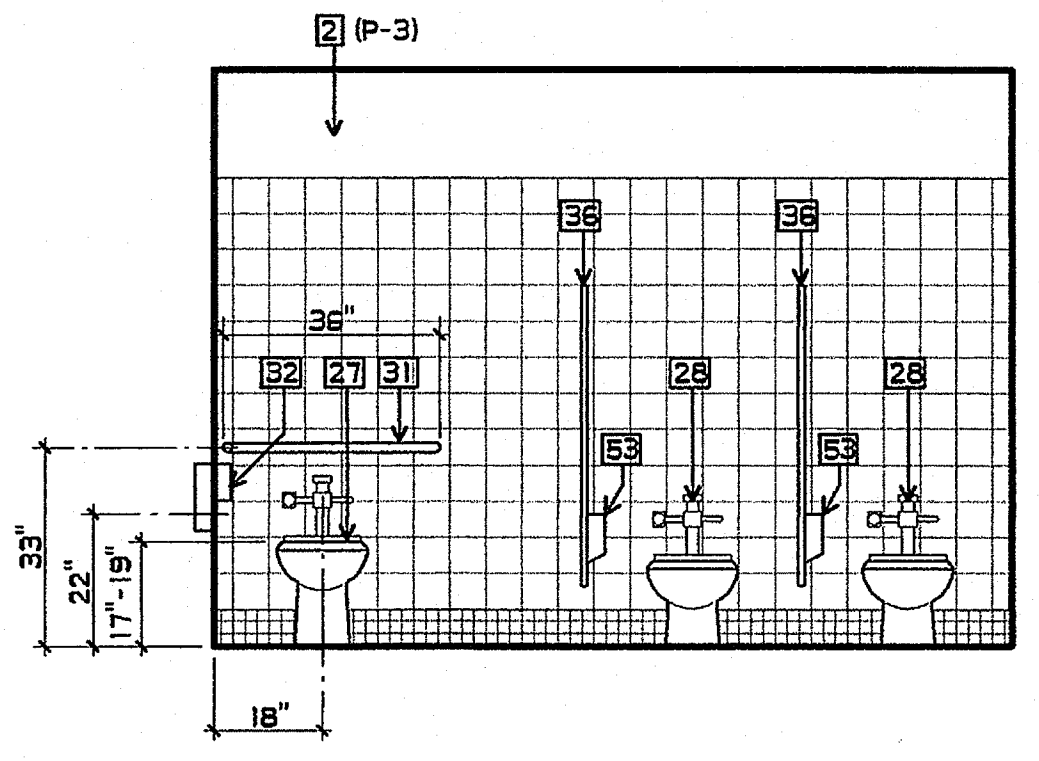
SHEET TITLE

BUILDING N
 INTERIOR
 ELEVATIONS

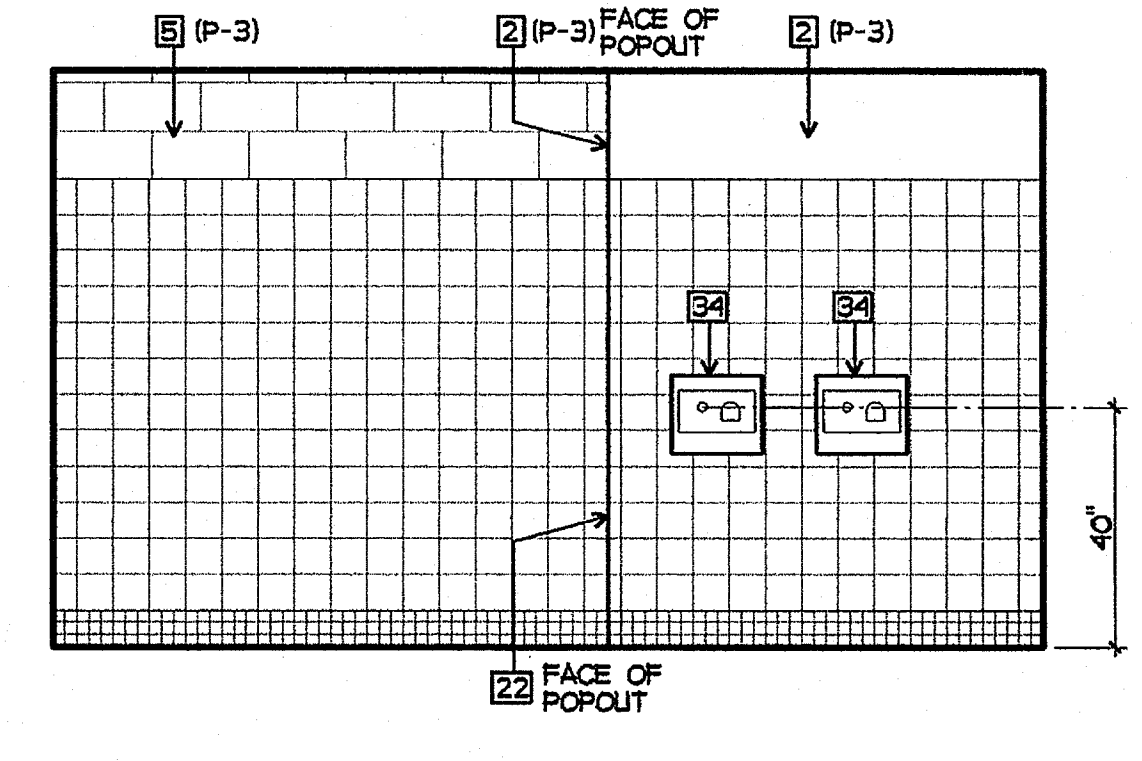
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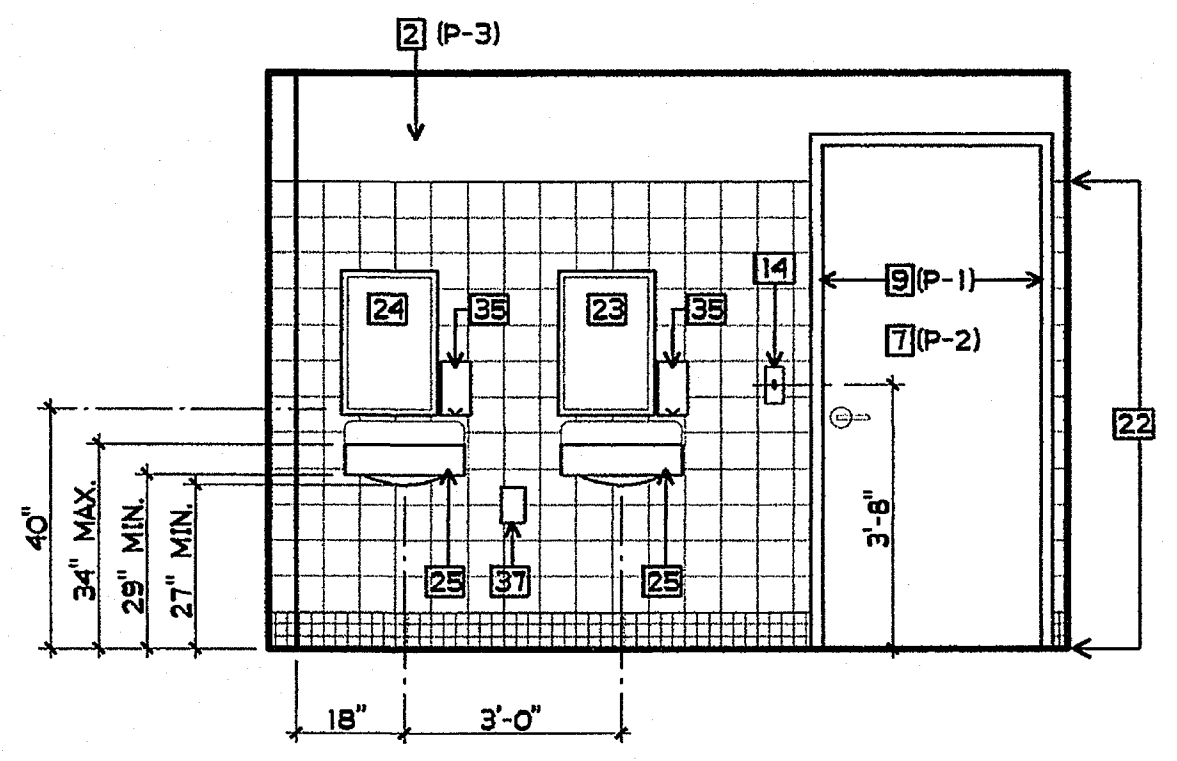
34 **GIRLS**
 3/8" = 1'-0"



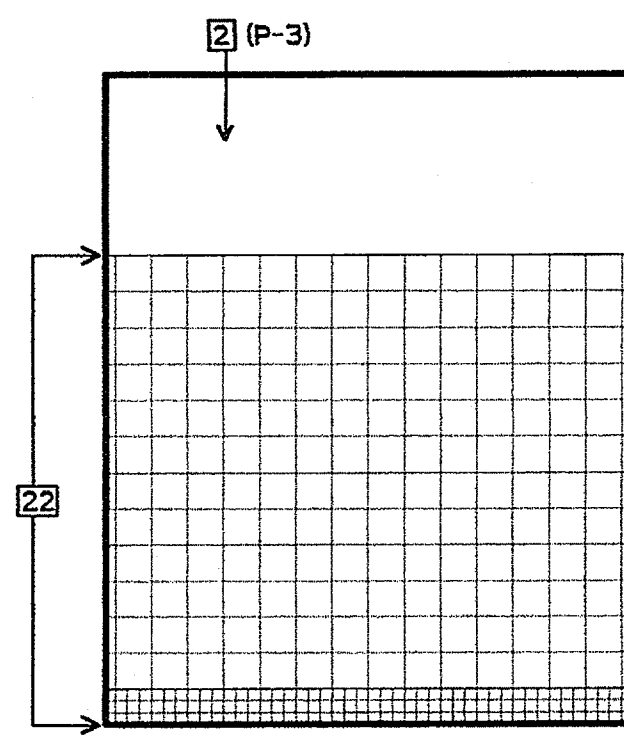
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 3/8" = 1'-0"



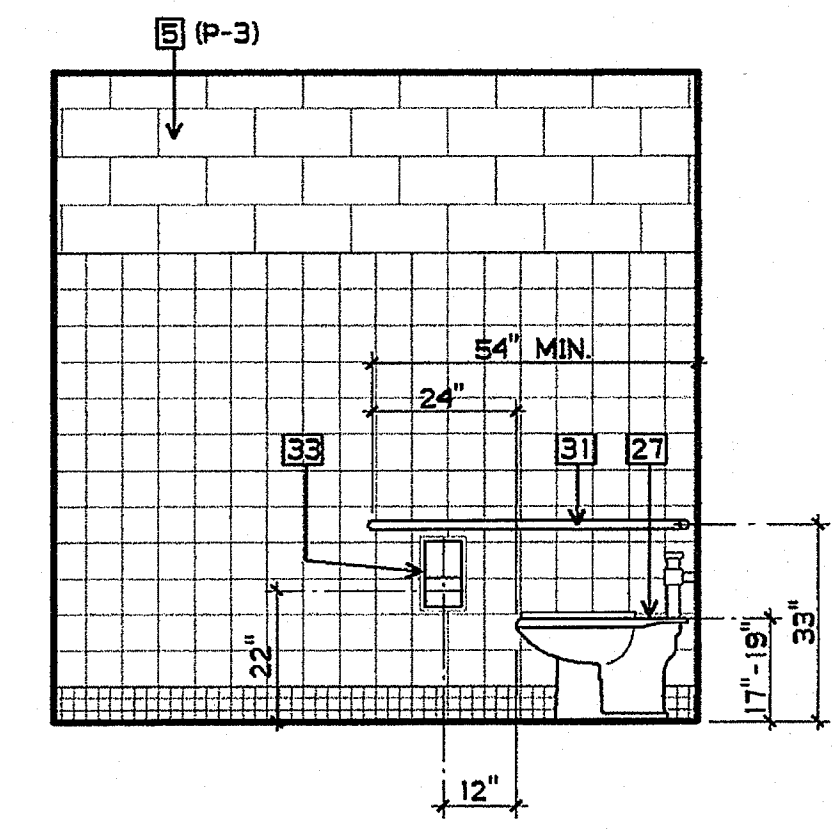
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 3/8" = 1'-0"



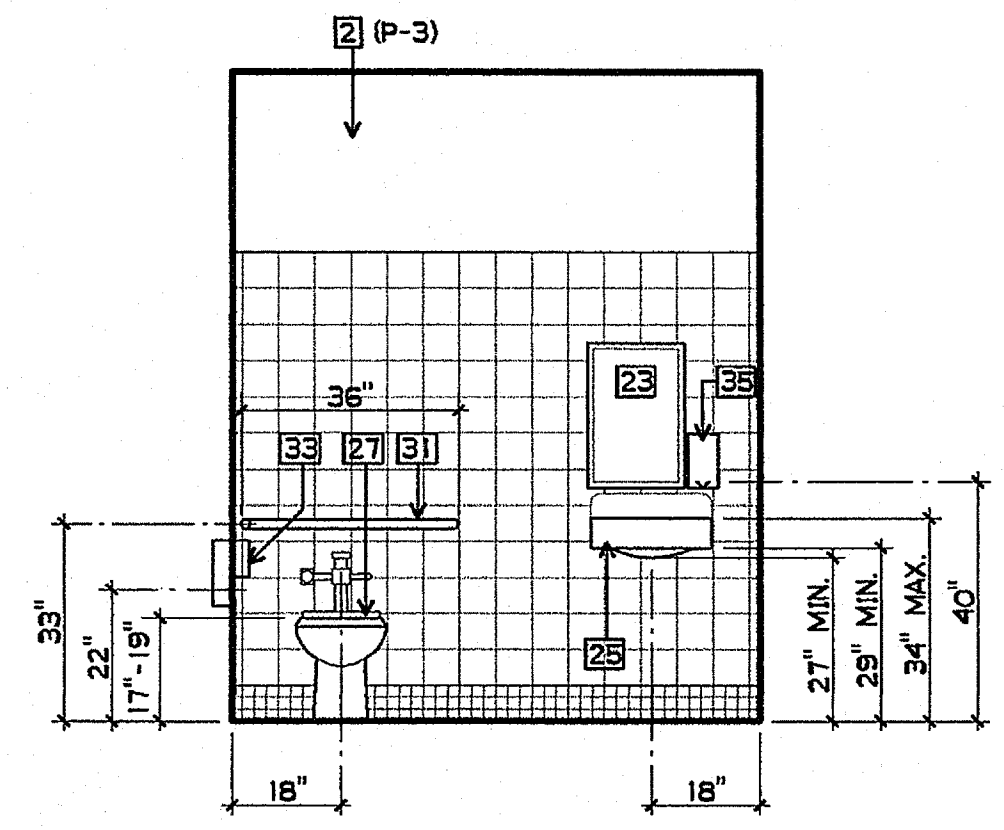
37 **GIRLS**
 3/8" = 1'-0"



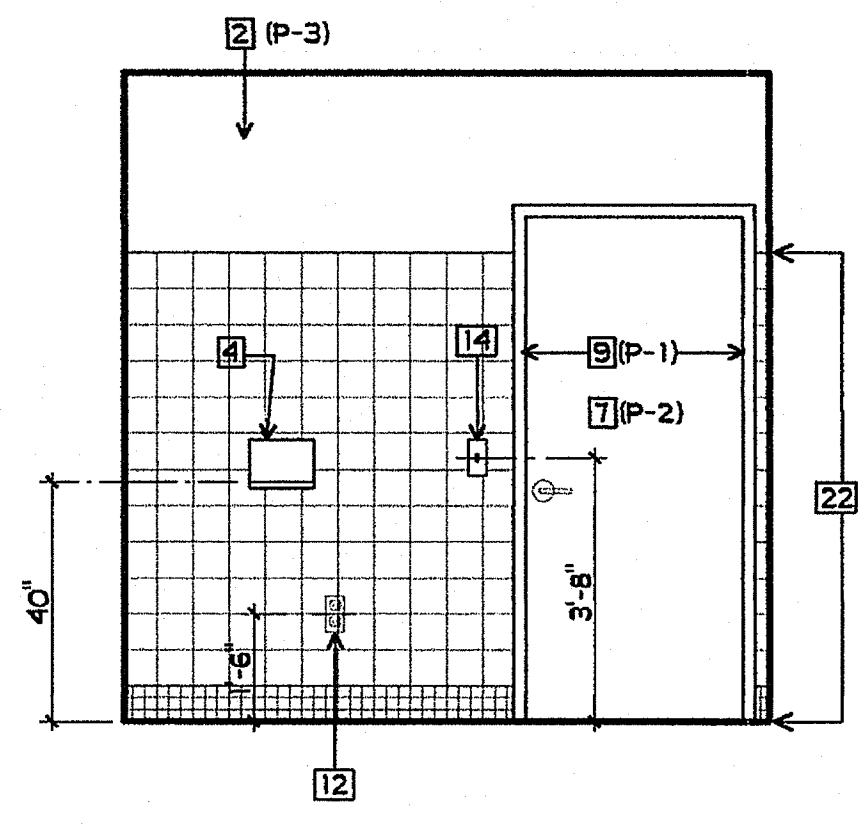
38 **WOMEN**
 3/8" = 1'-0"



39 **WOMEN**
 3/8" = 1'-0"



40 **WOMEN**
 3/8" = 1'-0"



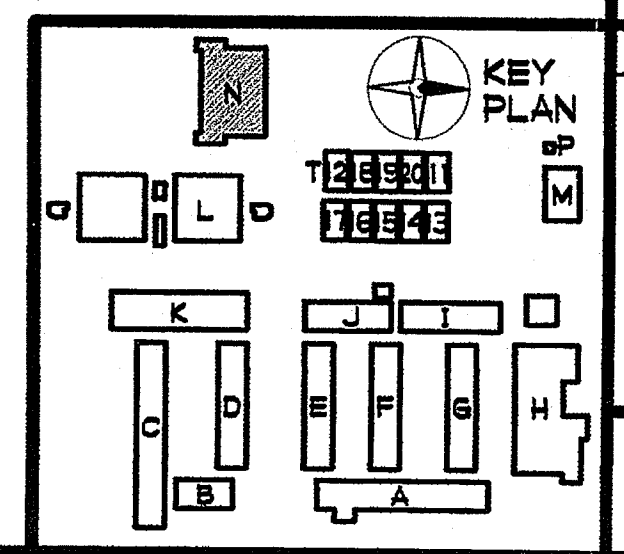
41 **WOMEN**
 3/8" = 1'-0"

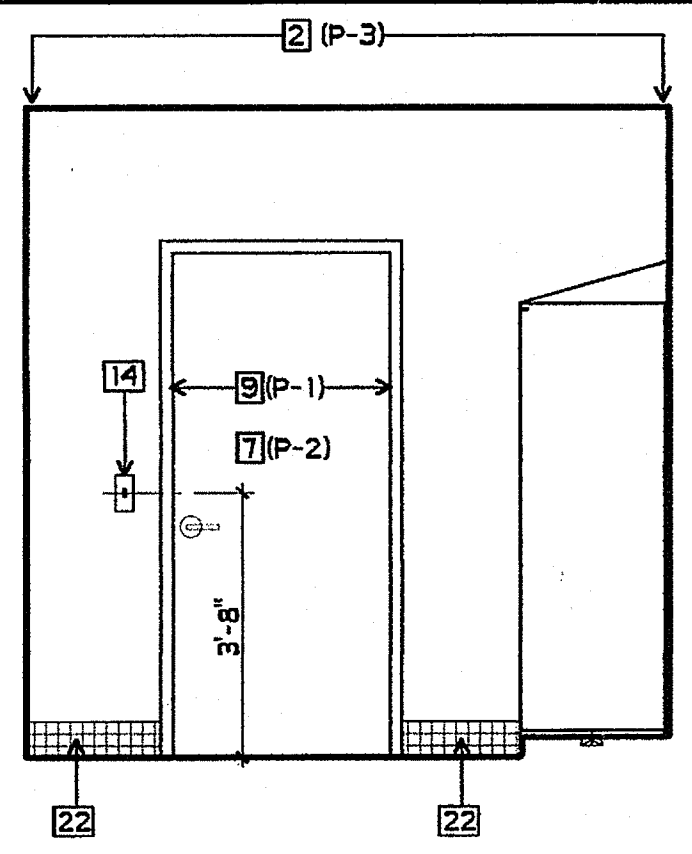
SHEET NOTES

- | | | |
|--|--|--|
| 2] 5/8" GBX - ORANGE PEAL TEXT. - SEMI-GLOSS PAINT | 23] ACCESSIBLE MIRROR 16" X 24" | 33] ACCESSIBLE TOILET TISSUE DISPENSER AT CMU WALL - SEE 10/A9-5 |
| 4] PAPER TOWEL DISPENSER | 24] MIRROR 16" X 24" | 34] RECESSED HAND DRYER |
| 5] CMU - SEMI-GLOSS PAINT | 25] ACCESSIBLE LAVATORY | 35] SOAP DISPENSER - 1/2" TO BE 1-1/2" FROM EDGE OF LAVATORY |
| 7] DOOR - SEMI-GLOSS PAINT | 27] ACCESSIBLE WATER CLOSET | 36] PHENOLIC TOILET PARTITIONS |
| 9] METAL FRAME - SEMI-GLOSS PAINT | 28] WATER CLOSET | 37] HOSE BIBB WITH COVER |
| 12] POWER OUTLET | 31] GRAB BARS - SEE 11/A9-5 AND 12/A9-5 | 38] TOILET TISSUE DISPENSER |
| 14] LIGHT CONTROLS | 32] ACCESSIBLE TOILET TISSUE DISPENSER AT TYP. INTERIOR PARTITION - SEE 9/A9-5 | |
| 22] CERAMIC TILE - PATTERN PER 29/AN7-4 | | |

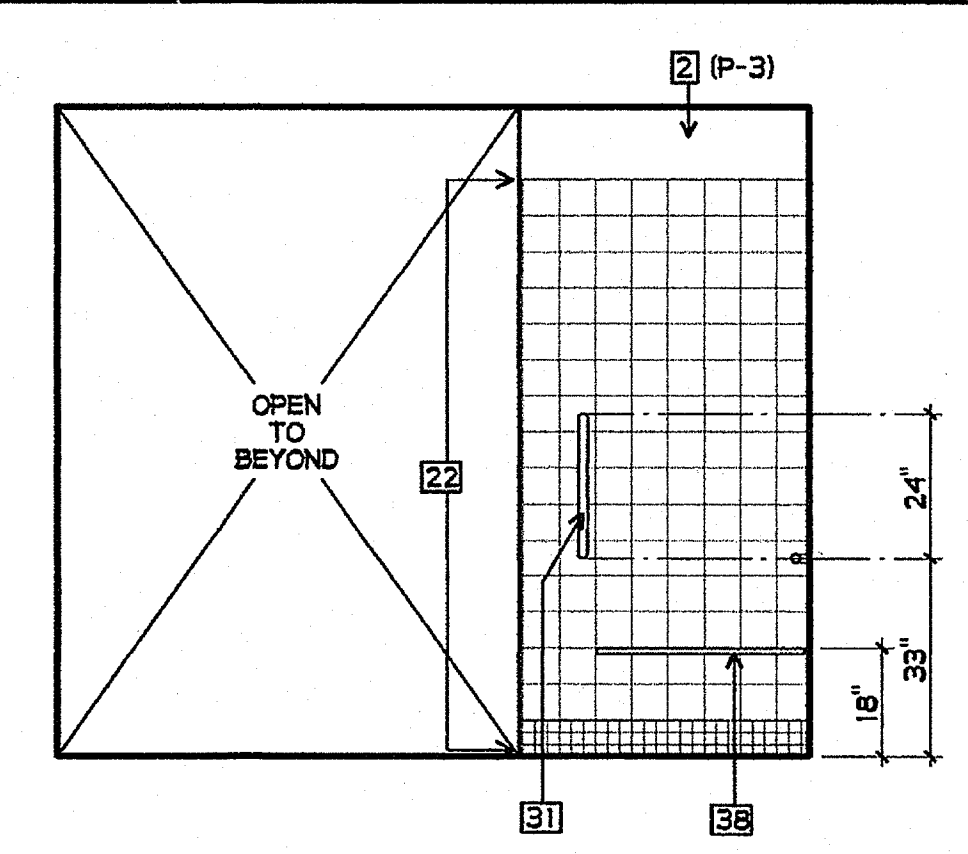
FINISH PALETTE

- (WT-1) CERAMIC WALL TILE, COLOR #1
 (WT-2) CERAMIC WALL TILE, COLOR #2
 (WT-3) CERAMIC WALL TILE, COLOR #3
 (FT-2) CERAMIC FLOOR TILE, COLOR #2
 (P-1) PAINT, COLOR #1
 (P-2) PAINT, COLOR #2
 (P-3) PAINT, COLOR #3

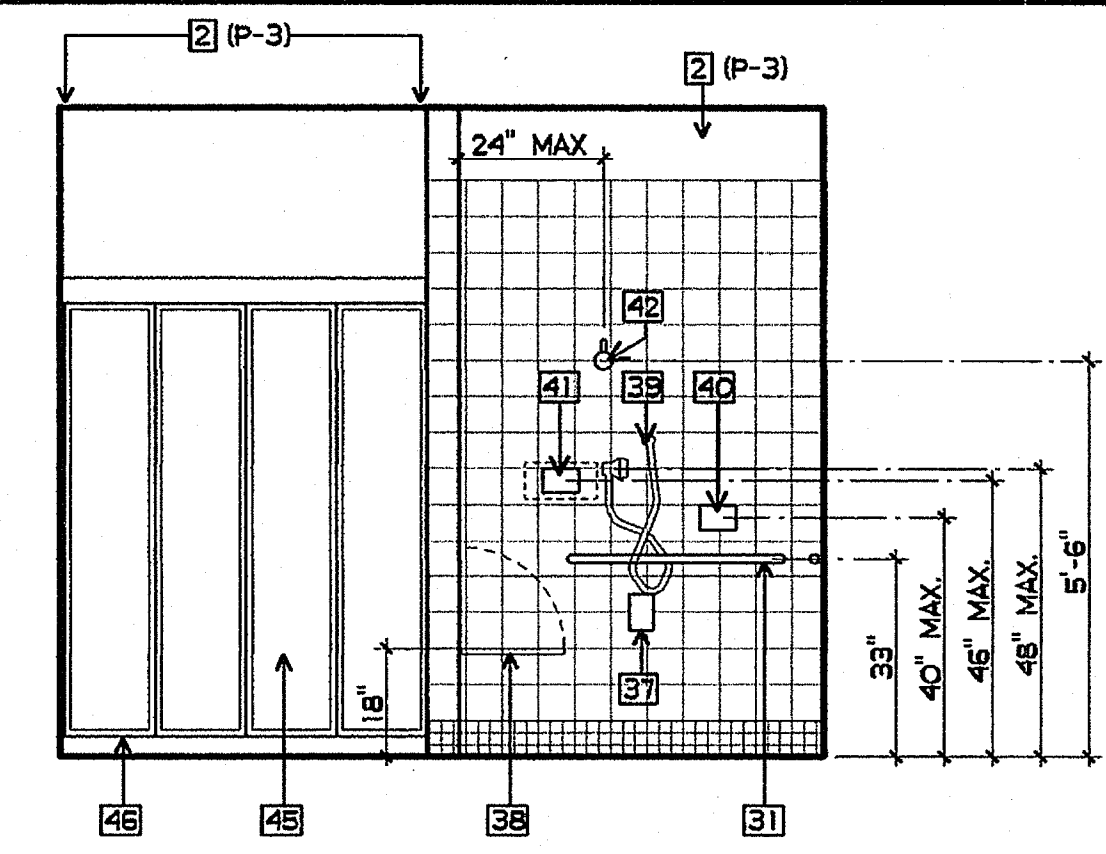




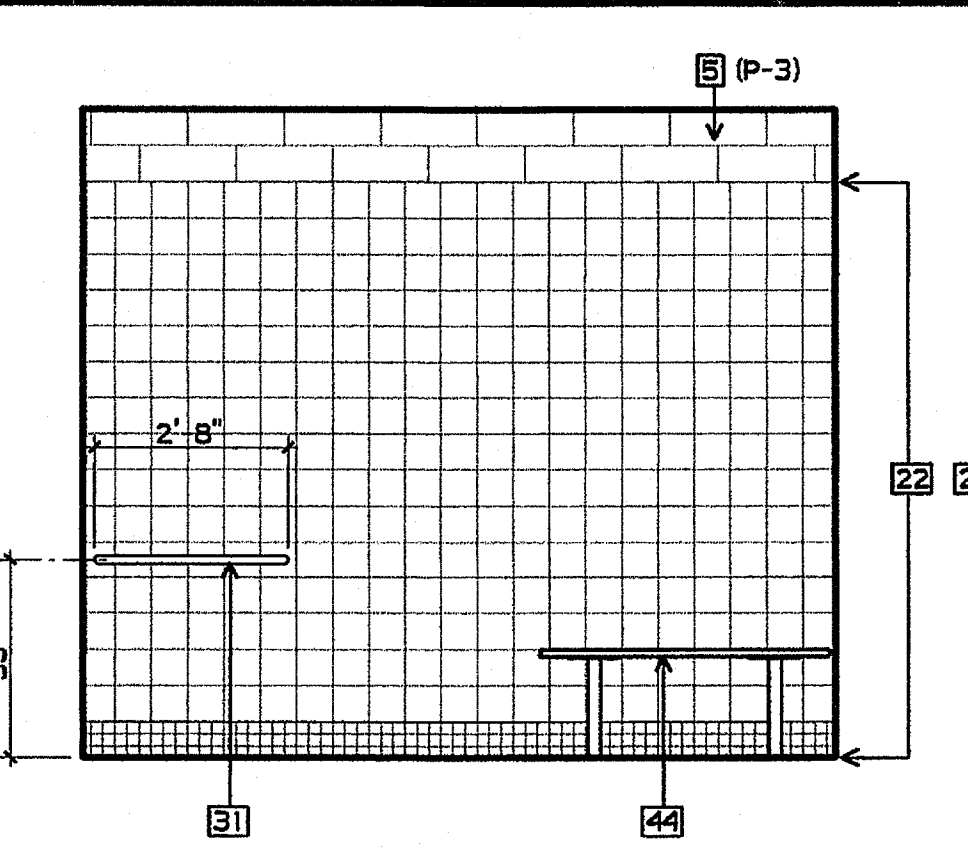
42 MENS DRESSING
3/8" = 1'-0"



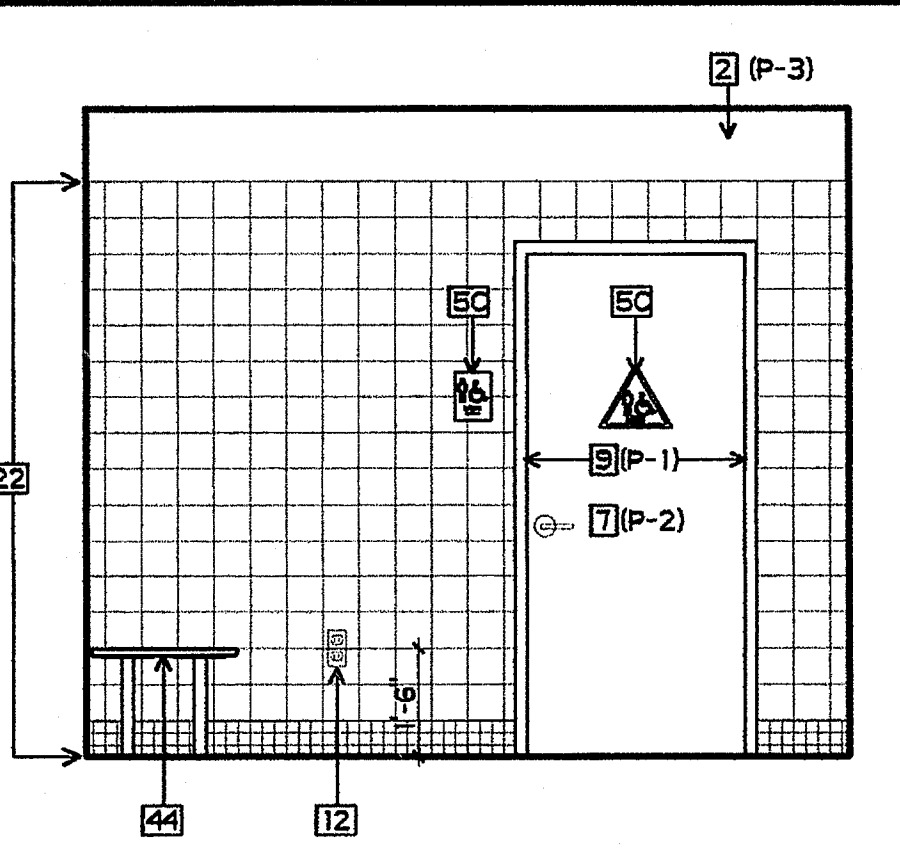
43 MENS DRESSING
3/8" = 1'-0"



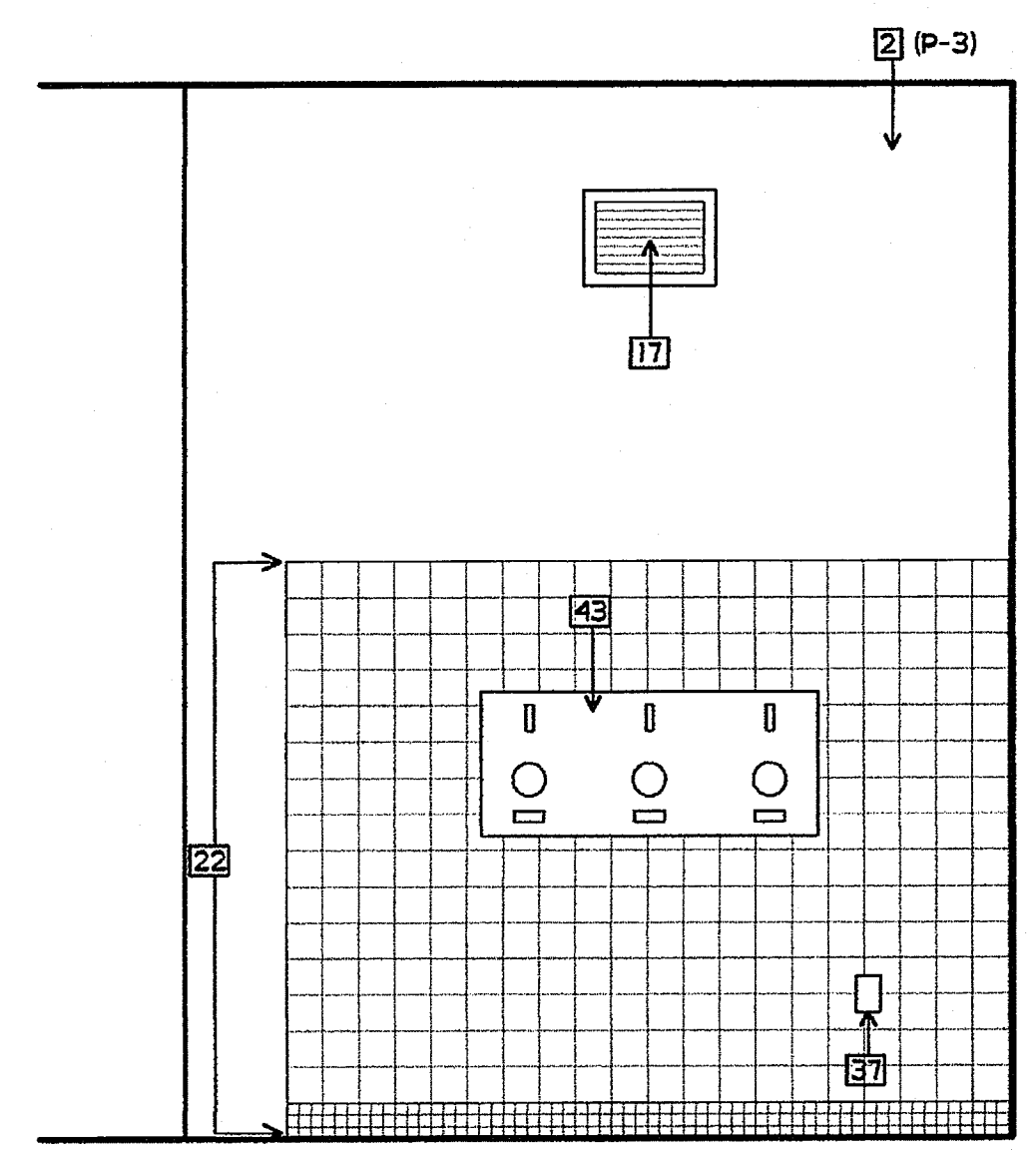
44 MENS DRESSING
3/8" = 1'-0"



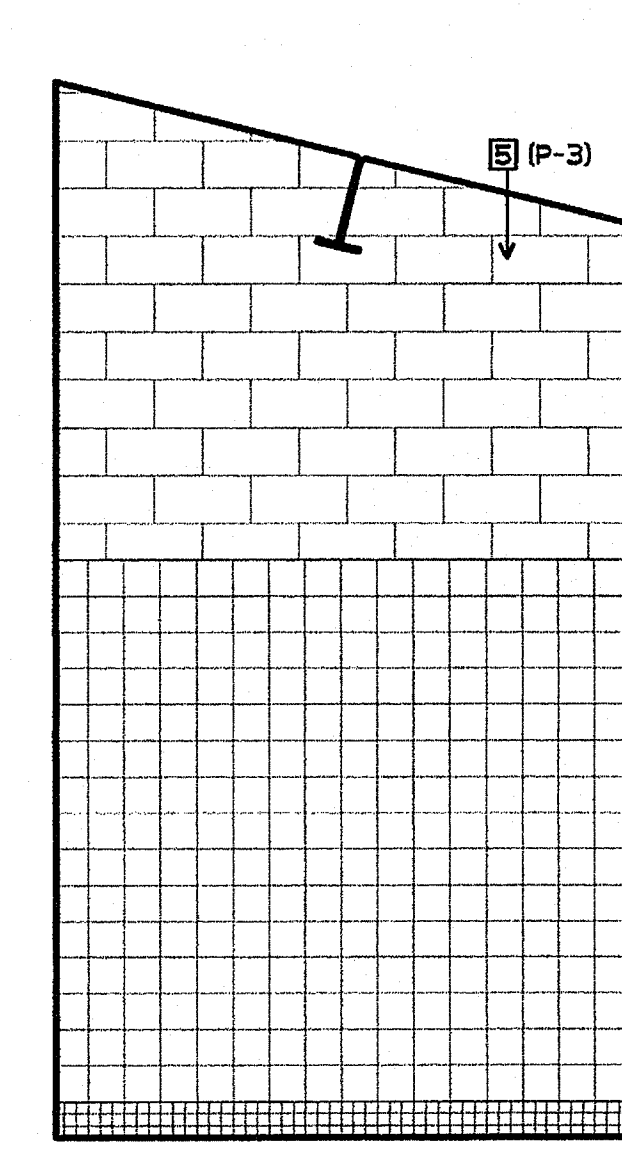
45 MENS DRESSING
3/8" = 1'-0"



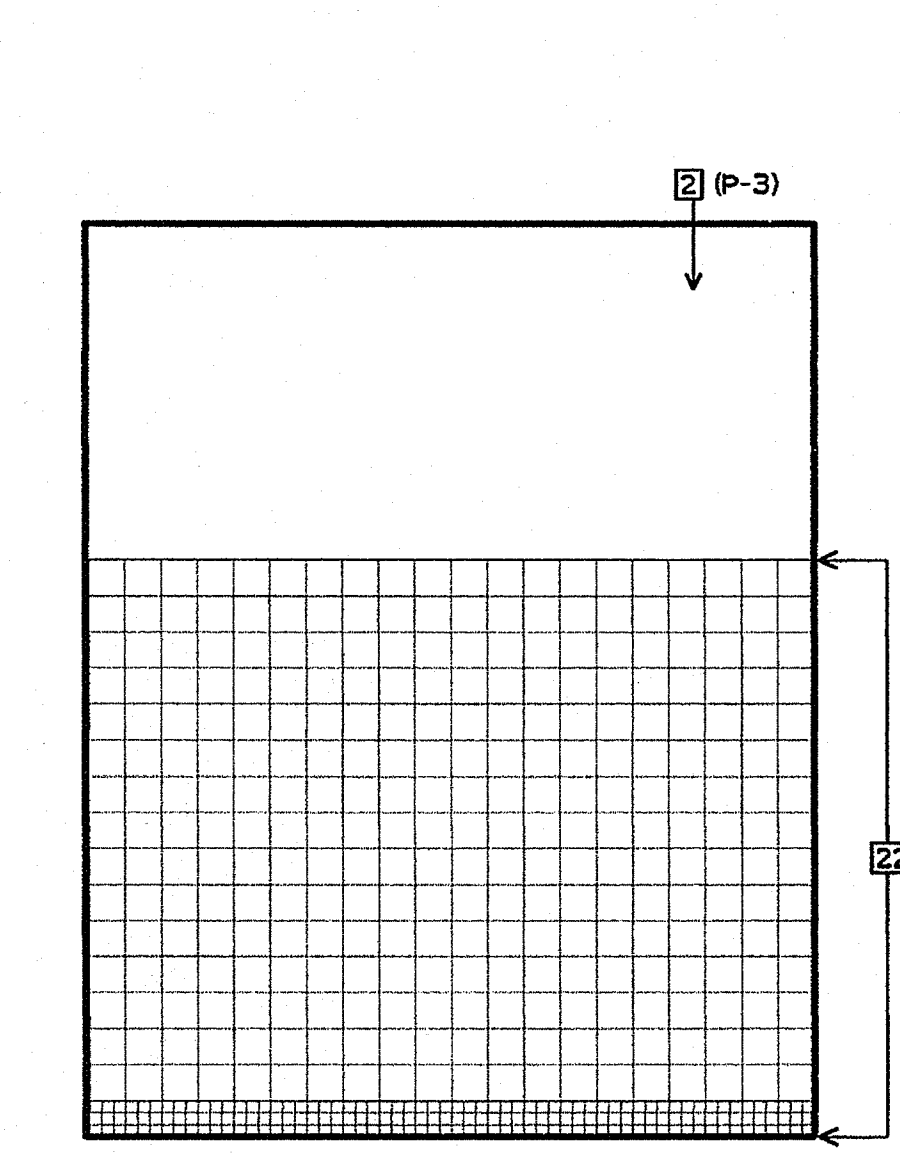
46 MENS DRESSING
3/8" = 1'-0"



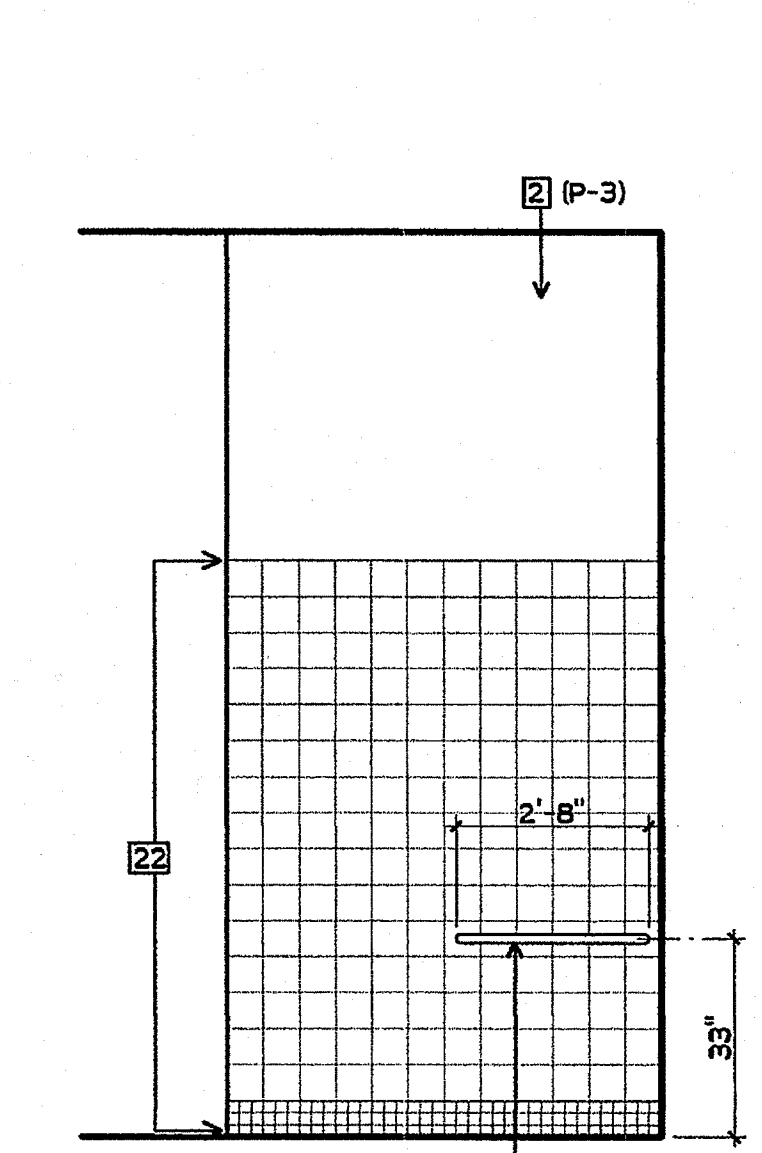
47 BOYS SHOWER
3/8" = 1'-0"



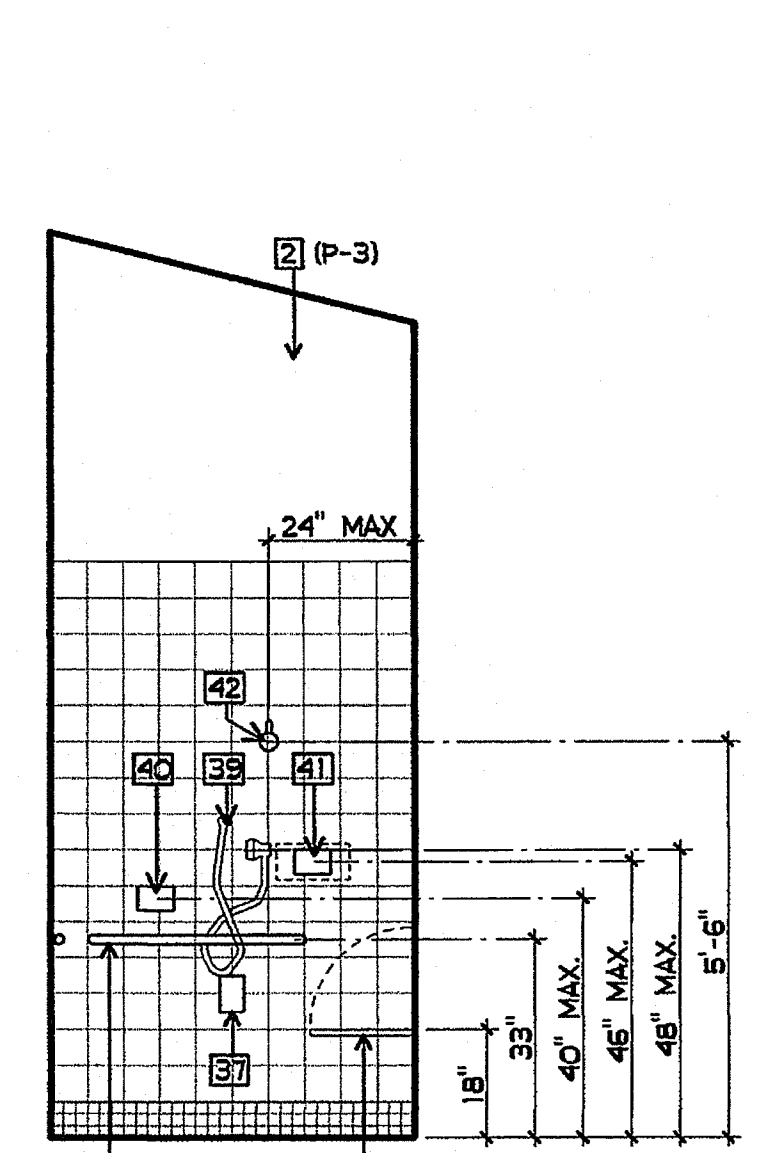
48 BOYS SHOWER
3/8" = 1'-0"



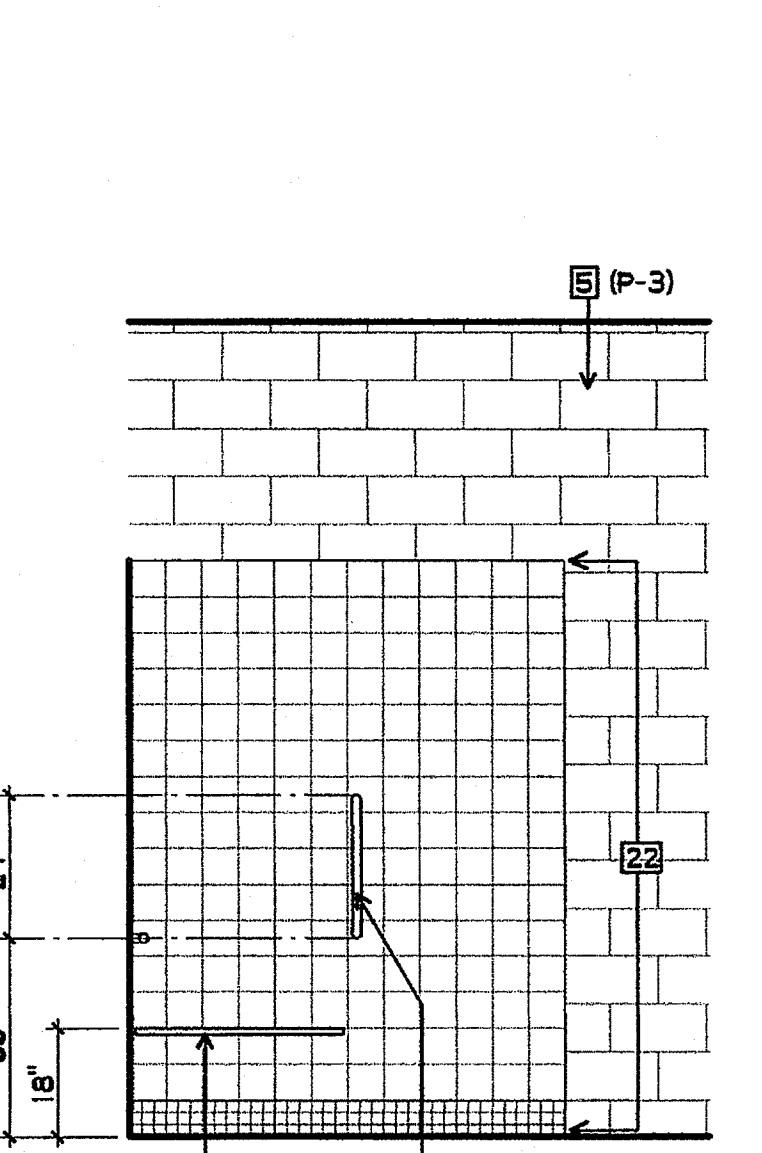
49 BOYS SHOWER
3/8" = 1'-0"



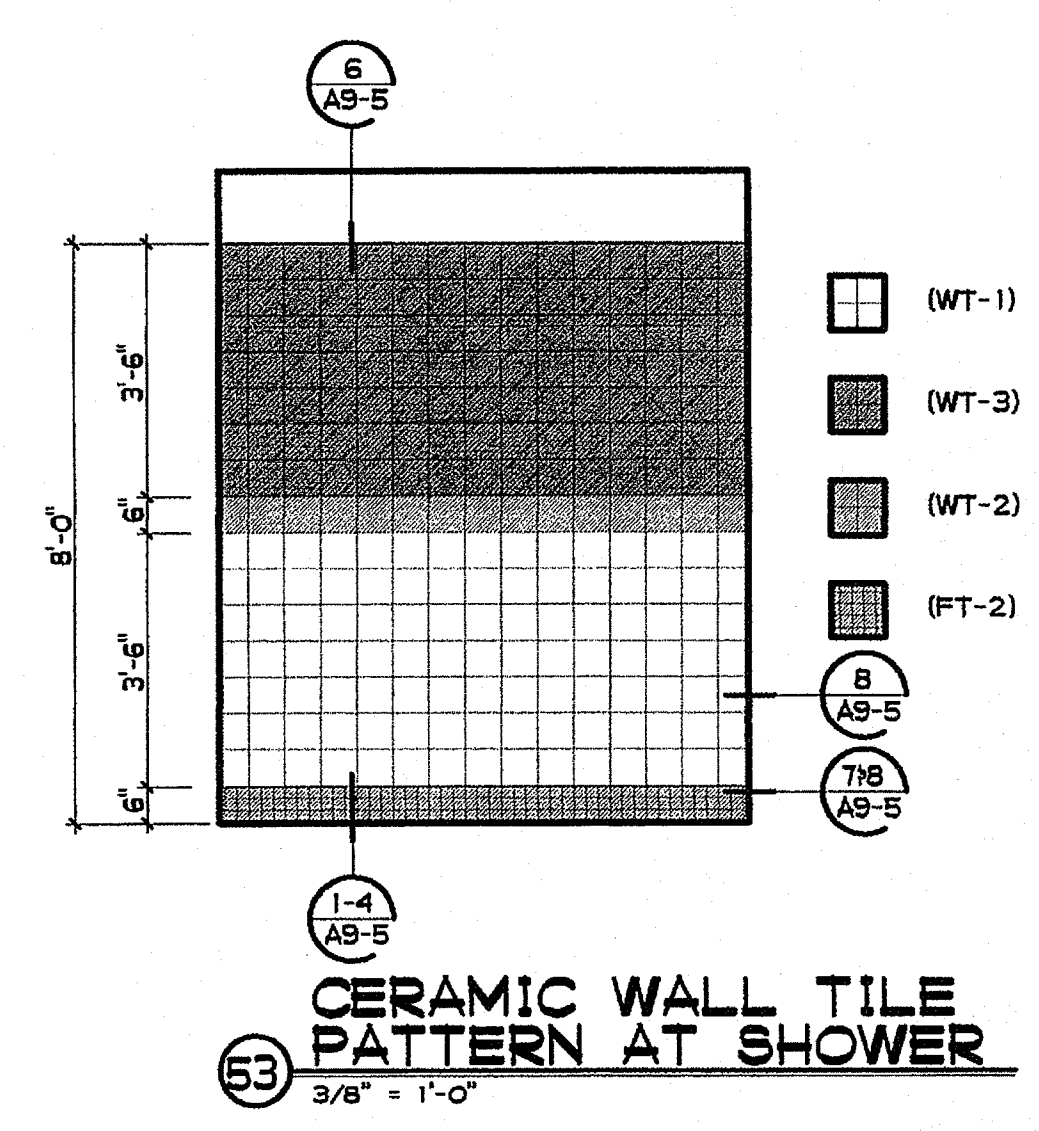
50 BOYS ACCESS. SHOWER
3/8" = 1'-0"



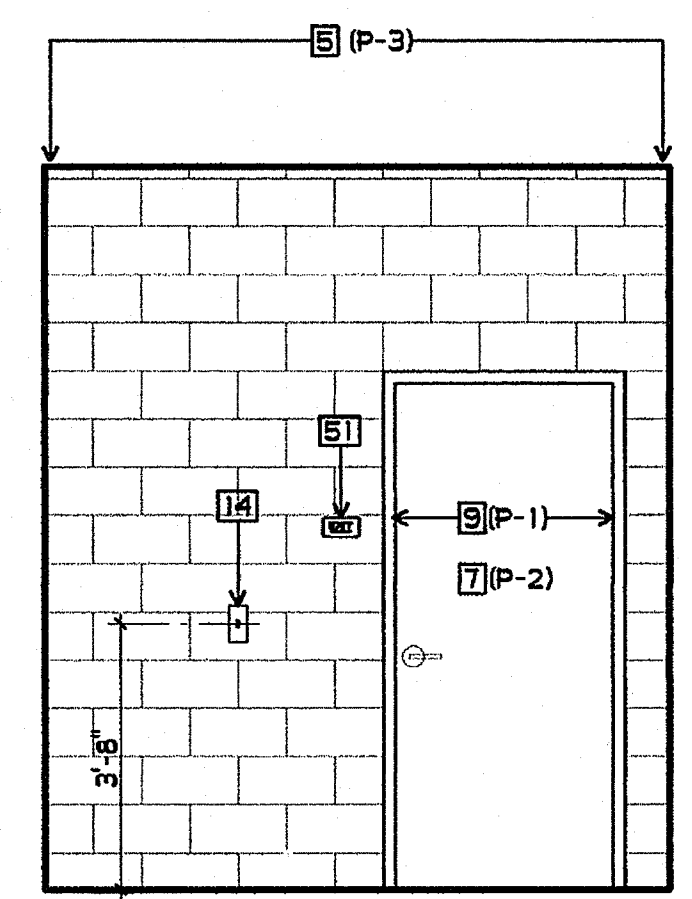
51 BOYS ACCESS. SHOWER
3/8" = 1'-0"



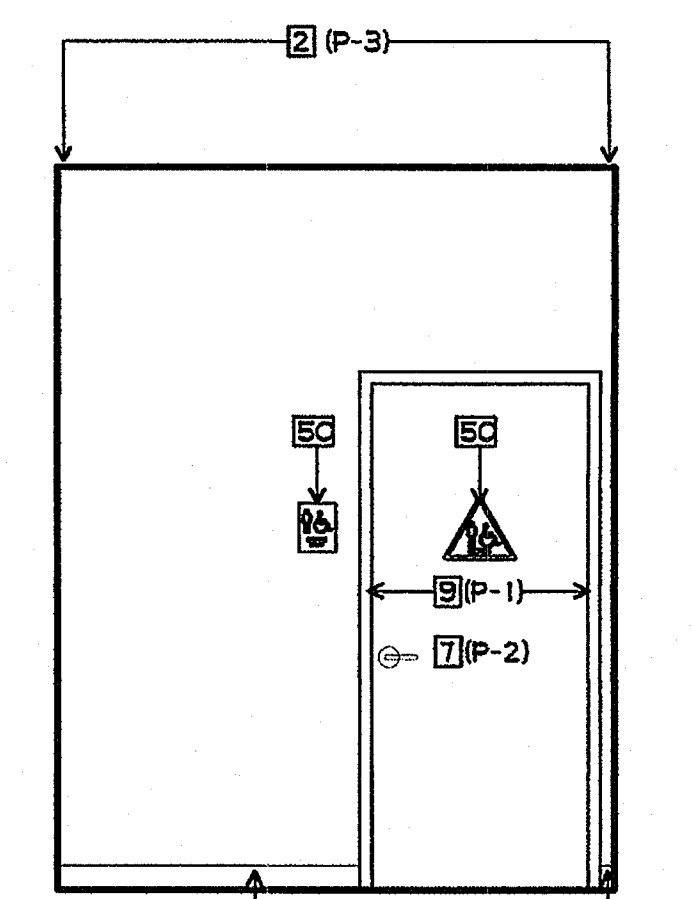
52 BOYS ACCESS. SHOWER
3/8" = 1'-0"



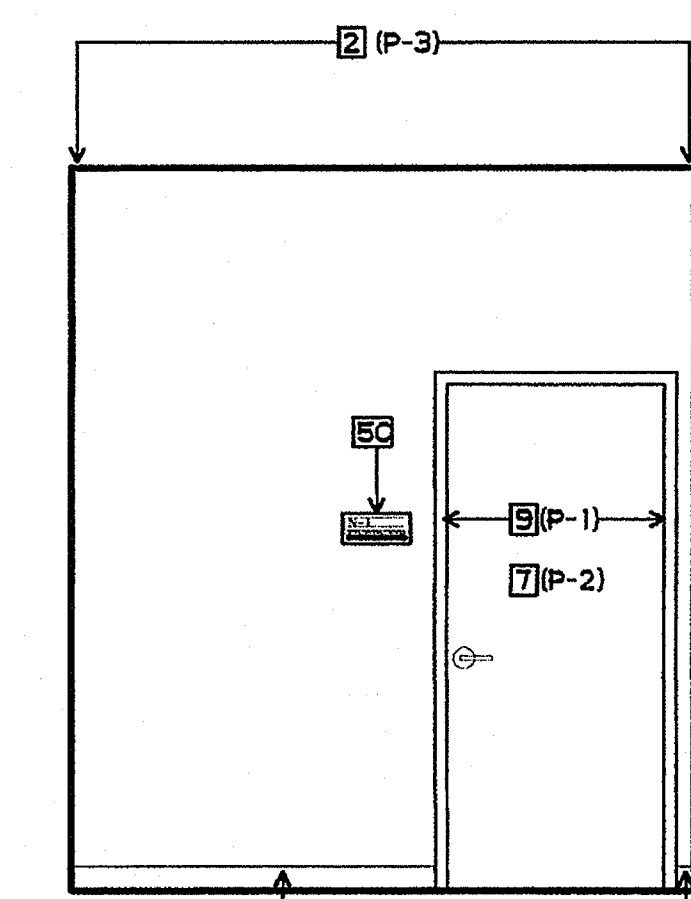
53 CERAMIC WALL TILE PATTERN AT SHOWER
3/8" = 1'-0"



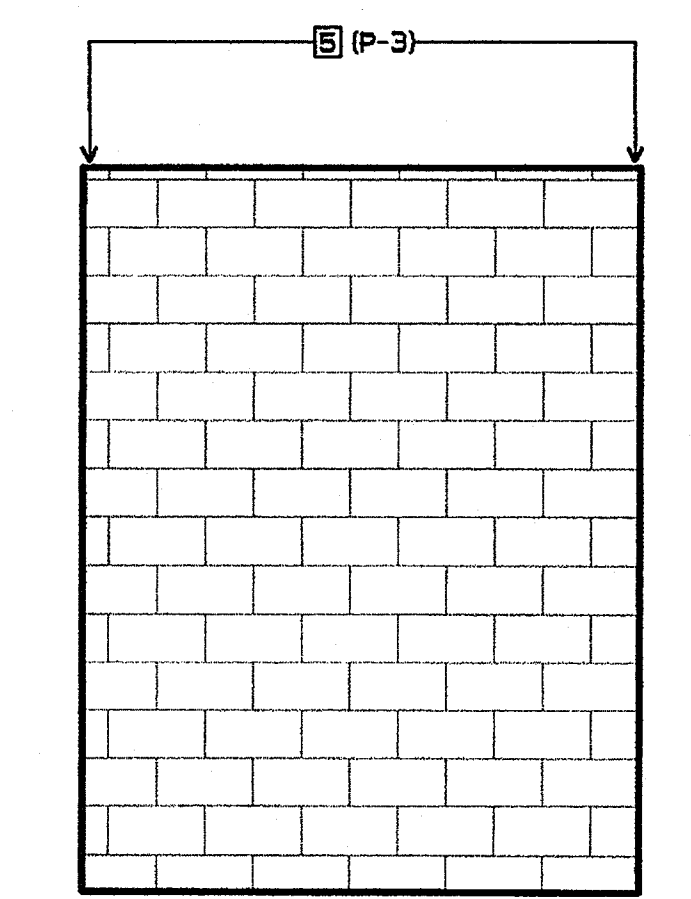
54 VESTIBULE
3/8" = 1'-0"



55 VESTIBULE
3/8" = 1'-0"



56 VESTIBULE
3/8" = 1'-0"



57 VESTIBULE
3/8" = 1'-0"

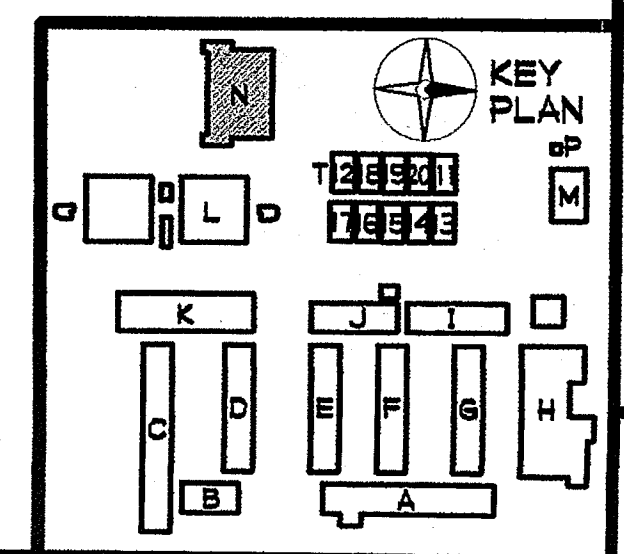
SHEET NOTES

- 2 5/8" GBX - ORANGE PEEL TEXT. - SEMI-GLOSS PAINT
- 5 CMU - SEMI-GLOSS PAINT
- 7 DOOR - SEMI-GLOSS PAINT
- 9 METAL FRAME - SEMI-GLOSS PAINT
- 10 BASE
- 12 POWER OUTLET
- 14 LIGHT CONTROLS
- 17 LOUVER - SEE MECHANICAL DRAWINGS
- 22 CERAMIC TILE - PATTERN PER 29/ANT-4
- 31 GRAB BARS - SEE 11/A9-5 AND 12/A9-5
- 37 HOSE BIBB WITH COVER
- 38 FOLDING SHOWER SEAT
- 39 FLEXIBLE SPRAY HOSE, 60" LONG
- 40 SOAP DISH
- 41 SINGLE LEVEL MIXING VALVE CONTROL
- 42 FIXED SHOWER HEAD

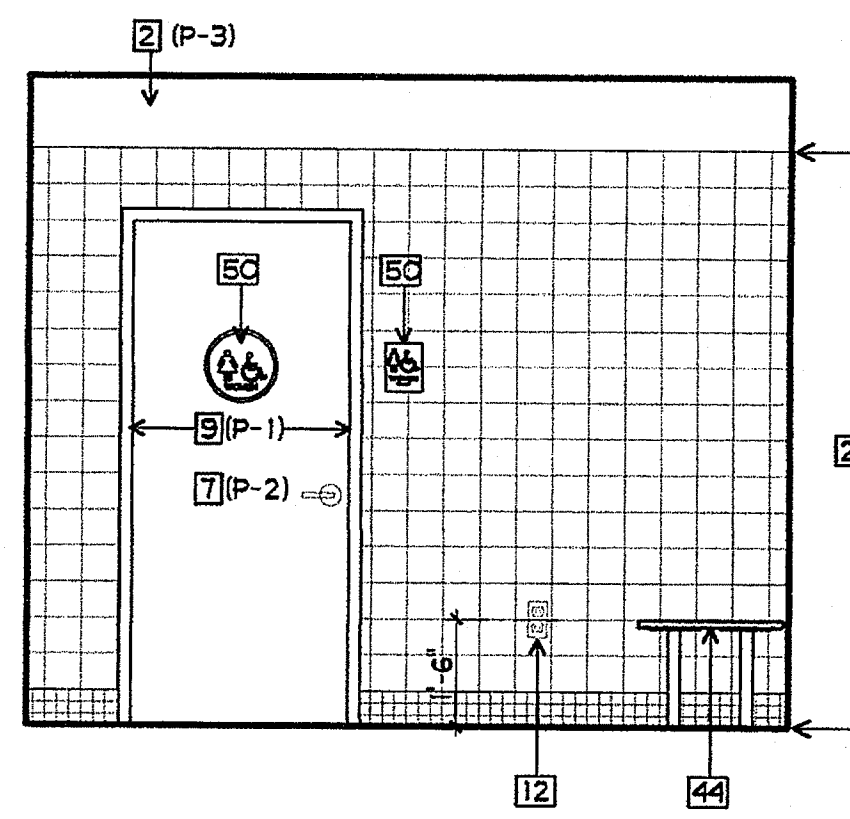
- 43 GANG SHOWER
- 44 BENCH
- 45 LOCKERS
- 46 CONCRETE CURB
- 50 DOOR SIGNAGE - SEE DOOR SCHEDULE
- 51 EXIT SIGN - SEE SIGNAGE PLAN AND 7/A9-5

FINISH PALETTE

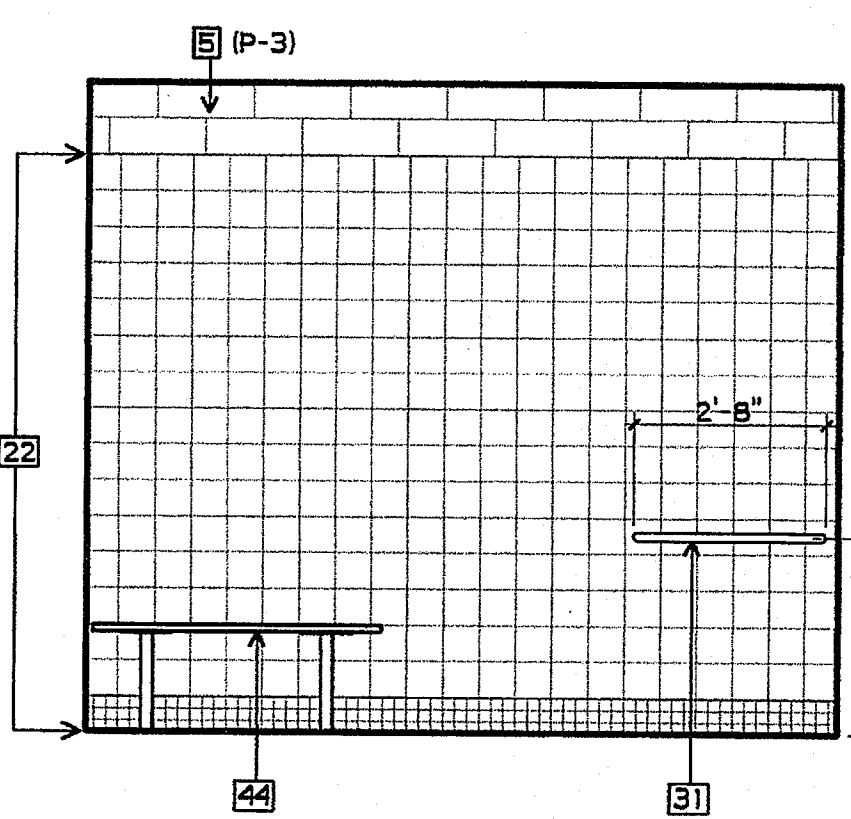
- (WT-1) CERAMIC WALL TILE, COLOR #1
- (WT-2) CERAMIC WALL TILE, COLOR #2
- (WT-3) CERAMIC WALL TILE, COLOR #3
- (FT-2) CERAMIC FLOOR TILE, COLOR #2
- (P-1) PAINT, COLOR #1
- (P-2) PAINT, COLOR #2
- (P-3) PAINT, COLOR #3



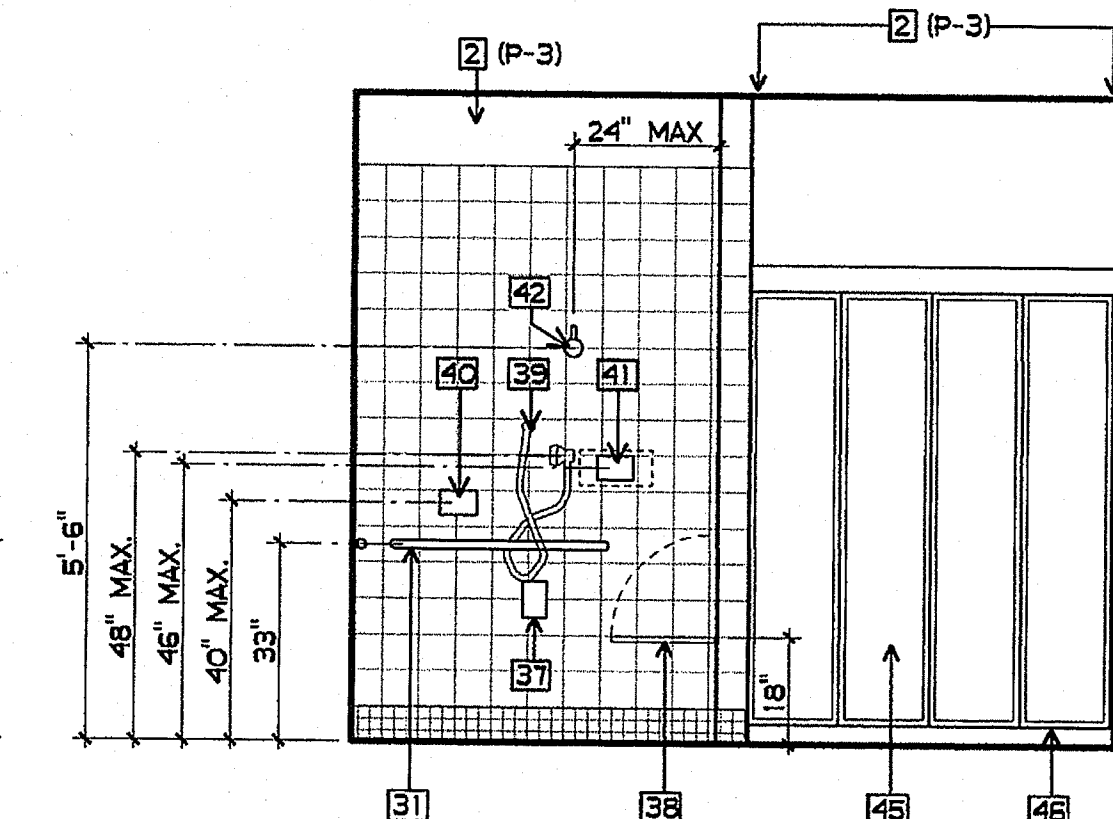
PLOTTED 3/18/2005 12:43 PM
 GROTH ARCHITECTS, INC.
 823 ACACIA STREET
 OCEANSIDE, CA 92054
 OCEANSIDE UNIFIED S.D.
 PHONE 760-754-8191
 FAX 760-754-8291
 OLSD NO. 758-000
 PROJECT NOS. 025
 P. T. N. 73569-9
 DATE
 REVISIONS
 JEFFERSON MS NEW CONSTRUCTION
 823 ACACIA STREET
 OCEANSIDE, CA 92054
 OCEANSIDE UNIFIED S.D.
 GROTH ARCHITECTS, INC.
 3355 MISSION AVE. SUITE 234
 OCEANSIDE, CALIFORNIA 92054
 DSA
 IDENTIFICATION STAMP
 DIV. OF THE STATE ARCHITECT
 OFFICE OF REGULATION SERVICES
 4-106494
 AC [initials] FL [initials] SS [initials]
 DATE MAR 28 2005
 LICENSED ARCHITECT
 JOHN SCOTT GROTH
 C-26609
 4/30/2007
 RENEWAL
 STATE OF CALIFORNIA
 BUILDING N
 INTERIOR
 ELEVATIONS
 AN7-6



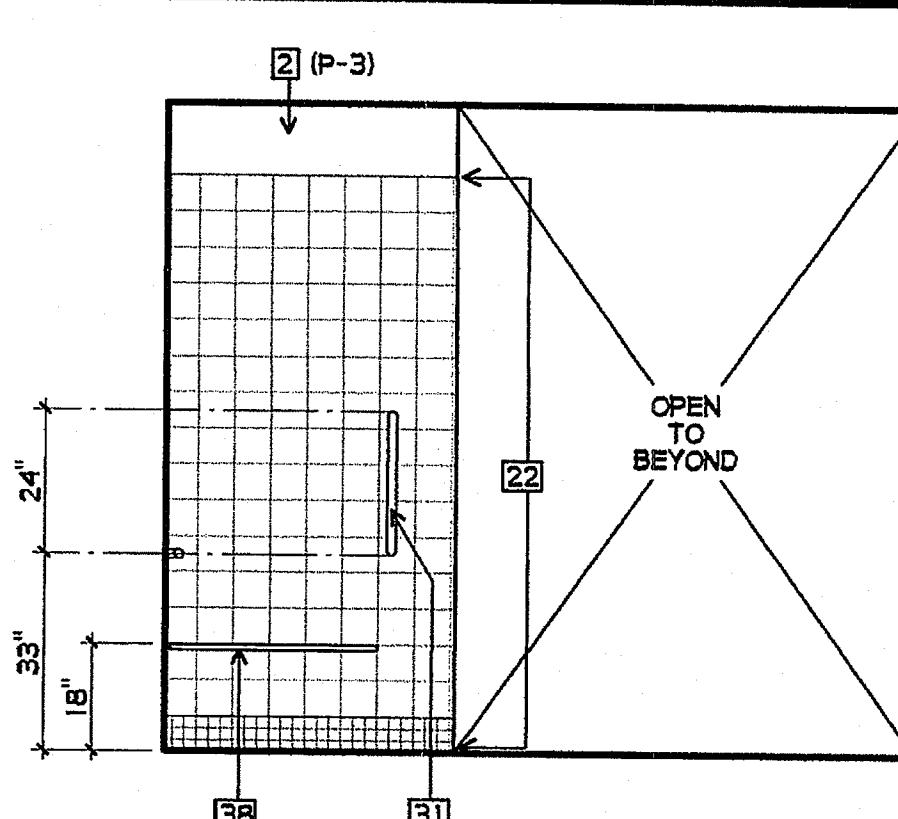
58 WOMENS DRESSING
3/8" = 1'-0"



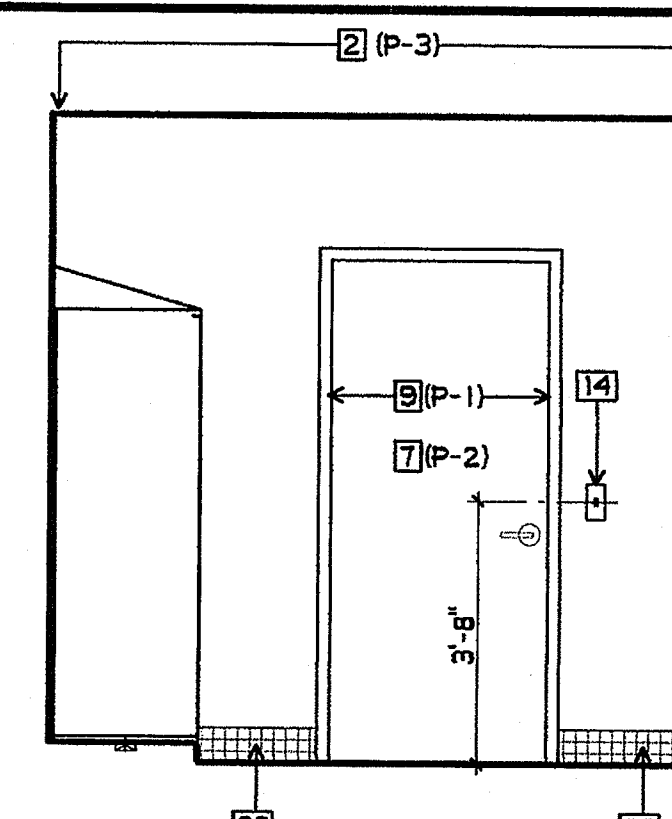
59 WOMENS DRESSING
3/8" = 1'-0"



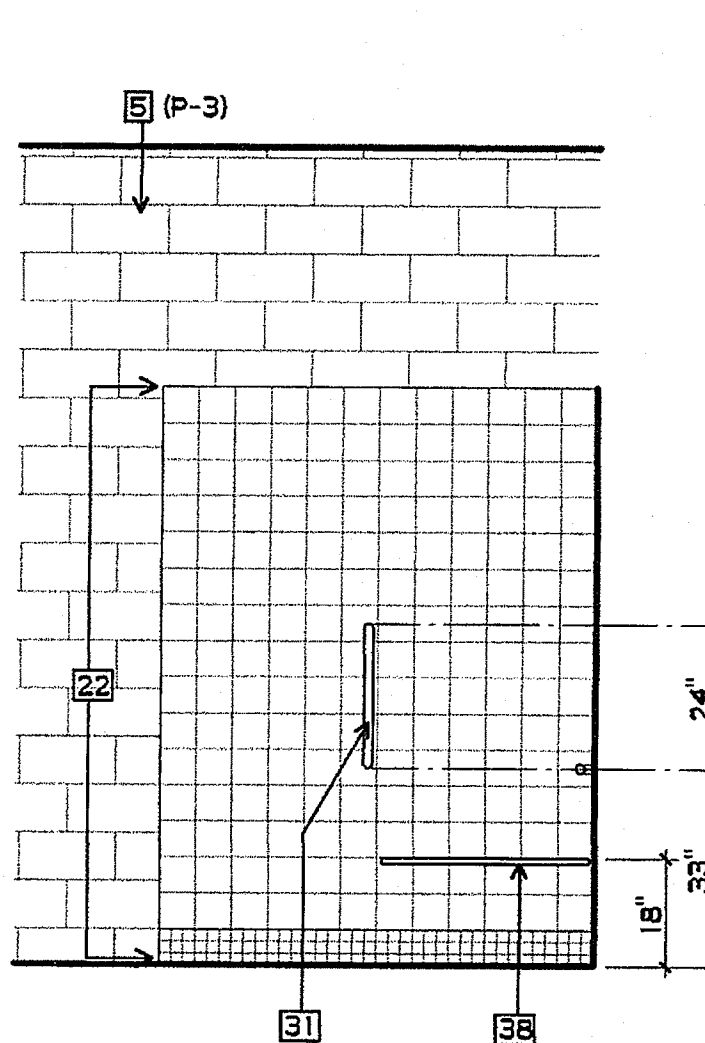
60 WOMENS DRESSING
3/8" = 1'-0"



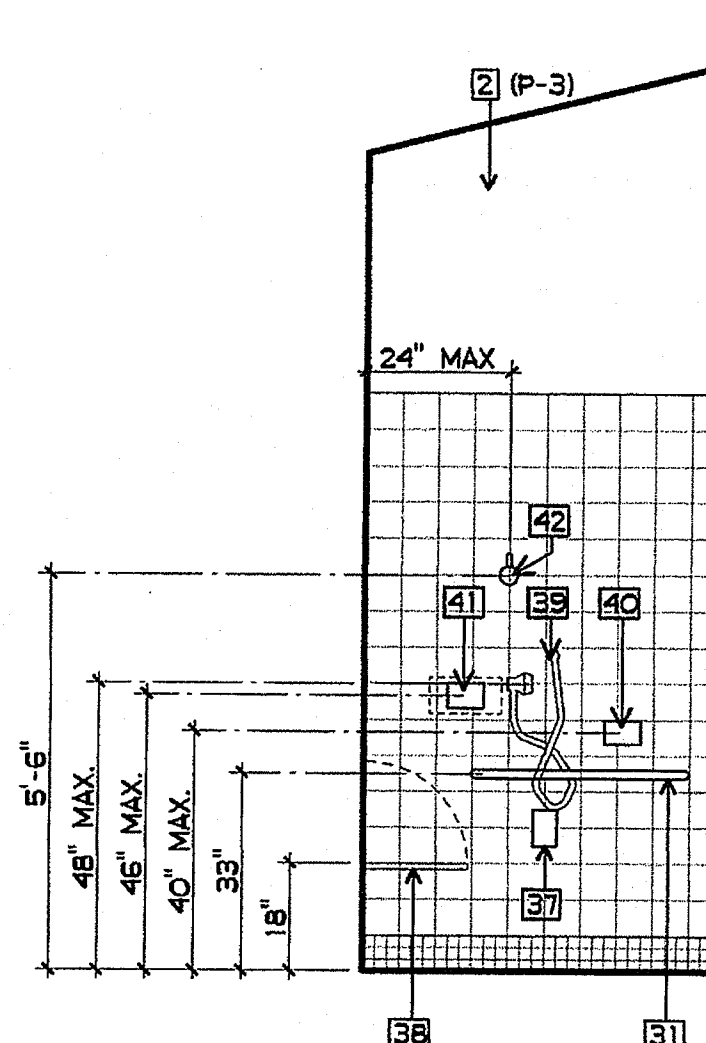
61 WOMENS DRESSING
3/8" = 1'-0"



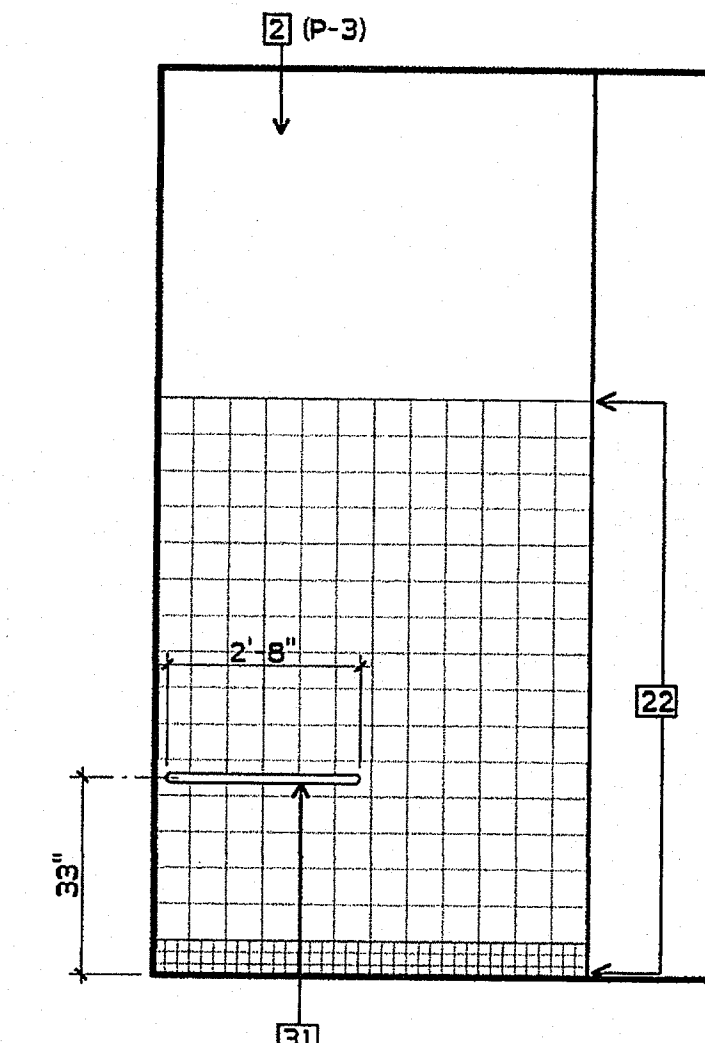
62 WOMENS DRESSING
3/8" = 1'-0"



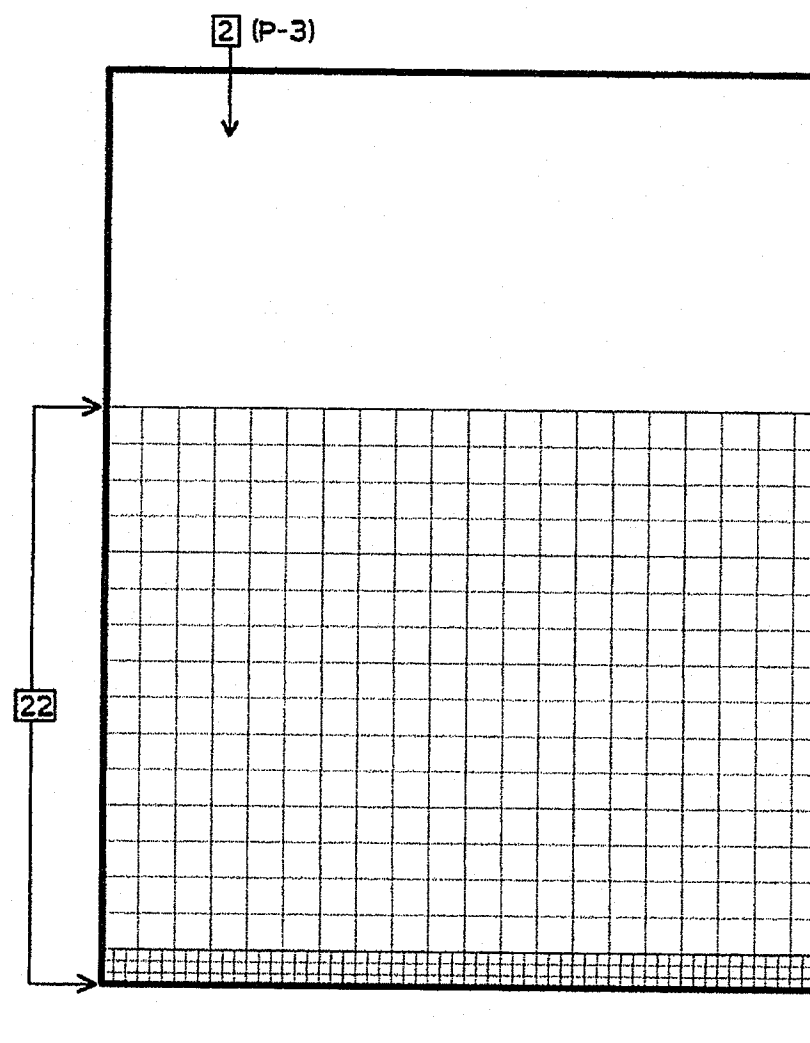
63 GIRLS ACCESS. SHOWER
3/8" = 1'-0"



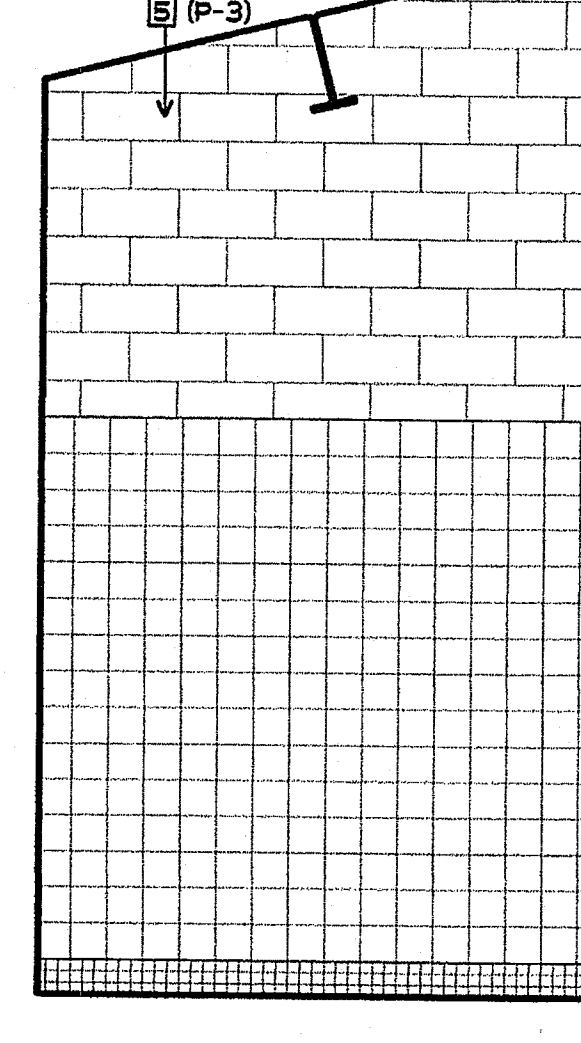
64 GIRLS ACCESS. SHOWER
3/8" = 1'-0"



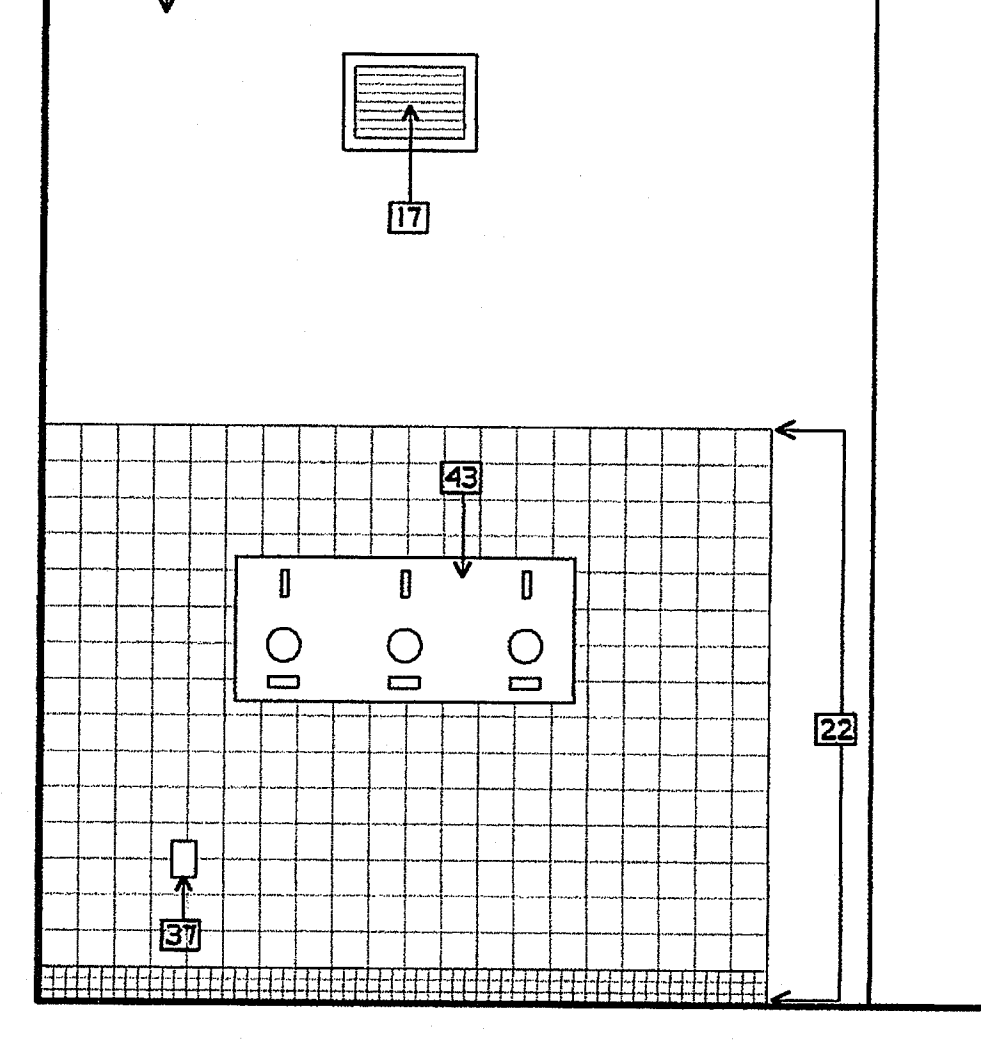
65 GIRLS ACCESS. SHOWER
3/8" = 1'-0"



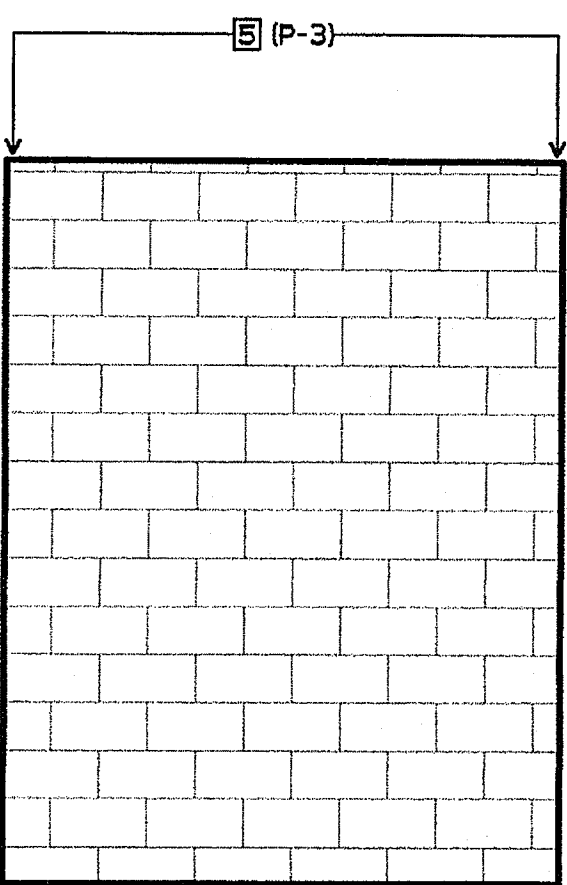
66 GIRLS SHOWER
3/8" = 1'-0"



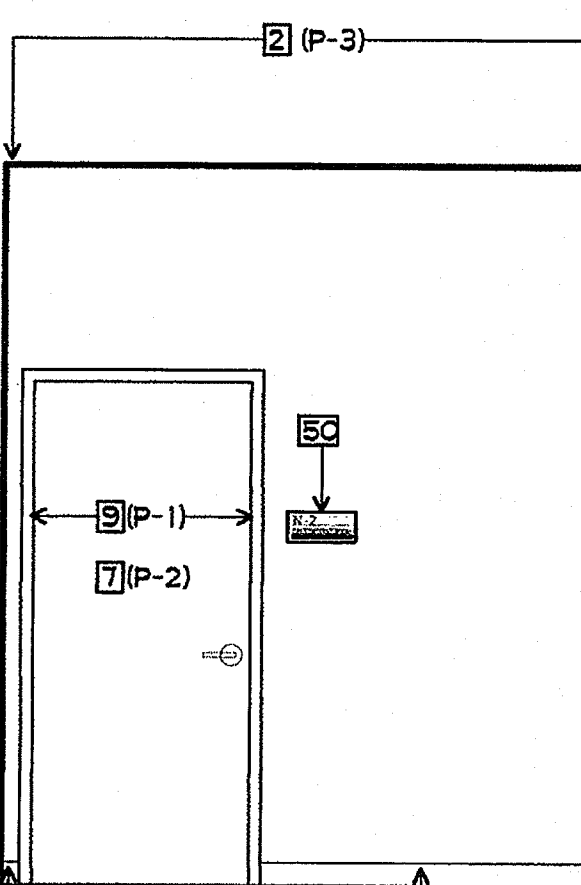
67 GIRLS SHOWER
3/8" = 1'-0"



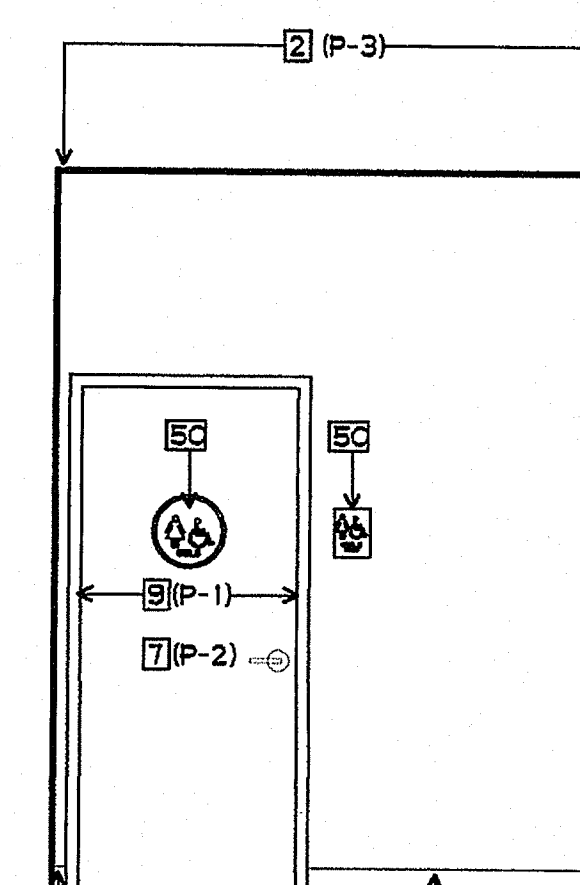
68 GIRLS SHOWER
3/8" = 1'-0"



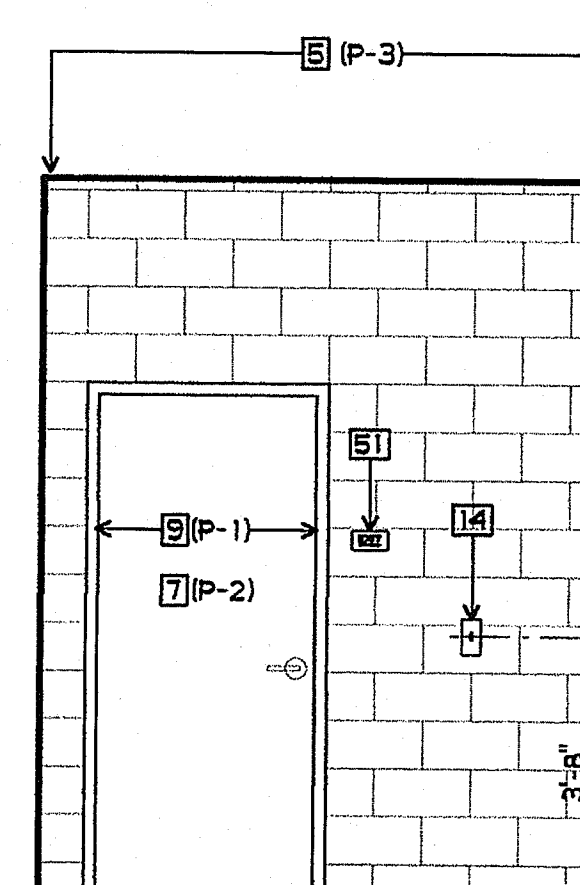
69 VESTIBULE
3/8" = 1'-0"



70 VESTIBULE
3/8" = 1'-0"



71 VESTIBULE
3/8" = 1'-0"



72 VESTIBULE
3/8" = 1'-0"

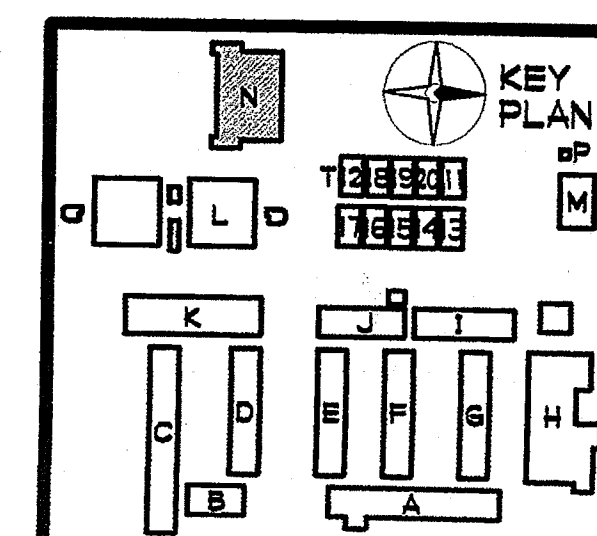
SHEET NOTES

- 2 5/8" GBX - ORANGE PEEL TEXT. - SEMI-GLOSS PAINT
- 5 CMU - SEMI-GLOSS PAINT
- 7 DOOR - SEMI-GLOSS PAINT
- 9 METAL FRAME - SEMI-GLOSS PAINT
- 10 BASE
- 12 POWER OUTLET
- 14 LIGHT CONTROLS
- 17 LOUVERS - SEE MECHANICAL DRAWINGS
- 22 CERAMIC TILE - PATTERN PER 29/AN7-4
- 31 GRAB BARS - SEE 11/A9-5 AND 12/A9-5
- 37 HOSE BIBB WITH COVER
- 38 FOLDING SHOWER SEAT
- 39 FLEXIBLE SPRAY HOSE, 60" LONG
- 40 SOAP DISH
- 41 SINGLE LEVEL MIXING VALVE CONTROL
- 42 FIXED SHOWER HEAD

- 43 GANG SHOWER
- 44 BENCH
- 45 LOCKERS
- 46 CONCRETE CURB
- 50 DOOR SIGNAGE - SEE DOOR SCHEDULE
- 51 EXIT SIGN - SEE SIGNAGE PLAN AND 7/A9-8

FINISH PALETTE

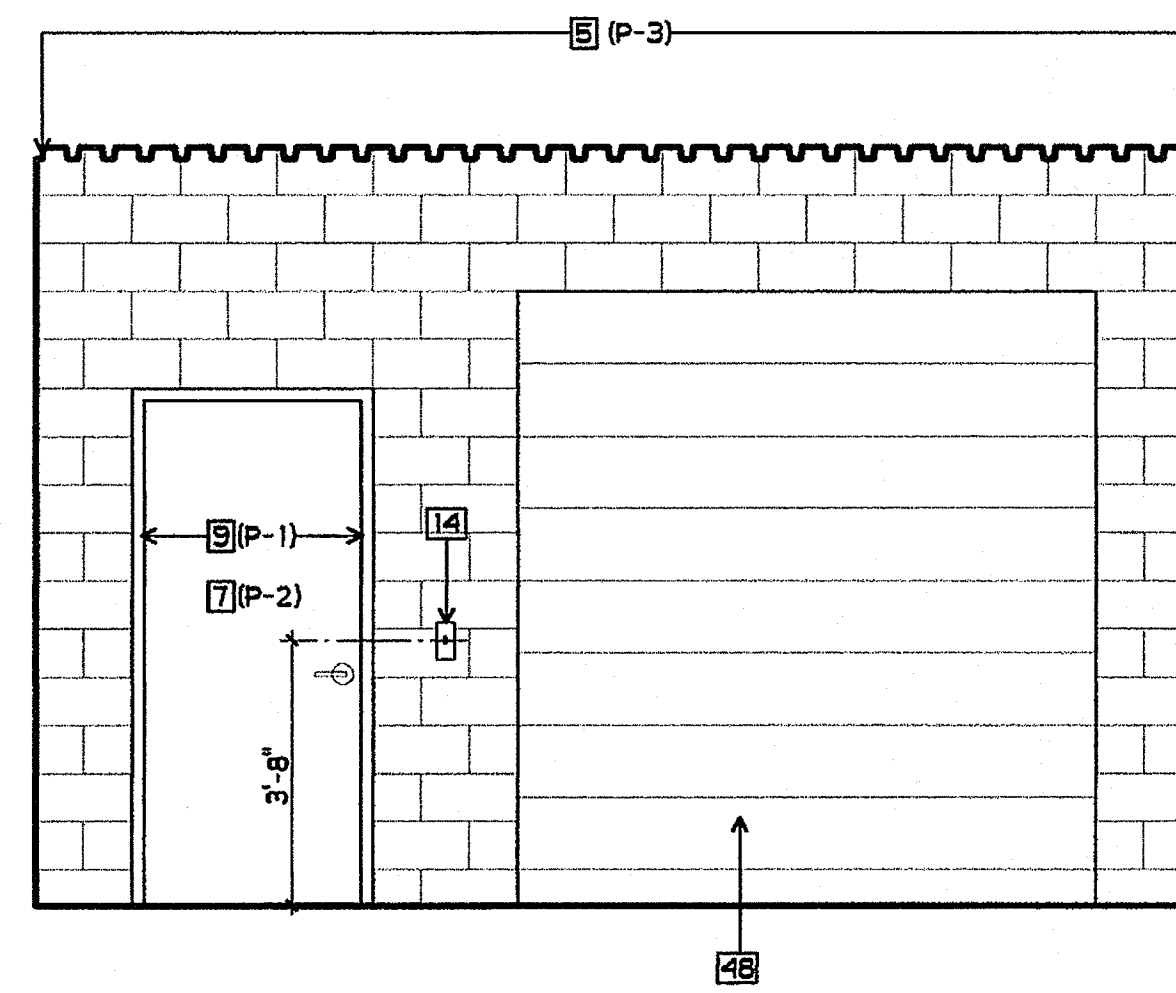
- (WT-1) CERAMIC WALL TILE, COLOR #1
- (WT-2) CERAMIC WALL TILE, COLOR #2
- (WT-3) CERAMIC WALL TILE, COLOR #3
- (FT-2) CERAMIC FLOOR TILE, COLOR #2
- (P-1) PAINT, COLOR #1
- (P-2) PAINT, COLOR #2
- (P-3) PAINT, COLOR #3



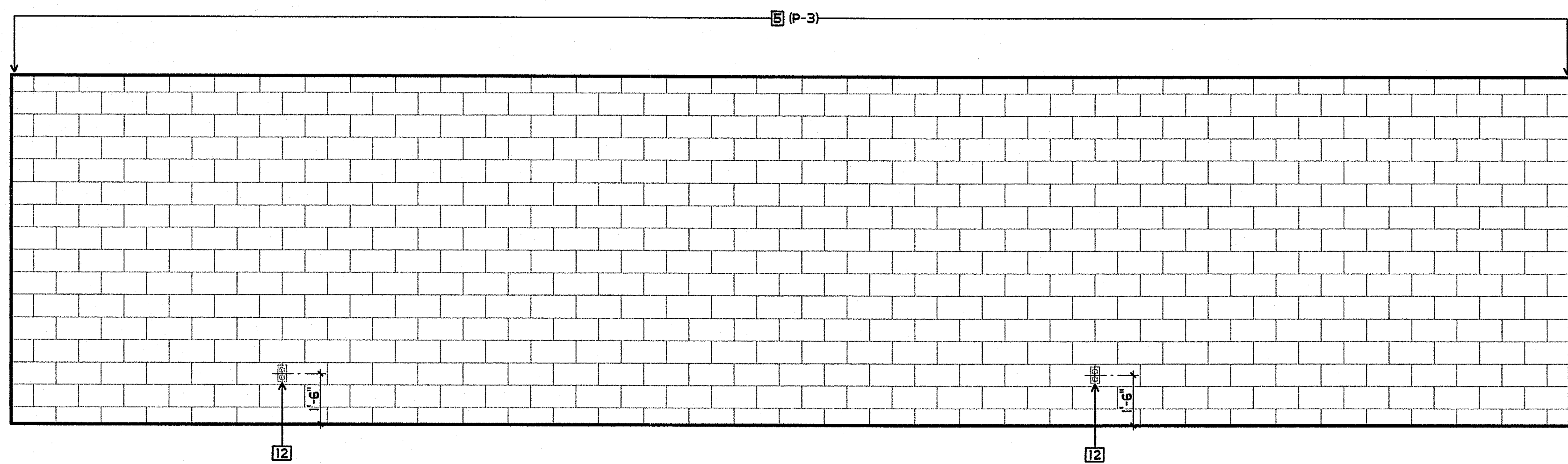
KEY PLAN
BUILDING N
INTERIOR
ELEVATIONS

AN7-7

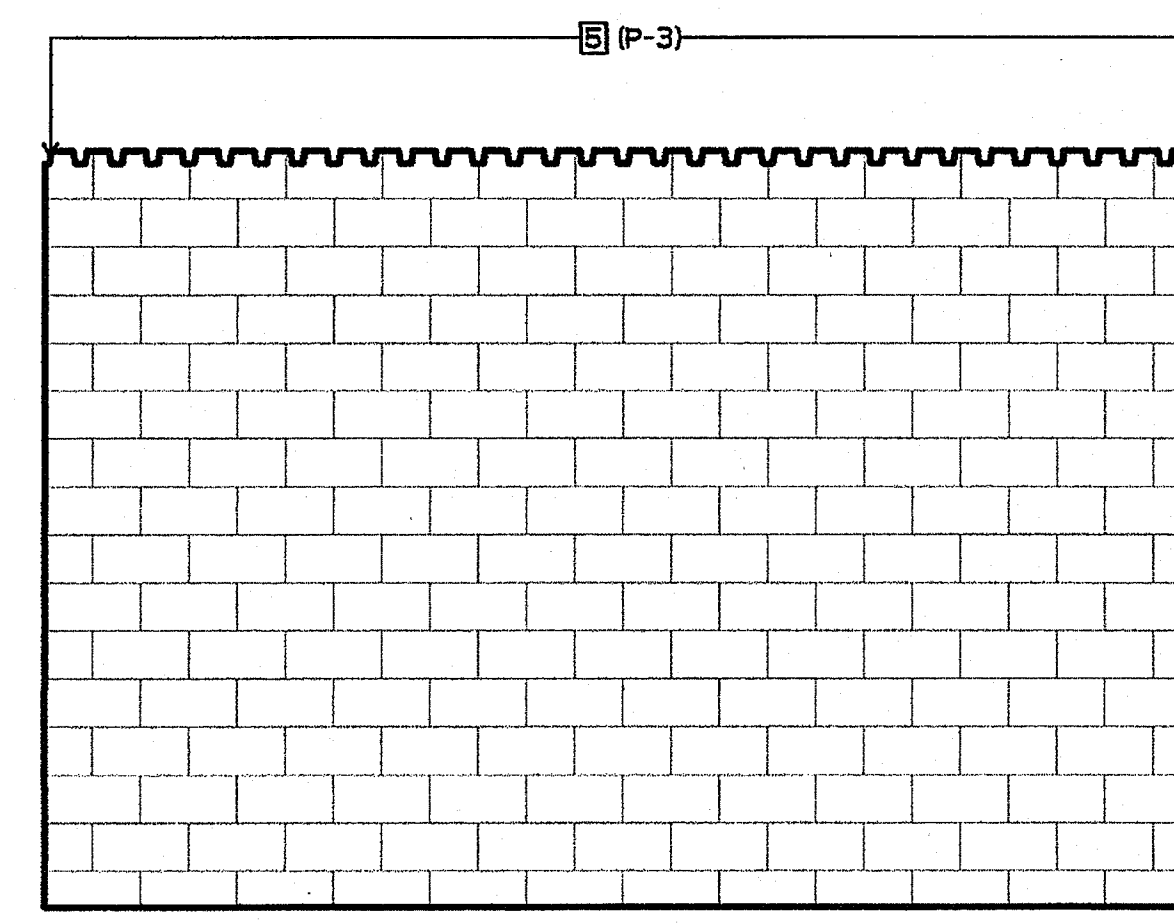
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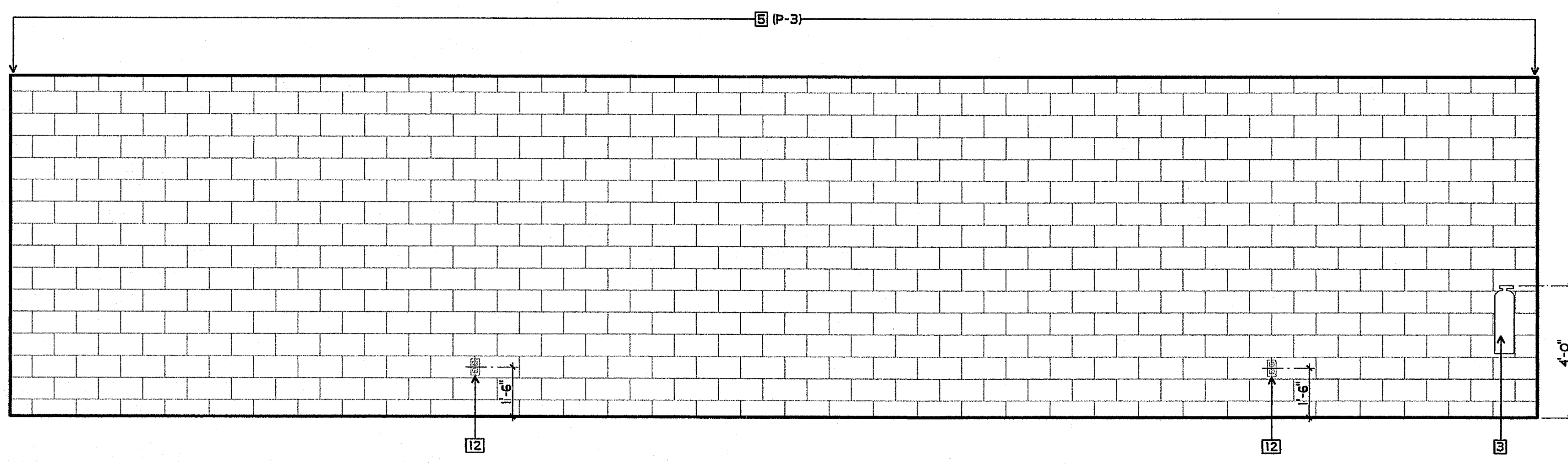
73 P.E. STORAGE
3/8" = 1'-0"



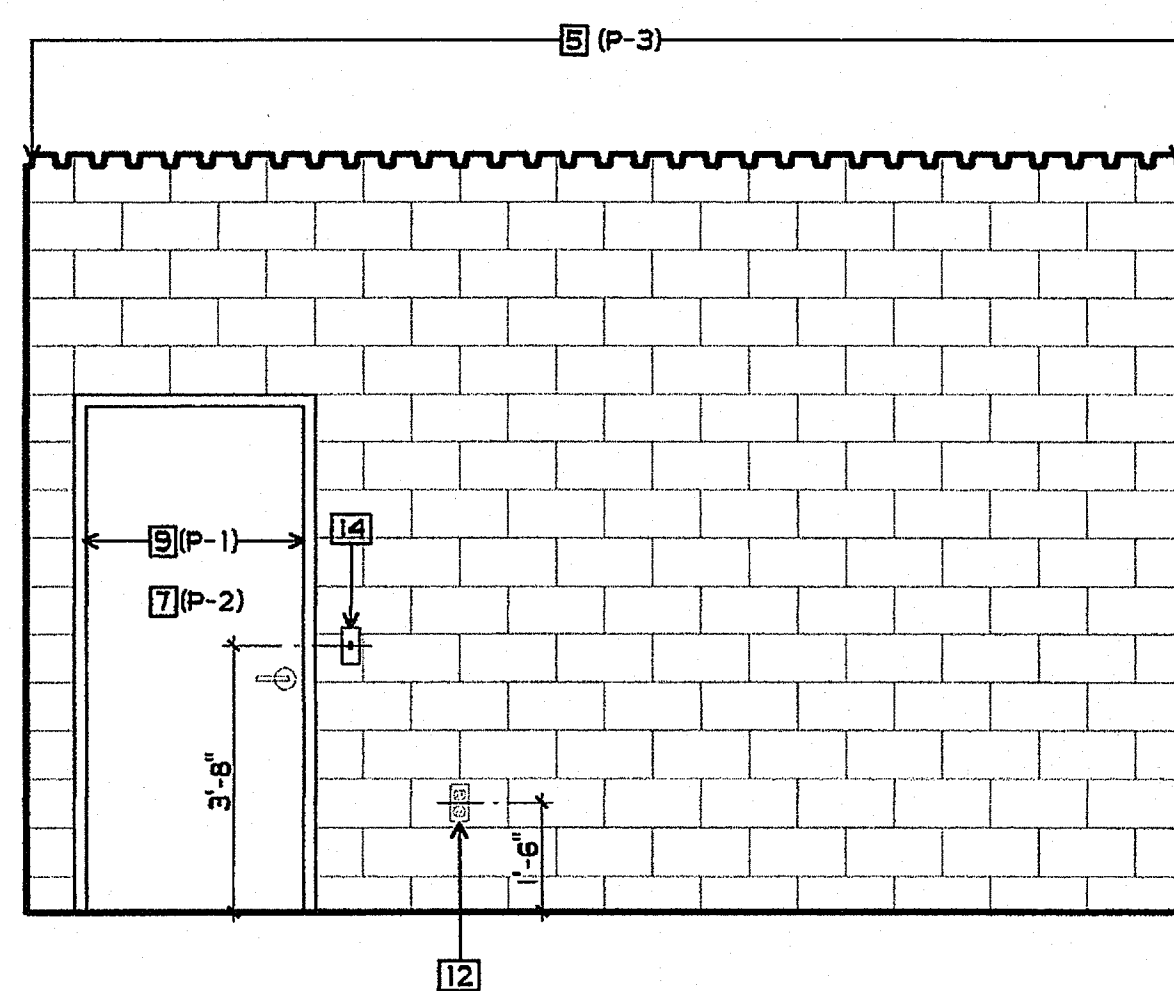
74 P.E. STORAGE
3/8" = 1'-0"



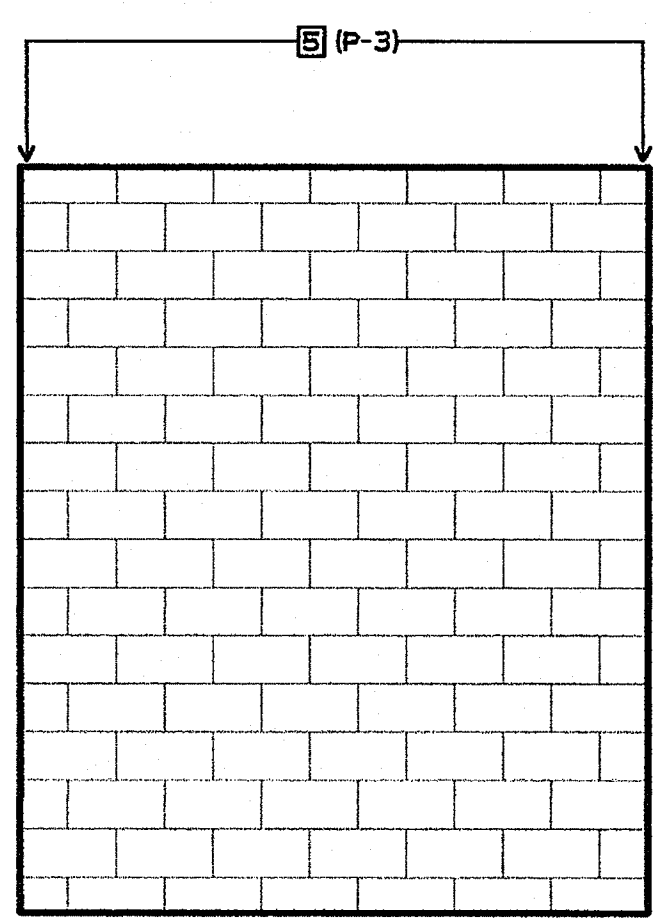
75 P.E. STORAGE
3/8" = 1'-0"



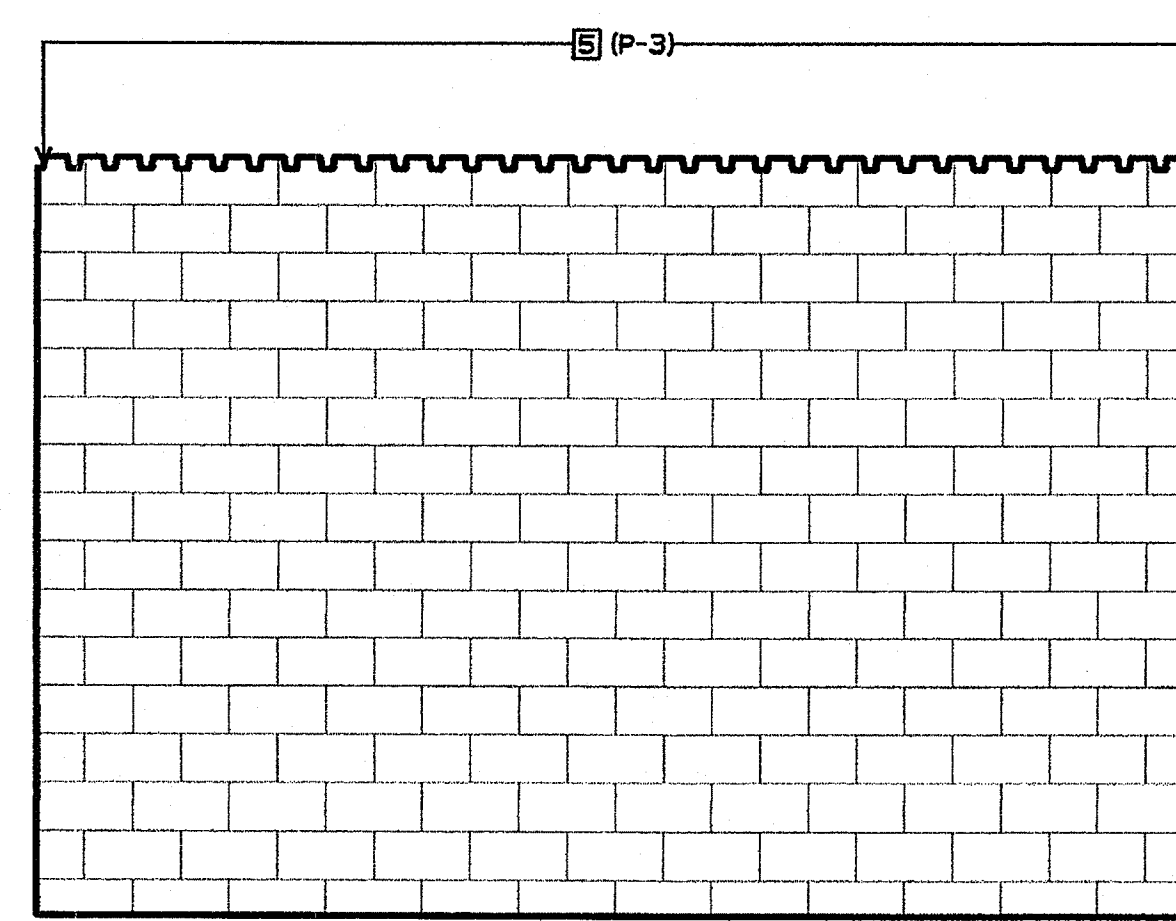
76 P.E. STORAGE
3/8" = 1'-0"



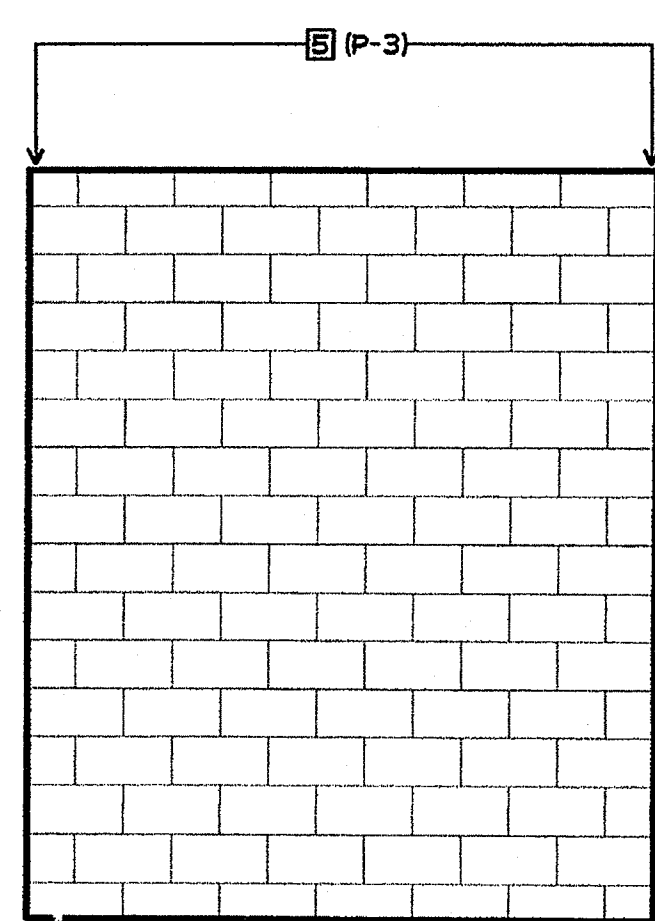
77 STORAGE
3/8" = 1'-0"



78 STORAGE
3/8" = 1'-0"



79 STORAGE
3/8" = 1'-0"



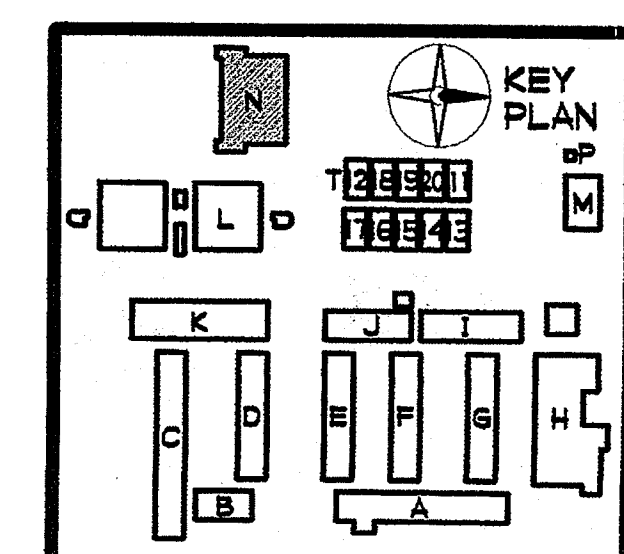
80 STORAGE
3/8" = 1'-0"

SHEET NOTES

- 46 BRACKET MOUNTED FIRE EXTINGUISHER
- 9 CMU - SEMI-GLOSS PAINT
- 7 DOOR - SEMI-GLOSS PAINT
- 9 METAL FRAME - SEMI-GLOSS PAINT
- 12 POWER OUTLET
- 14 LIGHT CONTROLS
- 46 ROLL UP DOOR

FINISH PALETTE

- (P-1) PAINT, COLOR #1
- (P-2) PAINT, COLOR #2
- (P-3) PAINT, COLOR #3



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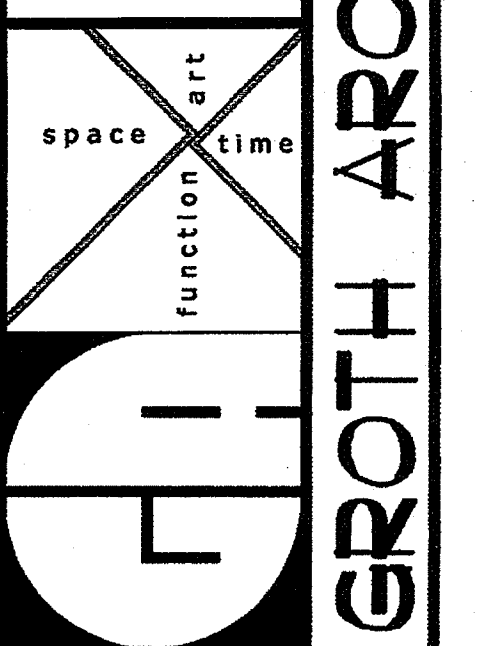
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 INTERIOR
 ELEVATIONS
AN7-8

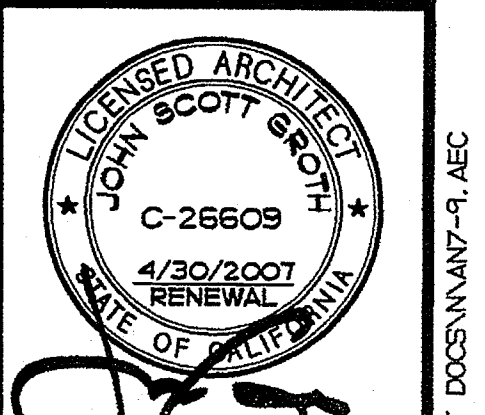
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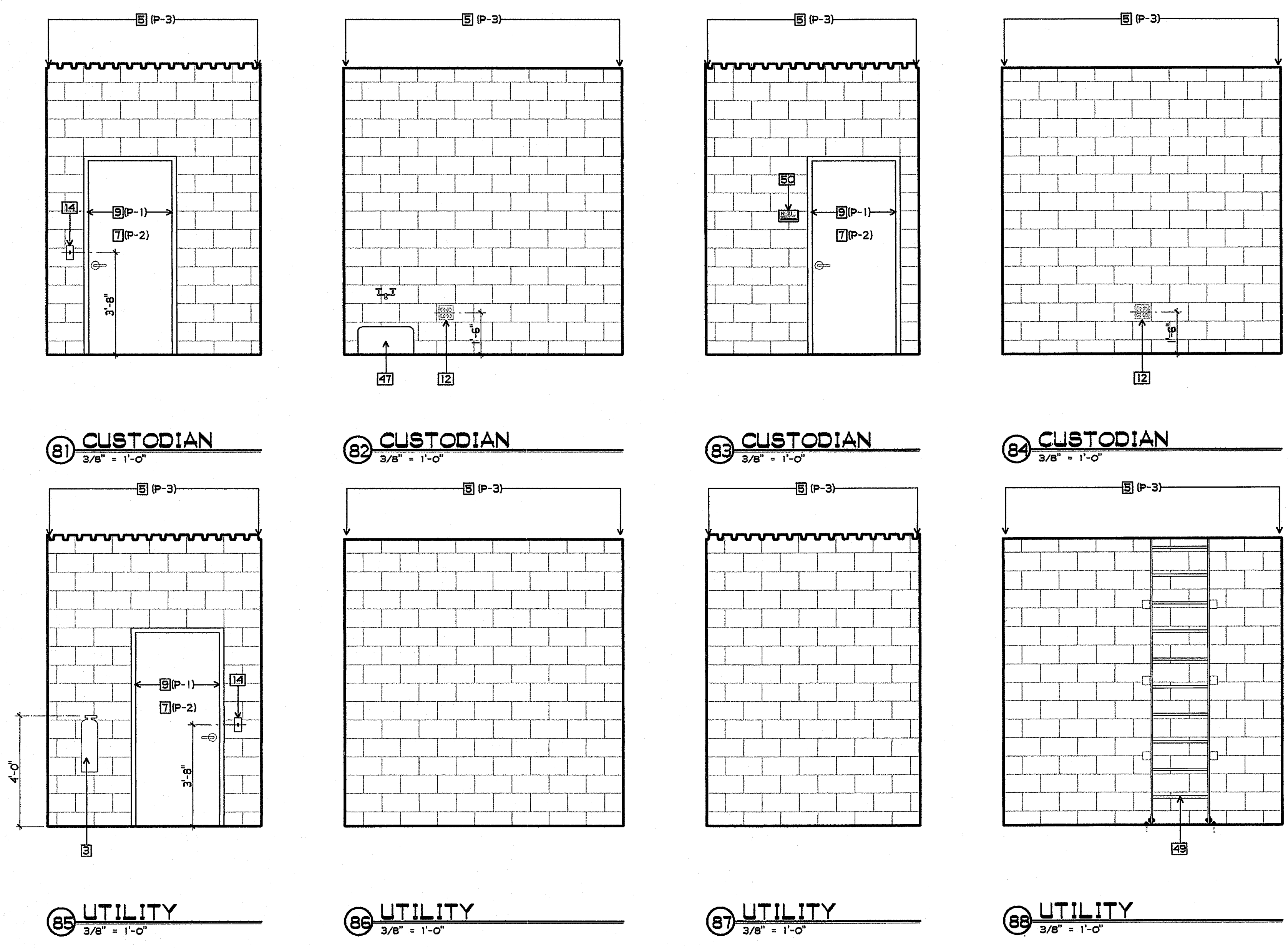
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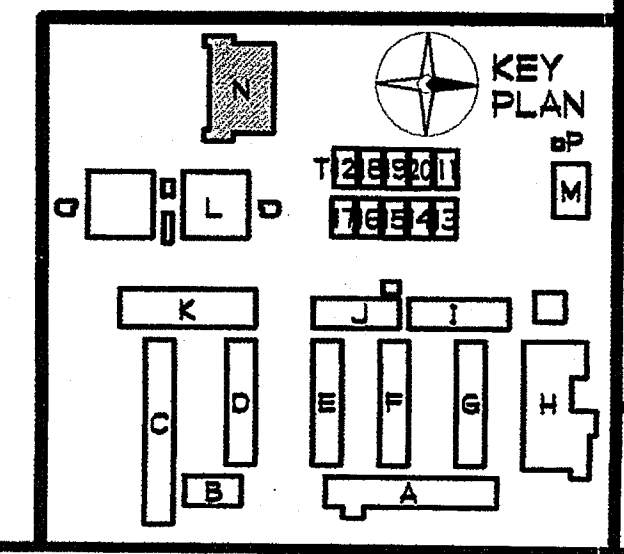


SHEET NOTES

- 5 BRACKET MOUNTED FIRE EXTINGUISHER
- 5 CMU - SEMI-GLOSS PAINT
- 7 DOOR - SEMI-GLOSS PAINT
- 9 METAL FRAME - SEMI-GLOSS PAINT
- 12 POWER OUTLET
- 14 LIGHT CONTROLS
- 47 ROOF SERVICE SINK
- 49 ROOF ACCESS LADDER, SEE 3/AN4-18
- 5C DOOR SIGNAGE - SEE DOOR SCHEDULE

FINISH PALETTE

- (P-1) PAINT, COLOR #1
- (P-2) PAINT, COLOR #2
- (P-3) PAINT, COLOR #3



WALL / PARTITION RESUME

EXTERIOR WALL: SEE 1/A9-3, 6" x 20 GA. (MIN)
 STEEL STUDS AT 16" OC - SEE STRUCTURAL DRAWINGS
 EXTERIOR FINISH: STUCCO SYSTEM PER 1/A9-3
 INTERIOR FINISH: 5/8" TYPE "X" GYP. BOARD
 USE MR GYP. BOARD WHERE REQUIRED

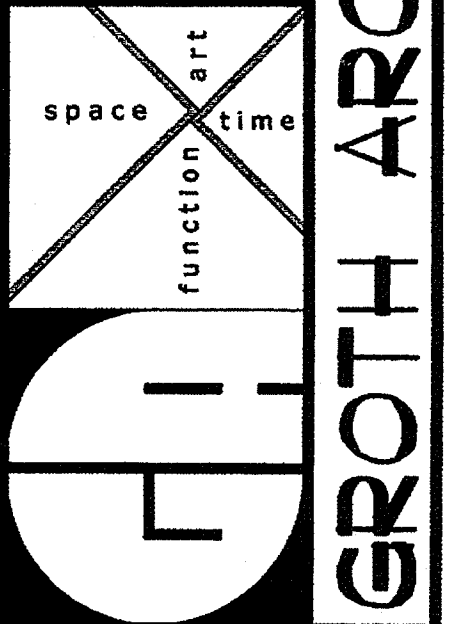
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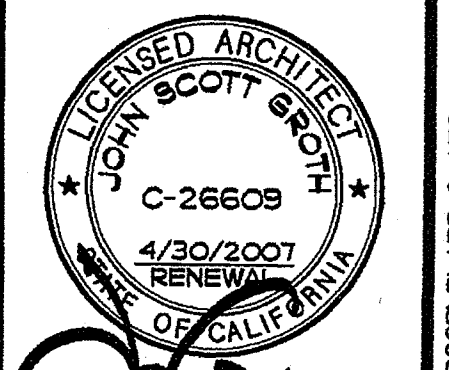
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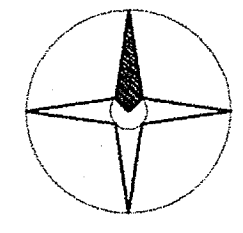
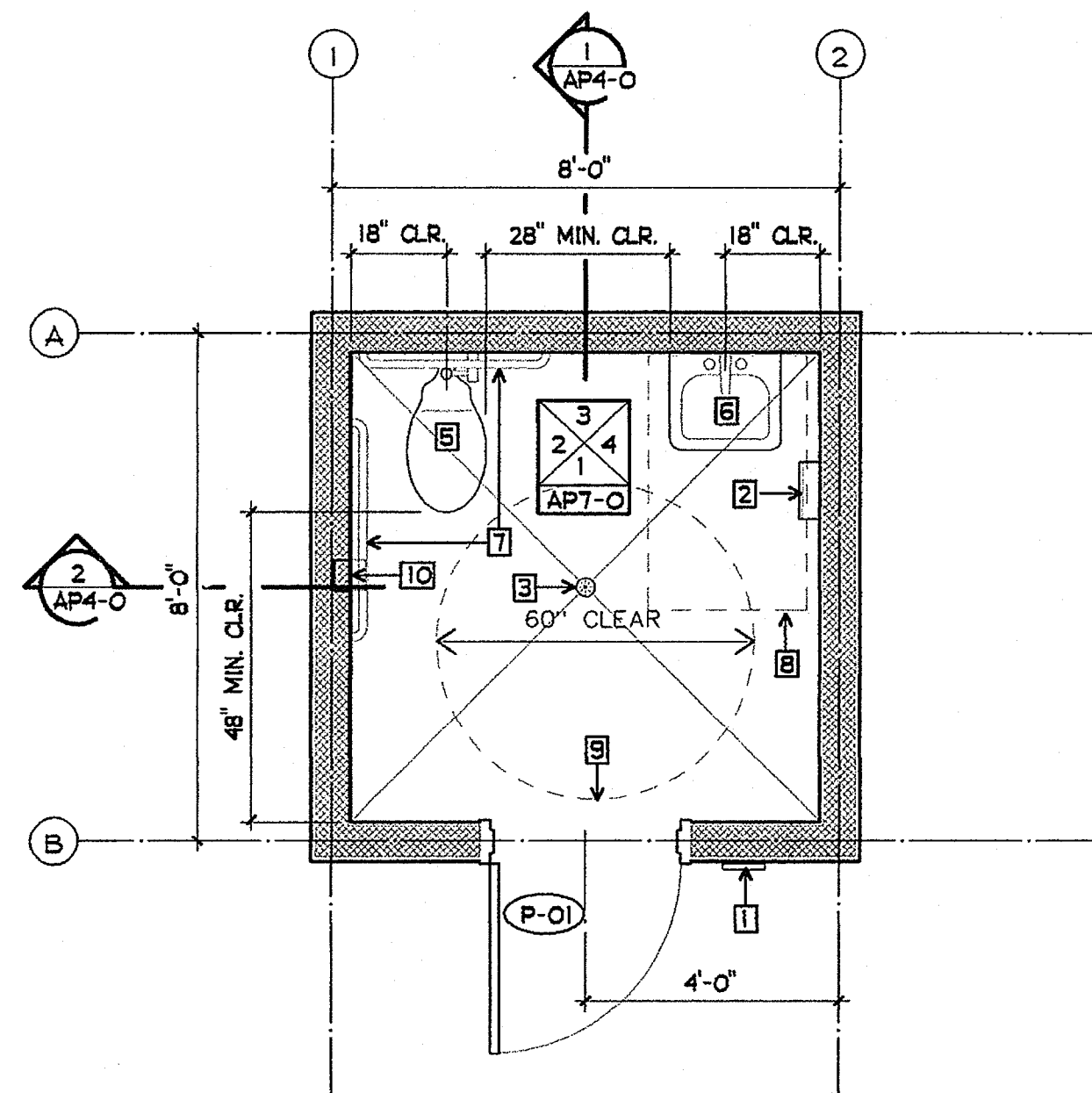
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KEY PLAN
 BUILDING P
 FLOOR, CEILING
 AND ROOF PLAN
 SHEET TITLE
 AP3-O

FLOOR PLAN NOTES

- (P-X) DOOR NO. - SEE DOOR AND HARDWARE SCHEDULE
- 1 DOOR SIGNAGE - SEE DOOR SCHEDULE
- 2 ACCESSIBLE PAPER TOWEL DISPENSER, 4" MAX PROJECTION
- 3 SLOPE FLOOR TO DRAIN - 1/4" PER FOOT MAXIMUM SLOPE
- 4 NOT USED
- 5 ACCESSIBLE WATER CLOSET - SEE PLUMBING DRAWINGS
- 6 ACCESSIBLE LAVATORY - SEE PLUMBING DRAWINGS
- 7 GRAB BARS - SEE 11/A9-5
- 8 30" x 48" x 27" HIGH CLEAR FLOOR SPACE "APPROACH" (DASHED)
- 9 60" x 27" HIGH CLEAR FLOOR SPACE "TURNING AREA" (DASHED)
- 10 ACCESSIBLE TOILET TISSUE DISPENSER - SEE 9/A9-5



BUILDING P - FLOOR PLAN

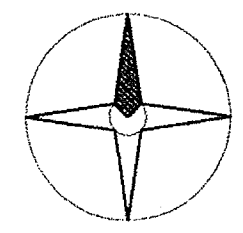
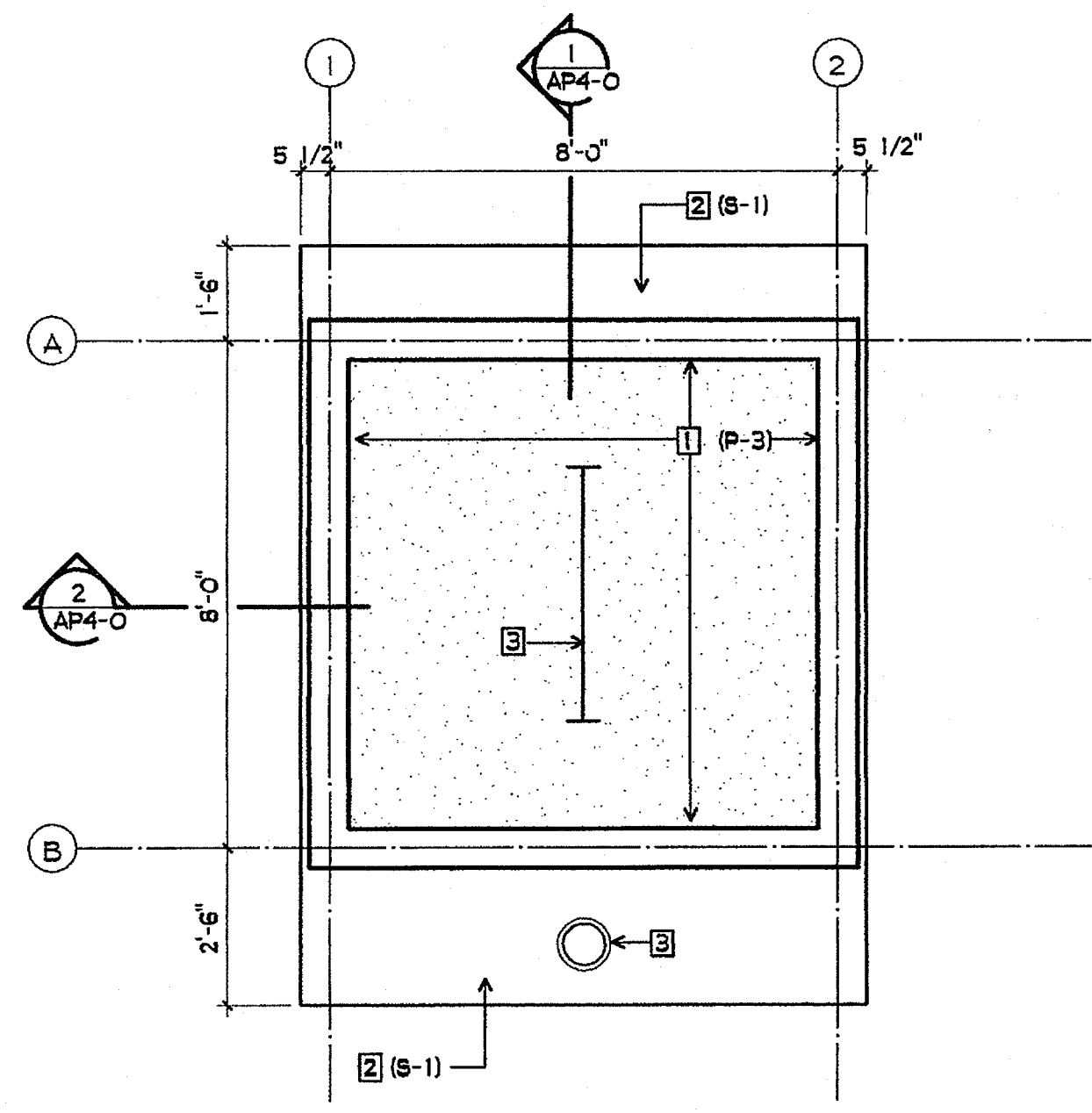
3/8" = 1'-0"

REFLECTED CEILING PLAN NOTES

- 1 GYP. BOARD CEILING - FLAT PAINT
- 2 STUCCO SOFFIT - SEE 5/A9-3
- 3 LIGHT FIXTURE - SEE ELECTRICAL DRAWINGS

FINISH PALETTE

- (P-1) PAINT, COLOR #1
- (S-1) STUCCO, COLOR #1

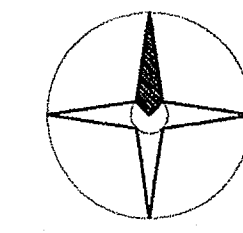
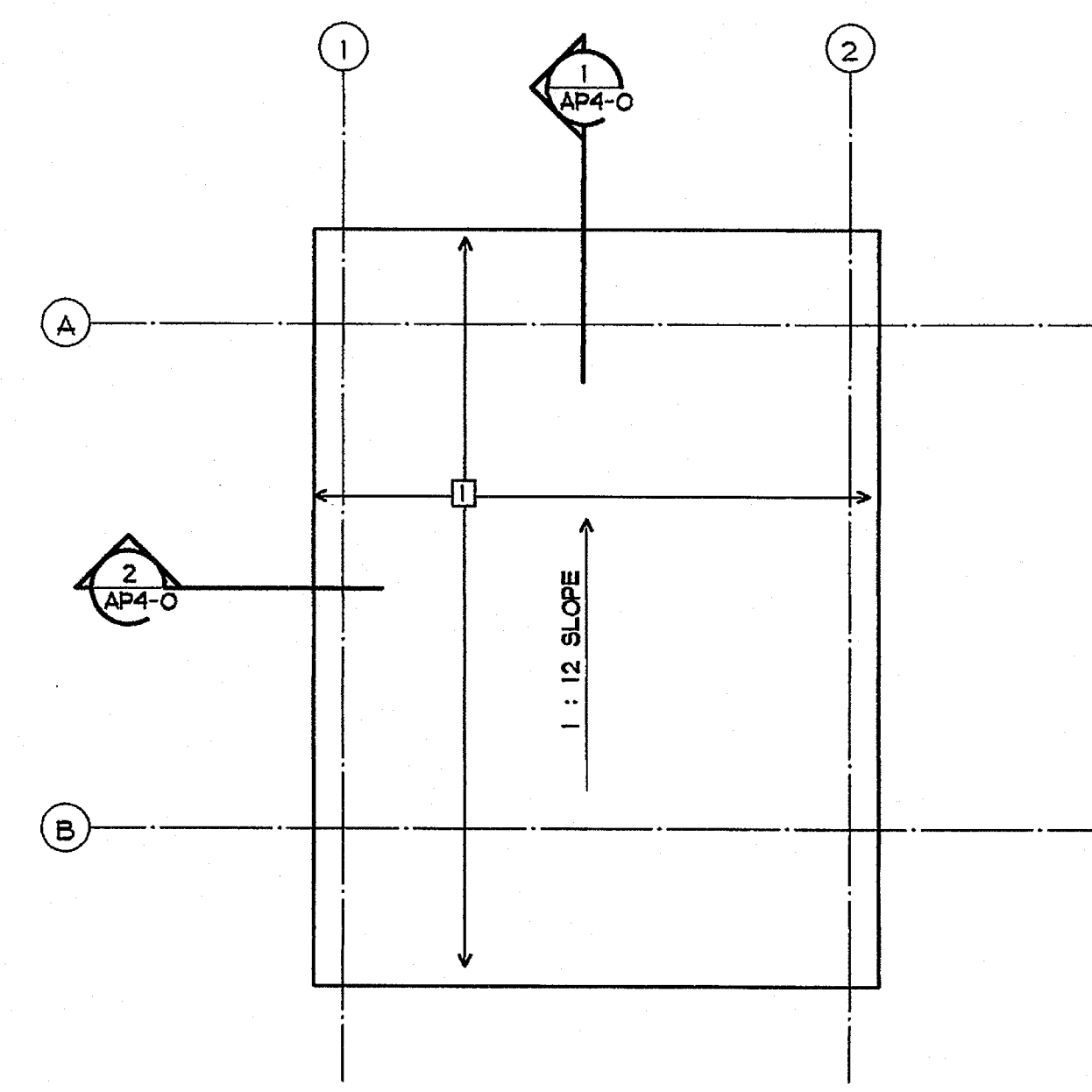


BUILDING P - REFLECTED CEILING PLAN

3/8" = 1'-0"

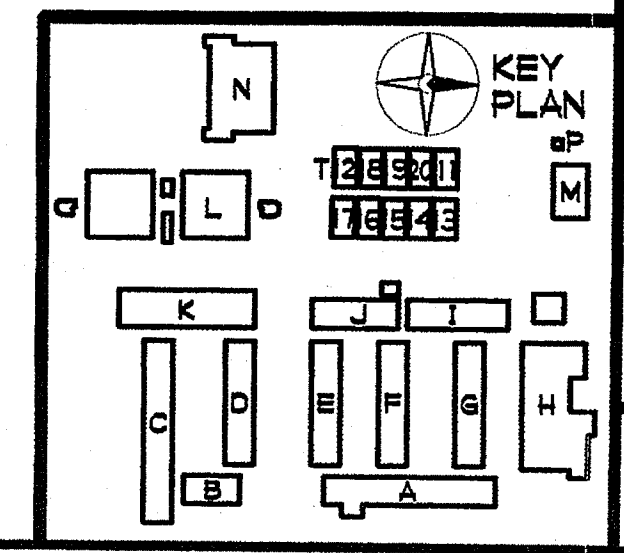
ROOF PLAN NOTES

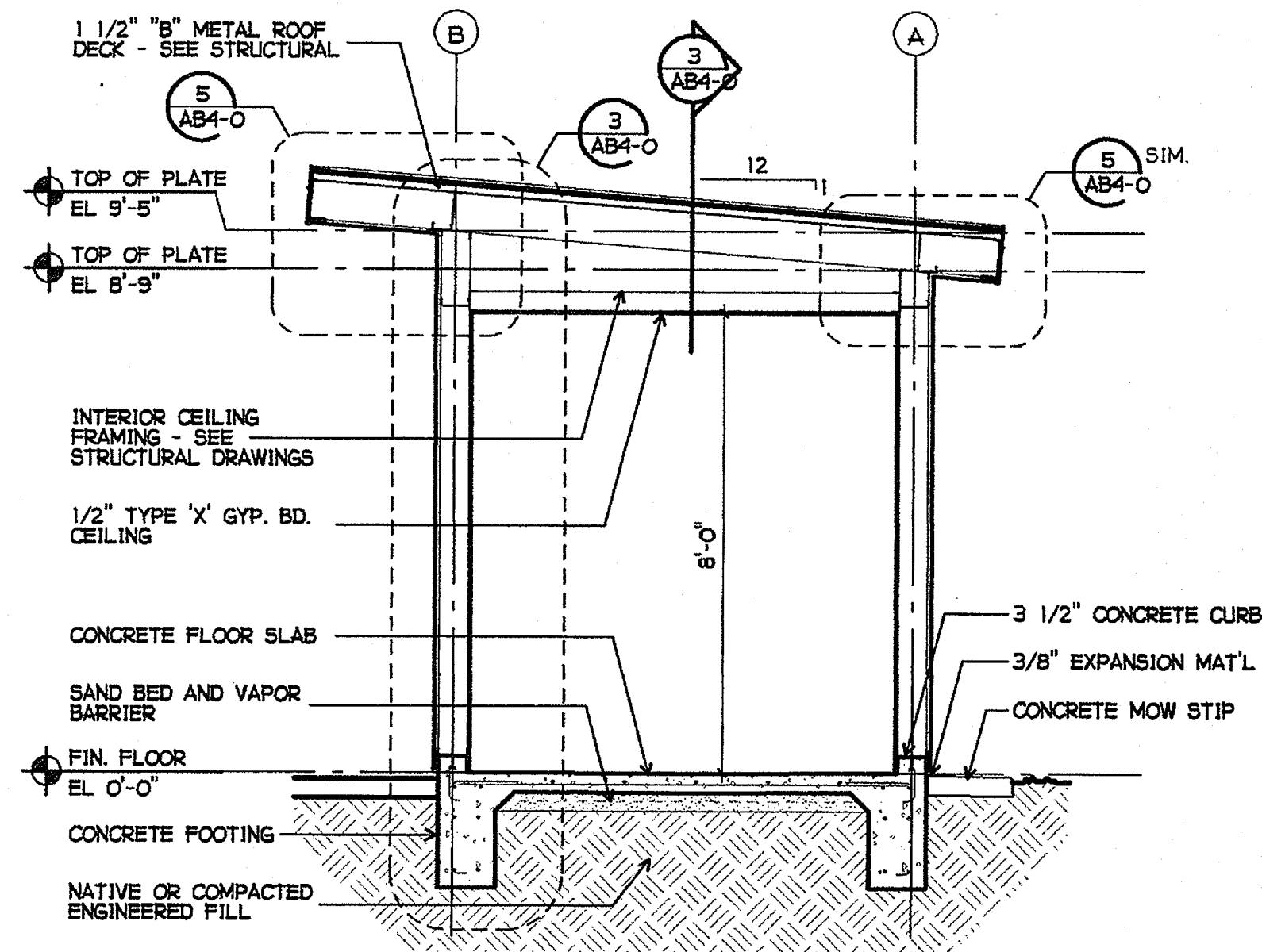
- 1 BUILT UP ROOFING



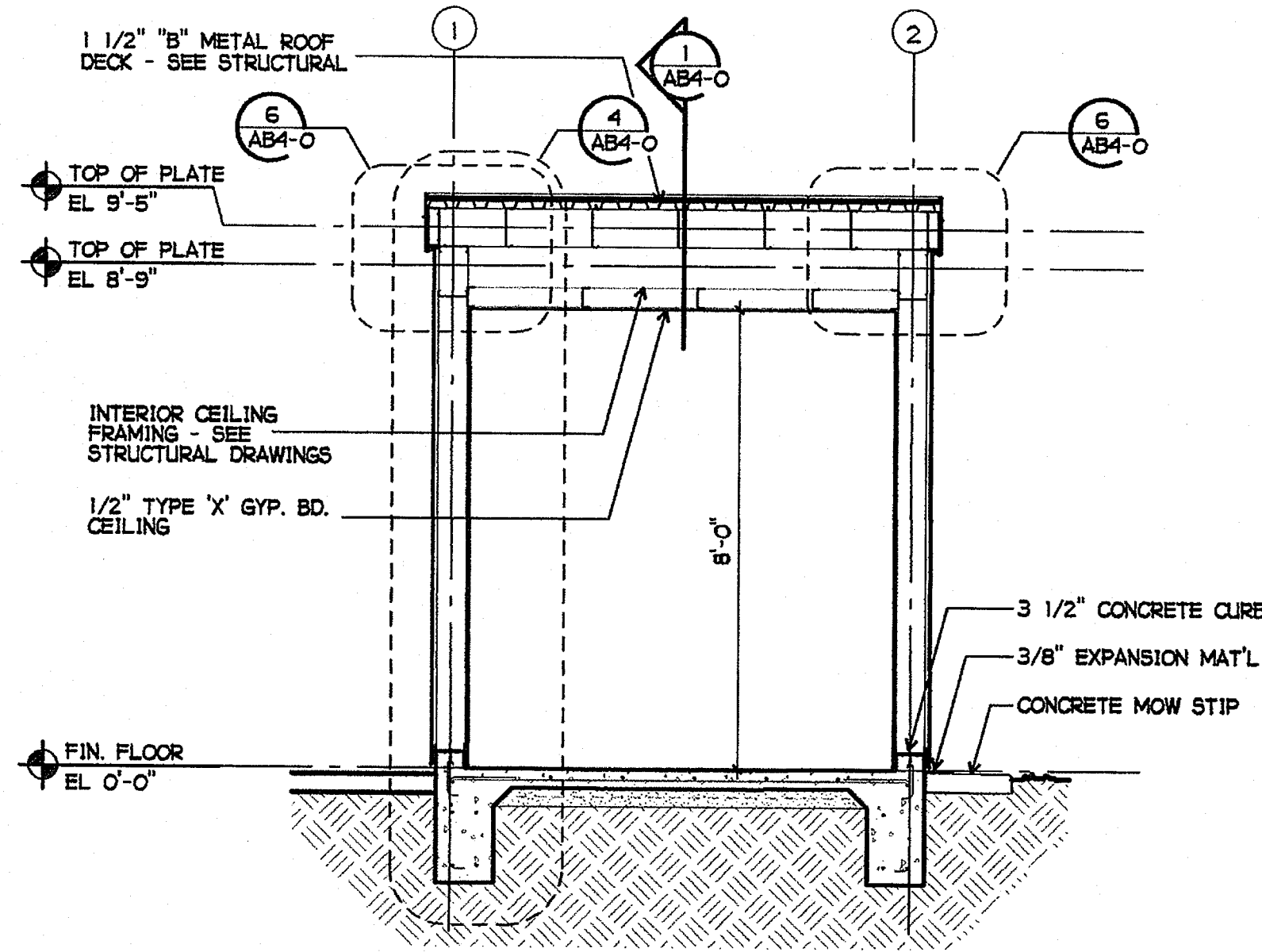
BUILDING P - ROOF PLAN

3/8" = 1'-0"

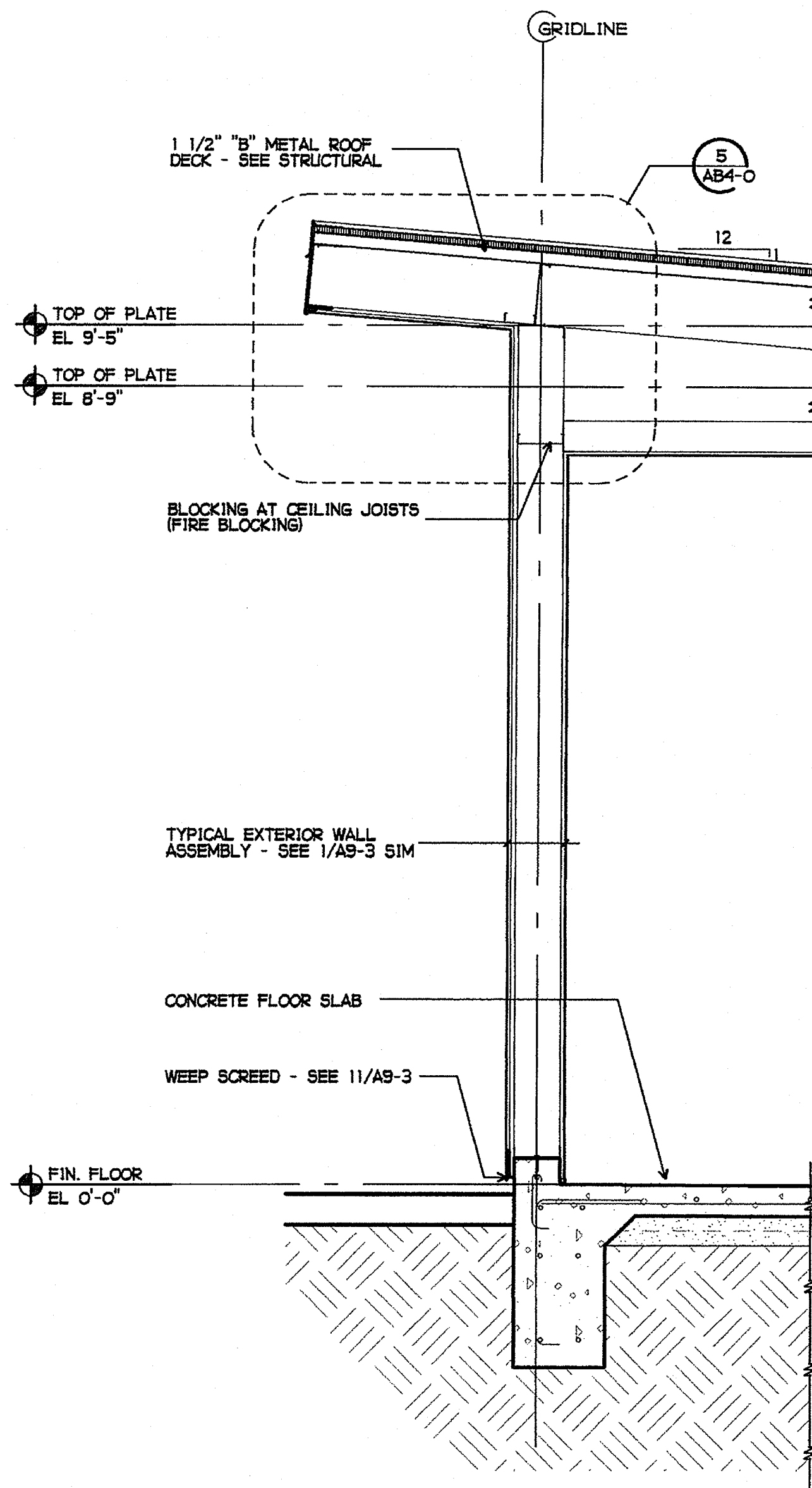




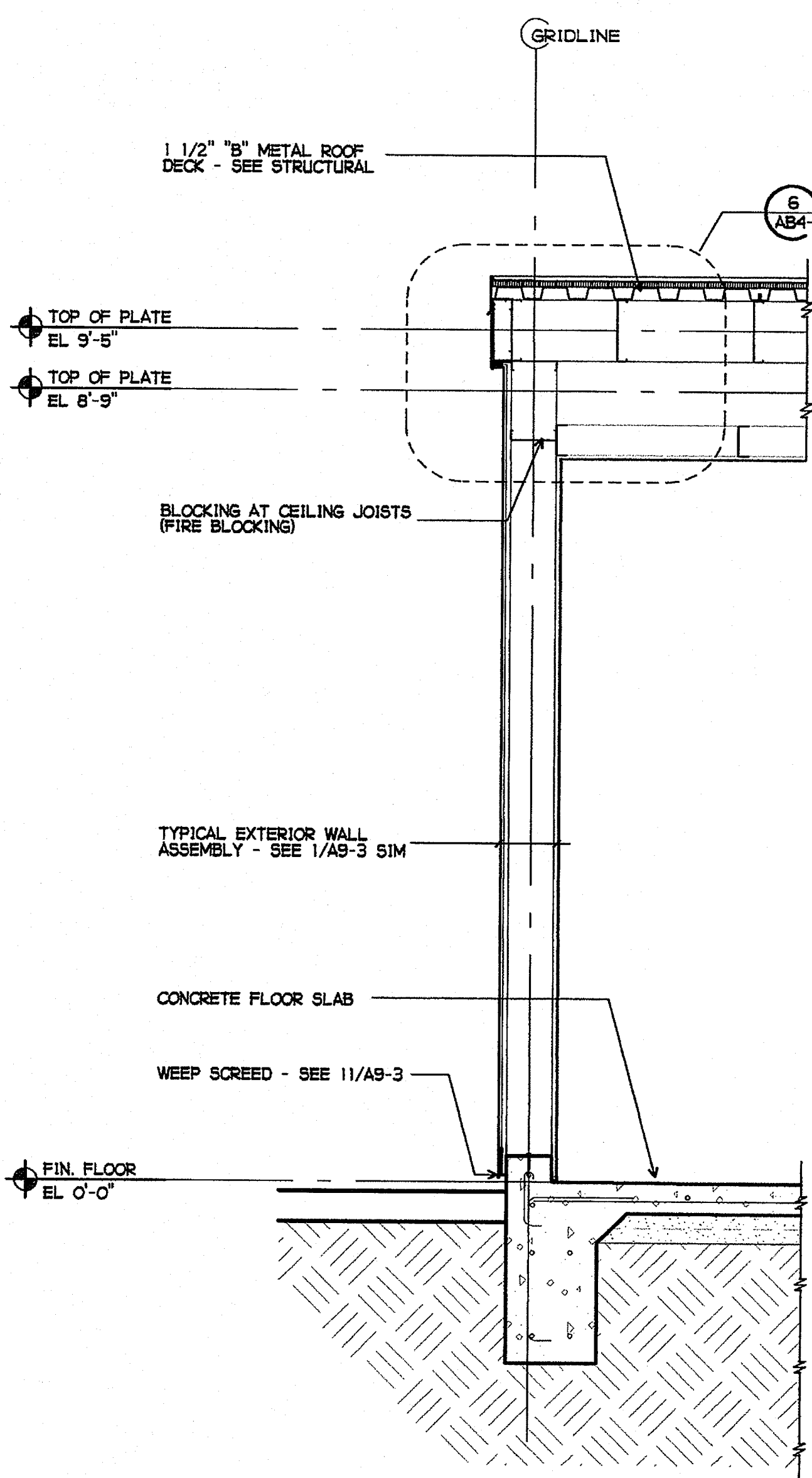
1 CROSS SECTION
3/8" = 1'-0"



2 CROSS SECTION
3/8" = 1'-0"

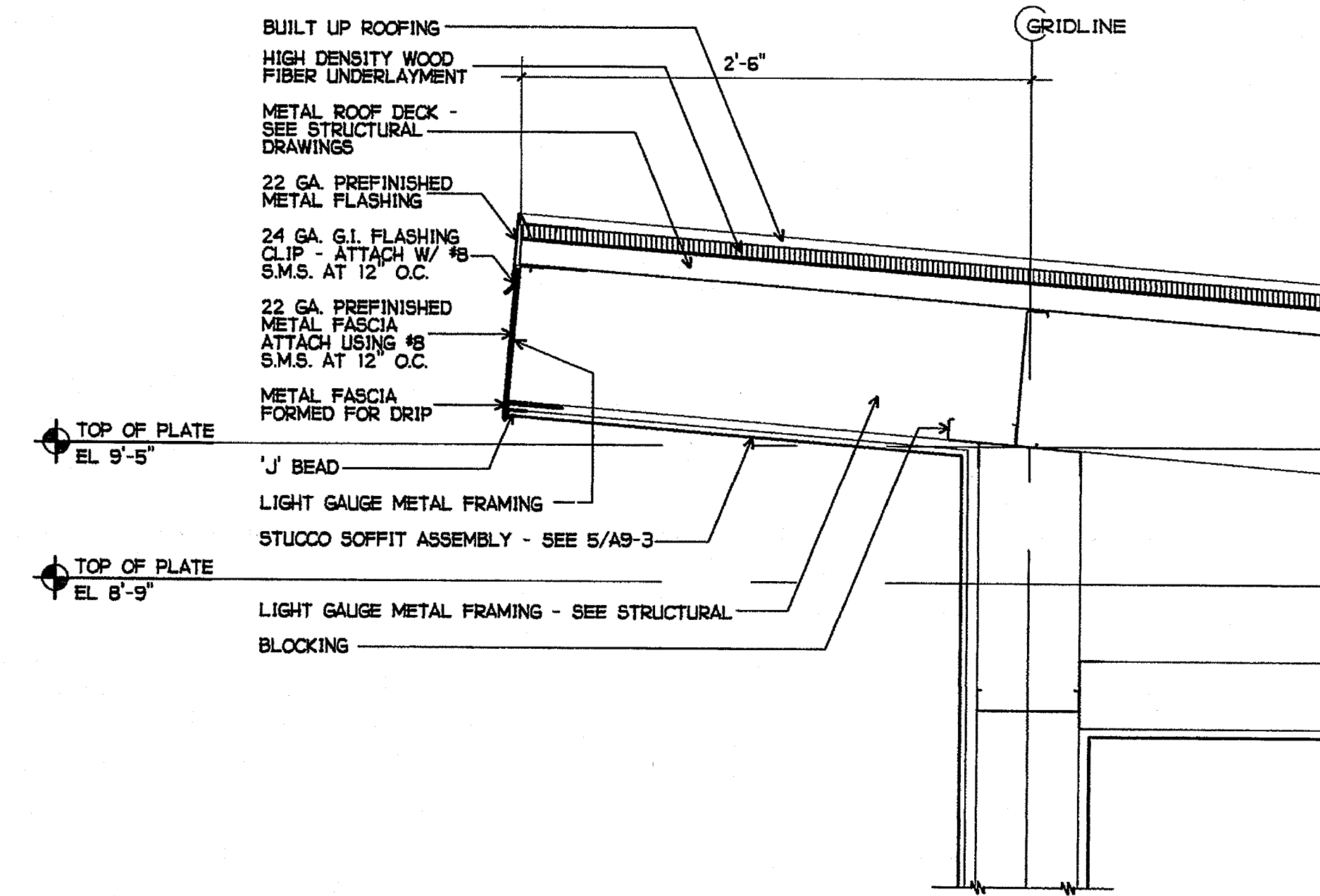


3 EXTERIOR WALL SECTION
3/4" = 1'-0"

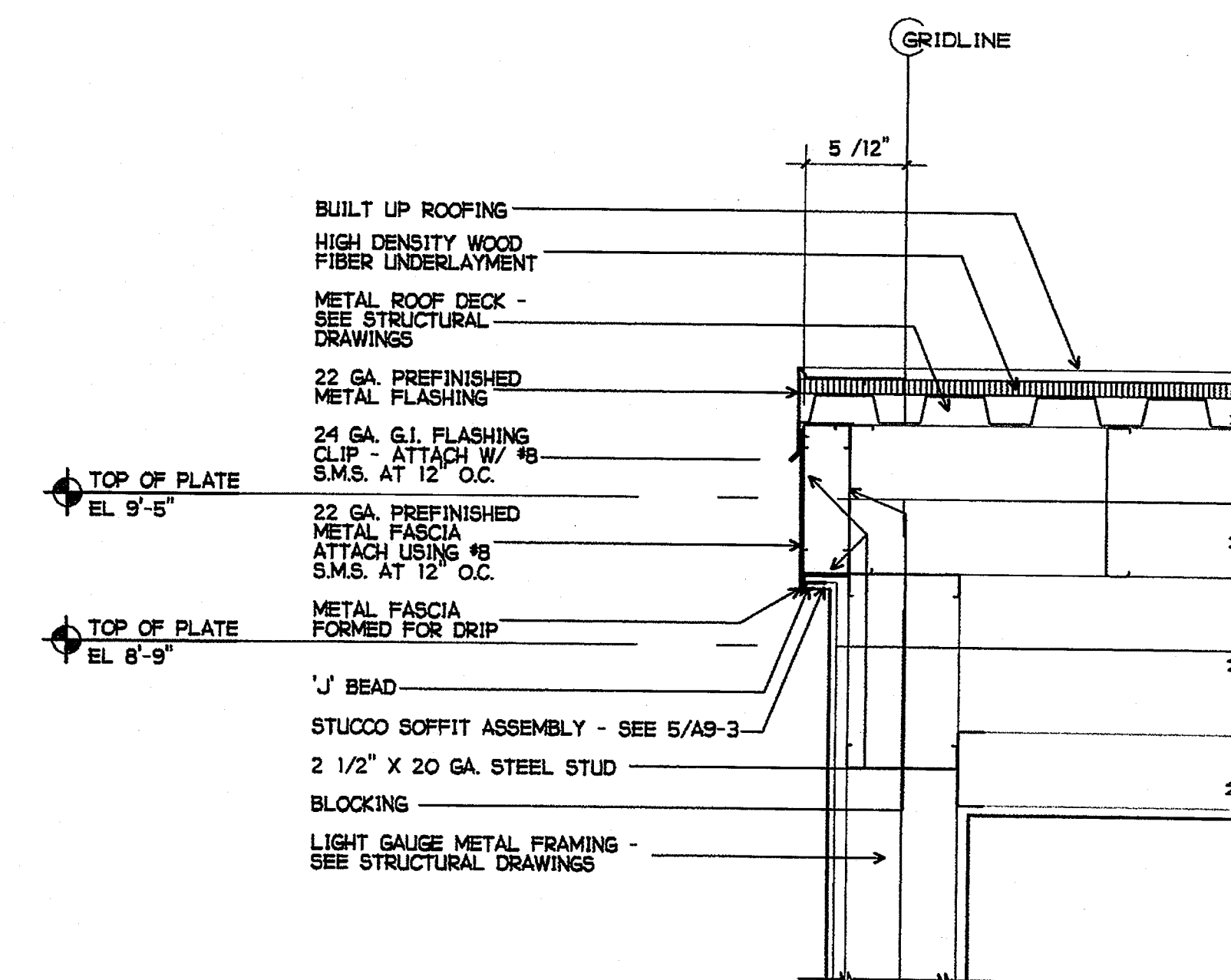


4 EXTERIOR WALL SECTION
3/4" = 1'-0"

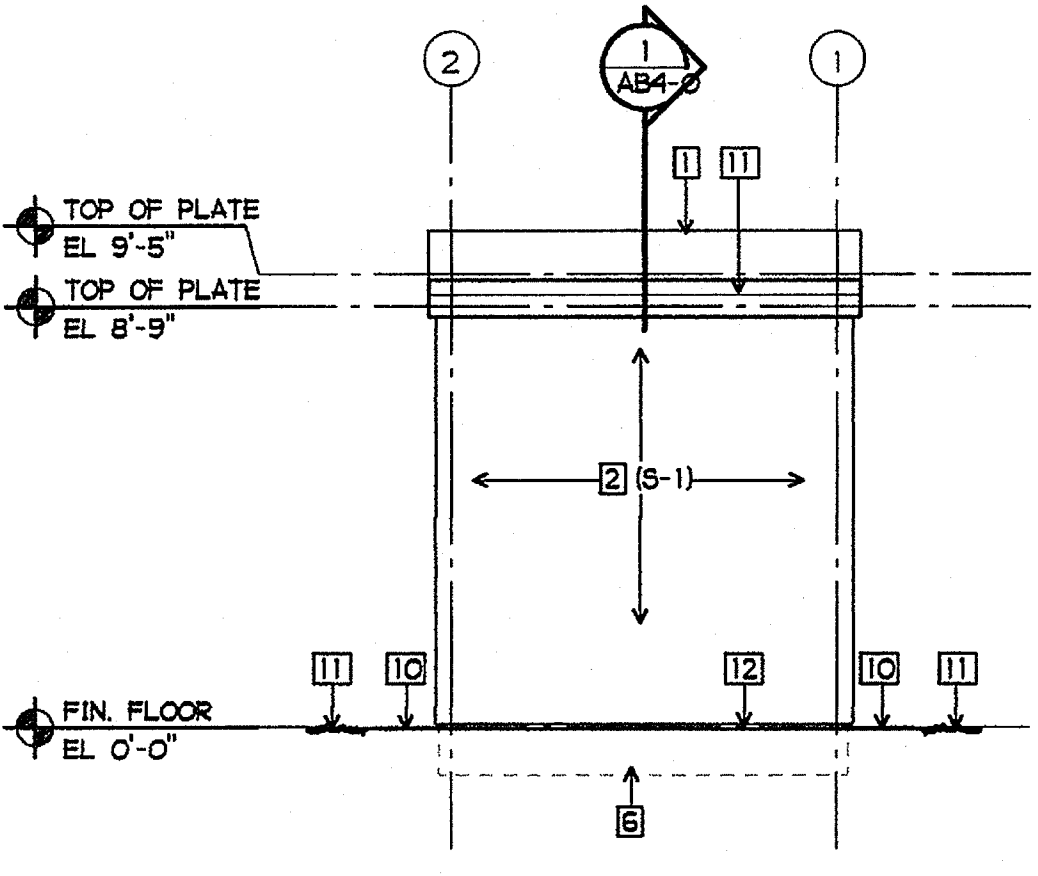
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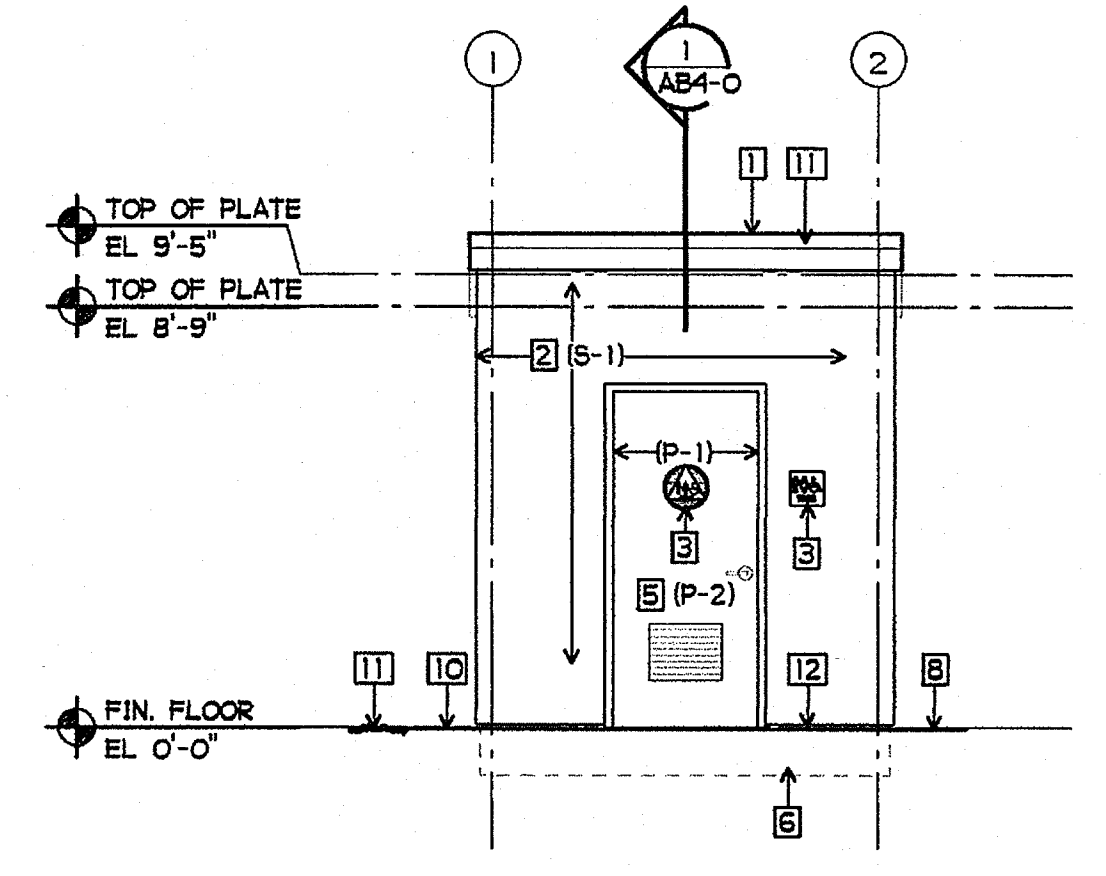
5 DETAIL
1 1/2" = 1'-0"



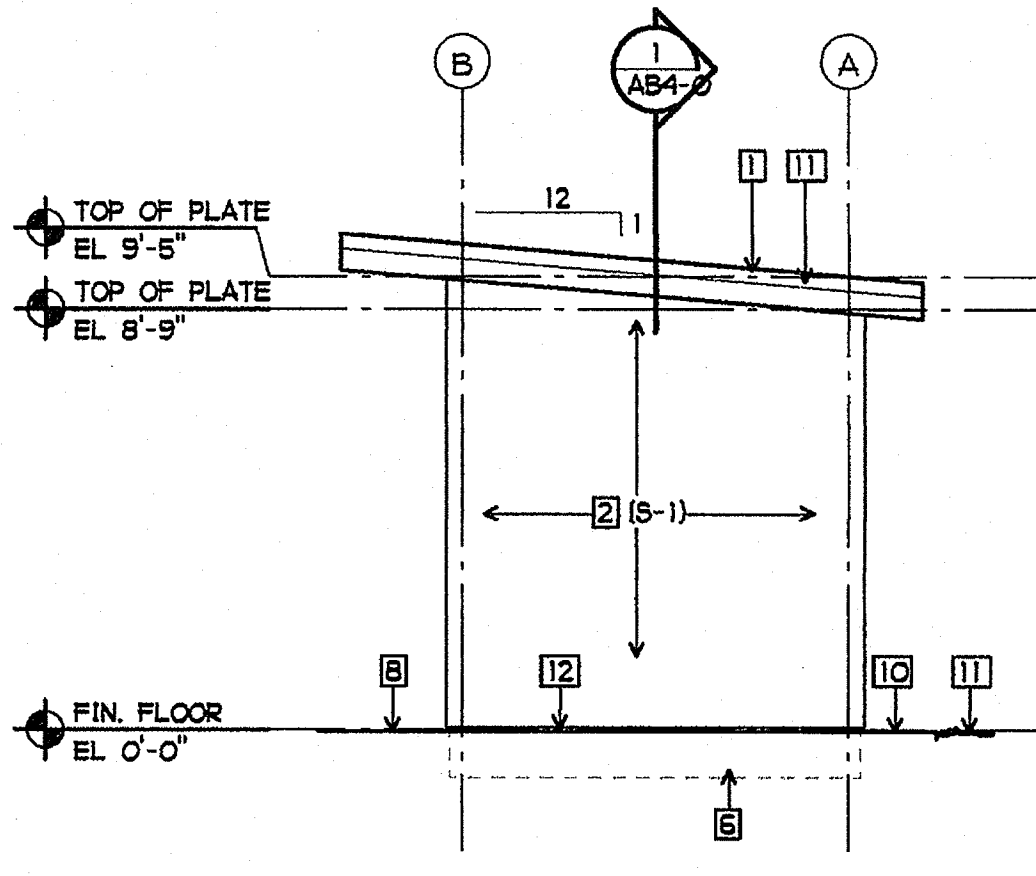
6 DETAIL
1 1/2" = 1'-0"



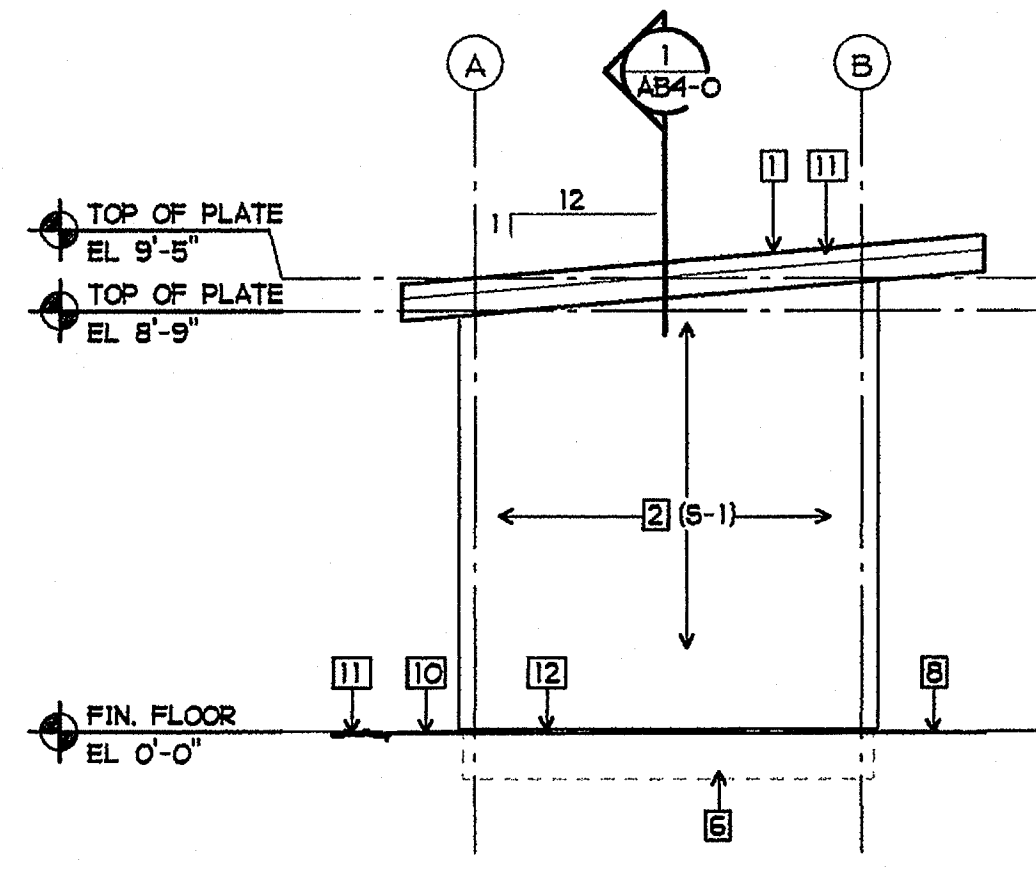
NORTH ELEVATION
 1/4" = 1'-0"



SOUTH ELEVATION
 1/4" = 1'-0"



EAST ELEVATION
 1/4" = 1'-0"



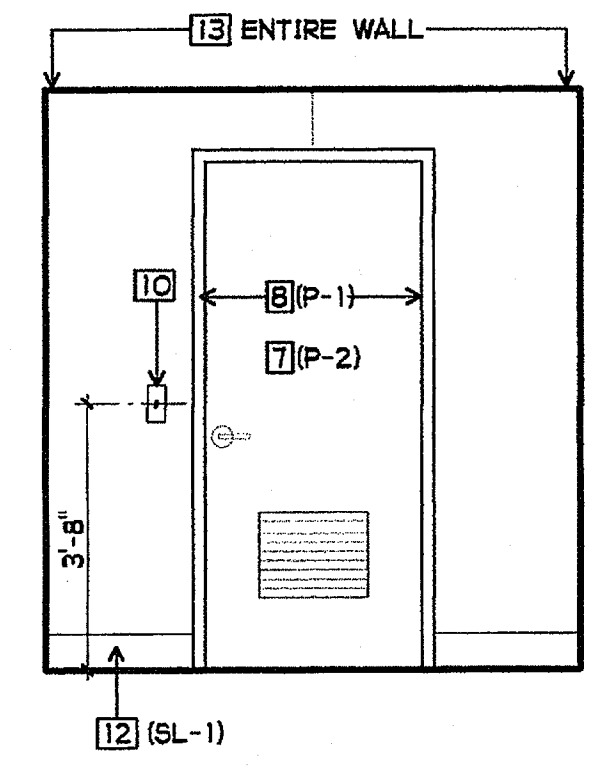
WEST ELEVATION
 1/4" = 1'-0"

EXTERIOR ELEVATION NOTES

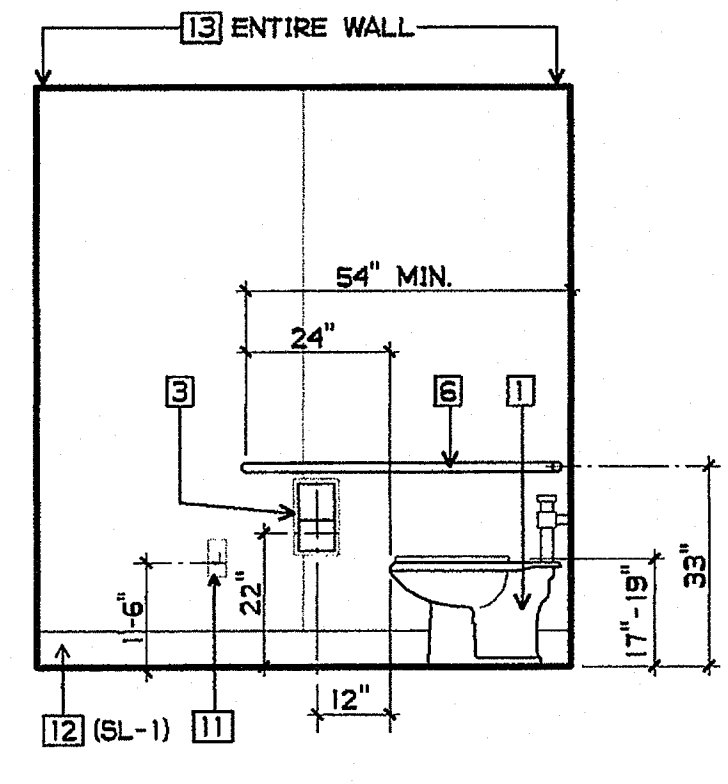
- 1 BUILT UP ROOFING
- 2 EXTERIOR STUCCO SYSTEM - SEE 1/A9-3
- 3 DOOR SIGN - SEE DOOR SCHEDULE
- 4 AIR GRILLE - SEE MECHANICAL DRAWINGS
- 5 DOOR AND FRAME - SEMI-GLOSS PAINT
- 6 FOOTING SHOWN DASHED
- 7 EXPOSED CONC. - STUCCO COLOR COAT
- 8 CONCRETE SIDEWALK
- 9 FINISH GRADE
- 10 MOW STRIP
- 11 FLASHING
- 12 WEEP SCREED - SEE 11/A9-3

FINISH PALETTE

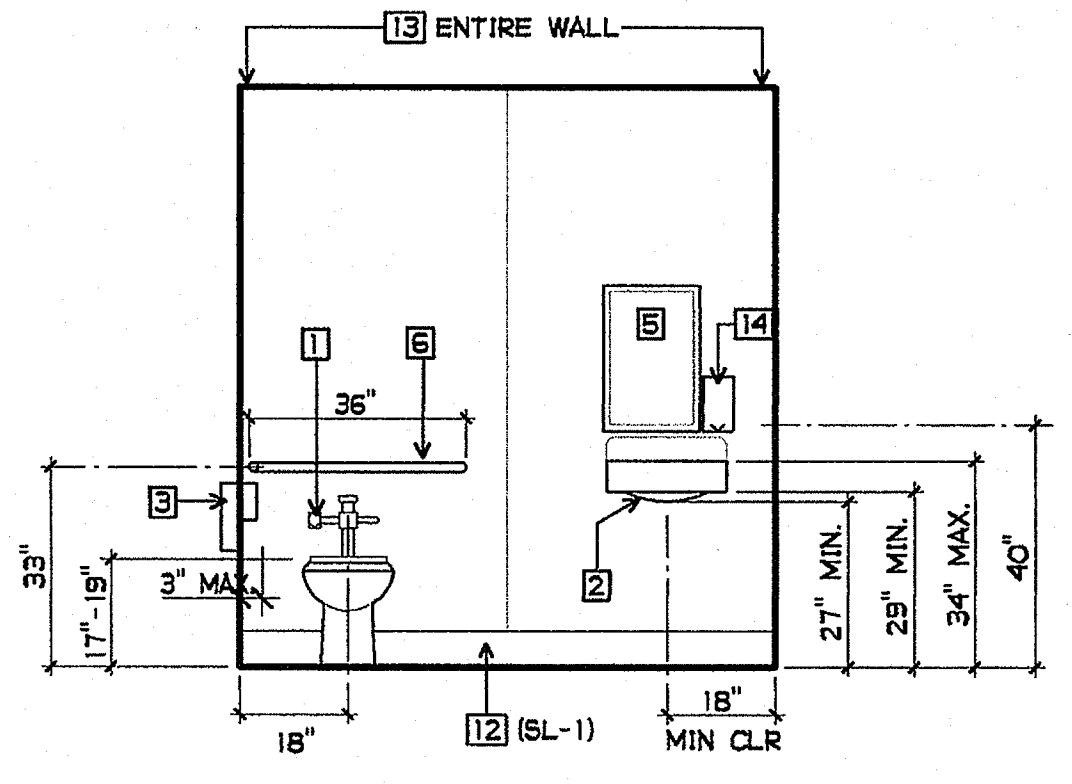
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- (P-1) PAINT, COLOR #1
- (P-2) PAINT, COLOR #2



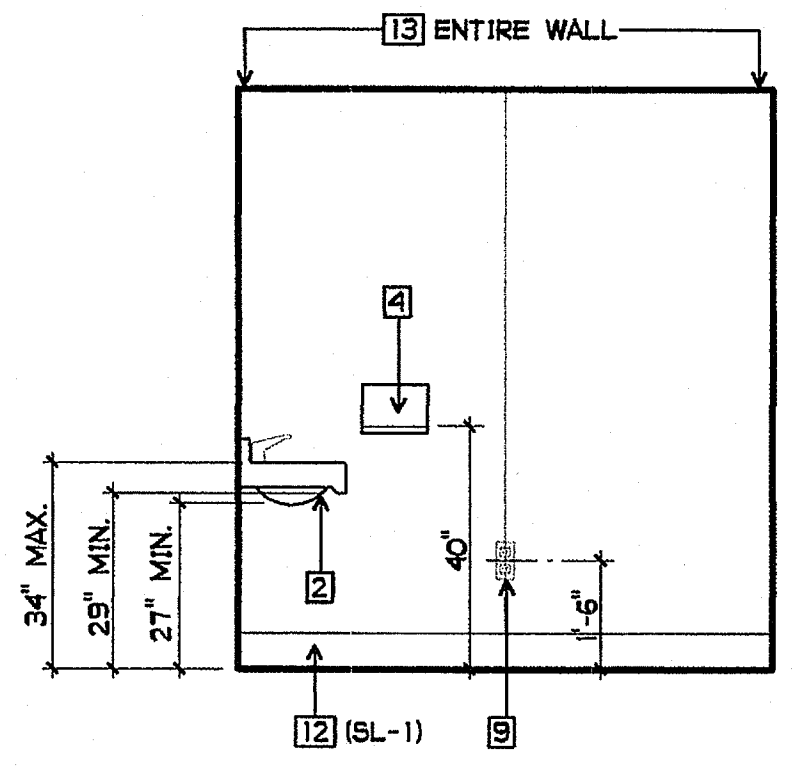
1 STAFF TOILET
 3/8" = 1'-0"



2 STAFF TOILET
 3/8" = 1'-0"



3 STAFF TOILET
 3/8" = 1'-0"



4 STAFF TOILET
 3/8" = 1'-0"

INTERIOR ELEVATION NOTES

- 1 ACCESSIBLE WATER CLOSET
- 2 ACCESSIBLE LAVATORY
- 3 ACCESSIBLE TOILET TISSUE DISPENSER - SEE 9/A9-5
- 4 ACCESSIBLE PAPER TOWEL DISPENSER, MAX. 4" PROJECTION
- 5 ACCESSIBLE MIRROR 16" X 24"
- 6 GRAB BARS - SEE 11/A9-5
- 7 DOOR - SEMI-GLOSS PAINT
- 8 METAL FRAME - SEMI-GLOSS PAINT
- 9 POWER OUTLET
- 10 LIGHT CONTROLS
- 11 J-BOX W/ COVER
- 12 COVE RESILIENT SHEET FLOORING UP WALL 6"
- 13 SANITARY WALL PANELS
- 14 SOAP DISPENSER - & TO BE 1-1/2" FROM EDGE OF LAVATORY

FINISH PALETTE

- (P-1) PAINT, COLOR #1
- (P-2) PAINT, COLOR #2

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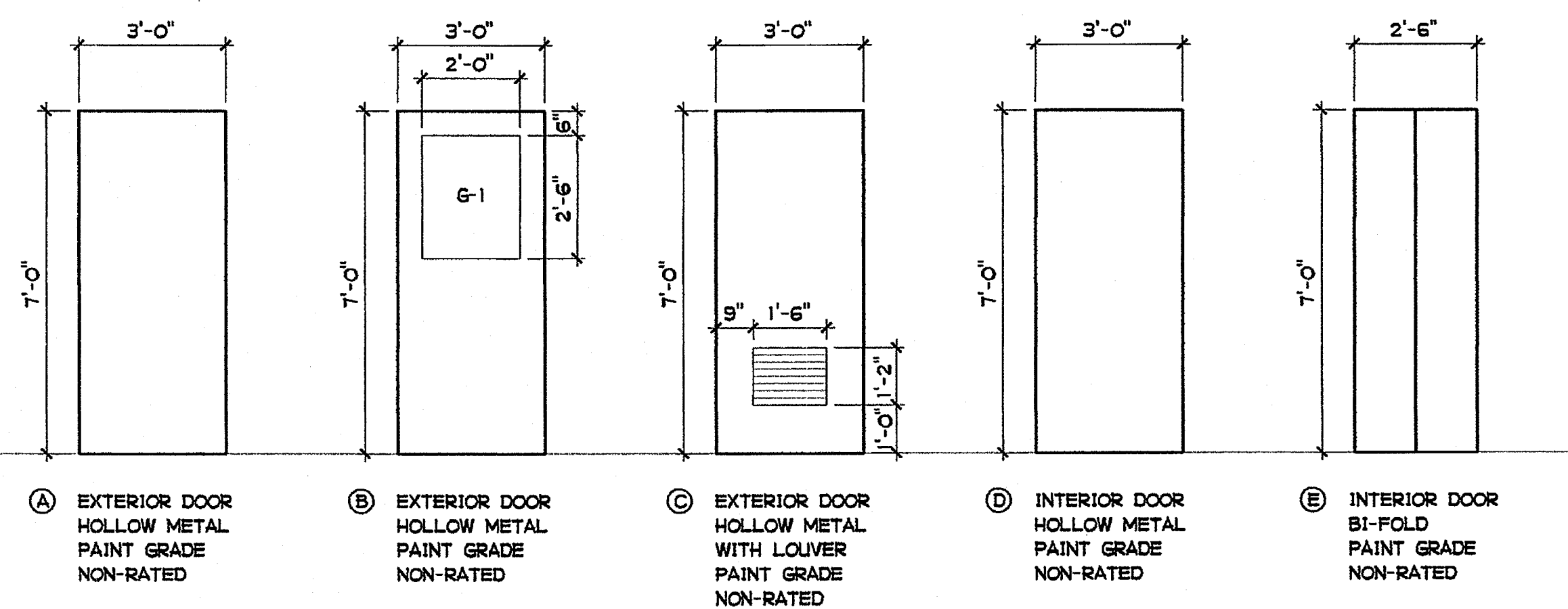
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C-26609
4/30/2007 RENEWAL
STATE OF CALIFORNIA

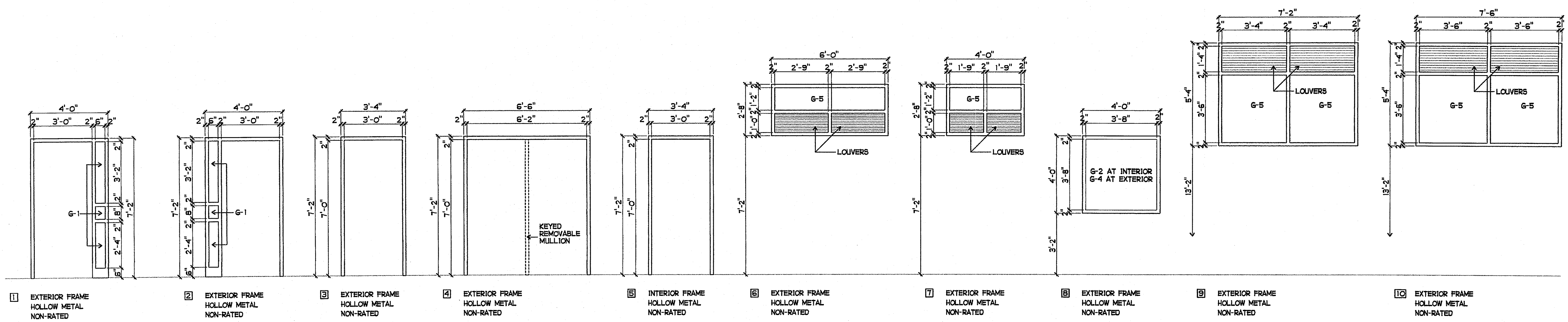
DOOR TYPES AND HOLLOW METAL FRAME ELEVATIONS
A8-0



A EXTERIOR DOOR HOLLOW METAL PAINT GRADE NON-RATED
B EXTERIOR DOOR HOLLOW METAL PAINT GRADE NON-RATED
C EXTERIOR DOOR HOLLOW METAL WITH LOUVER PAINT GRADE NON-RATED
D INTERIOR DOOR HOLLOW METAL PAINT GRADE NON-RATED
E INTERIOR DOOR BI-FOLD PAINT GRADE NON-RATED

SCALE: 3/8" = 1'-0" **DOOR TYPES**

LEGEND SEE SPECIFICATION SECTION 08800
G-1 CLEAR TEMPERED
G-2 CLEAR
G-4 REFLECTIVE
G-5 OBSCURED



1 EXTERIOR FRAME HOLLOW METAL NON-RATED
2 EXTERIOR FRAME HOLLOW METAL NON-RATED
3 EXTERIOR FRAME HOLLOW METAL NON-RATED
4 EXTERIOR FRAME HOLLOW METAL NON-RATED
5 INTERIOR FRAME HOLLOW METAL NON-RATED
6 EXTERIOR FRAME HOLLOW METAL NON-RATED
7 EXTERIOR FRAME HOLLOW METAL NON-RATED
8 EXTERIOR FRAME HOLLOW METAL NON-RATED
9 EXTERIOR FRAME HOLLOW METAL NON-RATED
10 EXTERIOR FRAME HOLLOW METAL NON-RATED

SCALE: 3/8" = 1'-0" **HOLLOW METAL FRAME ELEVATIONS**

HOLLOW METAL FRAME, DOOR & HARDWARE SCHEDULE

HOLLOW METAL FRAME, DOOR & HARDWARE SCHEDULE

DOOR NUMBER	BUILDING	FLOOR PLAN SHEET NUMBER	DOOR ELEVATION	FRAME ELEVATION	HARDWARE GROUP	PANIC DEVICE REQUIRED	DOOR SIZE			DETAIL				SIGN			
							WIDTH	HEIGHT	THICKNESS	HEAD	STRIKE JAMB	HINGE JAMB	SILL/ THRESHOLD	DOOR SIGNAGE	VINYL LETTERING	TEXT COPY (VERIFY TEXT WITH SCHOOL DISTRICT)	
																LINE 1 (TOP)	LINE 2 (BOTTOM)
(L-0)	L	AQL3-0	(A)	(2)	2	-	3'-0"	7'-0"	1-3/4"	6 7/AS-1	8/AS-1	6/AS-1	9 10/AS-1	2/AS-8	YES	L-1	CLASSROOM
(L-02)	L	AQL3-0	(A)	(1)	2	-	3'-0"	7'-0"	1-3/4"	6 7/AS-1	8/AS-1	6/AS-1	9 10/AS-1	2/AS-8	YES	L-2	CLASSROOM
(L-03)	L	AQL3-0	(A)	(1)	2	-	3'-0"	7'-0"	1-3/4"	6 7/AS-1	8/AS-1	6/AS-1	9 10/AS-1	2/AS-8	YES	L-3	CLASSROOM
(L-04)	L	AQL3-0	(A)	(2)	2	-	3'-0"	7'-0"	1-3/4"	6 7/AS-1	8/AS-1	6/AS-1	9 10/AS-1	2/AS-8	YES	L-4	CLASSROOM
(L-05)	L	AQL3-0	(A)	(2)	2	-	3'-0"	7'-0"	1-3/4"	6 7/AS-1	8/AS-1	6/AS-1	9 10/AS-1	2/AS-8	YES	L-5	CLASSROOM
(L-06)	L	AQL3-0	(A)	(1)	2	-	3'-0"	7'-0"	1-3/4"	6 7/AS-1	8/AS-1	6/AS-1	9 10/AS-1	2/AS-8	YES	L-6	CLASSROOM
(L-07)	L	AQL3-0	(A)	(1)	2	-	3'-0"	7'-0"	1-3/4"	6 7/AS-1	8/AS-1	6/AS-1	9 10/AS-1	2/AS-8	YES	L-7	CLASSROOM
(L-08)	L	AQL3-0	(A)	(2)	2	-	3'-0"	7'-0"	1-3/4"	6 7/AS-1	8/AS-1	6/AS-1	9 10/AS-1	2/AS-8	YES	L-8	CLASSROOM
(L-09)	L	AQL3-3	(A)	(2)	2	-	3'-0"	7'-0"	1-3/4"	6 7/AS-1	8/AS-1	6/AS-1	9 10/AS-1	2/AS-8	YES	L-9	CLASSROOM
(L-10)	L	AQL3-3	(A)	(2)	2	-	3'-0"	7'-0"	1-3/4"	6 7/AS-1	8/AS-1	6/AS-1	9 10/AS-1	2/AS-8	YES	L-10	CLASSROOM
(L-11)	L	AQL3-3	(A)	(2)	2	-	3'-0"	7'-0"	1-3/4"	6 7/AS-1	8/AS-1	6/AS-1	9 10/AS-1	2/AS-8	YES	L-11	CLASSROOM
(L-12)	L	AQL3-3	(A)	(1)	2	-	3'-0"	7'-0"	1-3/4"	6 7/AS-1	8/AS-1	6/AS-1	9 10/AS-1	2/AS-8	YES	L-12	CLASSROOM
(L-13)	L	AQL3-3	(A)	(1)	2	-	3'-0"	7'-0"	1-3/4"	6 7/AS-1	8/AS-1	6/AS-1	9 10/AS-1	2/AS-8	YES	L-13	CLASSROOM
(L-14)	L	AQL3-3	(A)	(2)	2	-	3'-0"	7'-0"	1-3/4"	6 7/AS-1	8/AS-1	6/AS-1	9 10/AS-1	2/AS-8	YES	L-14	CLASSROOM
(L-15)	L	AQL3-3	(A)	(2)	2	-	3'-0"	7'-0"	1-3/4"	6 7/AS-1	8/AS-1	6/AS-1	9 10/AS-1	2/AS-8	YES	L-15	CLASSROOM
(L-16)	L	AQL3-3	(A)	(1)	2	-	3'-0"	7'-0"	1-3/4"	6 7/AS-1	8/AS-1	6/AS-1	9 10/AS-1	2/AS-8	YES	L-16	CLASSROOM
(L-17)	L	AQL3-0	(A)	(3)	3	-	3'-0"	7'-0"	1-3/4"	6/AS-1	6/AS-1	6/AS-1	10/AS-1	1/AS-8	YES	L-17	FIRE RISER
(L-18)	L	AQL3-0	(C)	(3)	3	-	3'-0"	7'-0"	1-3/4"	6/AS-1	6/AS-1	6/AS-1	10/AS-1	1/AS-8	YES	L-18	ELECTRIC
(L-19)	L	AQL3-0	(C)	(3)	3	-	3'-0"	7'-0"	1-3/4"	6/AS-1	6/AS-1	6/AS-1	10/AS-1	1/AS-8	YES	L-20	ELEVATOR PUMP
(L-20)	L	AQL3-0	(C)	(3)	6	-	3'-0"	7'-0"	1-3/4"	6/AS-1	6/AS-1	6/AS-1	10/AS-1	3/AS-8, UNISEX	-	-	-
(L-21)	L	AQL3-0	(C)	(3)	3	-	3'-0"	7'-0"	1-3/4"	6/AS-1	6/AS-1	6/AS-1	10/AS-1	1/AS-8	YES	L-22	IDF
(L-22)	L	AQL3-0	(2B)	(4)	1	YES	3'-0"	7'-0"	1-3/4"	6/AS-1	6/AS-1	6/AS-1	10/AS-1, SIM	2/AS-8	-	L-49	HALL
(L-23)	L	AQL3-3	(2B)	(4)	1	YES	3'-0"	7'-0"	1-3/4"	6/AS-1	6/AS-1	6/AS-1	10/AS-1, SIM	2/AS-8	-	L-51	HALL
(L-24)	L	AQL3-3	(2B)	(4)	1	YES	3'-0"	7'-0"	1-3/4"	6/AS-1	6/AS-1	6/AS-1	10/AS-1, SIM	2/AS-8	-	L-51	HALL
(L-25)	L	AQL3-3	(2B)	(4)	1	YES	3'-0"	7'-0"	1-3/4"	6/AS-1	6/AS-1	6/AS-1	10/AS-1, SIM	2/AS-8	-	L-51	HALL
(L-26)	L	AQL3-0	(D)	(14)	-	-	3'-0"	7'-0"	1-3/4"	11/AS-1	11/AS-1	11/AS-1	-	1/AS-8	-	L-1 / L-23	CLASSROOM / ANTE
(L-27)	L	AQL3-0	(D)	(14)	-	-	3'-0"	7'-0"	1-3/4"	11/AS-1	11/AS-1	11/AS-1	-	1/AS-8	-	L-2 / L-23	CLASSROOM / ANTE
(L-28)	L	AQL3-0	(D)	(14)	-	-	3'-0"	7'-0"	1-3/4"	11/AS-1	11/AS-1	11/AS-1	-	1/AS-8	-	L-3 / L-23	CLASSROOM / ANTE
(L-29)	L	AQL3-0	(D)	(14)	-	-	3'-0"	7'-0"	1-3/4"	11/AS-1	11/AS-1	11/AS-1	-	1/AS-8	-	L-4 / L-23	CLASSROOM / ANTE
(L-30)	L	AQL3-0	(D)	(14)	-	-	3'-0"	7'-0"	1-3/4"	11/AS-1	11/AS-1	11/AS-1	13/AS-1	1/AS-8	-	L-5 / L-28	CLASSROOM / SHOWER
(L-31)	L	AQL3-0	(D)	(14)	-	-	3'-0"	7'-0"	1-3/4"	11/AS-1	11/AS-1	11/AS-1	-	1/AS-8	-	L-6 / L-27	CLASSROOM / ANTE
(L-32)	L	AQL3-0	(D)	(14)	-	-	3'-0"	7'-0"	1-3/4"	11/AS-1	11/AS-1	11/AS-1	13/AS-1	1/AS-8	-	L-7 / L-28	CLASSROOM / SHOWER
(L-33)	L	AQL3-0	(D)	(14)	-	-	3'-0"	7'-0"	1-3/4"	11/AS-1	11/AS-1	11/AS-1	-	1/AS-8	-	L-8 / L-28	CLASSROOM / ANTE
(L-34)	L	AQL3-3	(D)	(13)	-	-	3'-0"	7'-0"	1-3/4"	11/AS-1	11/AS-1	11/AS-1	-	1/AS-8	-	L-9 / L-49	CLASSROOM / HALL
(L-35)	L	AQL3-3	(D)	(13)	-	-	3'-0"	7'-0"	1-3/4"	11/AS-1	11/AS-1	11/AS-1	-	1/AS-8	-	L-10 / L-49	CLASSROOM / HALL
(L-36)	L	AQL3-3	(D)	(13)	-	-	3'-0"	7'-0"	1-3/4"	11/AS-1	11/AS-1	11/AS-1	-	1/AS-8	-	L-11 / L-49	CLASSROOM / HALL
(L-37)	L	AQL3-3	(D)	(13)	-	-	3'-0"	7'-0"	1-3/4"	11/AS-1	11/AS-1	11/AS-1	-	1/AS-8	-	L-12 / L-49	CLASSROOM / HALL
(L-38)	L	AQL3-3	(D)	(13)	-	-	3'-0"	7'-0"	1-3/4"	11/AS-1	11/AS-1	11/AS-1	-	1/AS-8	-	L-13 / L-51	CLASSROOM / HALL
(L-39)	L	AQL3-3	(D)	(13)	-	-	3'-0"	7'-0"	1-3/4"	11/AS-1	11/AS-1	11/AS-1	-	1/AS-8	-	L-14 / L-51	CLASSROOM / HALL
(L-40)	L	AQL3-3	(D)	(13)	-	-	3'-0"	7'-0"	1-3/4"	11/AS-1	11/AS-1	11/AS-1	-	1/AS-8	-	L-15 / L-51	CLASSROOM / HALL
(L-41)	L	AQL3-3	(D)	(13)	-	-	3'-0"	7'-0"	1-3/4"	11/AS-1	11/AS-1	11/AS-1	-	1/AS-8	-	L-16 / L-51	CLASSROOM / HALL
(L-42)	L	AQL3-0	(D)	(11)	-	-	3'-0"	7'-0"	1-3/4"	11/AS-1	11/AS-1	11/AS-1	-	1/AS-8	-	L-25	STORAGE
(L-43)	L	AQL3-0	(D)	(11)	-	-	3'-0"	7'-0"	1-3/4"	11/AS-1	11/AS-1	11/AS-1	-	1/AS-8	-	L-26	STORAGE
(L-44)	L	AQL3-0	(D)	(11)	-	-	3'-0"	7'-0"	1-3/4"	11/AS-1	11/AS-1	11/AS-1	-	1/AS-8	-	L-24	STORAGE
(L-45)	L	AQL3-0	(D)	(11)	-	-	3'-0"	7'-0"	1-3/4"	11/AS-1	11/AS-1	11/AS-1	-	1/AS-8	-	L-29	STORAGE
(L-46)	L	AQL3-0	(D)	(16)	-	-	3'-0"	7'-0"	1-3/4"	11/AS-1	11/AS-1	11/AS-1	12/AS-1	1/AS-8	-	-	-
(L-47)	L	AQL3-0	(D)	(16)	-	-	3'-0"	7'-0"	1-3/4"	11/AS-1	11/AS-1	11/AS-1	12/AS-1	1/AS-8	-	-	-
(L-48)	L	AQL3-0	(D)	(16)	-	-	3'-0"	7'-0"	1-3/4"	11/AS-1	11/AS-1	11/AS-1	12/AS-1	1/AS-8	-	-	-
(L-49)	L	AQL3-0	(D)	(16)	-	-	3'-0"	7'-0"	1-3/4"	11/AS-1	11/AS-1	11/AS-1	12/AS-1	1/AS-8	-	-	-
(L-50)	L	AQL3-0	(D)	(16)	-	-	3'-0"	7'-0"	1-3/4"	11/AS-1	11/AS-1	11/AS-1	12/AS-1	1/AS-8	-	-	-
(L-51)	L	AQL3-0	(D)	(16)	-	-	3'-0"	7'-0"	1-3/4"	11/AS-1	11/AS-1	11/AS-1	12/AS-1	1/AS-8	-	-	-
(L-52)	L	AQL3-0	(D)	(16)	-	-	3'-0"	7'-0"	1-3/4"	11/AS-1	11/AS-1	11/AS-1	12/AS-1	1/AS-8	-	-	-
(L-53)	L	AQL3-0	(D)	(16)	-	-	3'-0"	7'-0"	1-3/4"	11/AS-1	11/AS-1	11/AS-1	12/AS-1	1/AS-8	-	-	-
(L-54)	L	AQL3-3	(D)	(16)	-	-	3'-0"	7'-0"	1-3/4"	11/AS-1	11/AS-1	11/AS-1	12/AS-1	1/AS-8	-	-	-
(L-55)	L	AQL3-3	(D)	(16)	-	-	3'-0"	7'-0"	1-3/4"	11/AS-1	11/AS-1	11/AS-1	12/AS-1	1/AS-8	-	-	-
(L-56)	L	AQL3-3	(D)	(16)	-	-	3'-0"	7'-0"	1-3/4"	11/AS-1	11/AS-1	11/AS-1	12/AS-1	1/AS-8	-	-	-
(L-57)	L	AQL3-3	(D)	(16)	-	-	3'-0"	7'-0"	1-3/4"	11/AS-1	11/AS-1	11/AS-1	12/AS-1	1/AS-8	-	-	-
(L-58)	L	AQL3-3	(D)	(16)	-	-	3'-0"	7'-0"	1-3/4"	11/AS-1	11/AS-1	11/AS-1	12/AS-1	1/AS-8	-	-	-
(L-59)	L	AQL3-3	(D)	(16)	-	-	3'-0"	7'-0"	1-3/4"	11/AS-1	11/AS-1	11/AS-1	12/AS-1	1/AS-8	-	-	-
(L-60)	L	AQL3-3	(D)	(16)	-	-	3'-0"	7'-0"	1-3/4"	11/AS-1	11/AS-1	11/AS-1	12/AS-1	1/AS-8	-	-	-
(L-61)	L	AQL3-3	(D)	(16)	-	-	3'-0"	7'-0"	1-3/4"	11/AS-1	11/AS-1	11/AS-1	12/AS-1	1/AS-8	-	-	-
(L-62)	L	AQL3-0	(D)	(14)	-	-	3'-0"	7'-0"	1-3/4"	11/AS-1	11/AS-1	11/AS-1	13/AS-1	3/AS-8, UNISEX	-	-	-
(L-63)	L	AQL3-0	(D)	(14)	-	-	3'-0"	7'-0"	1-3/4"	11/AS-1	11/AS-1	11/AS-1	13/AS-1	3/AS-8, UNISEX	-	-	-
(L-64)	L	AQL3-0	(D)	(14)	-	-	3'-0"	7'-0"	1-3/4"	-	-	-	-	-	-	-	-

DOOR NUMBER	BUILDING	FLOOR PLAN SHEET NUMBER	DOOR ELEVATION	FRAME ELEVATION	HARDWARE GROUP	PANIC DEVICE REQUIRED	DOOR SIZE			DETAIL				SIGN			
							WIDTH	HEIGHT	THICKNESS	HEAD	STRIKE JAMB	HINGE JAMB	SILL/ THRESHOLD	DOOR SIGNAGE	VINYL LETTERING	TEXT COPY (VERIFY TEXT WITH SCHOOL DISTRICT)	
																LINE 1 (TOP)	LINE 2 (BOTTOM)
(N-01)	N	ANS-0	(A)	(7)	7	YES	3'-0"	7'-0"	1-3/4"	14/AS-1	15/AS-1	15/AS-1	10/AS-1	2/AS-8	YES	N-3	BOYS VESTIBULE
(N-02)	N	ANS-0	(A)	(7)	7	YES	3'-0"	7'-0"	1-3/4"	14/AS-1	15/AS-1	15/AS-1	10/AS-1	2/AS-8	YES	N-4	GIRLS VESTIBULE
(N-03)	N	ANS-0	(A)	(7)	7	YES	3'-0"	7'-0"	1-3/4"	14/AS-1	15/AS-1	15/AS-1	10/AS-1	1/AS-8	YES	N-19	UTILITY
(N-04)	N	ANS-0	(A)	(7)	3	-	3'-0"	7'-0"	1-3/4"	14/AS-1	15/AS-1	15/AS-1	10/AS-1	1/AS-8	YES	N-20	CUSTODIAN
(N-05)	N	ANS-0	(A)	(7)	7	YES	3'-0"	7'-0"	1-3/4"	14/AS-1	15/AS-1	15/AS-1	10/AS-1	2/AS-8	YES	N-11	BOYS RAMP
(N-06)	N	ANS-0	(A)	(7)	7	YES	3'-0"	7'-0"	1-3/4"	14/AS-1	15/AS-1	15/AS-1	10/AS-1	2/AS-8	YES	N-12	GIRLS RAMP
(N-07)	N	ANS-0	(A)	(7)	7	YES	3'-0"	7'-0"	1-3/4"	14/AS-1	15/AS-1	15/AS-1	10/AS-1	2/AS-8	YES	N-11	BOYS RAMP
(N-08)	N	ANS-0	(A)	(7)	7	YES	3'-0"	7'-0"	1-3/4"	14/AS-1	15/AS-1	15/AS-1	10/AS-1	2/AS-8	YES	N-12	GIRLS RAMP
(N-09)	N	ANS-0	(A)	(7)	5	-	3'-0"	7'-0"	1-3/4"	14/AS-1	15/AS-1	15/AS-1	10/AS-1	1/AS-8	YES	N-9	BOYS P.E. OFFICE
(N-10)	N	ANS-0	(A)	(7)	5	-	3'-0"	7'-0"	1-3/4"	14/AS-1	15/AS-1	15/AS-1	10/AS-1	1/AS-8	YES	N-10	GIRLS P.E. OFFICE
(N-11)	N	ANS-0	(A)	(7)	3	-	3'-0"	7'-0"	1-3/4"	14/AS-1	15/AS-1	15/AS-1	15/AS-1	1/AS-8	YES	N-22	P.E. STORAGE
(N-12)	N	ANS-0	(D)	(11)	-	-	3'-0"	7'-0"	1-3/4"	16/AS-1	1/AS-2	1/AS-2	-	1/AS-8	-	N-21	STORAGE
(N-13)	N	ANS-0	(D)	(15)	YES	3'-0"	7'-0"	1-3/4"	11/AS-1	11/AS-1	11/AS-1	-	1/AS-8	-	N-1	BOYS DRESSING ROOM	
(N-14)	N	ANS-0	(D)	(15)	YES												

WINDOW SCHEDULE

BUILDING L - FIRST FLOOR SOUTH

WINDOW SCHEDULE

BUILDING L - FIRST FLOOR NORTH

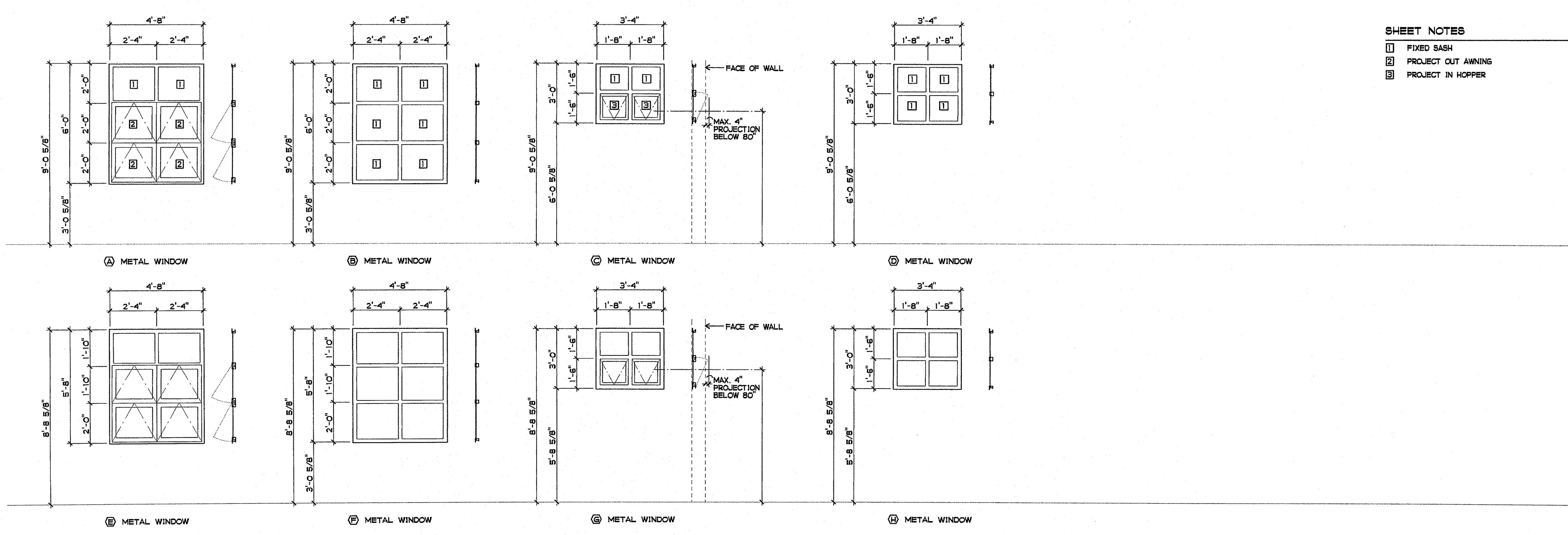
WINDOW SCHEDULE

BUILDING L - SECOND FLOOR SOUTH

WINDOW SCHEDULE

BUILDING L - SECOND FLOOR NORTH

WINDOW NUMBER	BUILDING	FLOOR PLAN SHEET NUMBER	WINDOW ELEVATION	GLAZING TYPE, SEE SPEC. CBS800/CB810	DETAIL			REMARKS	WINDOW NUMBER	BUILDING	FLOOR PLAN SHEET NUMBER	WINDOW ELEVATION	GLAZING TYPE, SEE SPEC. CBS800/CB810	DETAIL			REMARKS	WINDOW NUMBER	BUILDING	FLOOR PLAN SHEET NUMBER	WINDOW ELEVATION	GLAZING TYPE, SEE SPEC. CBS800/CB810	DETAIL			REMARKS	WINDOW NUMBER	BUILDING	FLOOR PLAN SHEET NUMBER	WINDOW ELEVATION	GLAZING TYPE, SEE SPEC. CBS800/CB810	DETAIL			REMARKS									
					HEAD	JAMB	SILL							HEAD	JAMB	SILL							HEAD	JAMB	SILL							HEAD	JAMB	SILL										
(L01)	L	AQL3-O	H	G-3	1/A9-1	3	4/A9-1	5/A9-1	(L33)	L	AQL3-O	H	G-3	1/A9-1	3	4/A9-1	5/A9-1	(L65)	L	AQL3-3	D	G-3	2/A9-1	3	4/A9-1	5/A9-1	(L97)	L	AQL3-3	D	G-3	2/A9-1	3	4/A9-1	5/A9-1									
(L02)	L	AQL3-O	G	G-3	1/A9-1	4/A9-1	5/A9-1	(L34)	L	AQL3-O	G	G-3	1/A9-1	4/A9-1	5/A9-1	(L66)	L	AQL3-3	C	G-3	2/A9-1	4/A9-1	5/A9-1	(L98)	L	AQL3-3	C	G-3	2/A9-1	4/A9-1	5/A9-1	(L98)	L	AQL3-3	C	G-3	2/A9-1	4/A9-1	5/A9-1					
(L03)	L	AQL3-O	G	G-3	1/A9-1	4/A9-1	5/A9-1	(L35)	L	AQL3-O	G	G-3	1/A9-1	4/A9-1	5/A9-1	(L67)	L	AQL3-3	C	G-3	2/A9-1	4/A9-1	5/A9-1	(L99)	L	AQL3-3	C	G-3	2/A9-1	4/A9-1	5/A9-1	(L99)	L	AQL3-3	C	G-3	2/A9-1	4/A9-1	5/A9-1					
(L04)	L	AQL3-O	H	G-3	1/A9-1	3	4/A9-1	5/A9-1	(L36)	L	AQL3-O	H	G-3	1/A9-1	3	4/A9-1	5/A9-1	(L68)	L	AQL3-3	D	G-3	2/A9-1	3	4/A9-1	5/A9-1	(L100)	L	AQL3-3	D	G-3	2/A9-1	3	4/A9-1	5/A9-1	(L100)	L	AQL3-3	D	G-3	2/A9-1	3	4/A9-1	5/A9-1
(L05)	L	AQL3-O	H	G-3	1/A9-1	3	4/A9-1	5/A9-1	(L37)	L	AQL3-O	H	G-3	1/A9-1	3	4/A9-1	5/A9-1	(L69)	L	AQL3-3	D	G-3	2/A9-1	3	4/A9-1	5/A9-1	(L101)	L	AQL3-3	D	G-3	2/A9-1	3	4/A9-1	5/A9-1	(L101)	L	AQL3-3	D	G-3	2/A9-1	3	4/A9-1	5/A9-1
(L06)	L	AQL3-O	G	G-3	1/A9-1	4/A9-1	5/A9-1	(L38)	L	AQL3-O	G	G-3	1/A9-1	4/A9-1	5/A9-1	(L70)	L	AQL3-3	C	G-3	2/A9-1	4/A9-1	5/A9-1	(L102)	L	AQL3-3	C	G-3	2/A9-1	4/A9-1	5/A9-1	(L102)	L	AQL3-3	C	G-3	2/A9-1	4/A9-1	5/A9-1					
(L07)	L	AQL3-O	G	G-3	1/A9-1	4/A9-1	5/A9-1	(L39)	L	AQL3-O	G	G-3	1/A9-1	4/A9-1	5/A9-1	(L71)	L	AQL3-3	C	G-3	2/A9-1	4/A9-1	5/A9-1	(L103)	L	AQL3-3	C	G-3	2/A9-1	4/A9-1	5/A9-1	(L103)	L	AQL3-3	C	G-3	2/A9-1	4/A9-1	5/A9-1					
(L08)	L	AQL3-O	H	G-3	1/A9-1	3	4/A9-1	5/A9-1	(L40)	L	AQL3-O	H	G-3	1/A9-1	3	4/A9-1	5/A9-1	(L72)	L	AQL3-3	D	G-3	2/A9-1	3	4/A9-1	5/A9-1	(L104)	L	AQL3-3	D	G-3	2/A9-1	3	4/A9-1	5/A9-1	(L104)	L	AQL3-3	D	G-3	2/A9-1	3	4/A9-1	5/A9-1
(L09)	L	AQL3-O	F	IG-1	1/A9-1	3	4/A9-1	5/A9-1	(L41)	L	AQL3-O	F	IG-1	1/A9-1	3	4/A9-1	5/A9-1	(L73)	L	AQL3-3	B	IG-1	2/A9-1	3	4/A9-1	5/A9-1	(L105)	L	AQL3-3	B	IG-1	2/A9-1	3	4/A9-1	5/A9-1	(L105)	L	AQL3-3	B	IG-1	2/A9-1	3	4/A9-1	5/A9-1
(L10)	L	AQL3-O	E	IG-1	1/A9-1	4/A9-1	5/A9-1	(L42)	L	AQL3-O	E	IG-1	1/A9-1	4/A9-1	5/A9-1	(L74)	L	AQL3-3	A	IG-1	2/A9-1	4/A9-1	5/A9-1	(L106)	L	AQL3-3	A	IG-1	2/A9-1	4/A9-1	5/A9-1	(L106)	L	AQL3-3	A	IG-1	2/A9-1	4/A9-1	5/A9-1					
(L11)	L	AQL3-O	E	IG-1	1/A9-1	4/A9-1	5/A9-1	(L43)	L	AQL3-O	E	IG-1	1/A9-1	4/A9-1	5/A9-1	(L75)	L	AQL3-3	A	IG-1	2/A9-1	4/A9-1	5/A9-1	(L107)	L	AQL3-3	A	IG-1	2/A9-1	4/A9-1	5/A9-1	(L107)	L	AQL3-3	A	IG-1	2/A9-1	4/A9-1	5/A9-1					
(L12)	L	AQL3-O	F	IG-1	1/A9-1	3	4/A9-1	5/A9-1	(L44)	L	AQL3-O	F	IG-1	1/A9-1	3	4/A9-1	5/A9-1	(L76)	L	AQL3-3	B	IG-1	2/A9-1	3	4/A9-1	5/A9-1	(L108)	L	AQL3-3	B	IG-1	2/A9-1	3	4/A9-1	5/A9-1	(L108)	L	AQL3-3	B	IG-1	2/A9-1	3	4/A9-1	5/A9-1
(L13)	L	AQL3-O	F	IG-1	1/A9-1	3	4/A9-1	5/A9-1	(L45)	L	AQL3-O	F	IG-1	1/A9-1	3	4/A9-1	5/A9-1	(L77)	L	AQL3-3	B	IG-1	2/A9-1	3	4/A9-1	5/A9-1	(L109)	L	AQL3-3	B	IG-1	2/A9-1	3	4/A9-1	5/A9-1	(L109)	L	AQL3-3	B	IG-1	2/A9-1	3	4/A9-1	5/A9-1
(L14)	L	AQL3-O	E	IG-1	1/A9-1	4/A9-1	5/A9-1	(L46)	L	AQL3-O	E	IG-1	1/A9-1	4/A9-1	5/A9-1	(L78)	L	AQL3-3	A	IG-1	2/A9-1	4/A9-1	5/A9-1	(L110)	L	AQL3-3	A	IG-1	2/A9-1	4/A9-1	5/A9-1	(L110)	L	AQL3-3	A	IG-1	2/A9-1	4/A9-1	5/A9-1					
(L15)	L	AQL3-O	E	IG-1	1/A9-1	4/A9-1	5/A9-1	(L47)	L	AQL3-O	E	IG-1	1/A9-1	4/A9-1	5/A9-1	(L79)	L	AQL3-3	A	IG-1	2/A9-1	4/A9-1	5/A9-1	(L111)	L	AQL3-3	A	IG-1	2/A9-1	4/A9-1	5/A9-1	(L111)	L	AQL3-3	A	IG-1	2/A9-1	4/A9-1	5/A9-1					
(L16)	L	AQL3-O	F	IG-1	1/A9-1	3	4/A9-1	5/A9-1	(L48)	L	AQL3-O	F	IG-1	1/A9-1	3	4/A9-1	5/A9-1	(L80)	L	AQL3-3	B	IG-1	2/A9-1	3	4/A9-1	5/A9-1	(L112)	L	AQL3-3	B	IG-1	2/A9-1	3	4/A9-1	5/A9-1	(L112)	L	AQL3-3	B	IG-1	2/A9-1	3	4/A9-1	5/A9-1
(L17)	L	AQL3-O	H	G-3	1/A9-1	3	4/A9-1	5/A9-1	(L49)	L	AQL3-O	H	G-3	1/A9-1	3	4/A9-1	5/A9-1	(L81)	L	AQL3-3	D	G-3	2/A9-1	3	4/A9-1	5/A9-1	(L113)	L	AQL3-3	D	G-3	2/A9-1	3	4/A9-1	5/A9-1	(L113)	L	AQL3-3	D	G-3	2/A9-1	3	4/A9-1	5/A9-1
(L18)	L	AQL3-O	G	G-3	1/A9-1	4/A9-1	5/A9-1	(L50)	L	AQL3-O	G	G-3	1/A9-1	4/A9-1	5/A9-1	(L82)	L	AQL3-3	C	G-3	2/A9-1	4/A9-1	5/A9-1	(L114)	L	AQL3-3	C	G-3	2/A9-1	4/A9-1	5/A9-1	(L114)	L	AQL3-3	C	G-3	2/A9-1	4/A9-1	5/A9-1					
(L19)	L	AQL3-O	G	G-3	1/A9-1	4/A9-1	5/A9-1	(L51)	L	AQL3-O	G	G-3	1/A9-1	4/A9-1	5/A9-1	(L83)	L	AQL3-3	C	G-3	2/A9-1	4/A9-1	5/A9-1	(L115)	L	AQL3-3	C	G-3	2/A9-1	4/A9-1	5/A9-1	(L115)	L	AQL3-3	C	G-3	2/A9-1	4/A9-1	5/A9-1					
(L20)	L	AQL3-O	H	G-3	1/A9-1	3	4/A9-1	5/A9-1	(L52)	L	AQL3-O	H	G-3	1/A9-1	3	4/A9-1	5/A9-1	(L84)	L	AQL3-3	D	G-3	2/A9-1	3	4/A9-1	5/A9-1	(L116)	L	AQL3-3	D	G-3	2/A9-1	3	4/A9-1	5/A9-1	(L116)	L	AQL3-3	D	G-3	2/A9-1	3	4/A9-1	5/A9-1
(L21)	L	AQL3-O	H	G-3	1/A9-1	3	4/A9-1	5/A9-1	(L53)	L	AQL3-O	H	G-3	1/A9-1	3	4/A9-1	5/A9-1	(L85)	L	AQL3-3	D	G-3	2/A9-1	3	4/A9-1	5/A9-1	(L117)	L	AQL3-3	D	G-3	2/A9-1	3	4/A9-1	5/A9-1	(L117)	L	AQL3-3	D	G-3	2/A9-1	3	4/A9-1	5/A9-1
(L22)	L	AQL3-O	G	G-3	1/A9-1	4/A9-1	5/A9-1	(L54)	L	AQL3-O	G	G-3	1/A9-1	4/A9-1	5/A9-1	(L86)	L	AQL3-3	C	G-3	2/A9-1	4/A9-1	5/A9-1	(L118)	L	AQL3-3	C	G-3	2/A9-1	4/A9-1	5/A9-1	(L118)	L	AQL3-3	C	G-3	2/A9-1	4/A9-1	5/A9-1					
(L23)	L	AQL3-O	G	G-3	1/A9-1	4/A9-1	5/A9-1	(L55)	L	AQL3-O	G	G-3	1/A9-1	4/A9-1	5/A9-1	(L87)	L	AQL3-3	C	G-3	2/A9-1	4/A9-1	5/A9-1	(L119)	L	AQL3-3	C	G-3	2/A9-1	4/A9-1	5/A9-1	(L119)	L	AQL3-3	C	G-3	2/A9-1	4/A9-1	5/A9-1					
(L24)	L	AQL3-O	H	G-3	1/A9-1	3	4/A9-1	5/A9-1	(L56)	L	AQL3-O	H	G-3	1/A9-1	3	4/A9-1	5/A9-1	(L88)	L	AQL3-3	D	G-3	2/A9-1	3	4/A9-1	5/A9-1	(L120)	L	AQL3-3	D	G-3	2/A9-1	3	4/A9-1	5/A9-1	(L120)	L	AQL3-3	D	G-3	2/A9-1	3	4/A9-1	5/A9-1
(L25)	L	AQL3-O	F	IG-1	1/A9-1	3	4/A9-1	5/A9-1	(L57)	L	AQL3-O	F	IG-1	1/A9-1	3	4/A9-1	5/A9-1	(L89)	L	AQL3-3	B	IG-1	2/A9-1	3	4/A9-1	5/A9-1	(L121)	L	AQL3-3	B	IG-1	2/A9-1	3	4/A9-1	5/A9-1	(L121)	L	AQL3-3	B	IG-1	2/A9-1	3	4/A9-1	5/A9-1
(L26)	L	AQL3-O	E	IG-1	1/A9-1	4/A9-1	5/A9-1	(L58)	L	AQL3-O	E	IG-1	1/A9-1	4/A9-1	5/A9-1	(L90)	L	AQL3-3	A	IG-1	2/A9-1	4/A9-1	5/A9-1	(L122)	L	AQL3-3	A	IG-1	2/A9-1	4/A9-1	5/A9-1	(L122)	L	AQL3-3	A	IG-1	2/A9-1	4/A9-1	5/A9-1					
(L27)	L	AQL3-O	E	IG-1	1/A9-1	4/A9-1	5/A9-1	(L59)	L	AQL3-O	E	IG-1	1/A9-1	4/A9-1	5/A9-1	(L91)	L	AQL3-3	A	IG-1	2/A9-1	4/A9-1	5/A9-1	(L123)	L	AQL3-3	A	IG-1	2/A9-1	4/A9-1	5/A9-1	(L123)	L	AQL3-3	A	IG-1	2/A9-1	4/A9-1	5/A9-1					
(L28)	L	AQL3-O	F	IG-1	1/A9-1	3	4/A9-1	5/A9-1	(L60)	L	AQL3-O	F	IG-1	1/A9-1	3	4/A9-1	5/A9-1	(L92)	L	AQL3-3	B	IG-1	2/A9-1	3	4/A9-1	5/A9-1	(L124)	L	AQL3-3	B	IG-1	2/A9-1	3	4/A9-1	5/A9-1	(L124)	L	AQL3-3	B	IG-1	2/A9-1	3	4/A9-1	5/A9-1
(L29)	L	AQL3-O	F	IG-1	1/A9-1	3	4/A9-1	5/A9-1	(L61)	L	AQL3-O	F	IG-1	1/A9-1	3	4/A9-1	5/A9-1	(L93)	L	AQL3-3	B	IG-1	2/A9-1	3	4/A9-1	5/A9-1	(L125)	L	AQL3-3	B	IG-1	2/A9-1	3	4/A9-1	5/A9-1	(L125)	L	AQL3-3	B	IG-1	2/A9-1	3	4/A9-1	5/A9-1
(L30)	L	AQL3-O	E	IG-1	1/A9-1	4/A9-1	5/A9-1	(L62)	L	AQL3-O	E	IG-1	1/A9-1	4/A9-1	5/A9-1	(L94)	L	AQL3-3	A	IG-1	2/A9-1	4/A9-1	5/A9-1	(L126)	L	AQL3-3	A	IG-1	2/A9-1	4/A9-1	5/A9-1	(L126)	L	AQL3-3	A	IG-1	2/A9-1	4/A9-1	5/A9-1					
(L31)	L	AQL3-O	E	IG-1	1/A9-1	4/A9-1	5/A9-1	(L63)	L	AQL3-O	E	IG-1	1/A9-1	4/A9-1	5/A9-1	(L95)	L	AQL3-3	A	IG-1	2/A9-1	4/A9-1	5/A9-1	(L127)	L	AQL3-3	A	IG-1	2/A9-1	4/A9-1	5/A9-1	(L127)	L	AQL3-3	A	IG-1	2/A9-1	4/A9-1	5/A9-1					
(L32)	L	AQL3-O	F	IG-1	1/A9-1	3	4/A9-1	5/A9-1	(L64)	L	AQL3-O	F	IG-1	1/A9-1	3	4/A9-1	5/A9-1	(L96)	L	AQL3-3	B	IG-1	2/A9-1	3	4/A9-1	5/A9-1	(L128)	L	AQL3-3	B	IG-1	2/A9-1	3	4/A9-1	5/A9-1	(L128)	L	AQL3-3	B	IG-1	2/A9-1	3	4/A9-1	5/A9-1



- SHEET NOTES**
- 1 FIXED SASH
 - 2 PROJECT OUT AWNING
 - 3 PROJECT IN HOPPER

PLOTTED 3/18/2005 10:30 PM

GROTH ARCHITECTS, INC.
823 ACACIA STREET
OCEANSIDE, CA 92054
OCEANSIDE UNIFIED S.D.

JEFFERSON MS NEW CONSTRUCTION
3355 MISSION AVE. SUITE 234
OCEANSIDE, CALIFORNIA 92054

PHONE 760-754-8191
FAX 760-754-8291

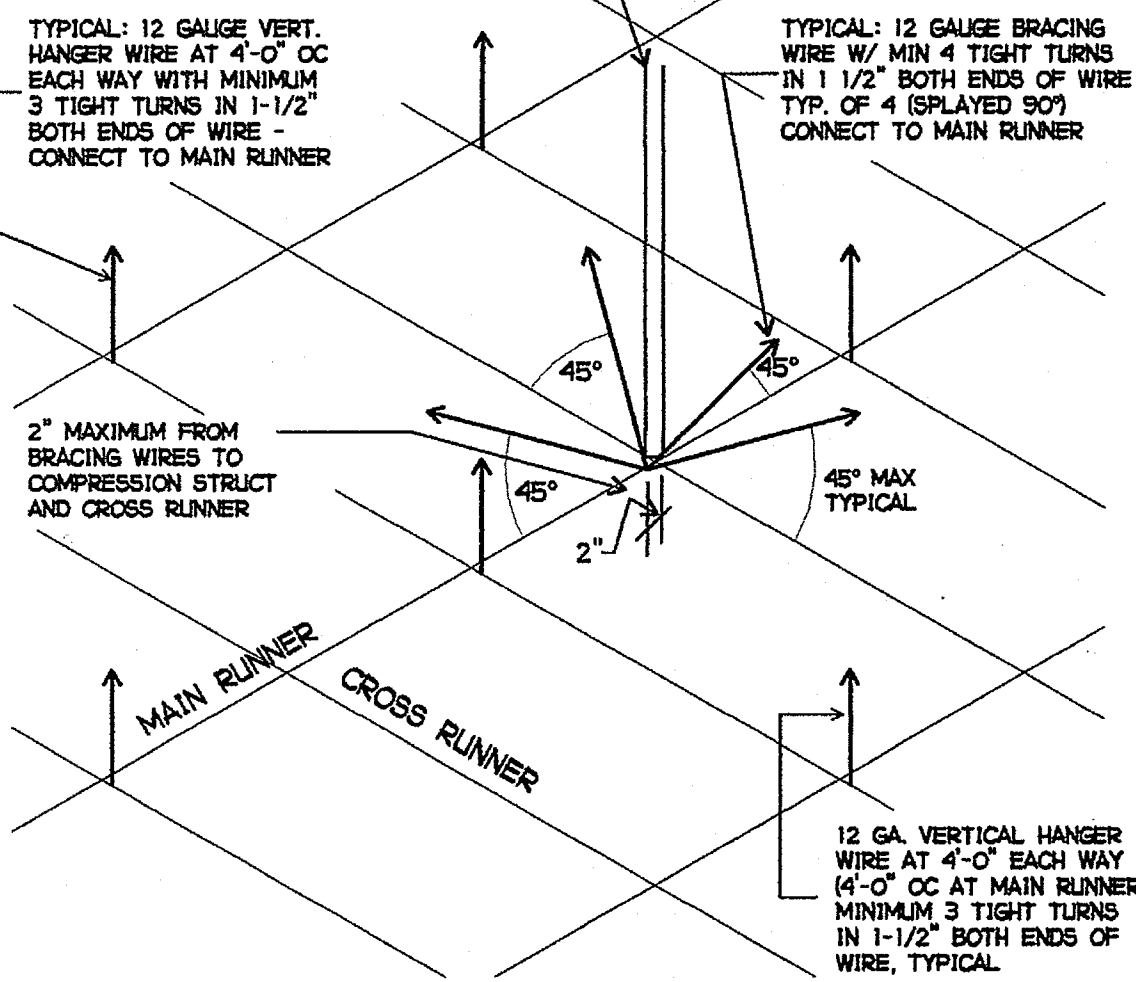
PROJECT NO. 025
DATE

REVISIONS

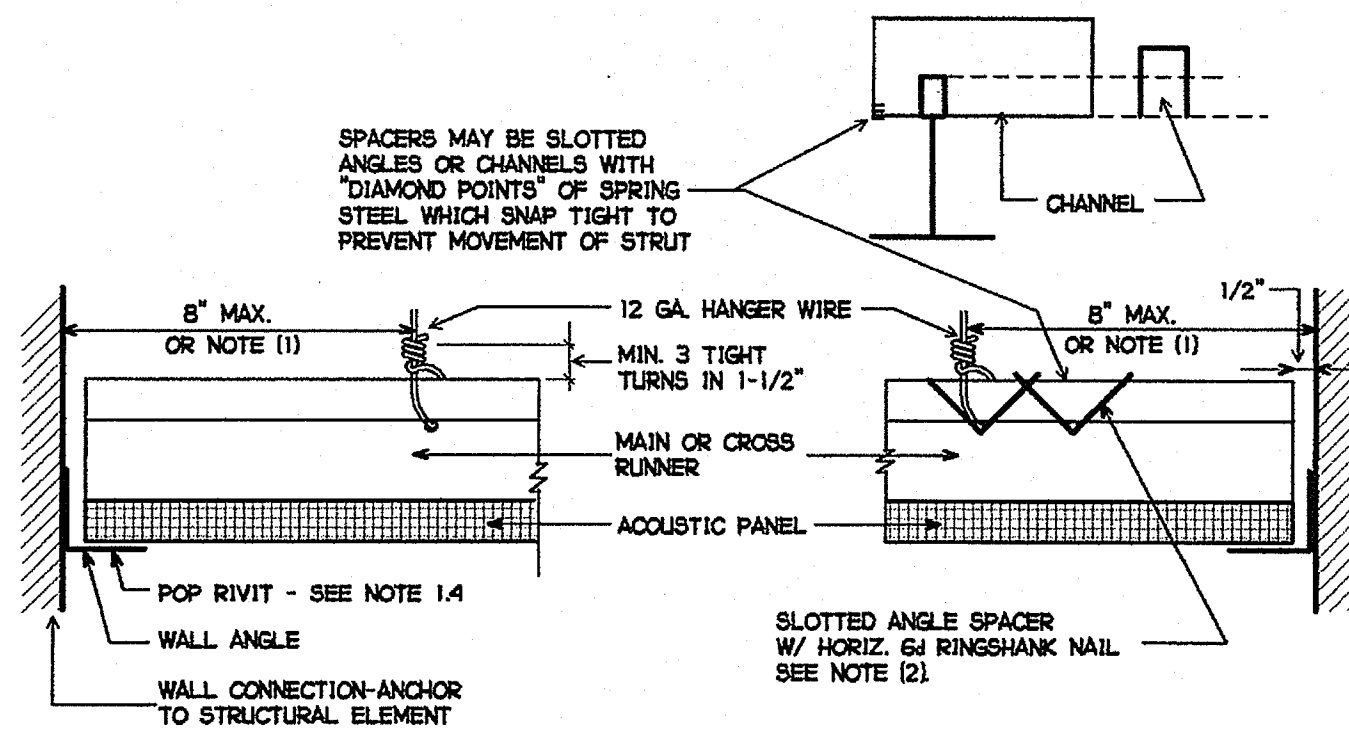
DBA IDENTIFICATION STAMP
DIV. OF THE STATE ARCHITECT
OFFICE OF REGULATION SERVICES
4-106494
AC [Signature] FLS [Signature] SS [Signature]
DATE MAR 2 8 2

NOTE: SEE FIGURES 3 AND 4 FOR CONNECTIONS OF BRACING AND HANGING WIRES TO THE STRUCTURE ABOVE.

COMPRESSION STRUTS: 4" x 2" FLANGE X 20 GA. STEEL STUD (L/R RATIO OF 200 MAXIMUM) ATTACH TO MAIN RUNNERS WITHIN 2" OF CROSS RUNNERS WITH 2-#12 SELF-DRILLING SELF-TAPPING (SDS) SCREWS AND TO STRUCTURE WITH 2-#12 x 2" SCREWS AT WOOD OR 3/16" DIAMETER ANCHOR AT CONCRETE/STEEL. COMPRESSION STRUT SHALL NOT REPLACE HANGER WIRE.

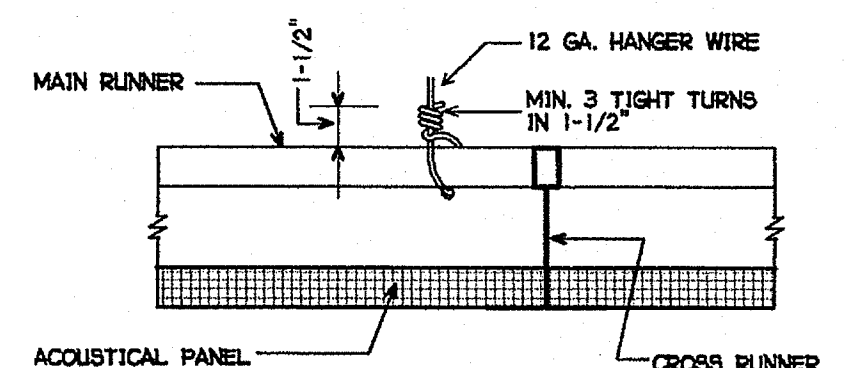


SUSPENDED CEILING BRACING ASSEMBLY
FIGURE NO. 1

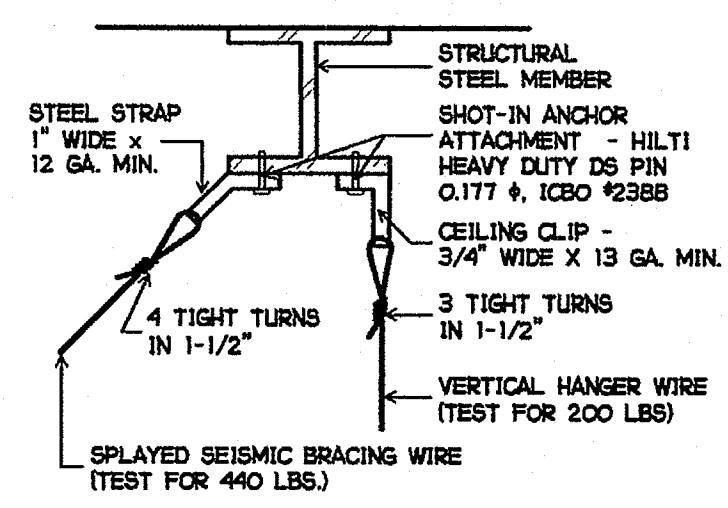


DETAIL (A) HORIZONTAL STRUT - TYPICAL (SEE NOTE 1.5)

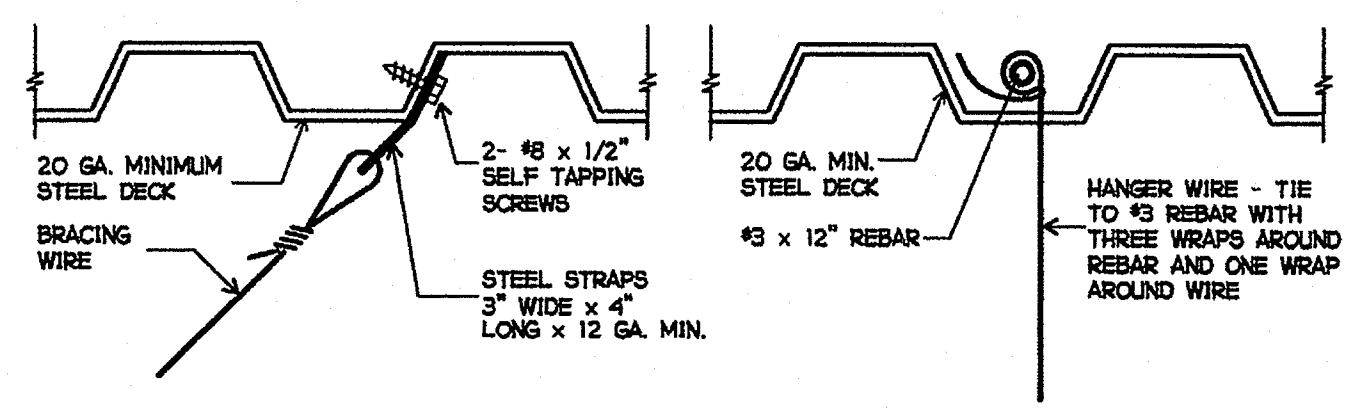
NOTES: (1) 1/4 OF THE LENGTH OF THE END OF RUNNER WHICHEVER IS LESS
(2) NAILS AT ENDS OF HORIZONTAL STRUTS ARE TO BE PLACED WITH NAIL HEAD TOWARD CENTERLINE OF SPAN OF STRUT.



DETAIL (B)
ACCEPTABLE HANGER WIRE CONNECTION TO GRID
FIGURE NO. 2



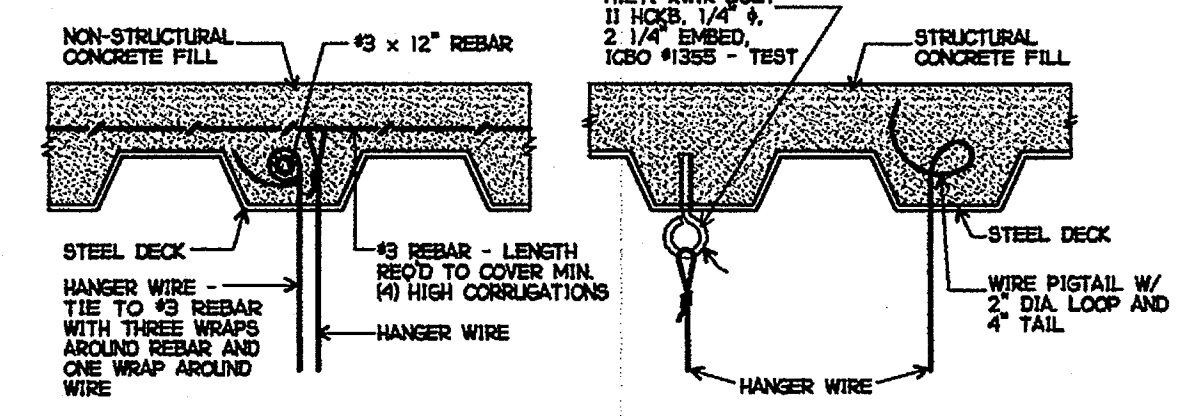
(A) AT STEEL BEAMS



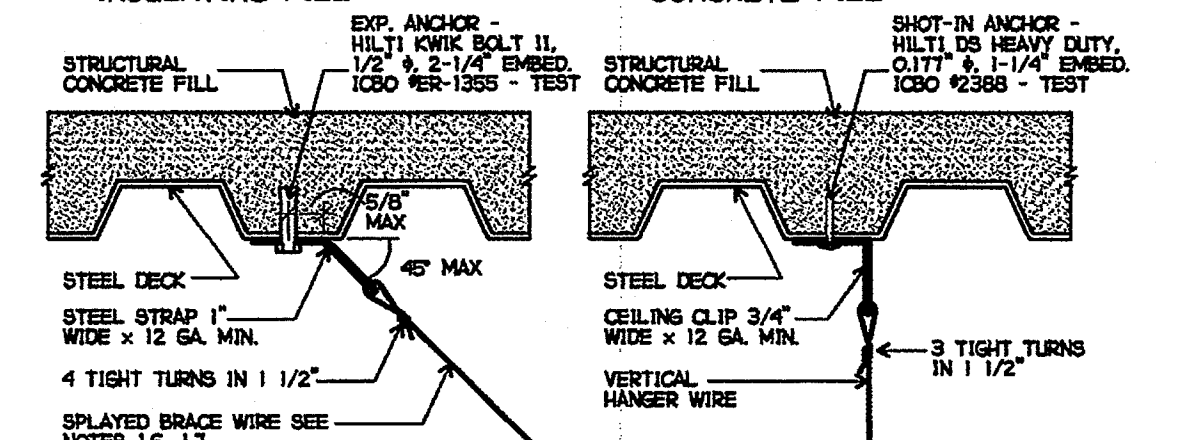
(B) AT STEEL ROOF DECK (C) AT STEEL ROOF DECK

NOTE: IF SELF TAPPING SCREWS ARE USED WITH CONCRETE FILL, SET SCREWS BEFORE PLACING CONCRETE.

ACCEPTABLE DETAILS - WIRE CONNECTIONS TO STEEL FRAMING
FIGURE NO. 3



(A) AT STEEL DECK WITH INSULATING FILL (B) AT STEEL DECK WITH CONCRETE FILL



(C) AT STEEL DECK WITH CONCRETE FILL (D) AT STEEL DECK WITH CONCRETE FILL

NOTE: IF SELF TAPPING SCREWS ARE USED WITH CONCRETE FILL, SET SCREWS BEFORE PLACING CONCRETE.

ACCEPTABLE DETAILS - WIRE CONNECTIONS TO STEEL FRAMING
FIGURE NO. 4

CEILING NOTES

- CEILING NOTES. THE FOLLOWING NOTES WILL BE ACCEPTABLE IN PLANS AND SPECIFICATIONS FOR CEILING SYSTEMS WHOSE TOTAL WEIGHT, INCLUDING AIR CONDITIONING GRILLES AND LIGHT FIXTURES, DOES NOT EXCEED FOUR (4) PSF. HEAVIER SYSTEMS AND THOSE SUPPORTING LATERAL LOADS FROM PARTITIONS, WILL REQUIRE SPECIAL DESIGN DETAILS.
- 12 GA. (MIN) HANGER WIRES MAY BE USED FOR UP TO AND INCLUDING 4'-0" x 4'-0" GRID SPACING AND SHALL BE ATTACHED TO MAIN RUNNERS.
- PROVIDE 12 GA. HANGER WIRES AT THE ENDS OF ALL MAIN AND CROSS RUNNERS WITHIN EIGHT INCHES (8") OF THE SUPPORT OR WITHIN ONE-FOURTH (1/4) OF THE LENGTH OF THE END TEE, WHICHEVER IS LEAST, FOR THE PERIMETER OF THE CEILING AREA. END CONNECTIONS FOR RUNNERS WHICH ARE DESIGNED AND DETAILED TO RESIST THE APPLIED VERTICAL AND HORIZONTAL FORCES MAY BE USED IN LIEU OF THE 12 GA. HANGER WIRES, SUBJECT TO DIVISION OF THE STATE ARCHITECT DSA REVIEW AND APPROVAL.
- PROVIDE TRAPEZE OR OTHER SUPPLEMENTARY SUPPORT MEMBERS AT OBSTRUCTIONS TO TYPICAL HANGER SPACING. PROVIDE ADDITIONAL HANGERS, STRUTS OR BRACES AS REQUIRED AT ALL CEILING BREAKS, SOFFITS OR DISCONTINUOUS AREAS. HANGER WIRES THAT ARE MORE THAN 1 IN 6 OUT OF PLUMB ARE TO HAVE COUNTERSLOPING WIRES.
- CEILING GRID MEMBERS MAY BE ATTACHED TO NOT MORE THAN TWO (2) ADJACENT WALLS. CEILING GRID MEMBERS SHALL BE AT LEAST 1/2" FREE OF OTHER WALLS. IF WALLS RUN DIAGONALLY TO CEILING GRID SYSTEM RUNNERS, ONE END OF MAIN AND CROSS RUNNERS SHOULD BE FREE, AND A MINIMUM OF 1/2" CLEAR OF WALL.

- AT THE PERIMETER OF THE CEILING AREA WHERE MAIN OR CROSS RUNNERS ARE NOT CONNECTED TO THE ADJACENT WALL, PROVIDE INTERCONNECTION BETWEEN THE RUNNERS AT THE FREE END TO PREVENT LATERAL SPREADING. A METAL STRUT OR A 16 GA. WIRE WITH A POSITIVE MECHANICAL CONNECTION TO THE RUNNER MAY BE USED. WHERE THE PERPENDICULAR DISTANCE FROM THE WALL TO THE FIRST PARALLEL RUNNER IS 12" OR LESS, THIS INTERLOCK IS NOT REQUIRED.
- PROVIDE BRACING ASSEMBLIES CONSISTING OF A COMPRESSION STRUT AND FOUR (4) 12 GA. SPLAYED BRACING WIRES ORIENTED 90 DEGREES FROM EACH OTHER (SEE FIGURE 1) AT THE FOLLOWING SPACINGS:
 - FOR SCHOOL BUILDINGS, PLACE BRACING ASSEMBLIES AT A SPACING NOT MORE THAN 12' BY 12' ON CENTER.
 - FOR ESSENTIAL SERVICES BUILDINGS, PLACE BRACING ASSEMBLIES NOT MORE THAN 8' BY 12' ON CENTER.
 - PROVIDE BRACING ASSEMBLIES AT LOCATIONS NOT MORE THAN ONE HALF (1/2) THE SPACINGS GIVEN ABOVE, FROM EACH PERIMETER WALL AND AT THE EDGE OF VERTICAL CEILING OFFSETS.

THE SLOPE OF THESE WIRES SHALL NOT EXCEED 45 DEGREES FROM THE PLANE OF THE CEILING AND SHALL BE TAUT. SPLICES IN BRACING WIRES ARE NOT TO BE PERMITTED WITHOUT SPECIAL DSA APPROVAL.
 - SUSPENDED ACOUSTICAL CEILING SYSTEMS WITH A CEILING AREA OF 144 SQUARE FEET OR LESS, AND FIRE RATED SUSPENDED ACOUSTICAL CEILING SYSTEMS WITH A CEILING AREA OF 96 SQUARE FEET OR LESS, SURROUNDED BY WALLS WHICH CONNECT DIRECTLY TO THE STRUCTURE ABOVE, DO NOT REQUIRE BRACING ASSEMBLIES WHEN ATTACHED TO TWO ADJACENT WALLS.

- FASTEN HANGER WIRES WITH NOT LESS THAN THREE (3) TIGHT TURNS. FASTEN BRACING WIRES WITH FOUR (4) TIGHT TURNS. MAKE ALL TIGHT TURNS WITHIN A DISTANCE OF 1 1/2" INCHES. HANGER OR BRACING WIRE ANCHORS TO THE STRUCTURE SHOULD BE INSTALLED IN SUCH A MANNER THAT THE DIRECTION OF THE WIRE ALIGNS AS CLOSELY AS POSSIBLE WITH THE DIRECTION OF THE FORCES ACTING ON THE WIRE. NOTE: WIRE TURNS MADE BY MACHINE WHERE BOTH STRANDS HAVE BEEN DEFORMED OR BENT IN WRAPPING CAN WAIVE THE 1" REQUIREMENT 1 1/2" REQUIREMENT, BUT THE NUMBER OF TURNS SHOULD BE MAINTAINED, AND BE AS TIGHT AS POSSIBLE.
- SEPARATE ALL CEILING HANGING AND BRACING WIRES AT LEAST SIX INCHES (6") FROM ALL UNBRACED DUCTS, PIPES, CONDUIT, ETC. IT IS ACCEPTABLE TO ATTACH LIGHTWEIGHT ITEMS, SUCH AS SINGLE ELECTRICAL CONDUIT NOT EXCEEDING 3/4" NOMINAL DIAMETER, TO HANGER WIRES USING CONNECTORS ACCEPTABLE TO DSA.
- WHEN DRILLED-IN CONCRETE ANCHORS OR SHOT-IN ANCHORS ARE USED IN REINFORCED CONCRETE FOR HANGER WIRES, 1 OUT OF 10 MUST BE FIELD TESTED FOR 200 LBS. IN TENSION. WHEN DRILLED-IN CONCRETE ANCHORS ARE USED FOR BRACING WIRES, 1 OUT OF 2 MUST BE FIELD TESTED FOR 440 LBS. IN TENSION. SHOT-IN ANCHORS IN CONCRETE ARE NOT PERMITTED FOR BRACING WIRES, IF ANY SHOT IN OR DRILLED-IN ANCHOR FAILS, SEE CBC, SECTION 1923A.3.5. NOTE: DRILLED-IN OR SHOT-IN ANCHORS REQUIRE SPECIAL DSA APPROVAL WHEN USED IN PRESTRESSED CONCRETE.
- ATTACH ALL LIGHT FIXTURES AND CEILING MOUNTED AIR TERMINALS OR SERVICES, TO THE CEILING GRID RUNNERS TO RESIST A HORIZONTAL FORCE EQUAL TO THE WEIGHT OF THE FIXTURES, SCREWS OR APPROVED FASTENERS ARE REQUIRED.

- FLUSH OR RECESSED LIGHT FIXTURES AND AIR TERMINALS OR SERVICES, WEIGHING LESS THAN 56 LBS., MAY BE SUPPORTED DIRECTLY ON THE RUNNERS OF A HEAVY DUTY GRID SYSTEM BUT, IN ADDITION, THEY MUST HAVE A MINIMUM OF TWO 12 GA. SLACK SAFETY WIRES ATTACHED TO THE FIXTURE AT DIAGONAL CORNERS AND ANCHORED TO THE STRUCTURE ABOVE. ALL 4' x 4' LIGHT FIXTURES MUST HAVE SLACK SAFETY WIRES AT EACH CORNER.

ALL FLUSH OR RECESSED LIGHT FIXTURES AND AIR TERMINALS OR SERVICES WEIGHING 56 LBS. OR MORE MUST BE INDEPENDENTLY SUPPORTED BY NOT LESS THAN FOUR (4) TAUT 12 GA. WIRES EACH ATTACHED TO THE FIXTURE AND TO THE STRUCTURE ABOVE REGARDLESS OF THE TYPE OF CEILING SYSTEM USED.

THE FOUR (4) TAUT 12 GA. WIRES INCLUDING THEIR ATTACHMENT TO THE STRUCTURE ABOVE MUST BE CAPABLE OF SUPPORTING FOUR (4) TIMES THE WEIGHT OF THE UNIT.
- ALL FIXTURES AND AIR TERMINALS OR SERVICES SUPPORTED ON INTERMEDIATE DUTY GRID SYSTEMS MUST BE INDEPENDENTLY SUPPORTED BY NOT LESS THAN FOUR (4) TAUT 12 GA. WIRES EACH ATTACHED TO THE FIXTURE OR TERMINAL, AND TO THE STRUCTURE ABOVE.
- SUPPORT SURFACE MOUNTED LIGHT FIXTURES BY AT LEAST TWO POSITIVE DEVICES WHICH SURROUND THE CEILING RUNNER AND WHICH ARE EACH SUPPORTED FROM THE STRUCTURE ABOVE BY A 12 GA. WIRE. SPRINGS CLIPS OR CLAMPS THAT CONNECT ONLY TO THE RUNNER ARE NOT ACCEPTABLE.

PROVIDE ADDITIONAL SUPPORTS WHEN LIGHT FIXTURES ARE 6'-0" OR LONGER.

- SUPPORT PENDANT MOUNTED LIGHT FIXTURES DIRECTLY FROM THE STRUCTURE ABOVE WITH HANGER WIRES OR CABLES PASSING THROUGH EACH PENDANT HANGER AND CAPABLE OF SUPPORTING FOUR (4) TIMES THE WEIGHT OF THE FIXTURE. A BRACING ASSEMBLY PER FIGURE 1, IS REQUIRED WHERE THE PENDANT HANGER PENETRATES THE CEILING. SPECIAL DETAILS ARE REQUIRED TO ATTACH THE PENDANT HANGER TO THE BRACING ASSEMBLY TO TRANSMIT HORIZONTAL FORCES.
- CLASSIFICATION OF CEILING GRID.

CLASSIFICATION OF CEILING GRID IS HEAVY DUTY. MANUFACTURER'S CATALOG NUMBER - MAIN RUNNER: ARMSTRONG: 7301 DOWN : DX/DXL26 CELOTEX: C12-12-20

MANUFACTURER'S CATALOG NUMBER - CROSS RUNNER: ARMSTRONG: XL1341 DOWN : DX/DXL424 CELOTEX: Q44-12-15

MANUFACTURER'S CATALOG NUMBER OF DETAIL FOR RUNNER SPLICE: ALL SPLICES UTILIZE BUILT-IN CLIPS FROM MANUFACTURER
- ADDITIONAL REQUIREMENTS FOR FIRE RATED CEILING.
 - PROVIDE UNDERWRITER LABORATORY (U.L.) DESIGN NUMBER OR STATE FIRE MARSHAL (SFM) LISTING NUMBER. THE COMPONENTS AND INSTALLATION DETAILS MUST CONFORM IN EVERY RESPECT WITH THE U.L. OR SFM APPROVAL FOR THE DESIGN NUMBER SPECIFIED. CUSTOM DESIGNS WHICH COMBINE COMPONENTS FROM DIFFERENT APPROVED DESIGNS BUT HAVE NOT BEEN TESTED AS A COMPLETE ASSEMBLY ARE NOT ACCEPTABLE.
 - FOR SCHOOLS AND ESSENTIAL SERVICES BUILDINGS, BRACING ASSEMBLIES ARE REQUIRED FOR EACH 96 SQUARE FEET. THE FIRST BRACING ASSEMBLY IS REQUIRED NOT MORE THAN FOUR FEET (4'-0") FROM EACH WALL. A MINIMUM OF ONE BRACING ASSEMBLY IS REQUIRED BETWEEN ANY TWO ADJACENT EXPANSION CUT-OUTS ON RUNNERS BEING BRACED.

- POP RIVETS, SCREWS, OR OTHER ATTACHMENTS ARE NOT ACCEPTABLE UNLESS SPECIFICALLY DETAILED ON THE DRAWINGS AND APPROVED BY U.L. AND SFM.
- ADDITIONAL REQUIREMENTS FOR METAL PANELS, METAL PANELS AND PANELS WEIGHING MORE THAN 1/2 PSF, OTHER THAN ACOUSTICAL TILE, ARE TO BE POSITIVELY ATTACHED TO THE CEILING SUSPENSION RUNNERS.
- SUSPENDED ACOUSTICAL CEILING BELOW GYPSUM BOARD CEILING. WHERE GYPSUM BOARD OR OTHER CEILING FINISHES ARE ATTACHED TO THE FRAMING, SPECIAL DETAILS WILL BE REQUIRED FOR THE VERTICAL HANGER WIRE AND LATERAL BRACING WIRE SUPPORT CONNECTIONS TO THE FRAMING.
- REUSE OF EXISTING CEILING HANGER WIRES AND SPLAY WIRES.
- THE GAGE AND SPACING OF THE WIRES MUST COMPLY WITH THE CURRENT APPLICABLE CODES.
- ALL EXISTING CEILING HANGER WIRES MUST BE TESTED TO 200 LBS. IN TENSION.
- ALL EXISTING SPLAYED BRACING WIRES MUST BE FIELD TESTED TO 440 LBS. IN TENSION.
- IF A NEW WIRE IS TO BE SPLICED TO AN EXISTING WIRE, THE FOLLOWING IS REQUIRED:
 - THE ARCHITECT OR STRUCTURAL ENGINEER IN GENERAL RESPONSIBLE CHARGE MUST SUBMIT TO DSA A DETAIL AND SPECIFICATION DESCRIBING HOW THE SPLICE IS TO BE MADE.
 - ALL NEW WIRES, AFTER BEING SPLICED TO THE EXISTING WIRES, MUST BE FIELD TESTED PER ITEMS 5.2 AND 5.3 ABOVE.
 - ALL FIELD TESTS MUST BE PERFORMED IN THE PRESENCE OF THE PROJECT INSPECTOR.

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JEFFERSON MS NEW CONSTRUCTION
823 ACACIA STREET
OCEANSIDE, CA 92054
OCEANSIDE UNIFIED S.D.

GROTH ARCHITECTS, INC.
3355 MISSION AVE. SUITE 234
OCEANSIDE, CALIFORNIA 92054

PHONE 760-754-8191
FAX 760-754-8291

CLSD NO. 758-000
PROJECT NOS. 025
P. T. N. 73569-9
DATE
REVISIONS

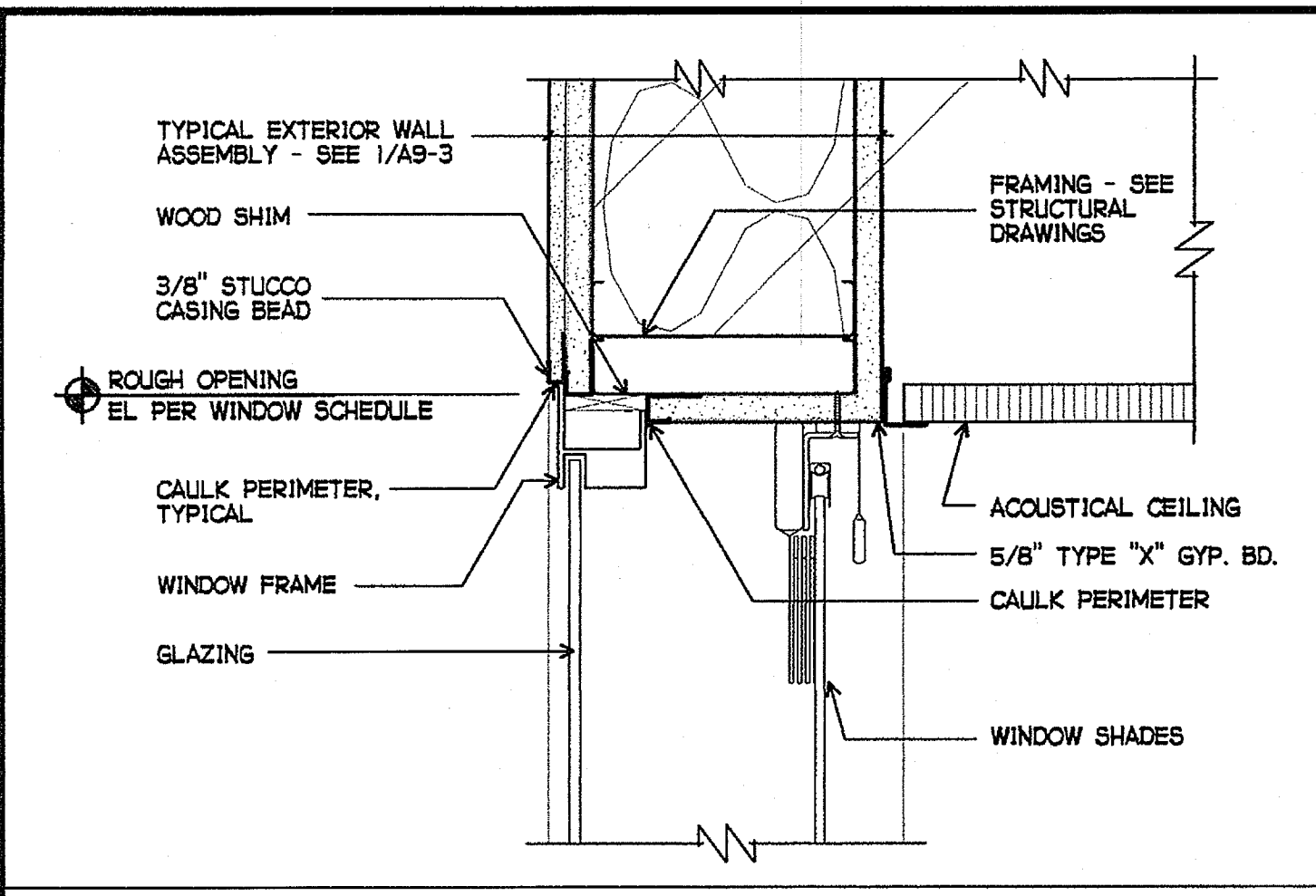
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4-106494
AC. [initials] FS. [initials] SS. [initials]
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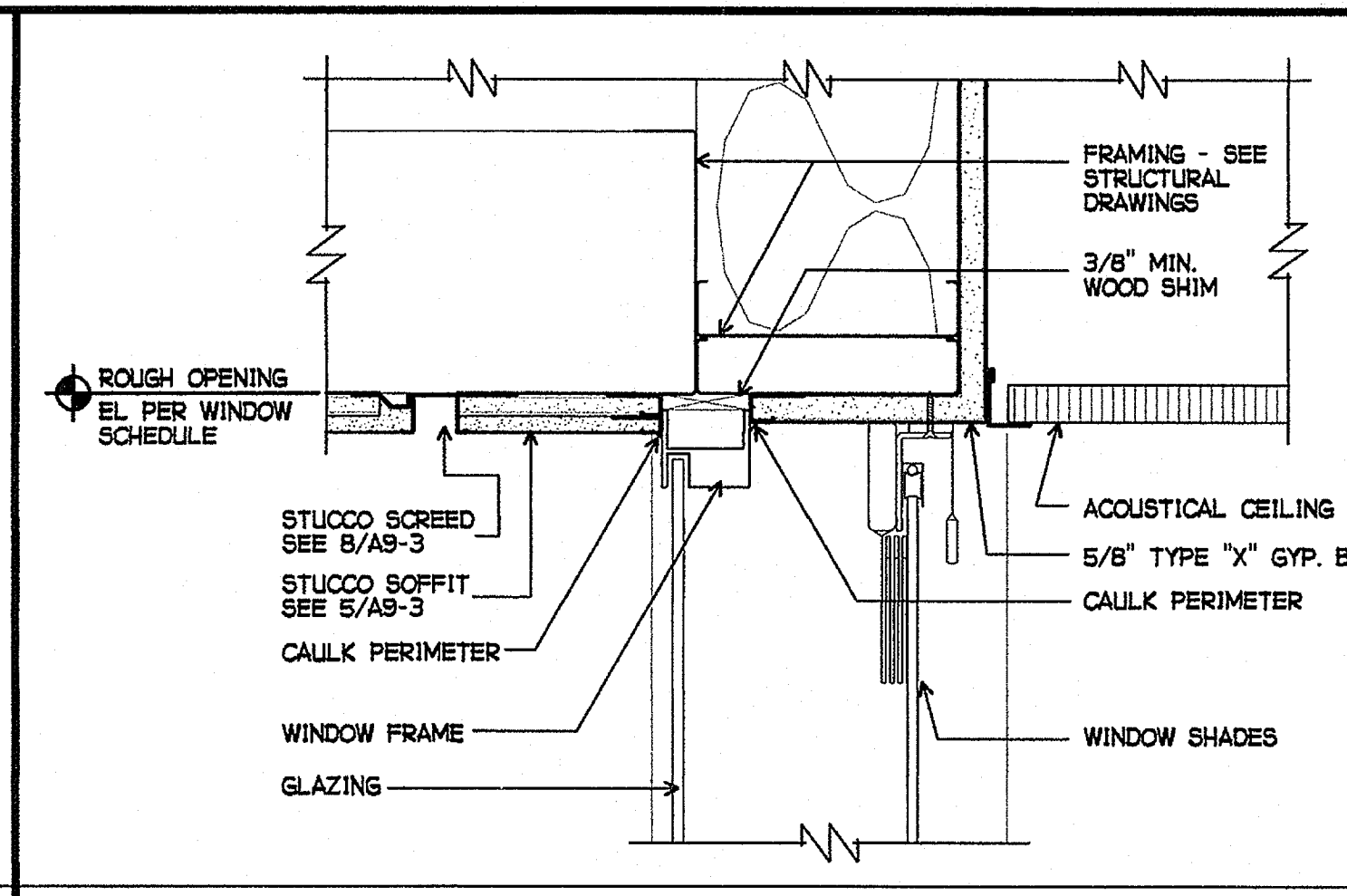
REGISTERED ARCHITECT
JOHN SCOTT BROTHERS
C-26609
4/30/2007 RENEWAL
STATE OF CALIFORNIA

SHEET TITLE
TYPICAL
SUSPENDED
CEILING DETAILS
A9-0

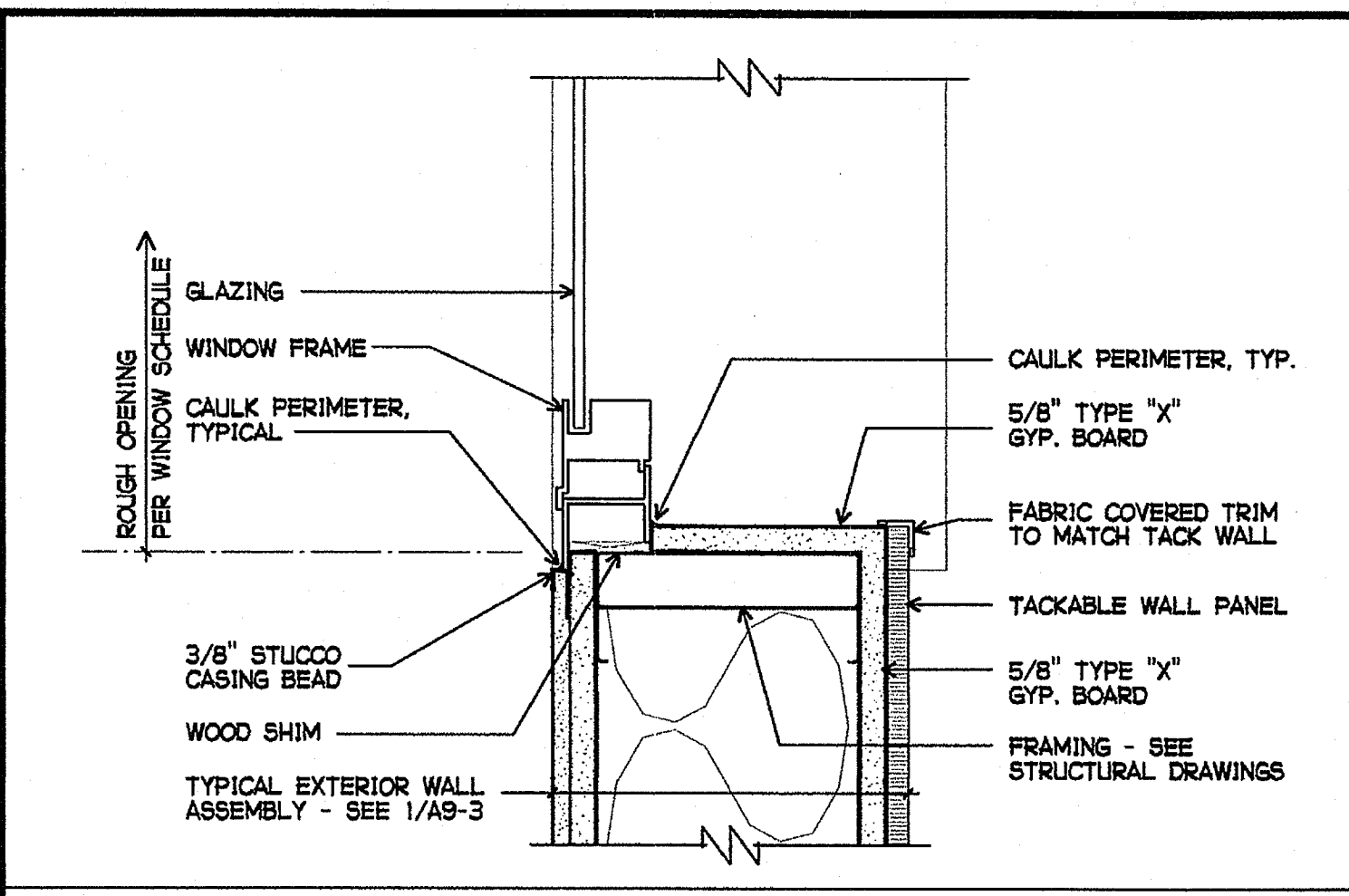
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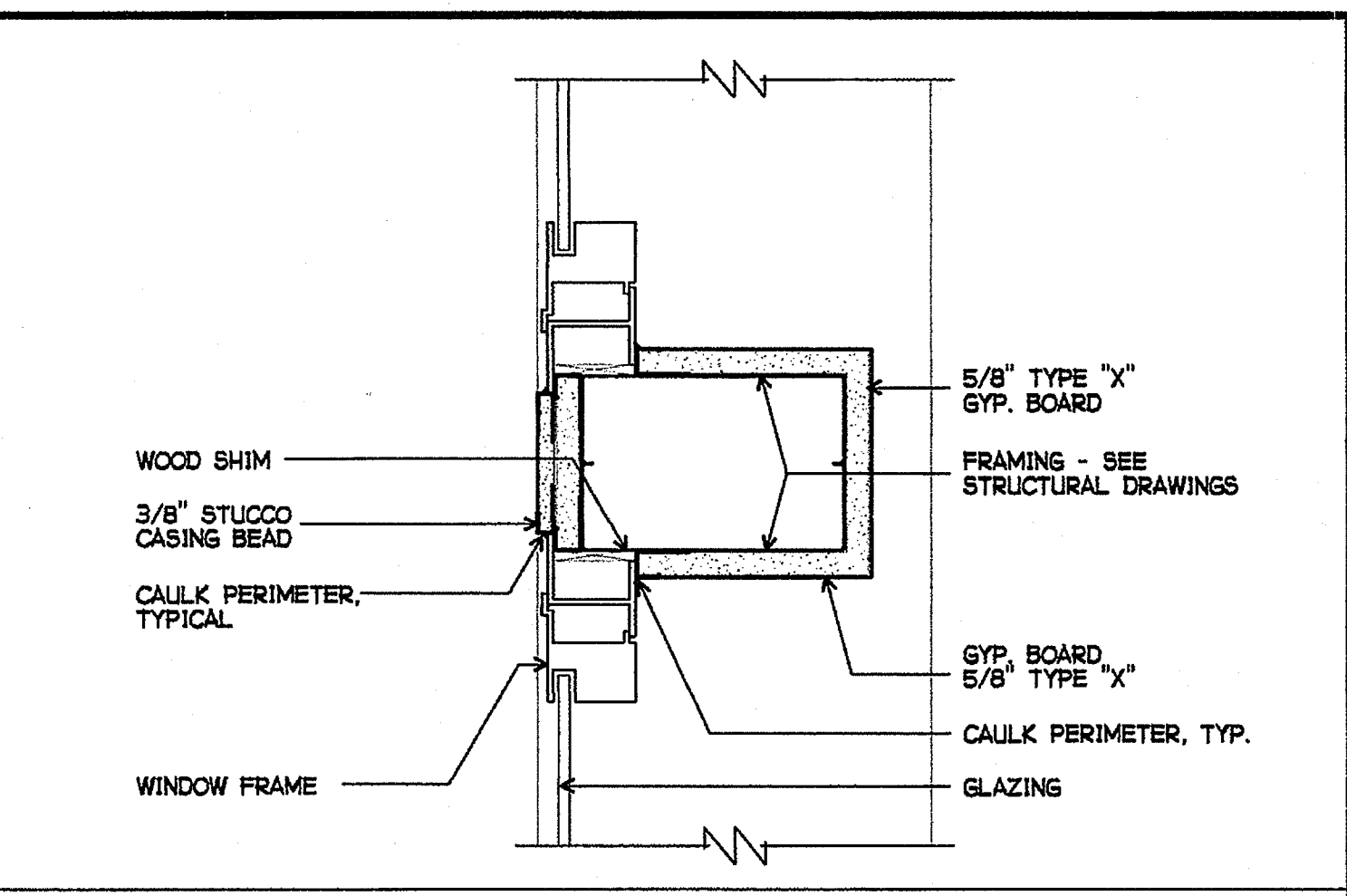
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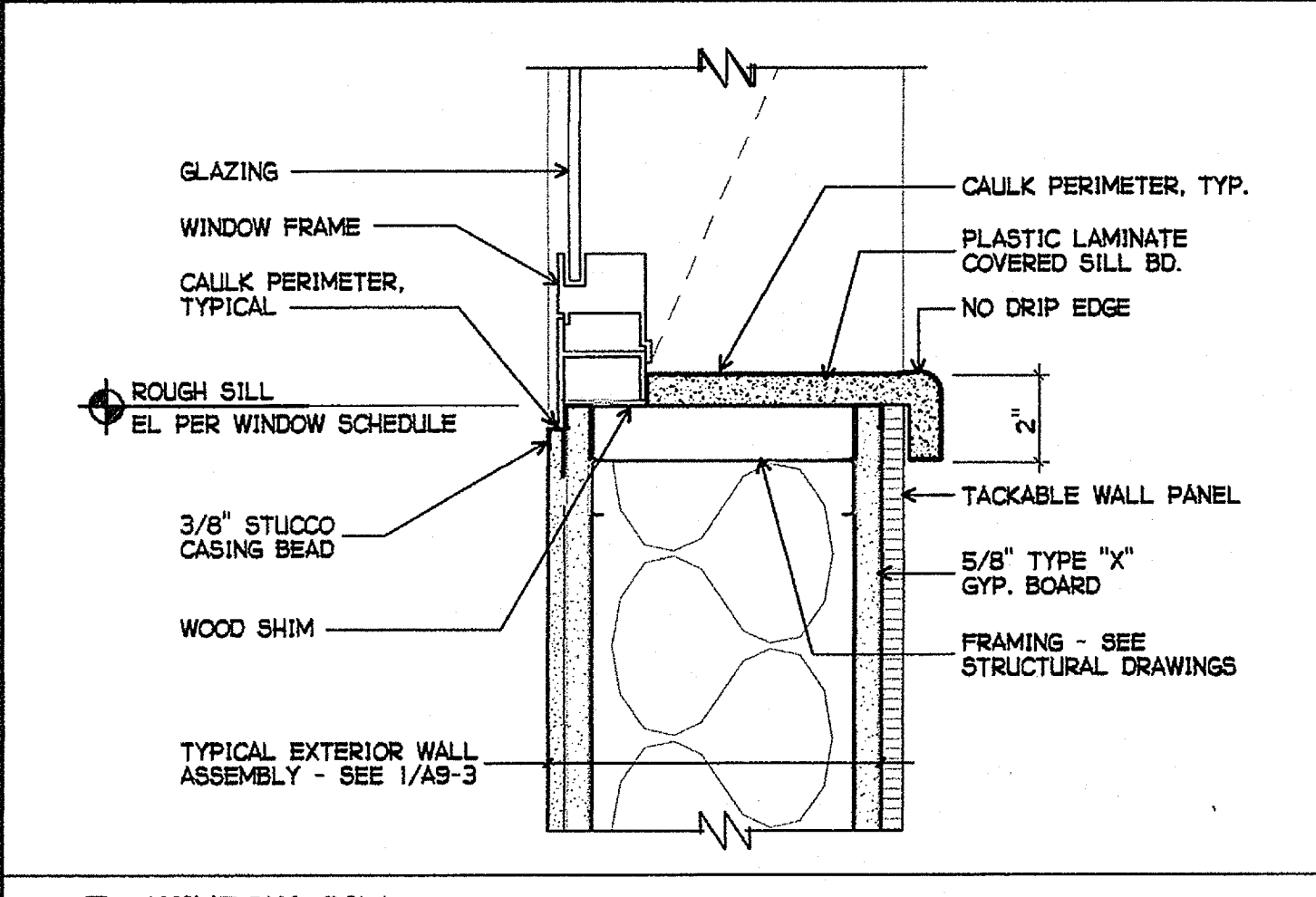
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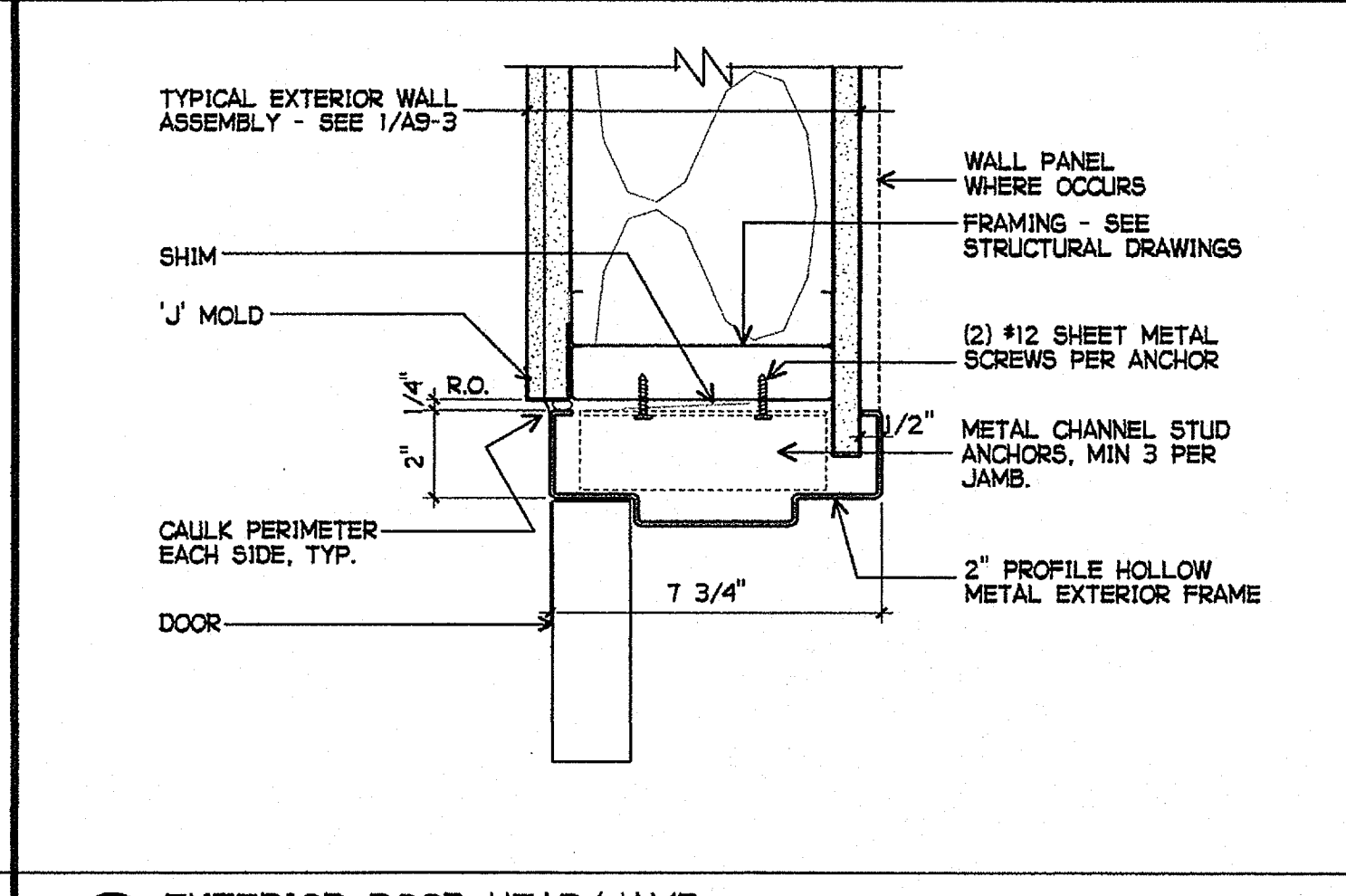
3 WINDOW JAMB
3" = 1'-0"



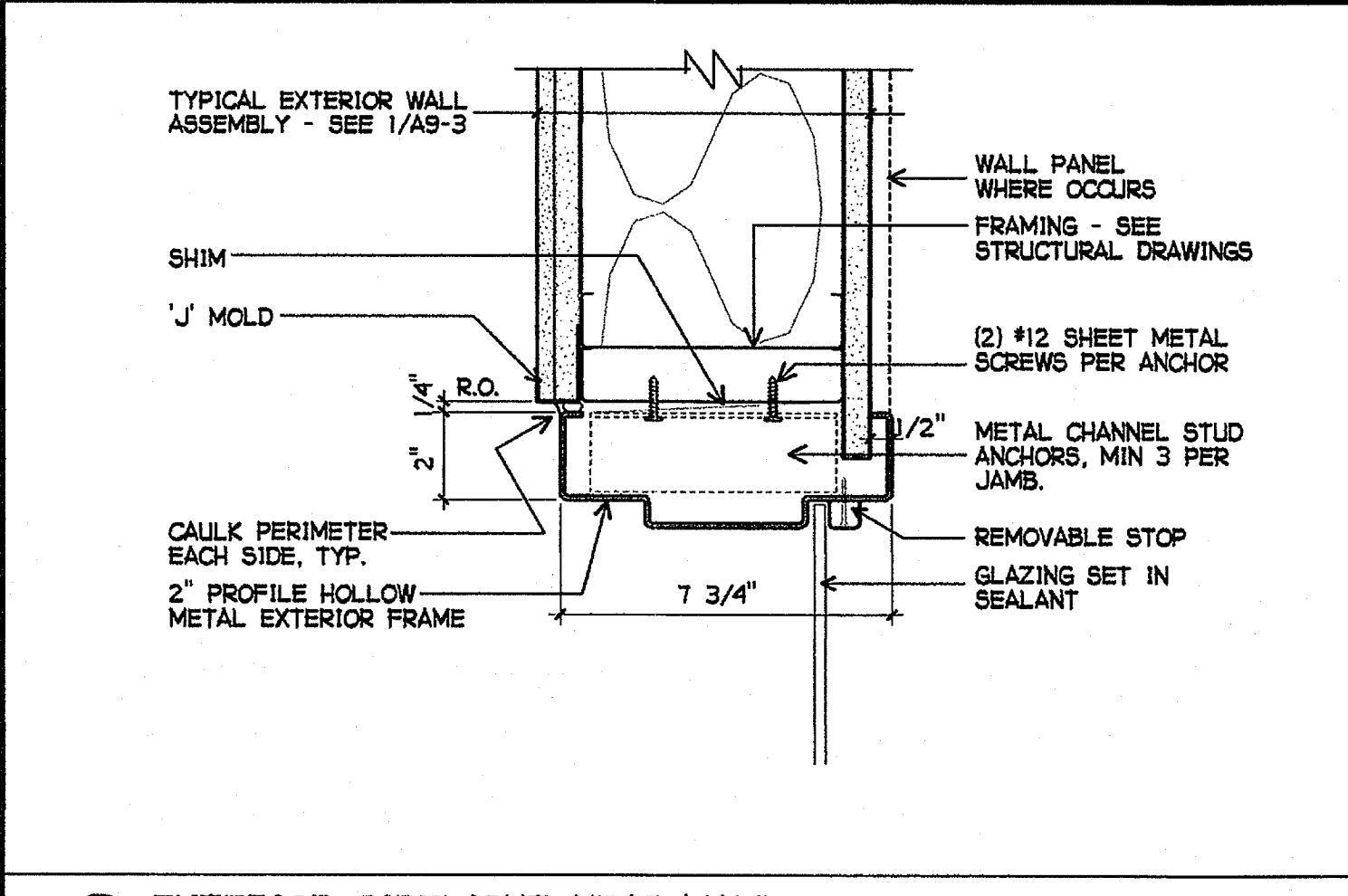
4 WINDOW JAMB - INTERMEDIATE
3" = 1'-0"



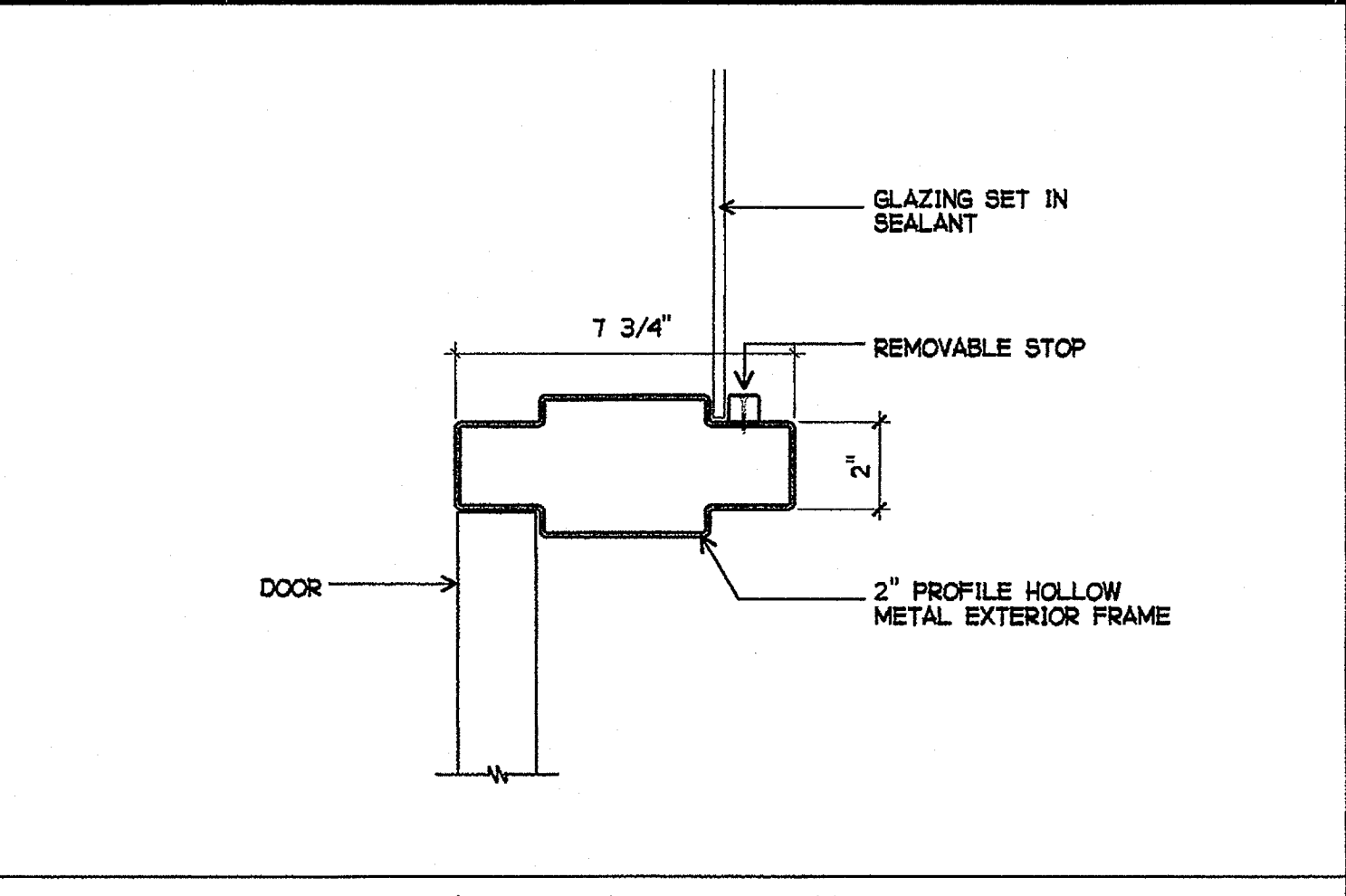
5 WINDOW SILL
3" = 1'-0"



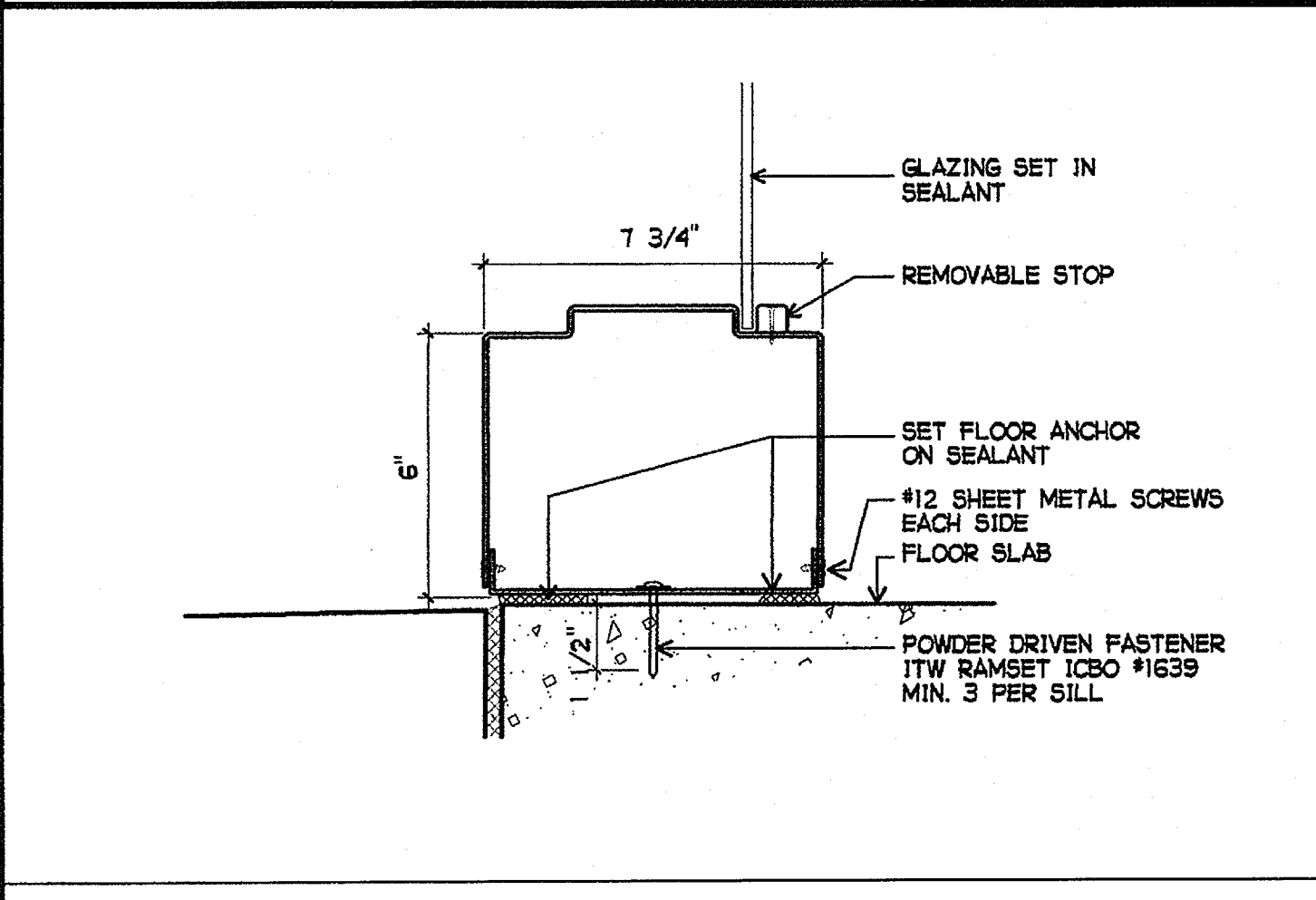
6 EXTERIOR DOOR HEAD/JAMB
3" = 1'-0"



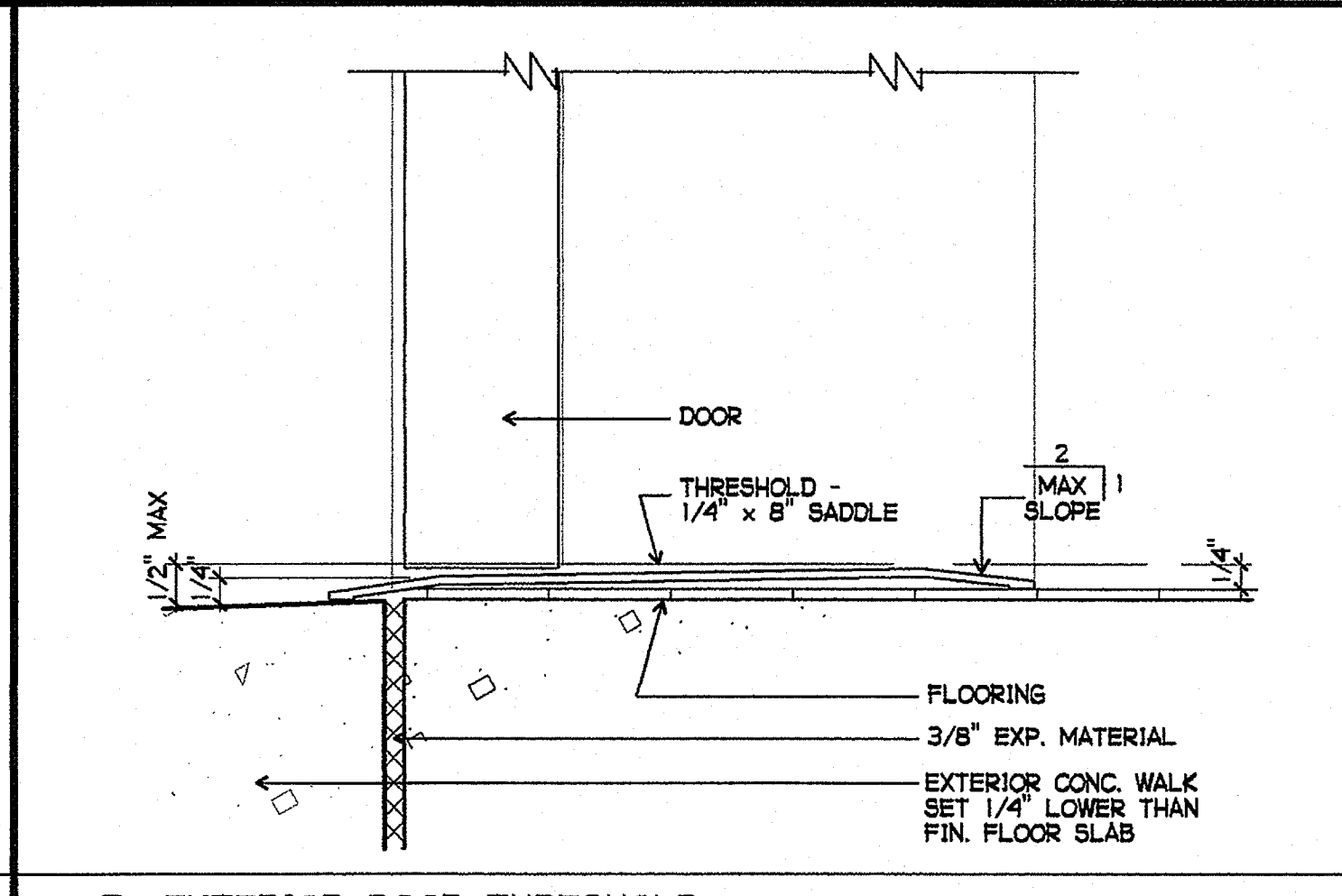
7 EXTERIOR SIDELIGHT HEAD/JAMB
3" = 1'-0"



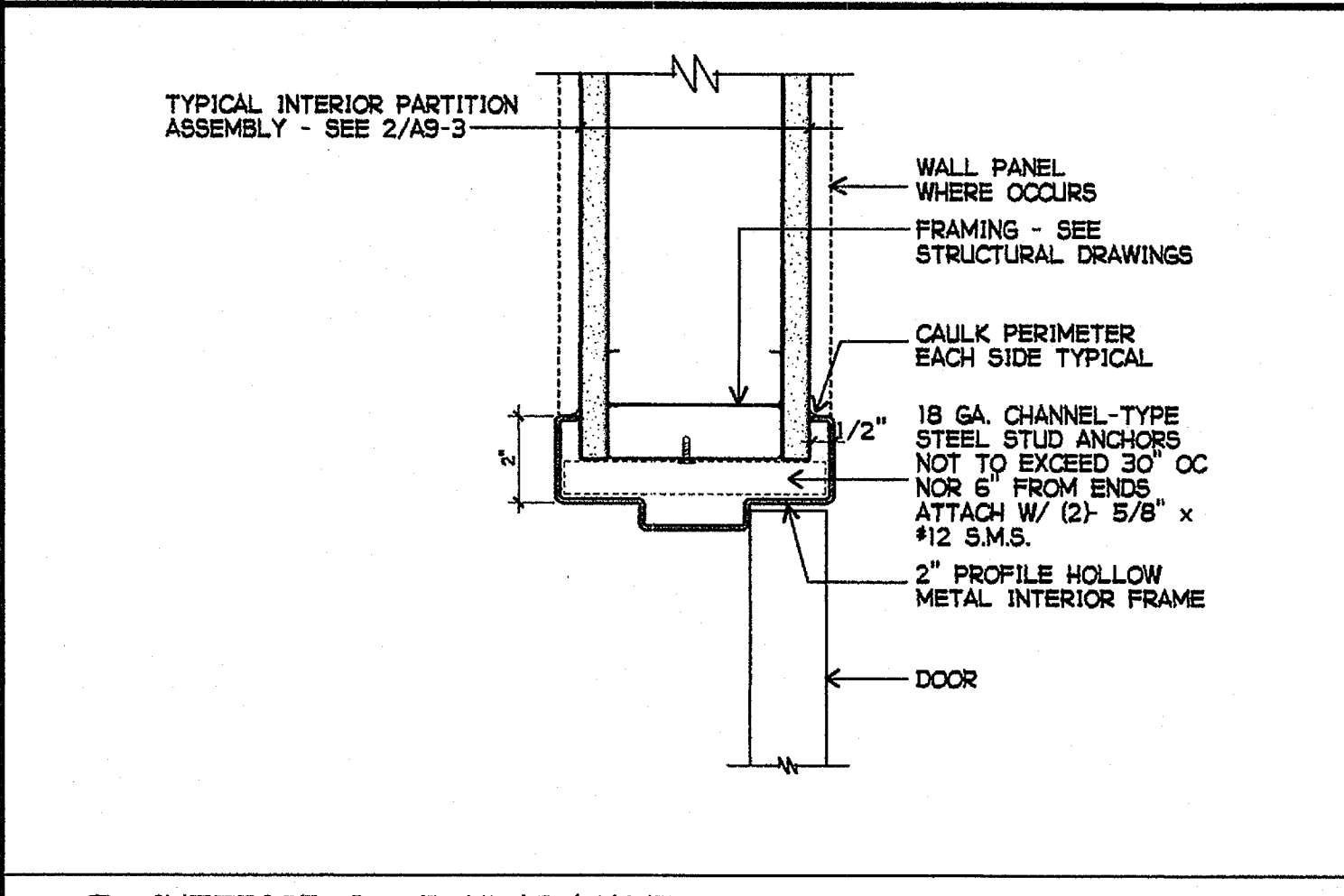
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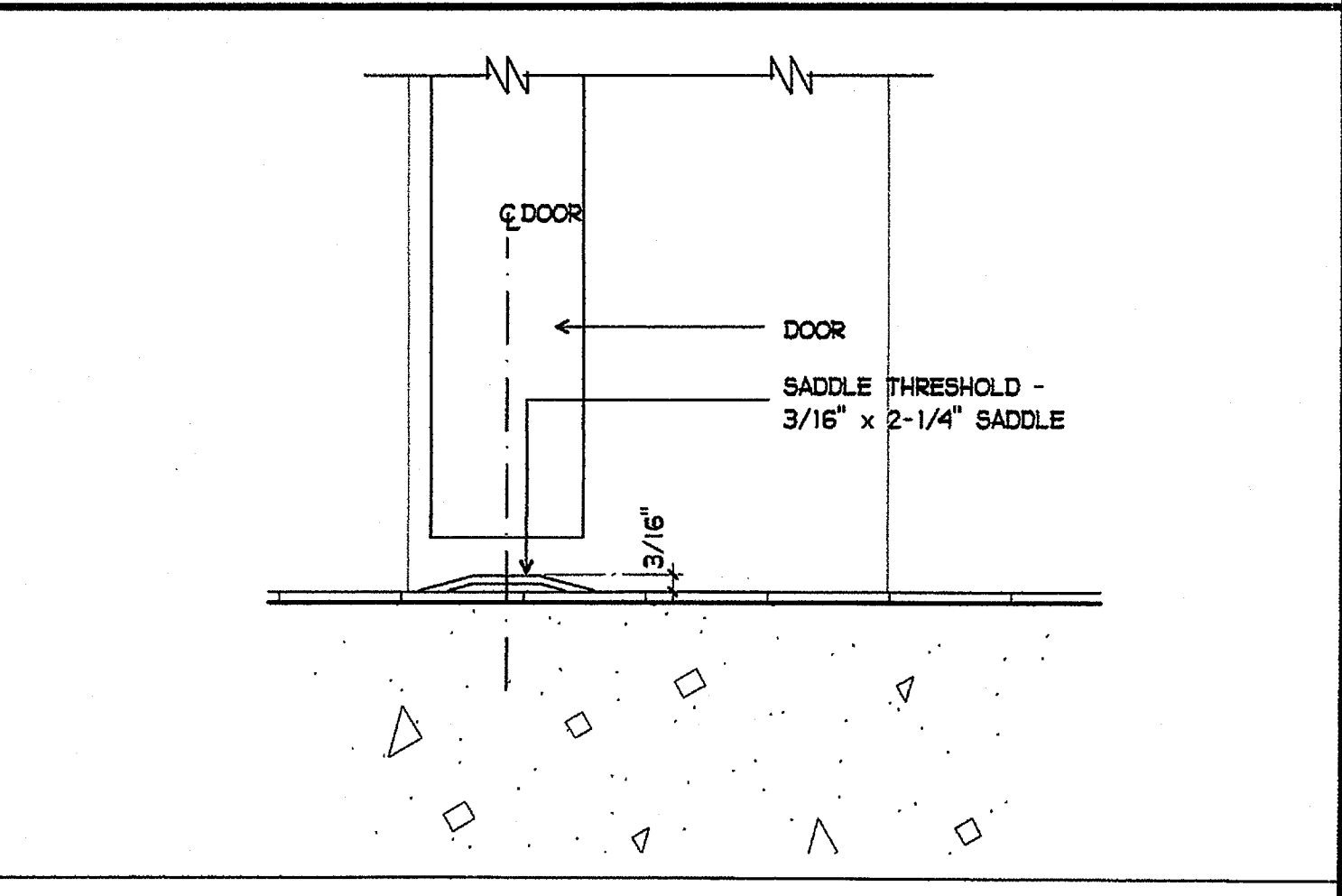
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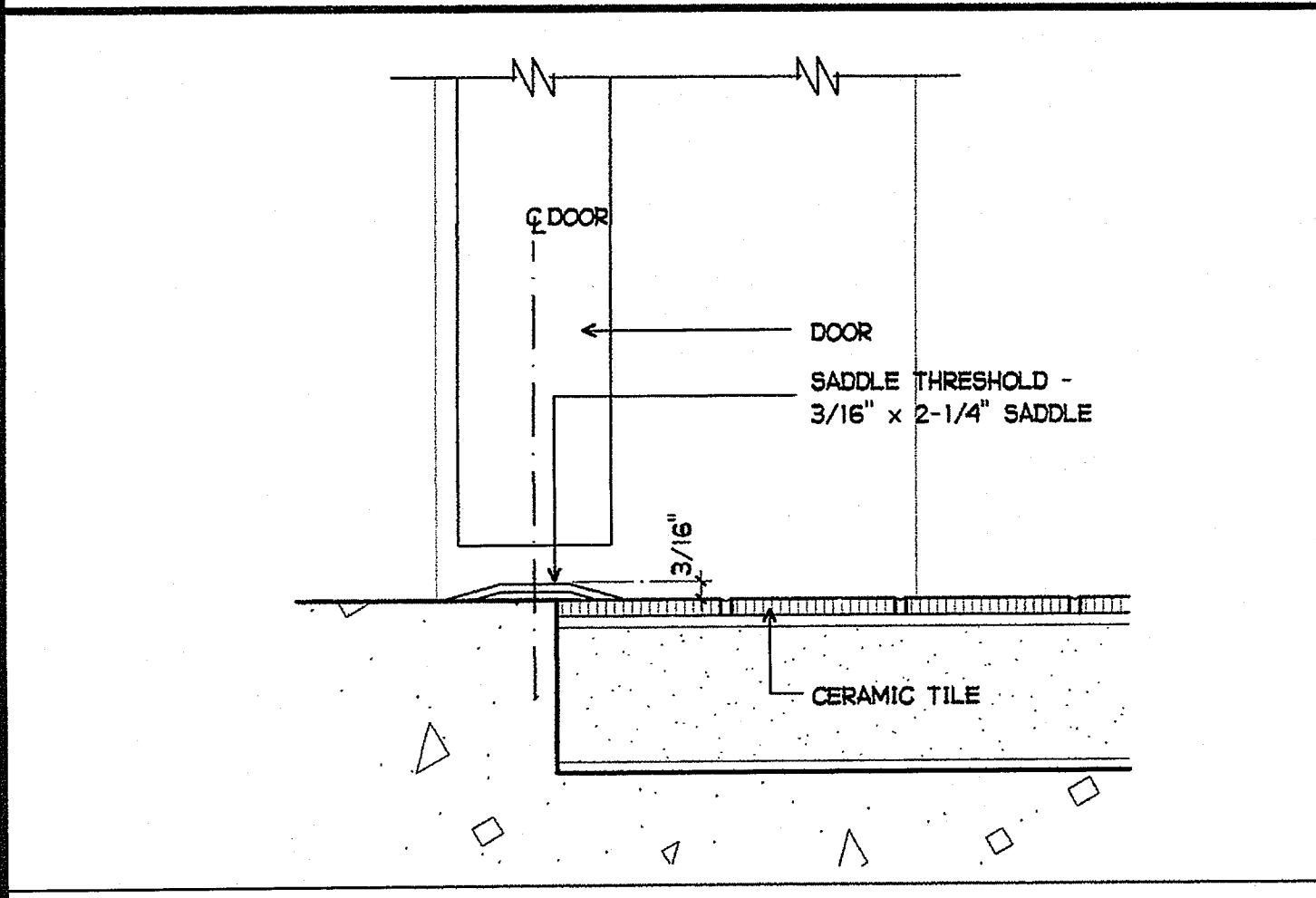
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6" = 1'-0"



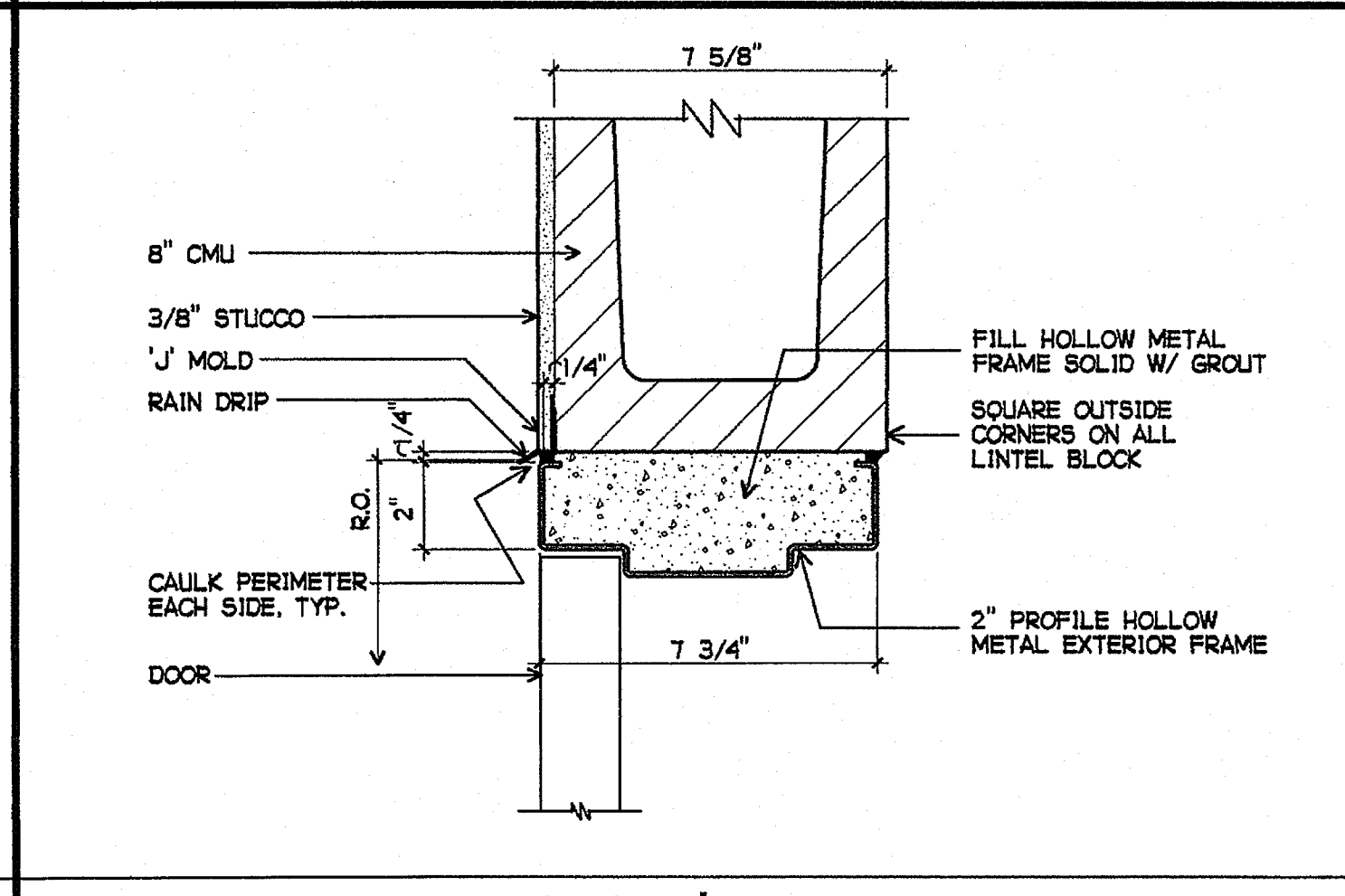
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3" = 1'-0"



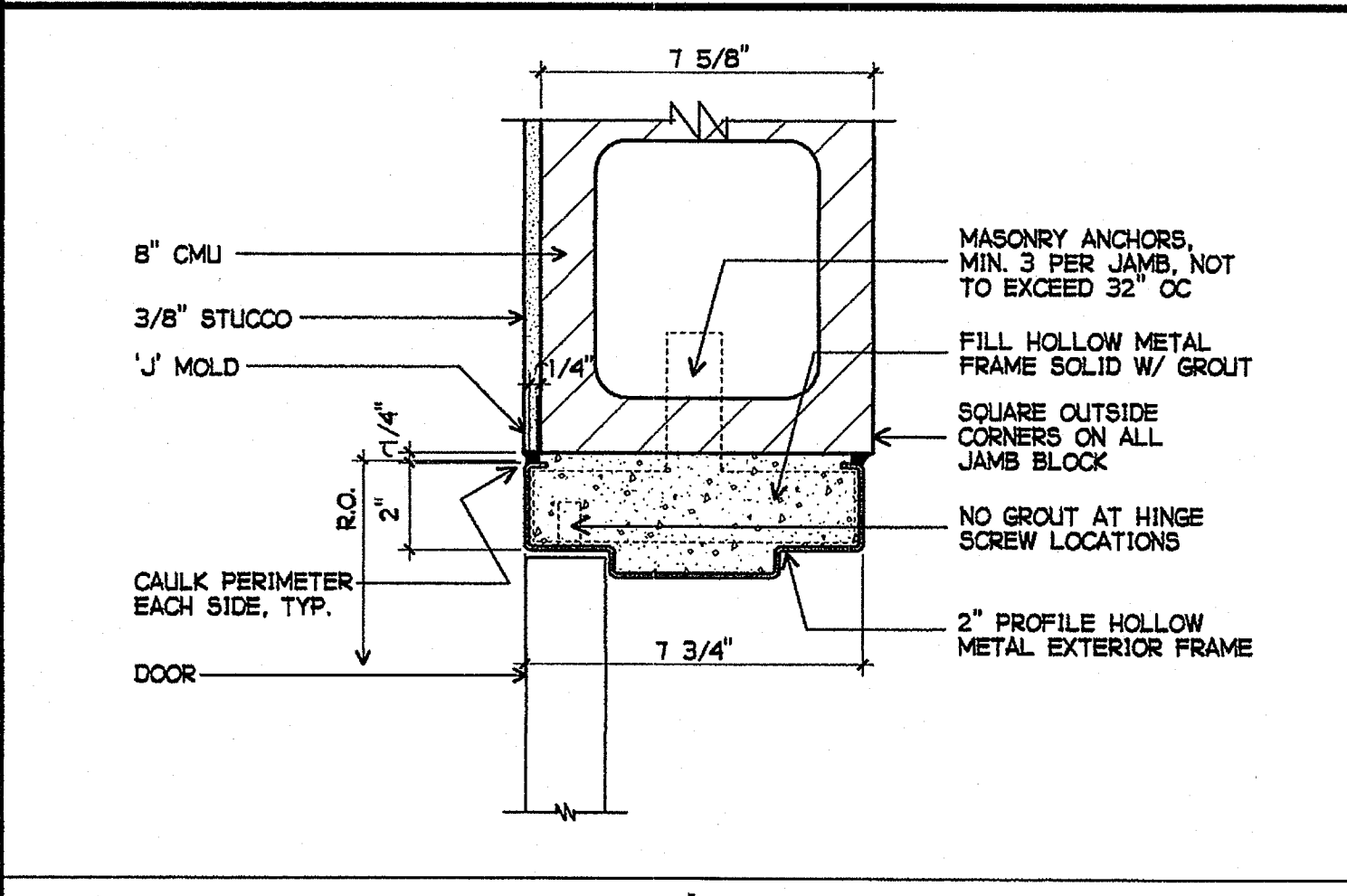
12 INTERIOR DOOR THRESHOLD AT FLOOR FINISH CHANGE
3" = 1'-0"



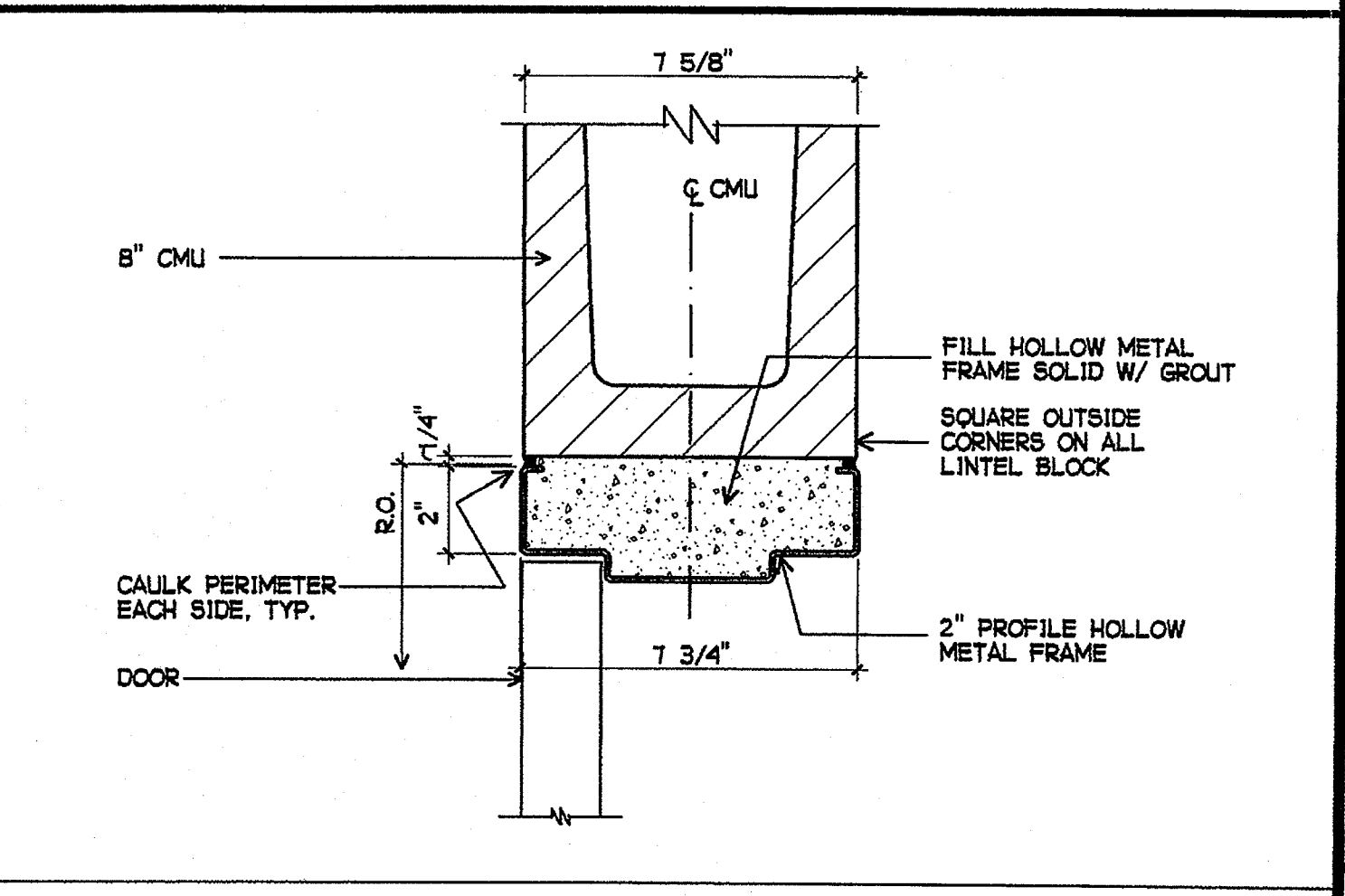
13 INTERIOR DOOR THRESHOLD AT CERAMIC TILE FLOOR
3" = 1'-0"



14 EXTERIOR DOOR HEAD AT 8\"/>



15 EXTERIOR DOOR JAMB AT 8\"/>



16 INTERIOR DOOR HEAD AT 8\"/>

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GROTH ARCHITECTS, INC.
823 ACACIA STREET
OCEANSIDE, CA 92054
OCEANSIDE UNIFIED S.D.

PHONE 760-754-8191
FAX 760-754-8291

CUSD NO. 758-000
PROJECT NOS. 025
P. T. N. 73569-9
DATE

REVISIONS

JEFFERSON MS NEW CONSTRUCTION
823 ACACIA STREET
OCEANSIDE, CA 92054
OCEANSIDE UNIFIED S.D.

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functional time

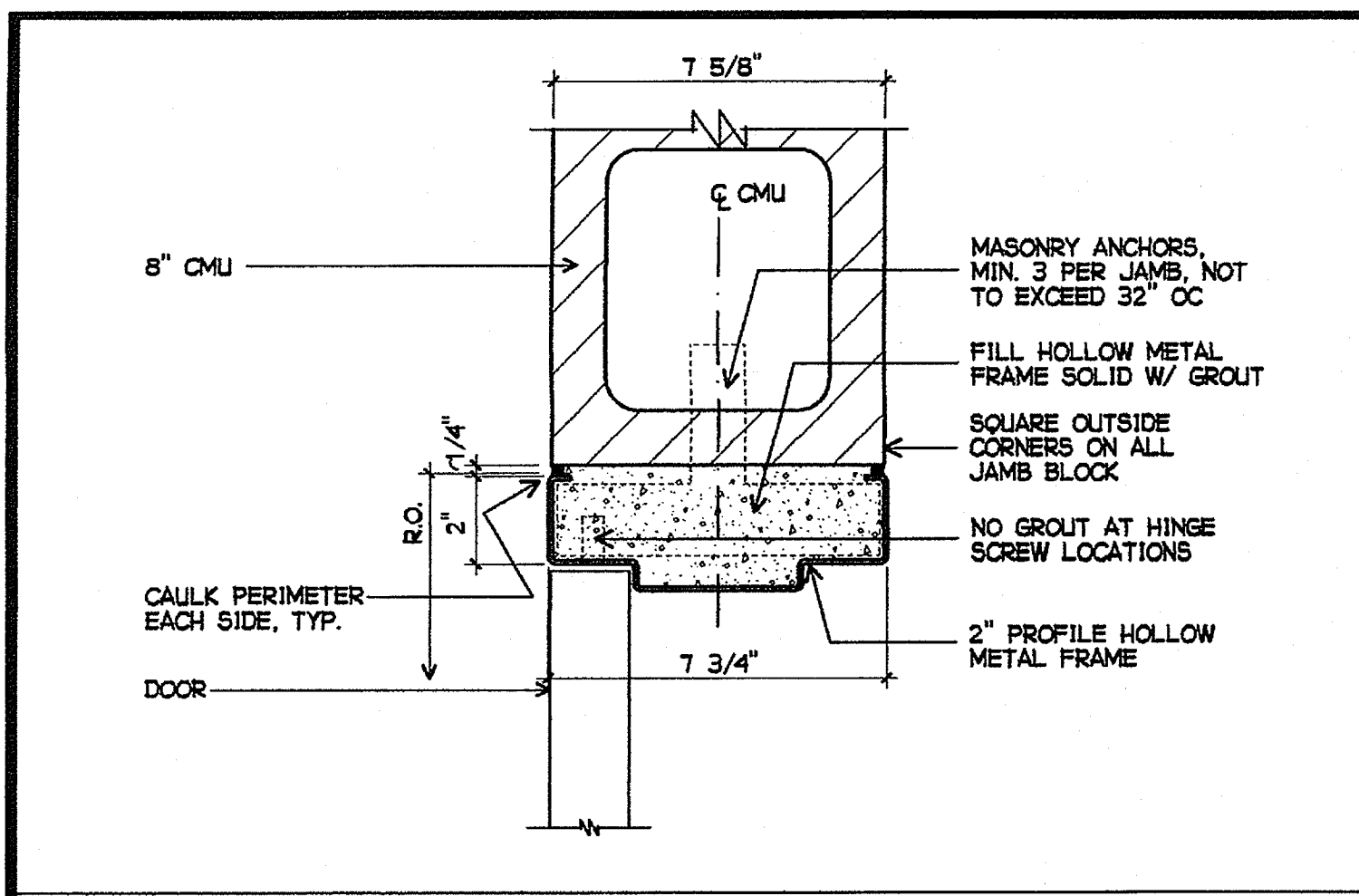
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DIV. OF THE STATE ARCHITECT
OFFICE OF REGULATION SERVICES
4-106494
AC 1/8 FLS 1/8 SS 1/8
DATE MAR 2 8 2005

LICENSURE ARCHITECT
JOHN SCOTT GROTH
C-26609
4/30/2007
RENEWAL

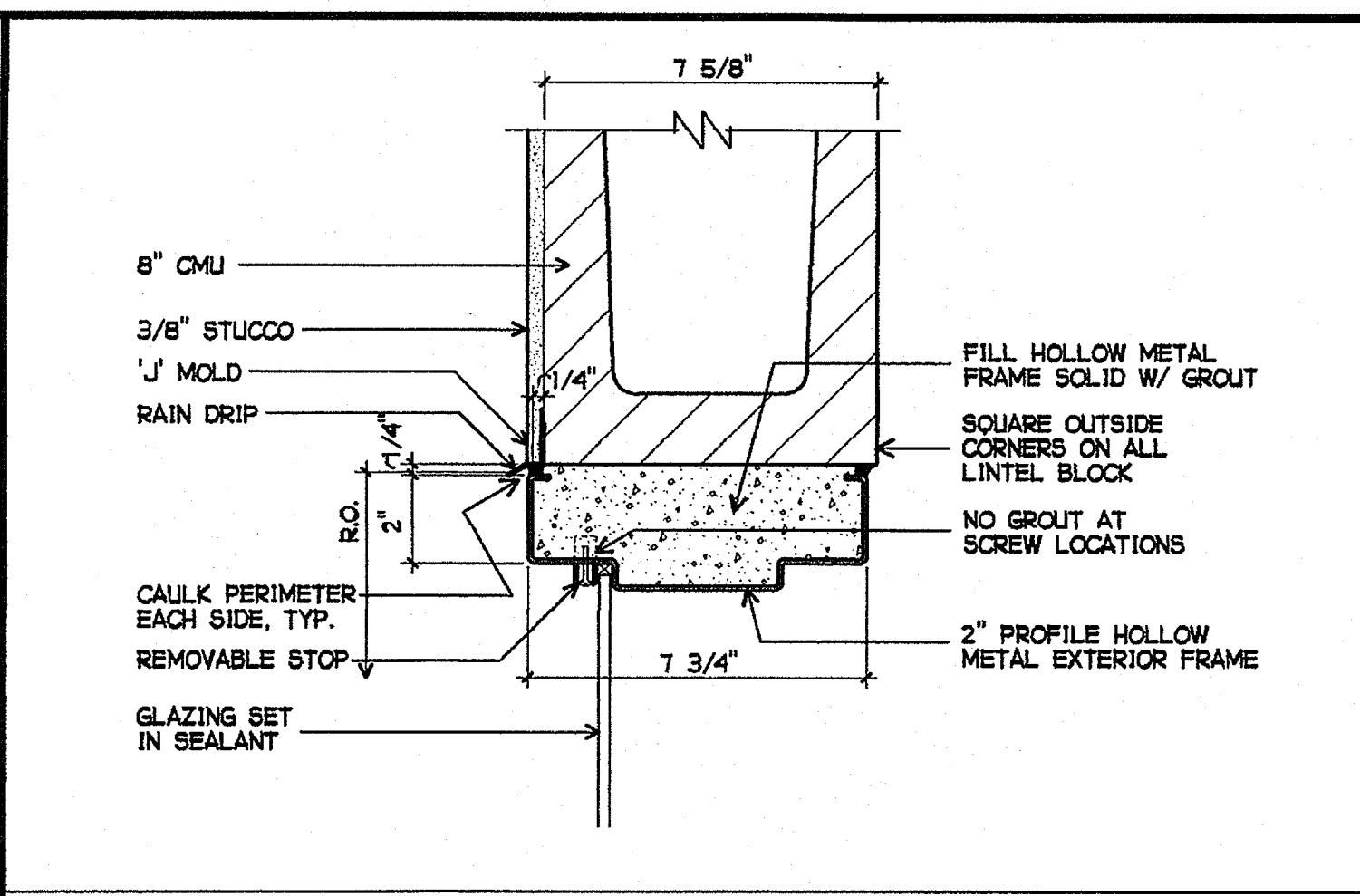
WINDOW DETAILS,
HOLLOW METAL
FRAME DETAILS

A9-1

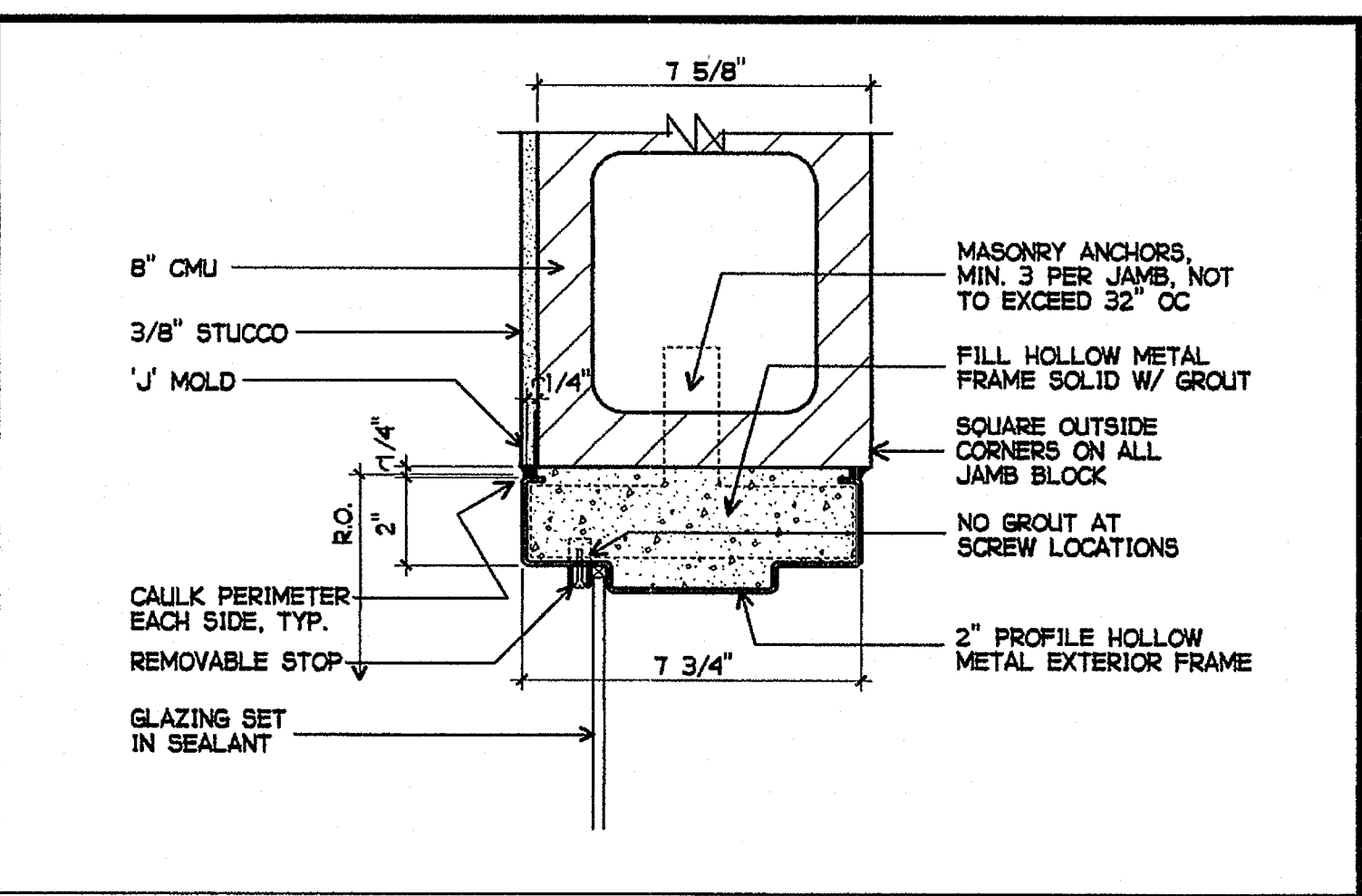
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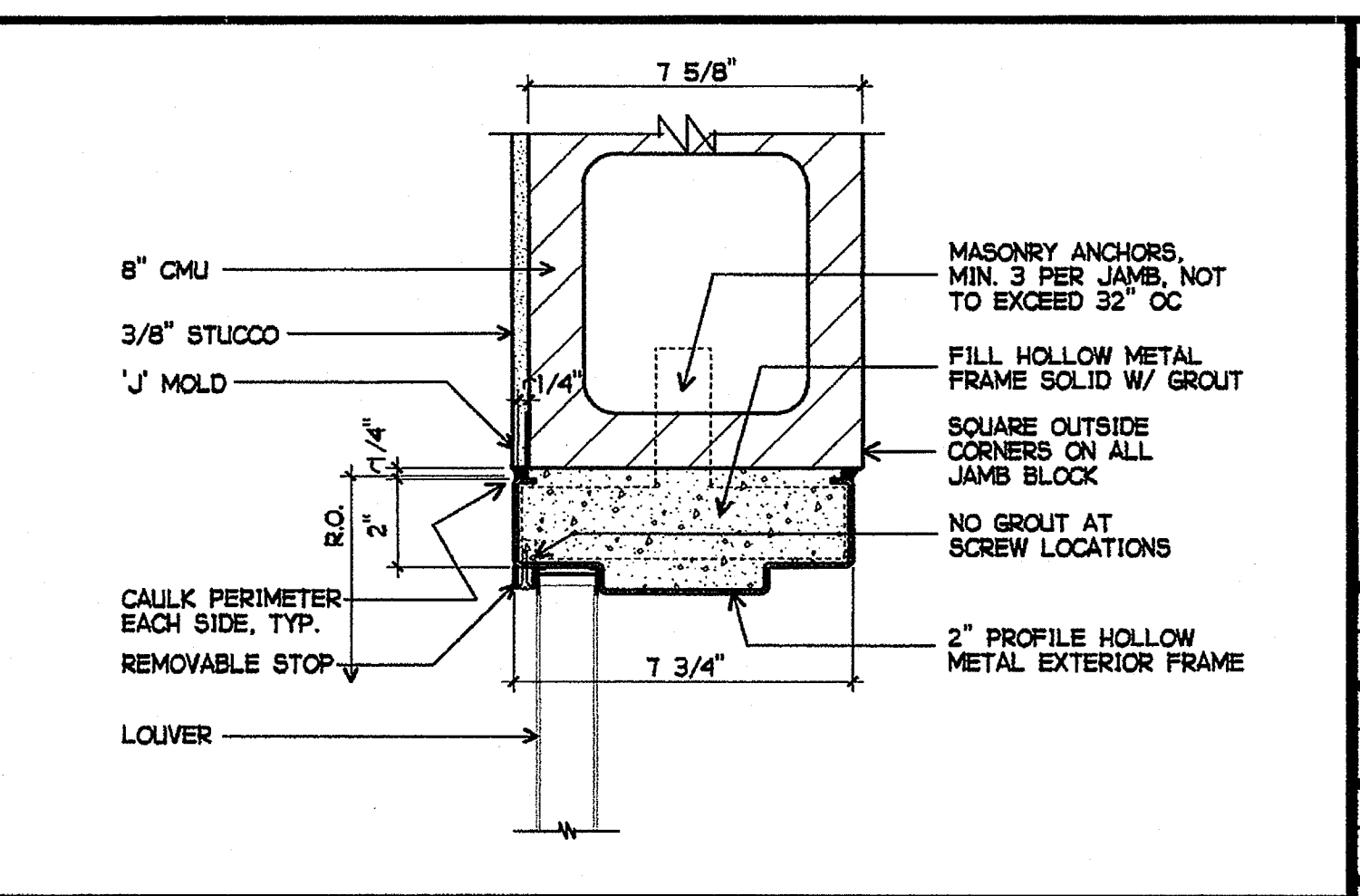
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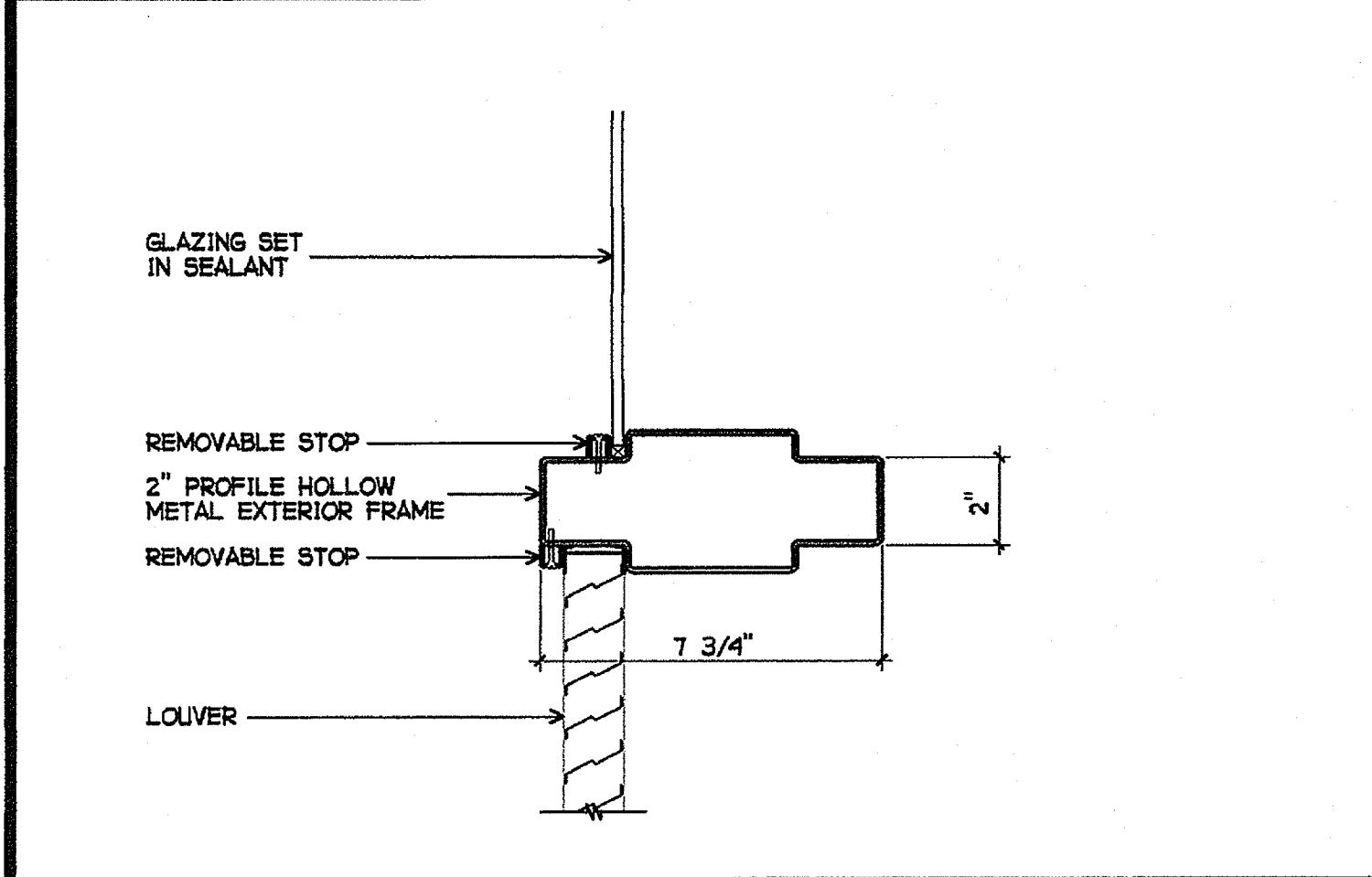
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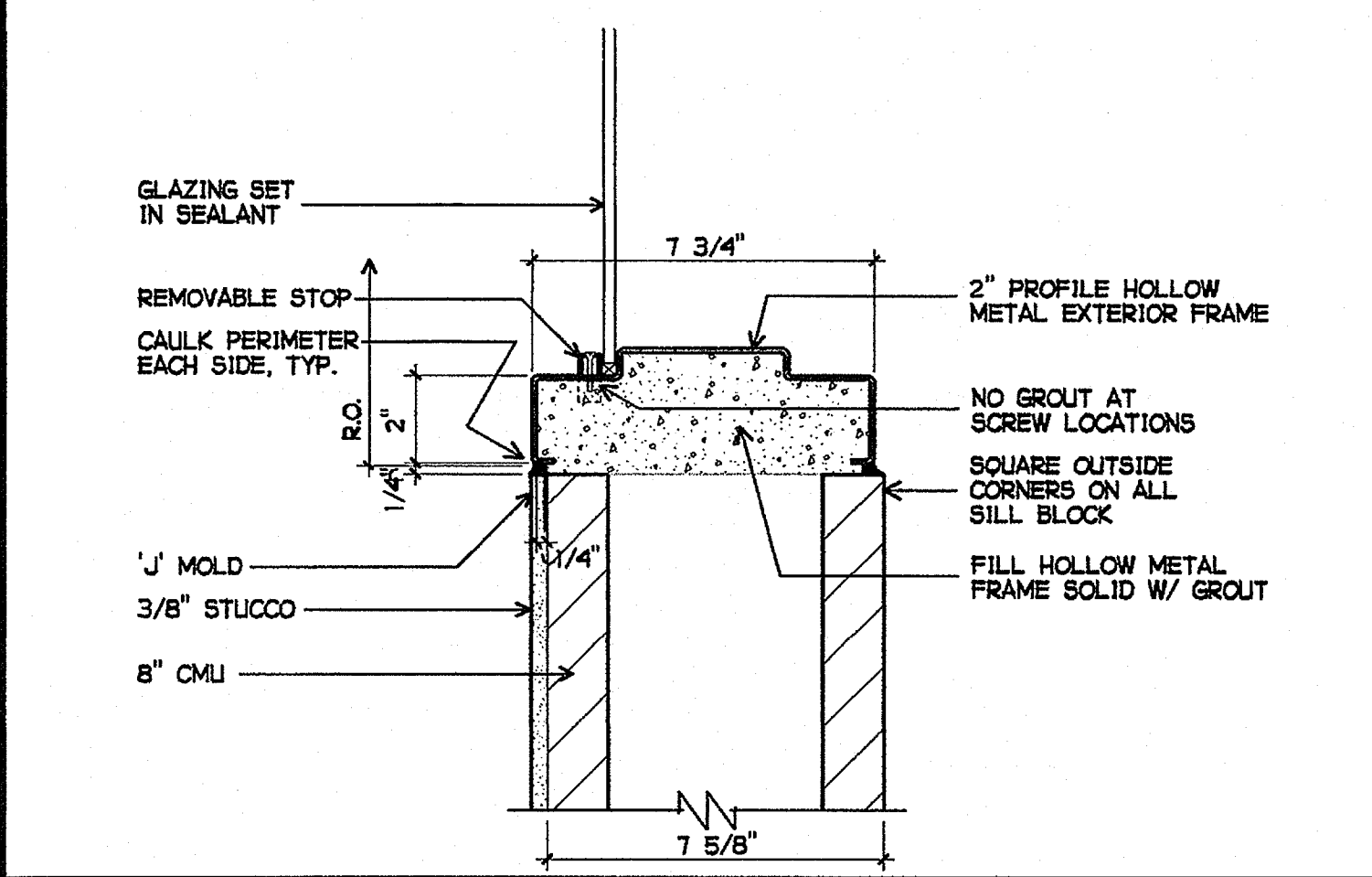
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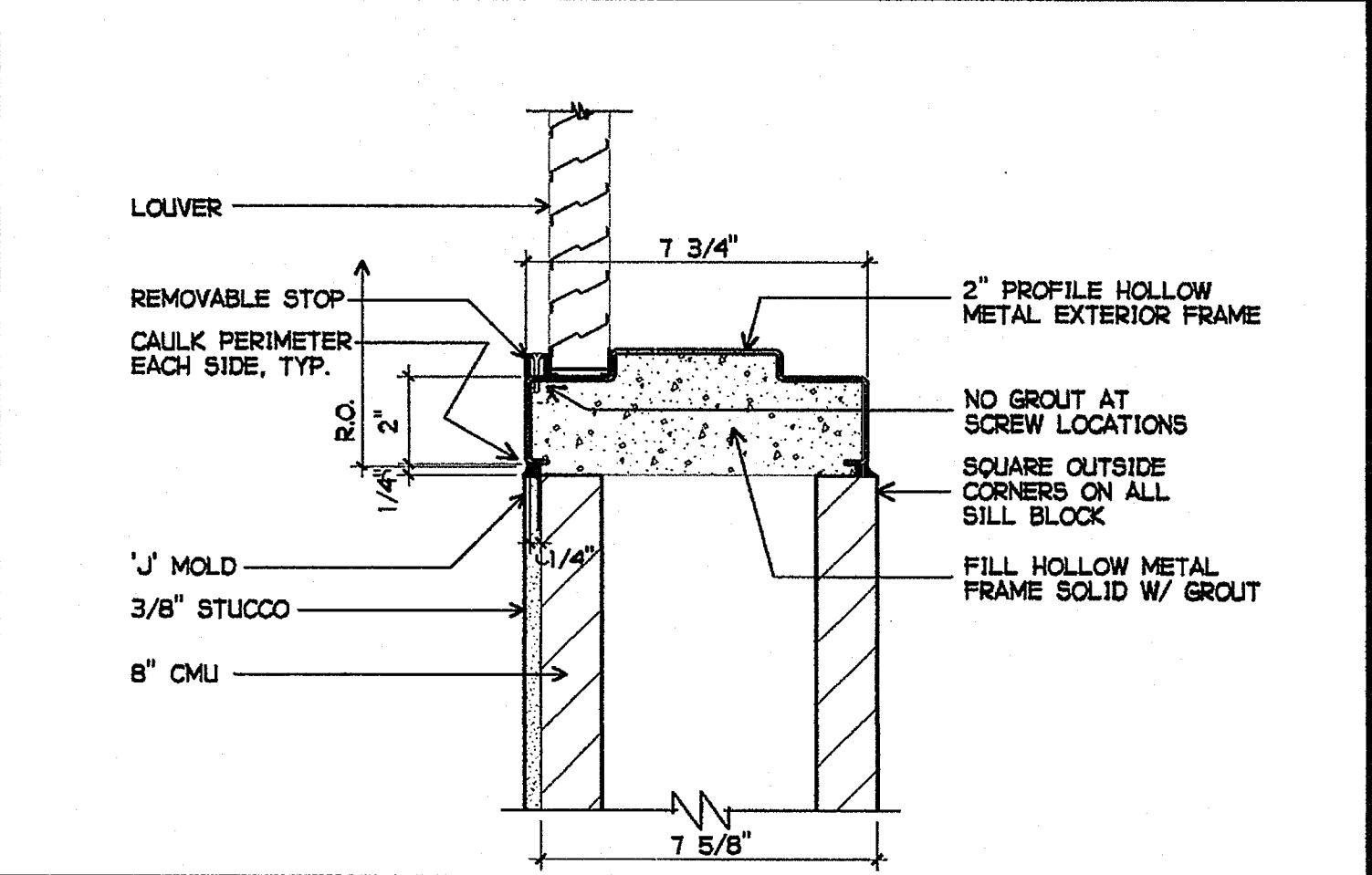
4 EXTERIOR LOUVER JAMB AT 8" CMU
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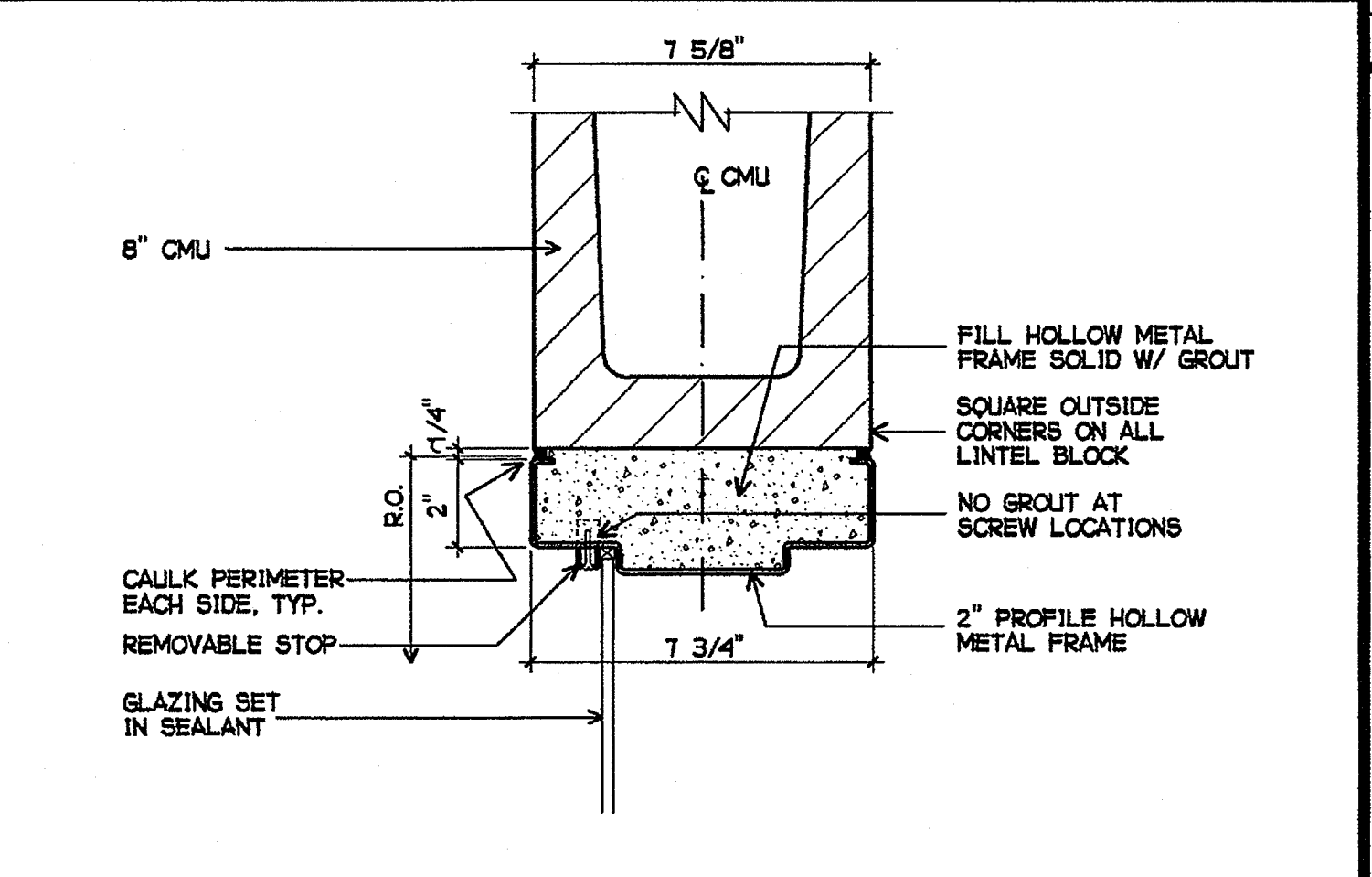
5 EXTERIOR GLAZING/LOUWER HORIZONTAL MULLION
3" = 1'-0"



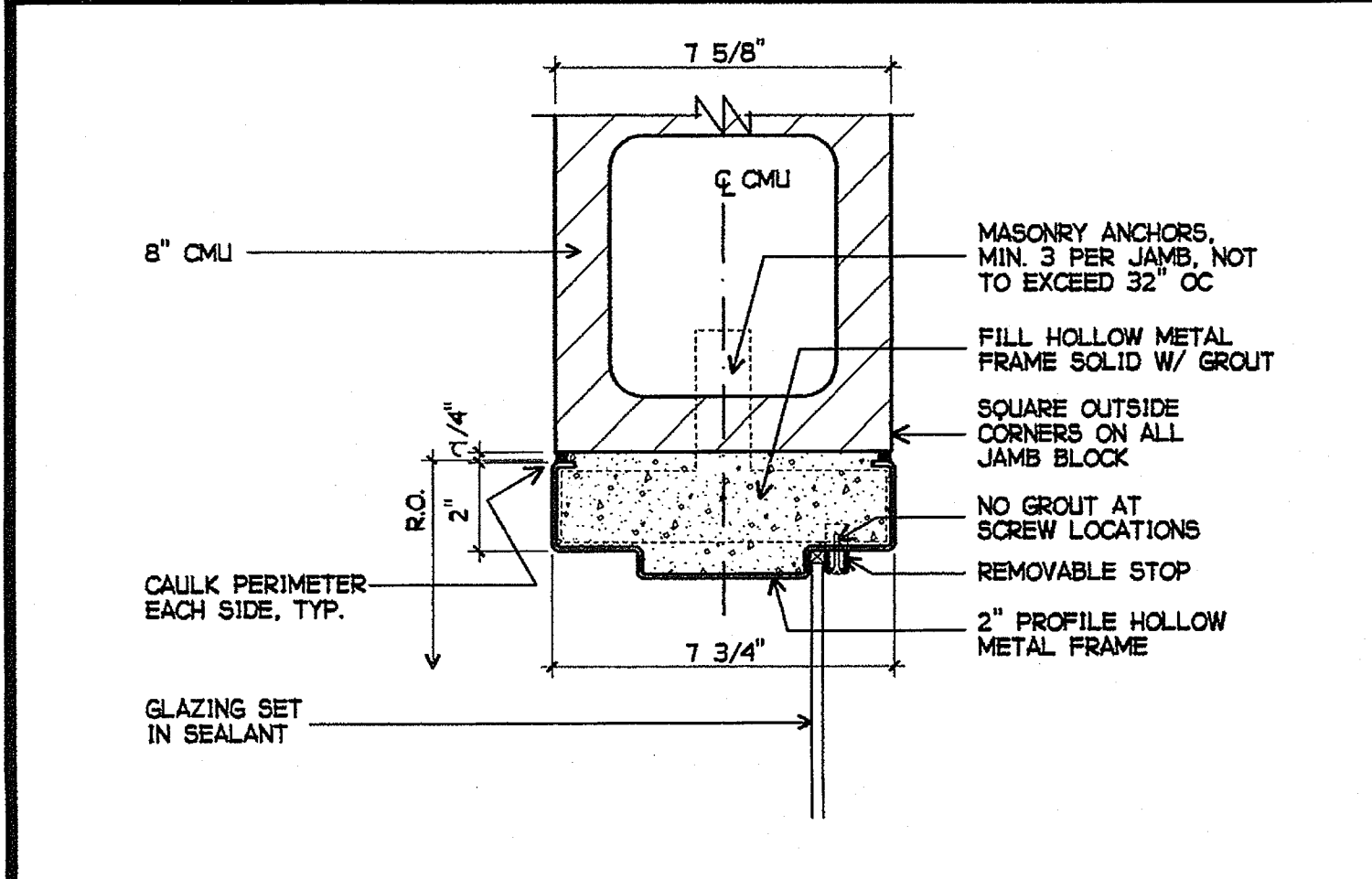
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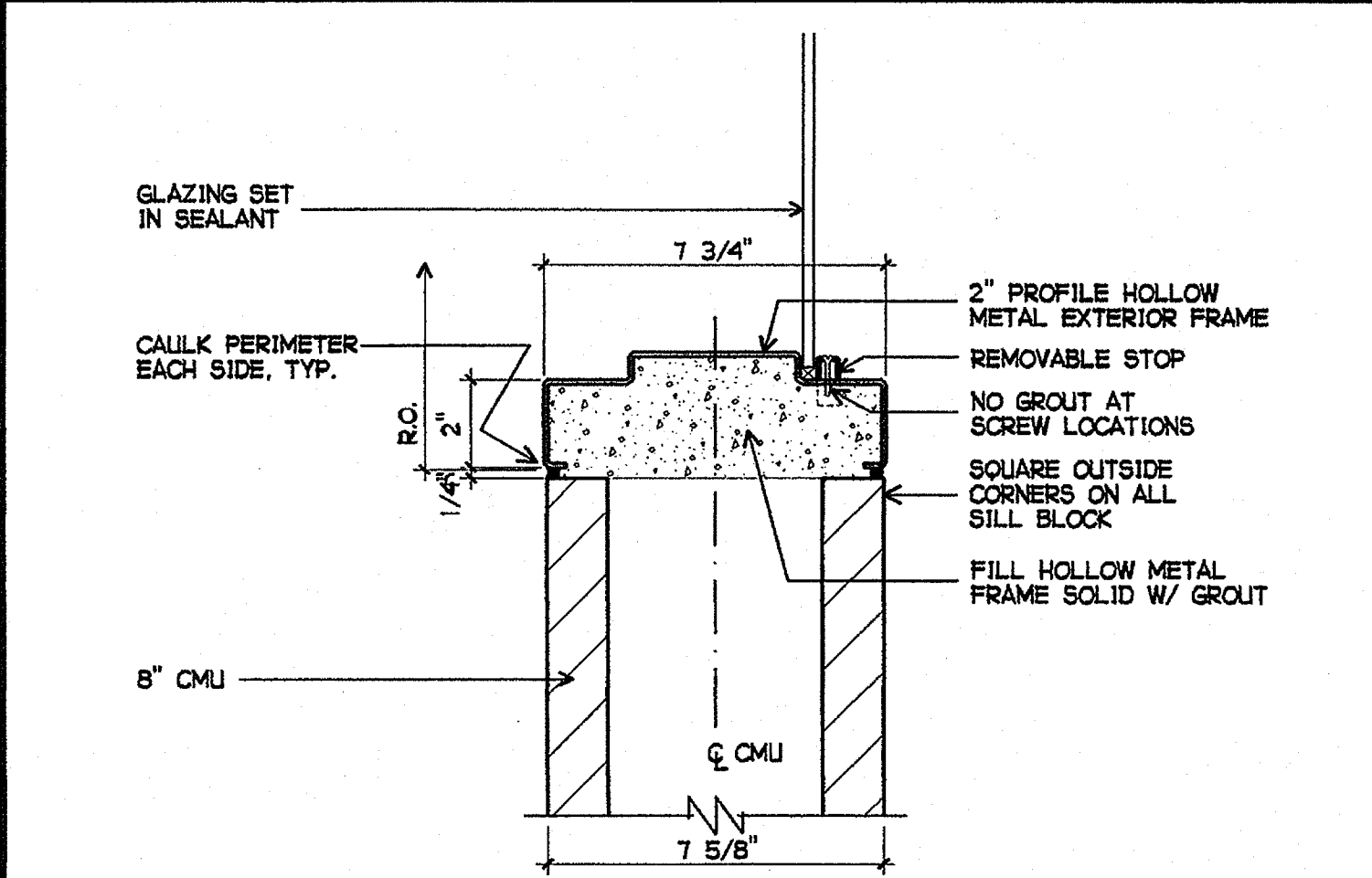
7 EXTERIOR LOUVER SILL AT 8" CMU
3" = 1'-0"



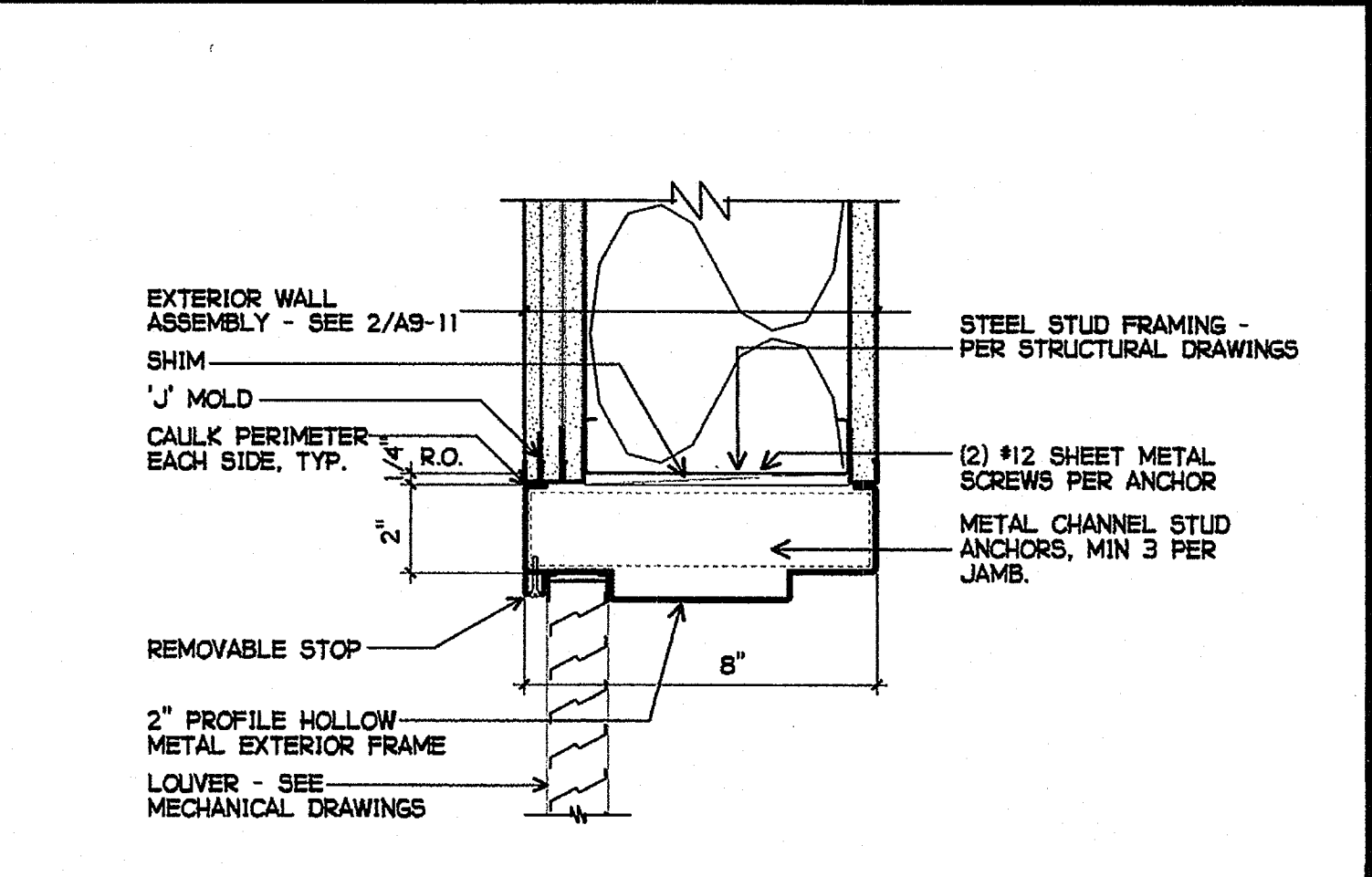
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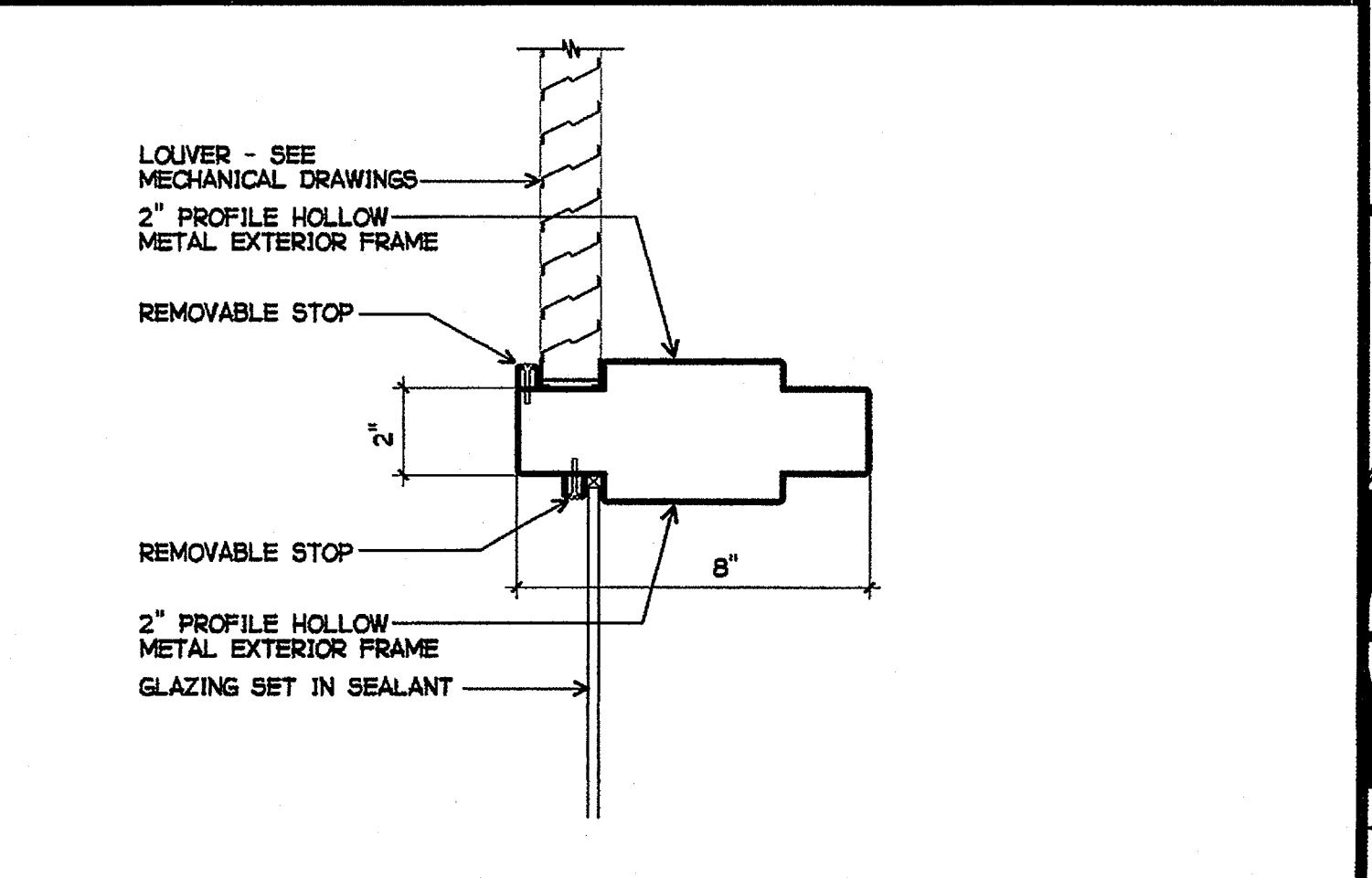
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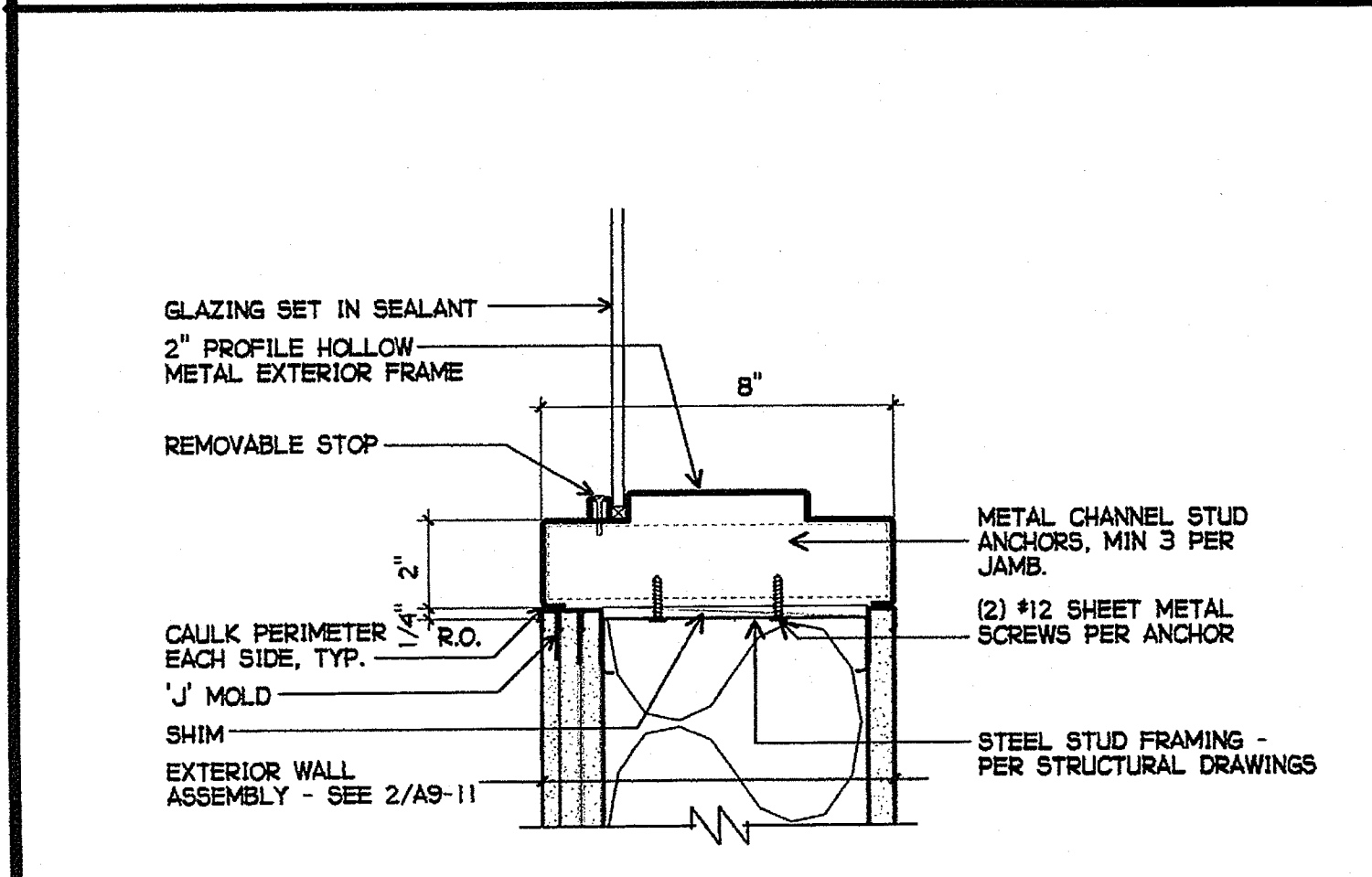
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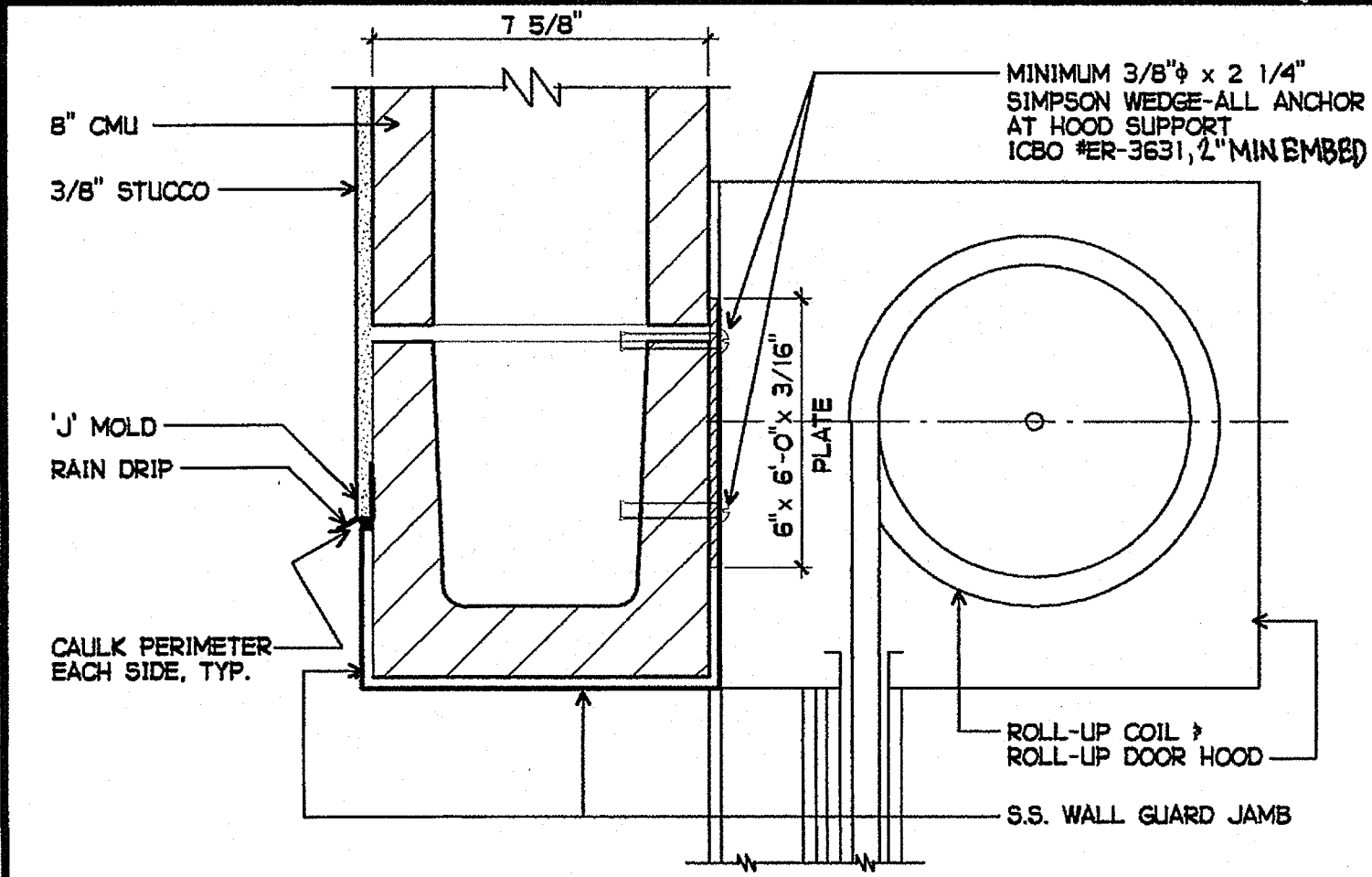
11 EXTERIOR LOUVER HEAD/JAMB AT CLERESTORY
3" = 1'-0"



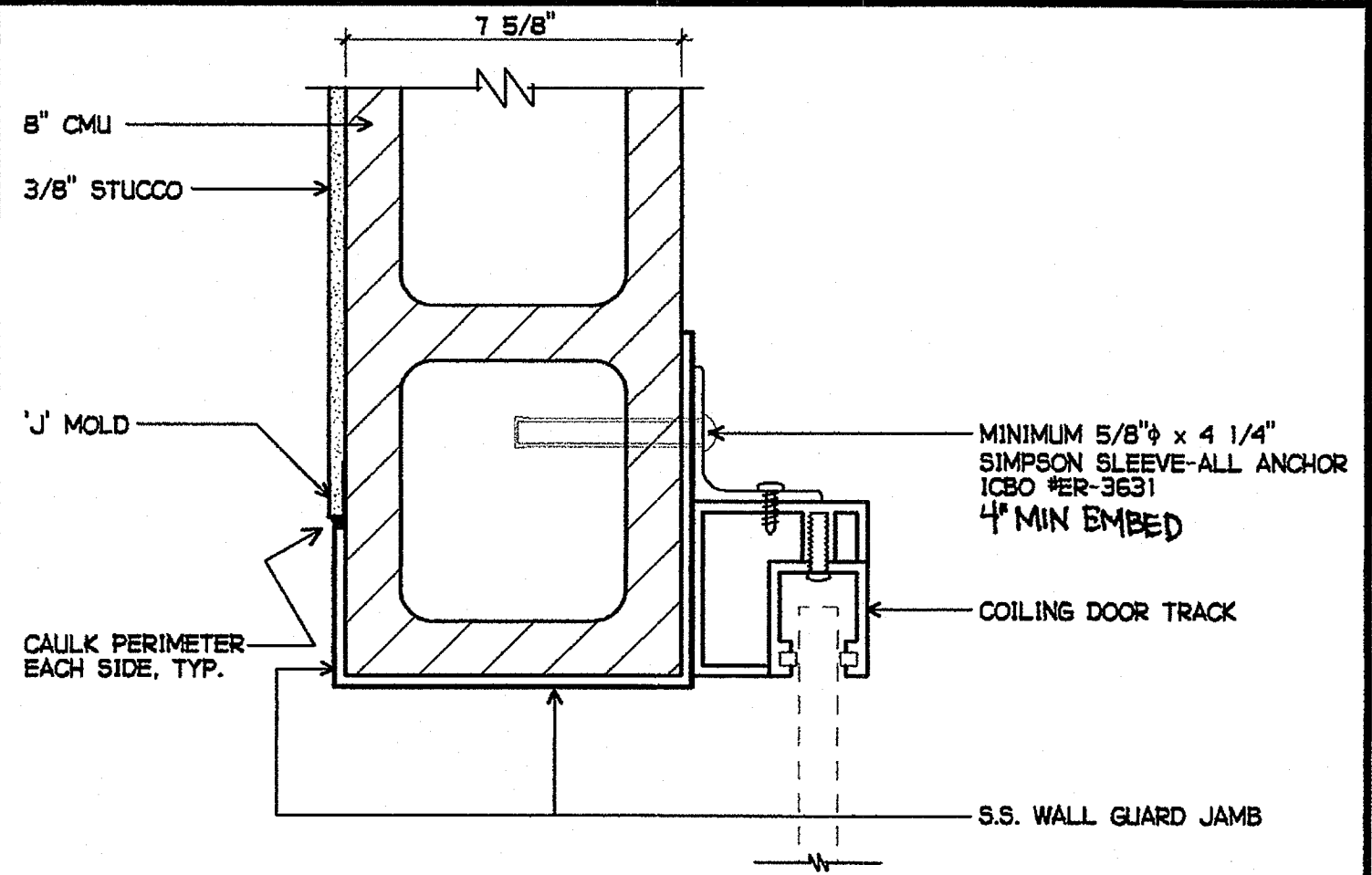
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3" = 1'-0"



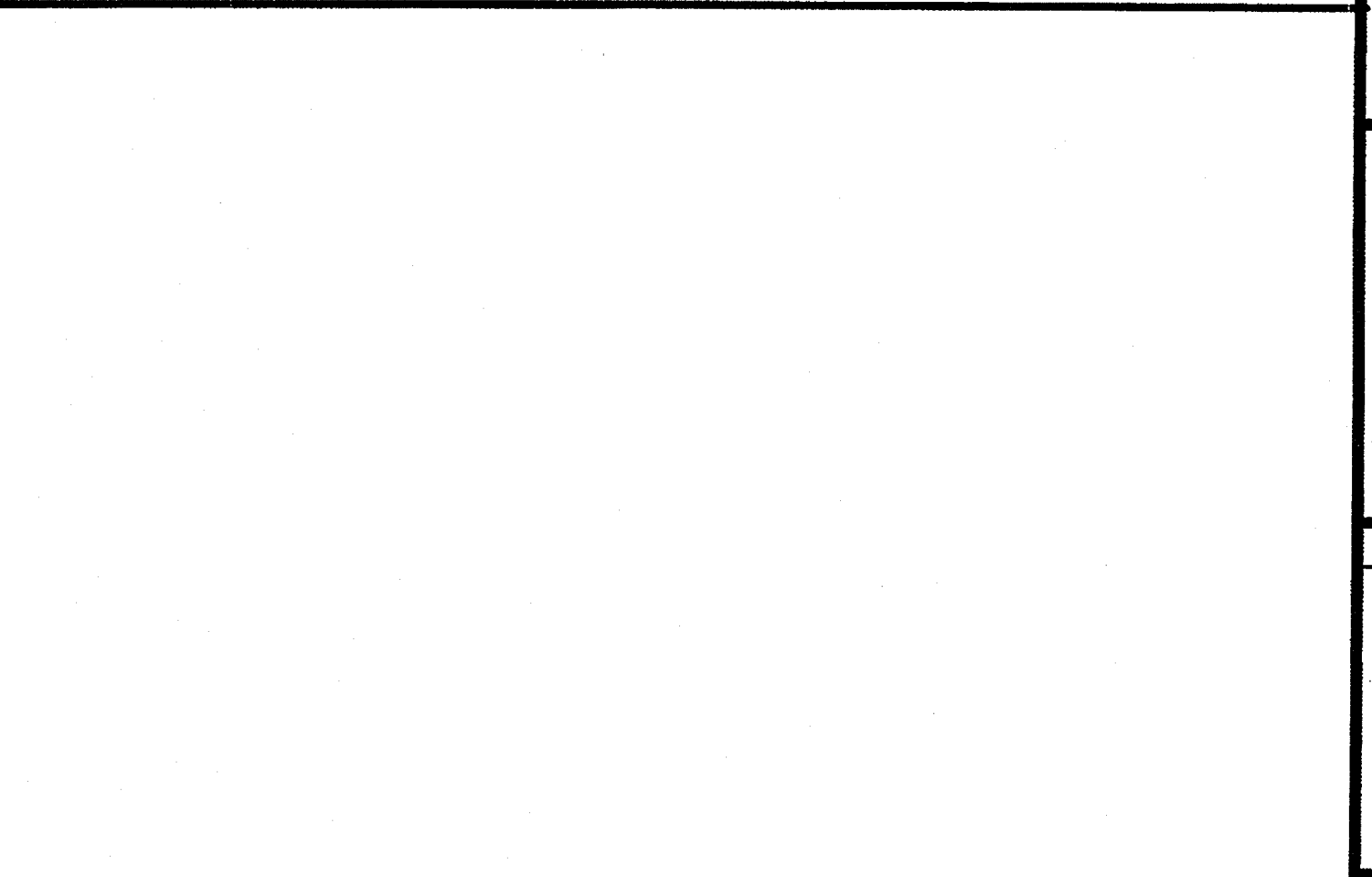
13 EXTERIOR GLAZING SILL/JAMB AT CLERESTORY
3" = 1'-0"



14 EXTERIOR COIL DOOR HEAD AT 8" CMU
3" = 1'-0"



15 EXTERIOR COIL DOOR JAMB AT 8" CMU
3" = 1'-0"



16 NOT USED

GROTH ARCHITECTS, INC.
 823 ACACIA STREET
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P. T. N.
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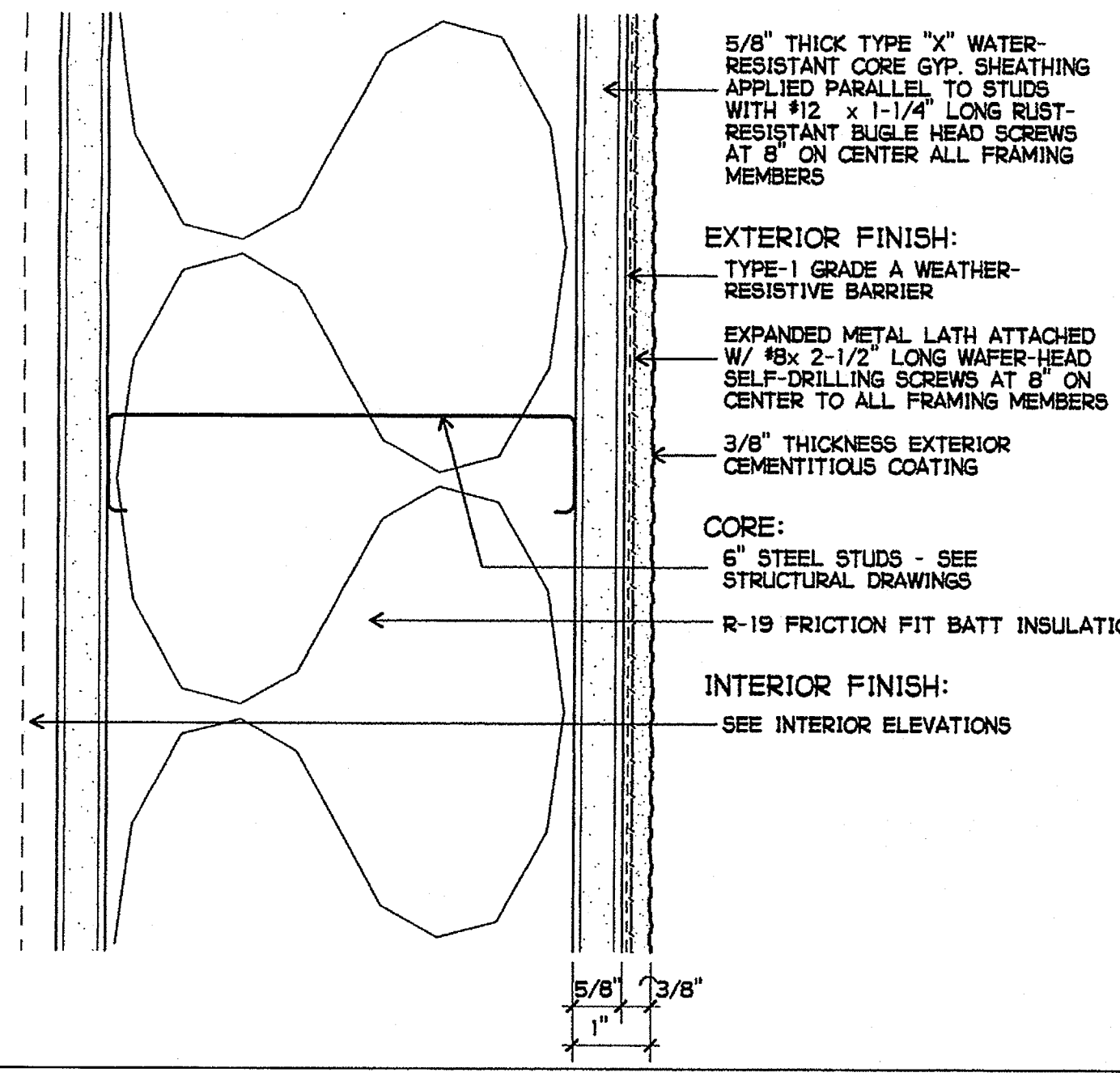
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 AC. VLS. FLS. SS.
 DATE: MAR 28 2005

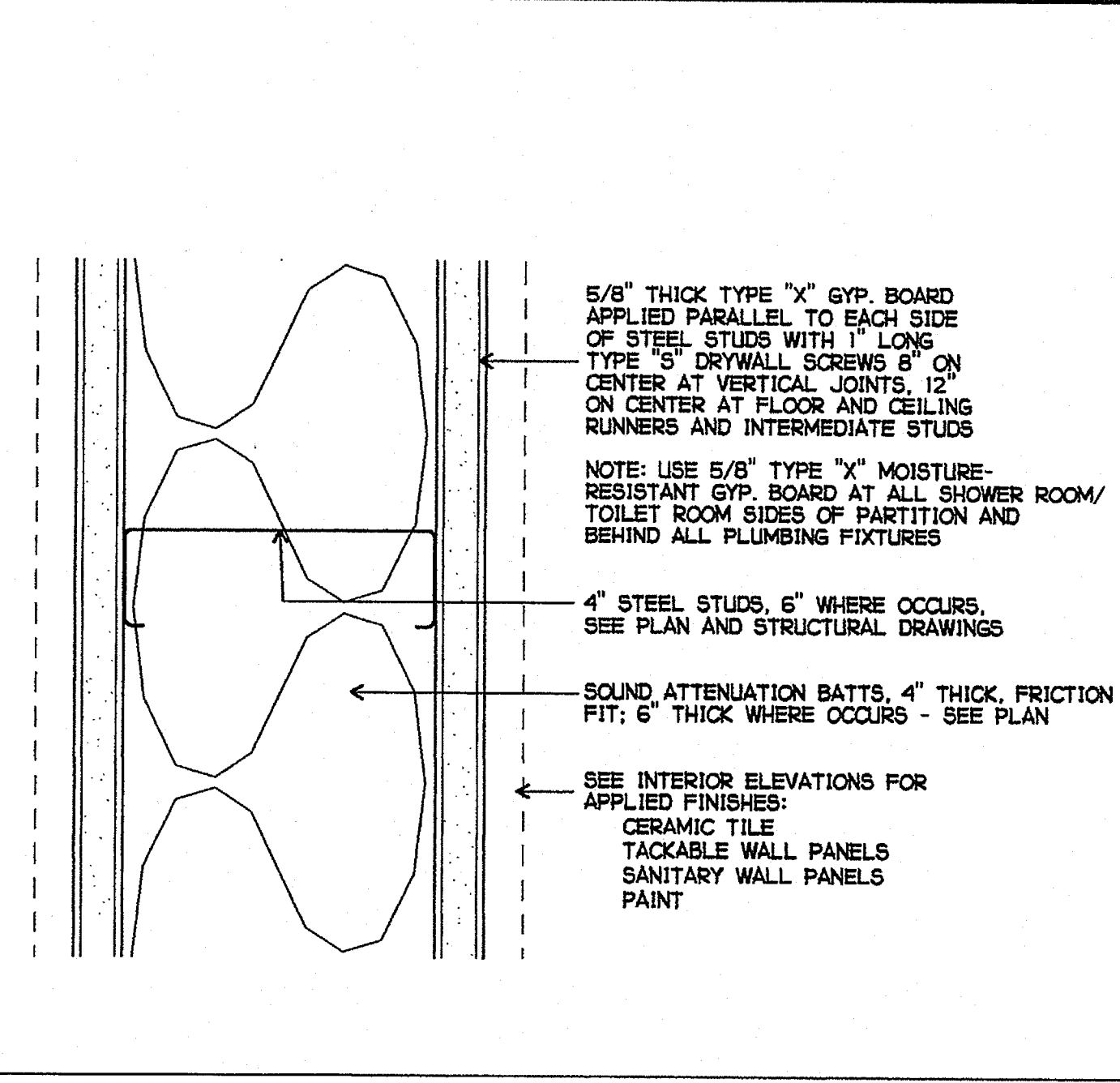
LICENSED ARCHITECT
 JOHN SCOTT GROTH
 C-26609
 4/30/2007 RENEWAL
 STATE OF CALIFORNIA

SHEET TITLE
 HOLLOW METAL
 FRAME, COIL DOOR
 DETAILS

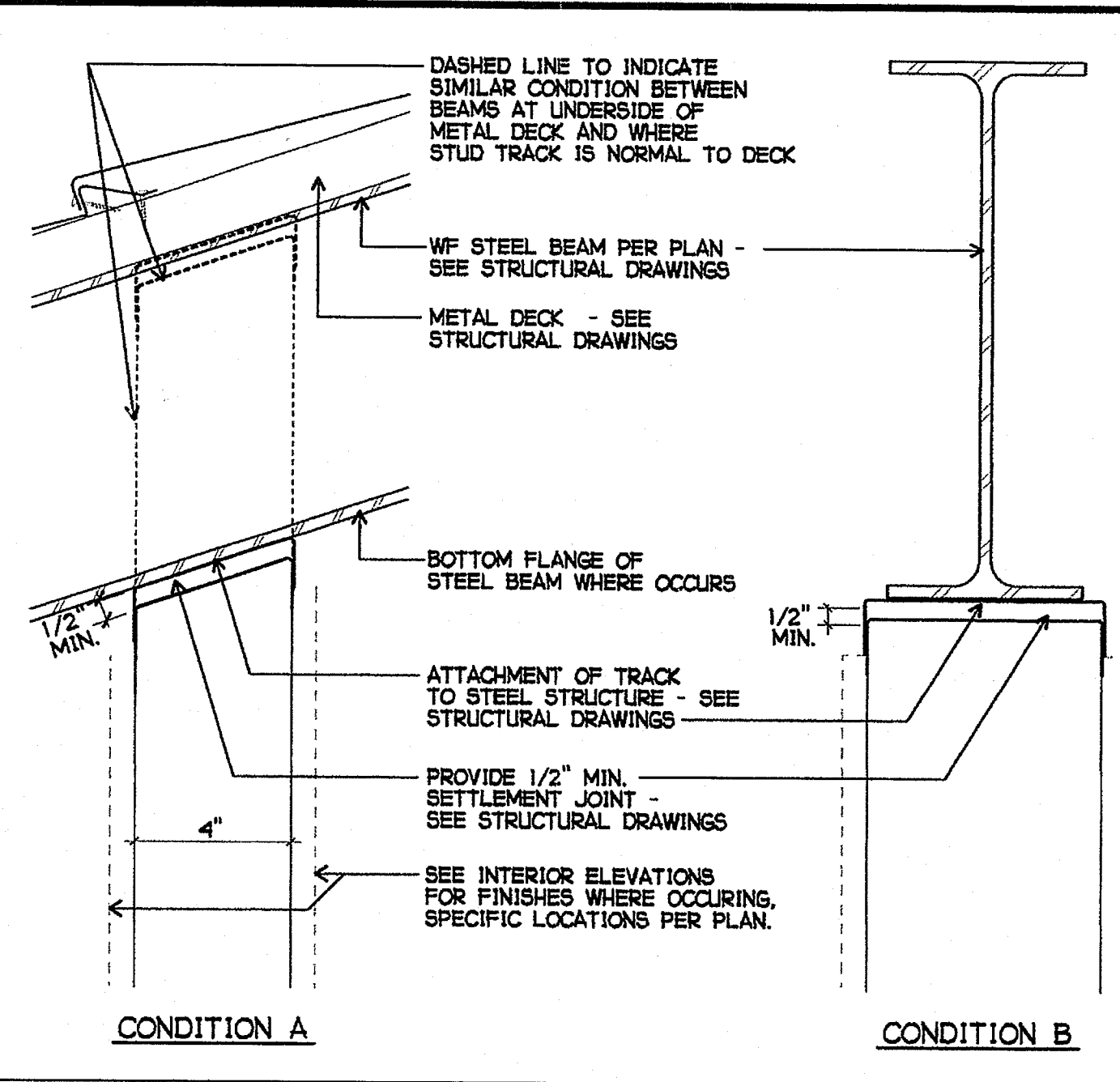
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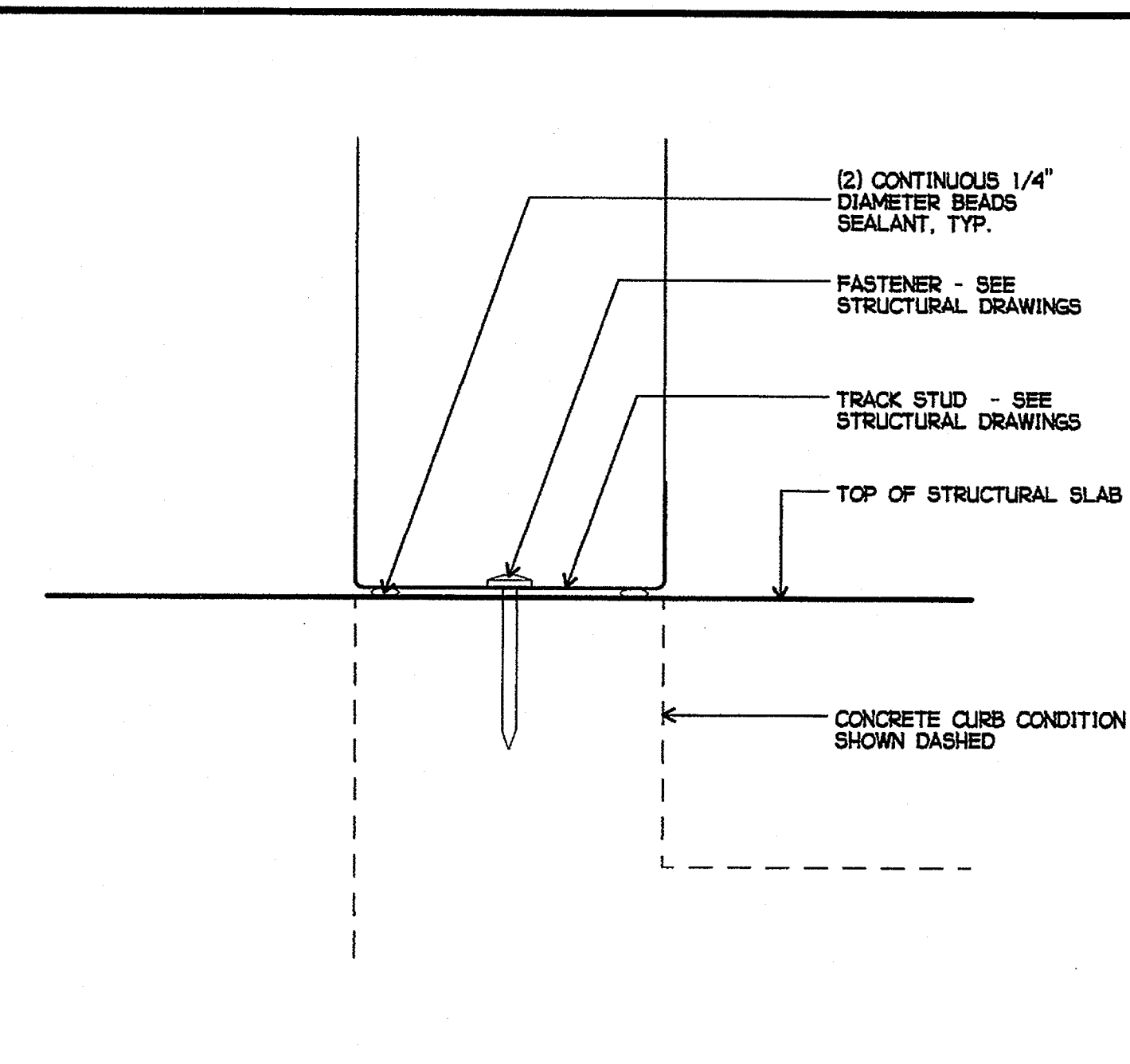
① 1-HR. EXTERIOR WALL ASSEMBLY ICB04: ESR-1994/ER-4226/ER-4368
6" = 1'-0"



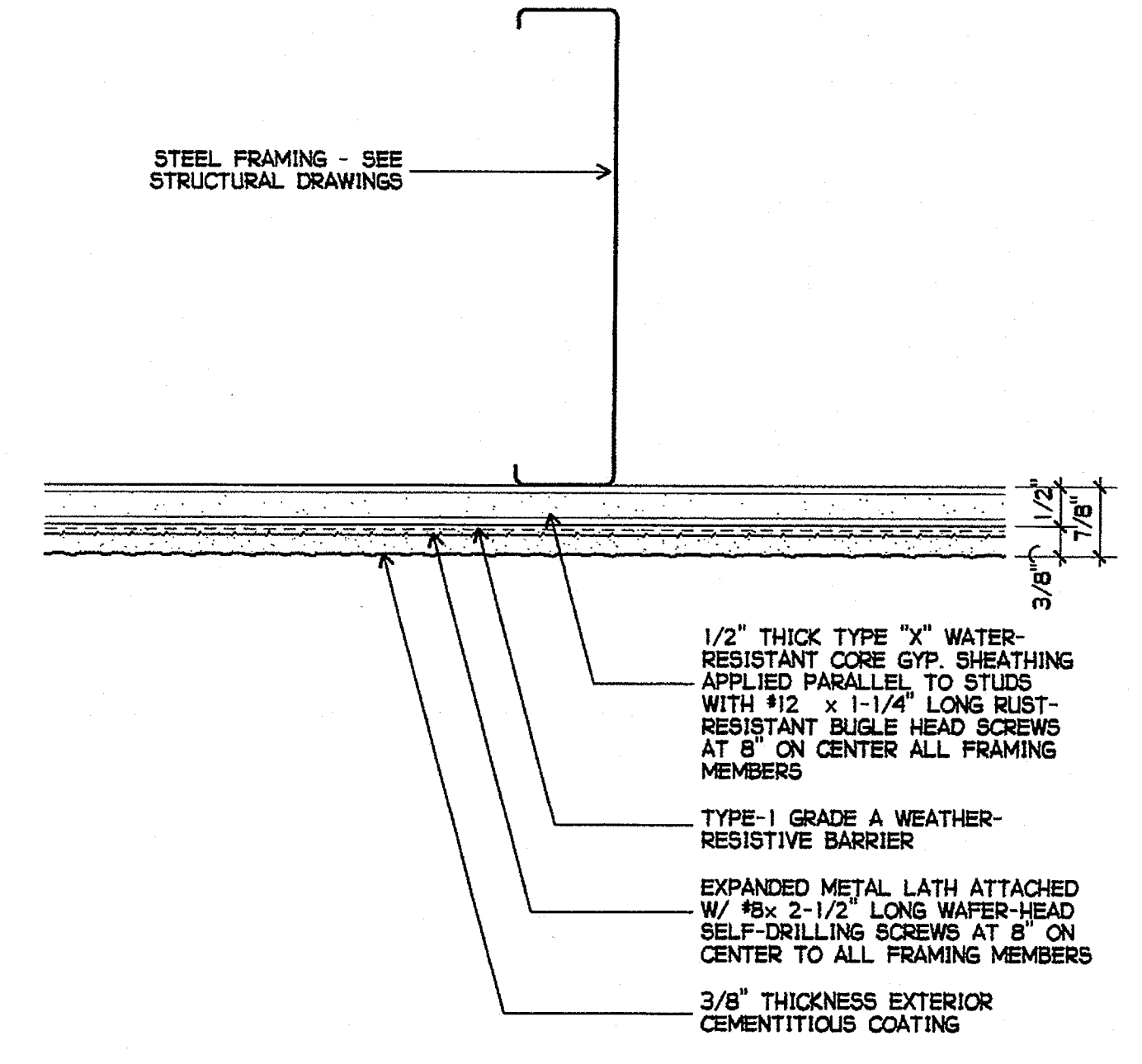
② TYPICAL INTERIOR PARTITION ASSEMBLY
6" = 1'-0"



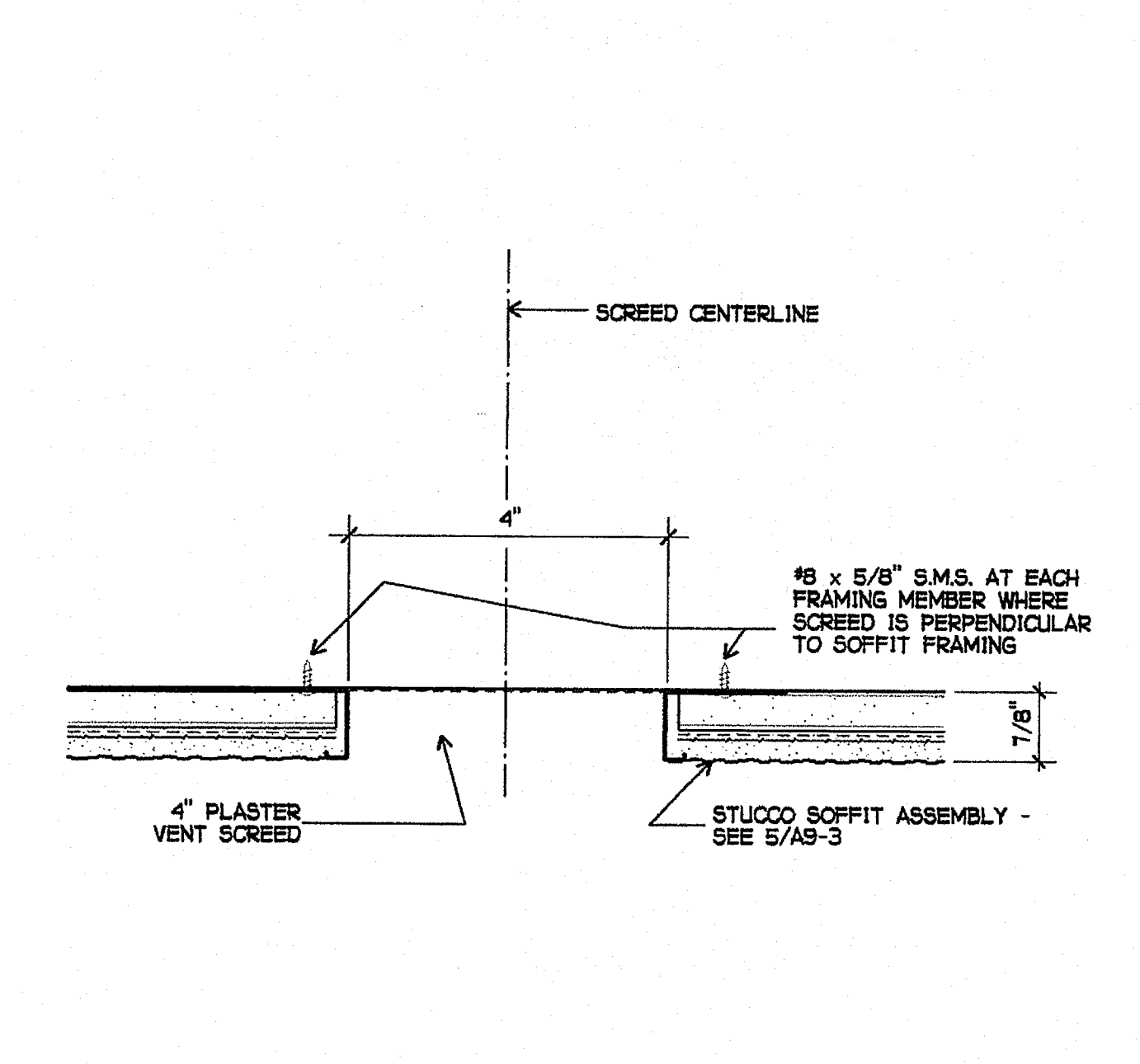
③ 1/2" STEEL STUD FRAMING SETTLEMENT DETAILS
3" = 1'-0"



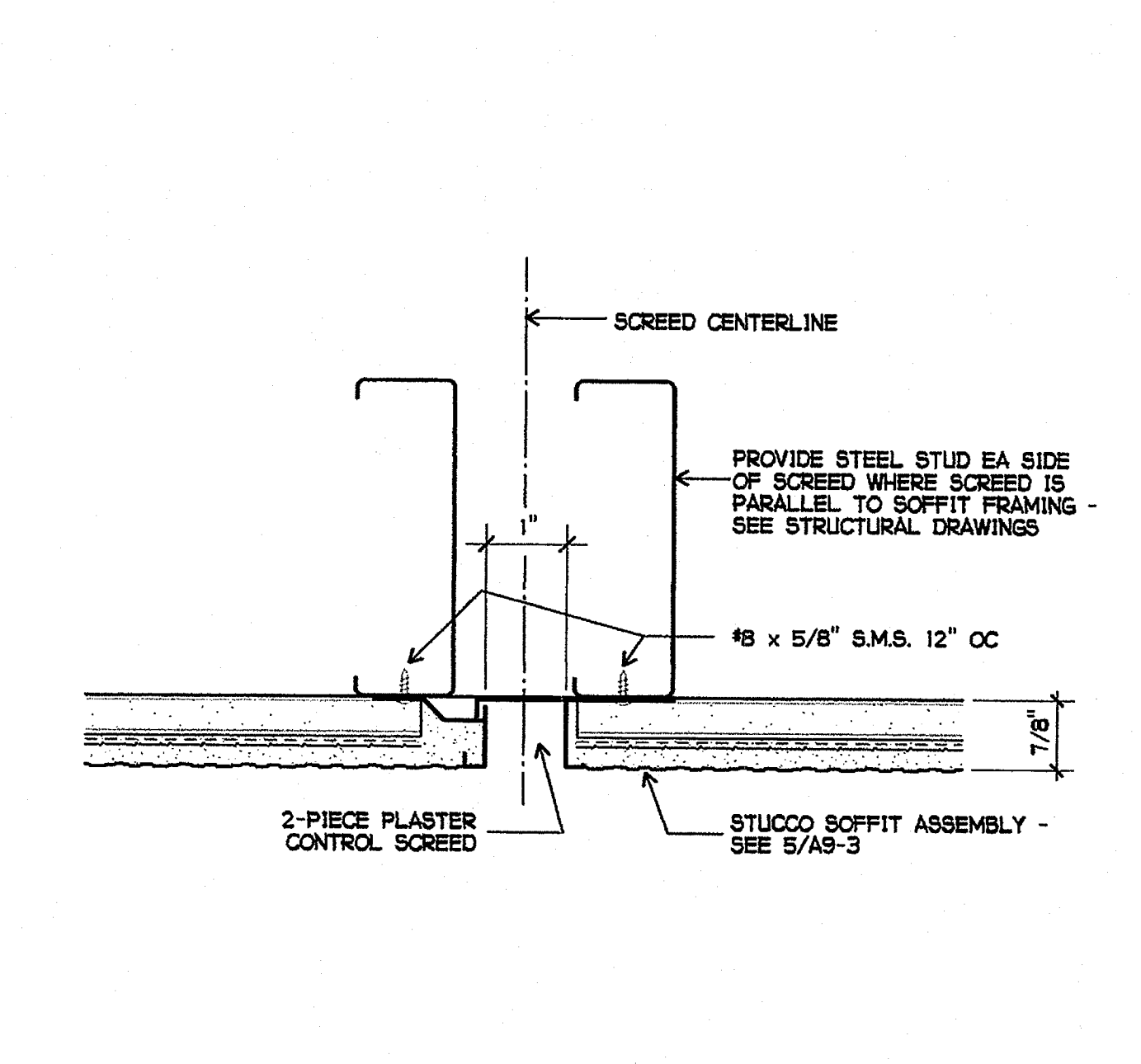
④ TRACK STUD TO FLOOR SLAB
6" = 1'-0"



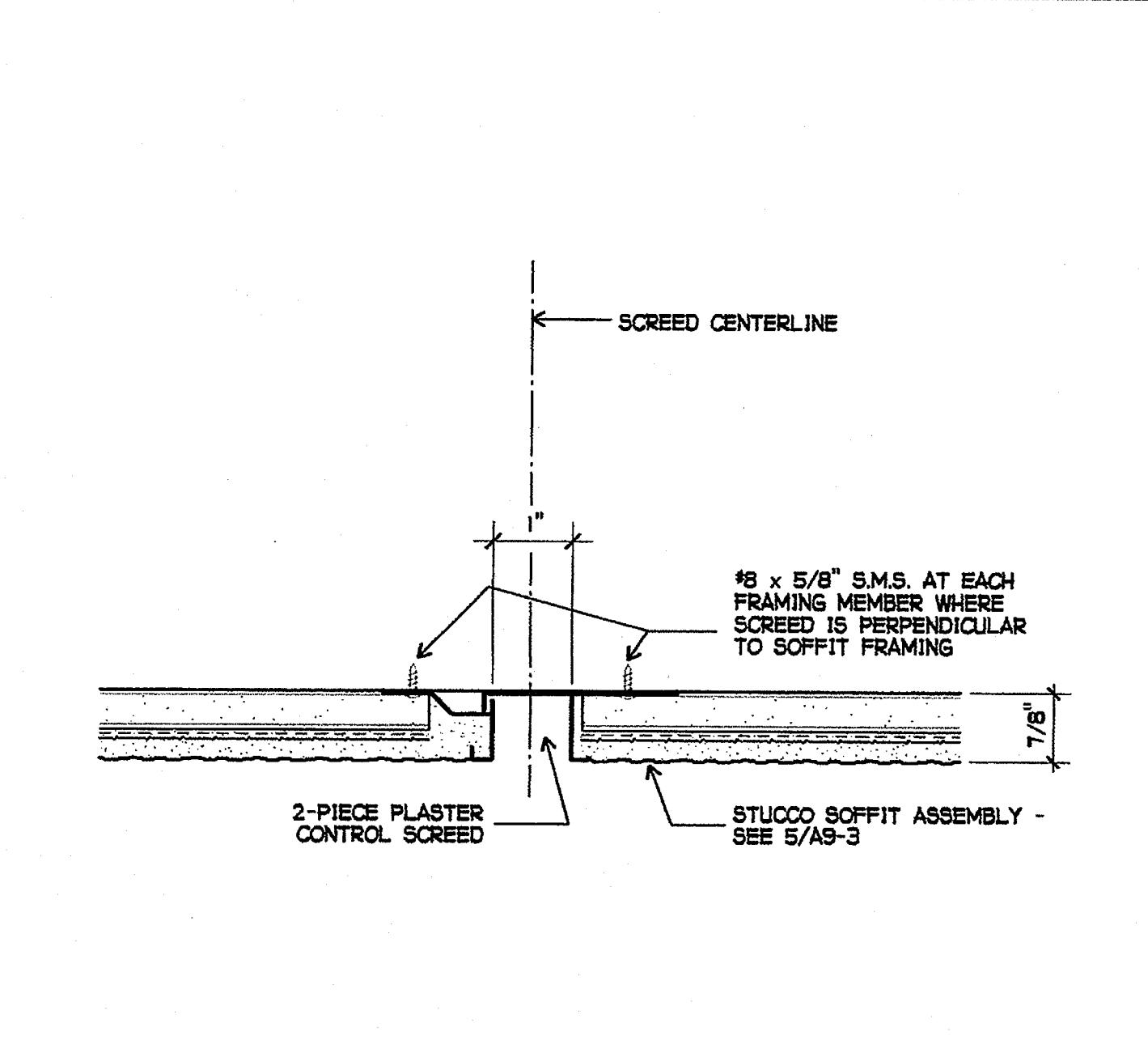
⑤ HORIZONTAL STUCCO SOFFIT ASSEMBLY
6" = 1'-0"



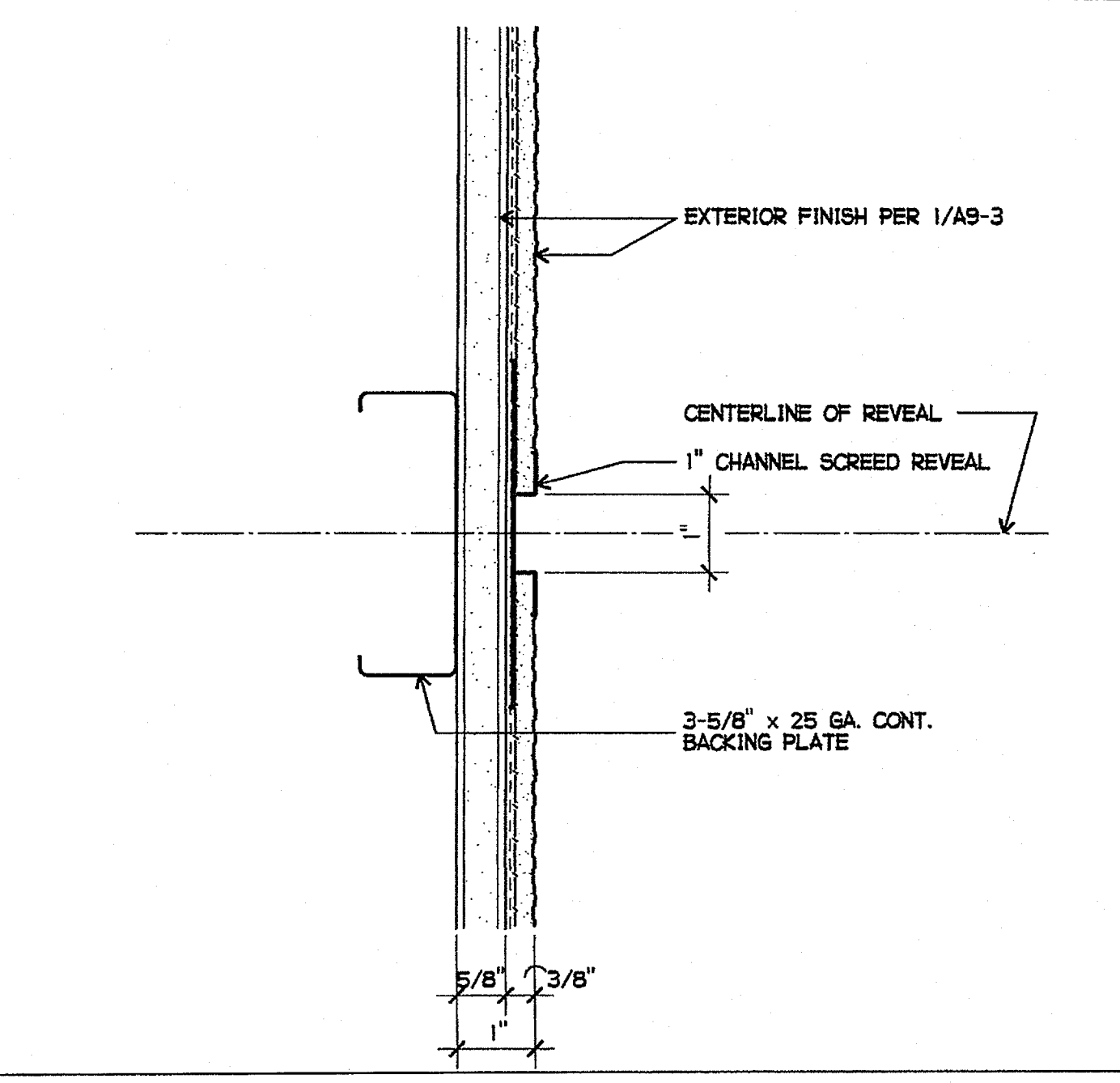
⑥ SOFFIT VENT SCREED
6" = 1'-0"



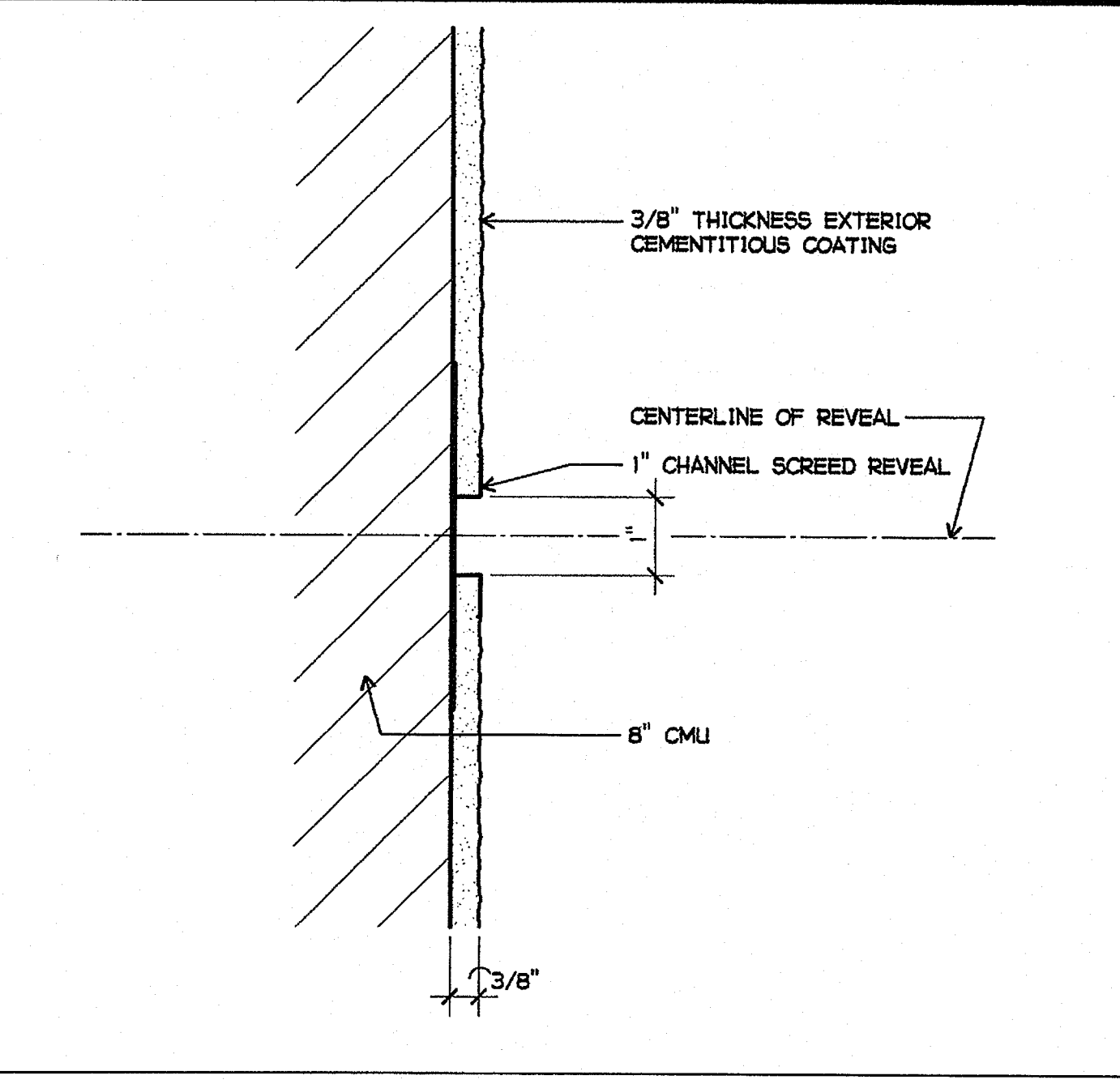
⑦ STUCCO SOFFIT EXPANSION CONTROL SCREED PARALLEL TO FRAMING
6" = 1'-0"



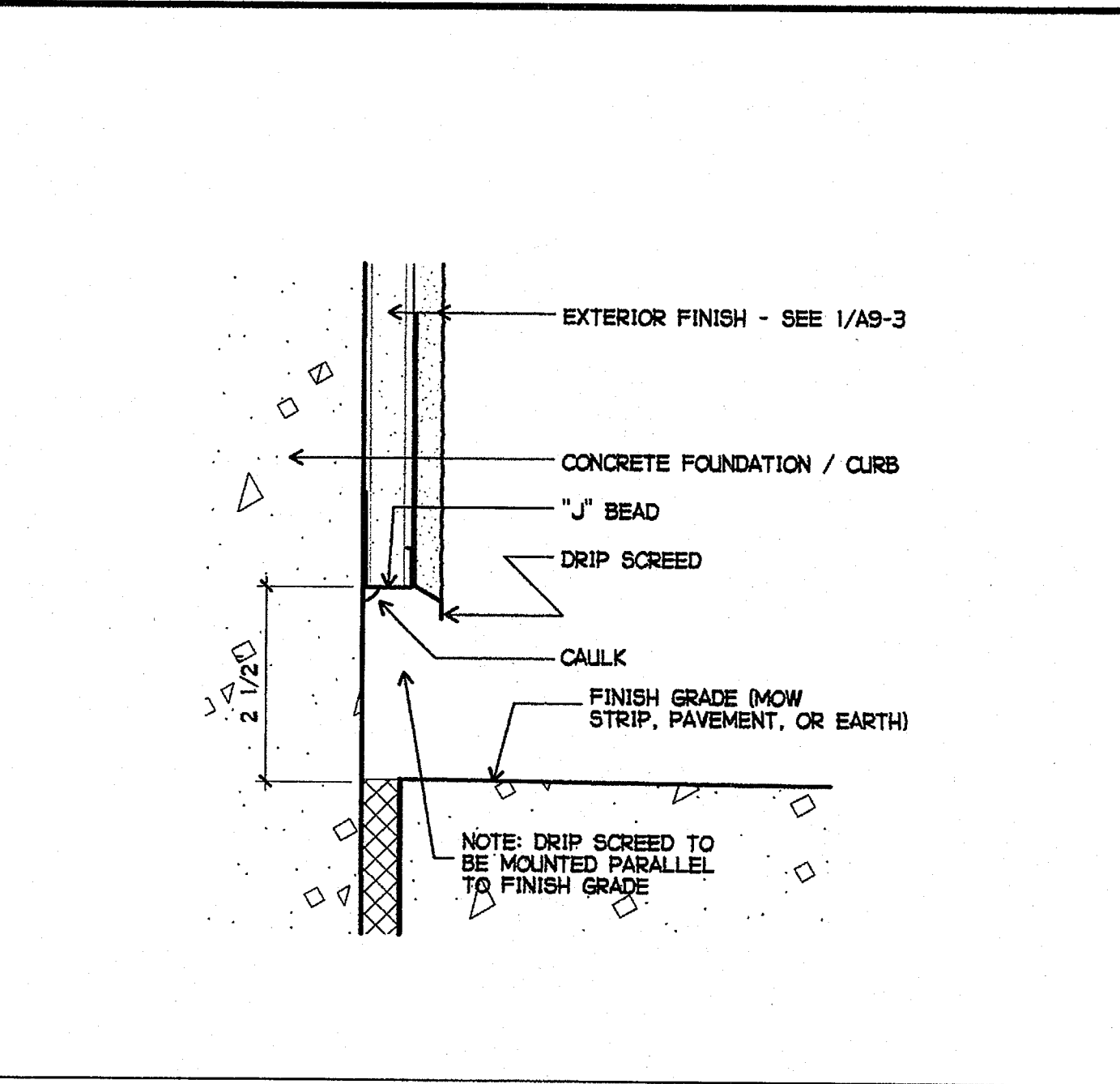
⑧ STUCCO SOFFIT EXPANSION CONTROL SCREED PERPENDICULAR TO FRAMING
6" = 1'-0"



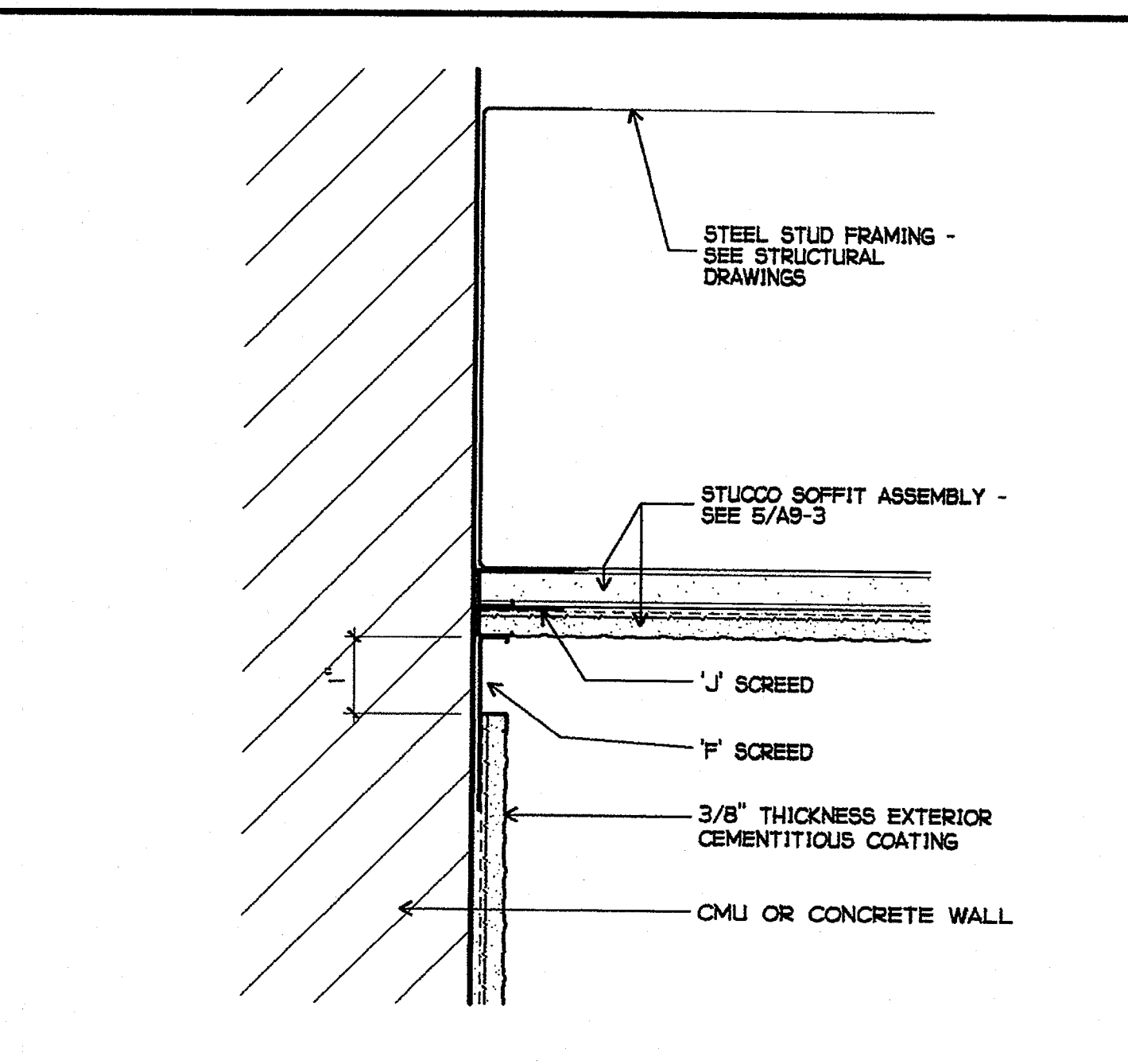
⑨ VERTICAL STUCCO SCREED
1" = 1'-0"



⑩ VERTICAL STUCCO SCREED AT CML
6" = 1'-0"



⑪ WEEP SCREED
6" = 1'-0"



⑫ STUCCO SOFFIT REVEAL AT CML/CONCRETE
6" = 1'-0"

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 P. T. N. 73569-9
 DATE

REVISIONS

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 OCEANSIDE, CA 92054
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space art function time

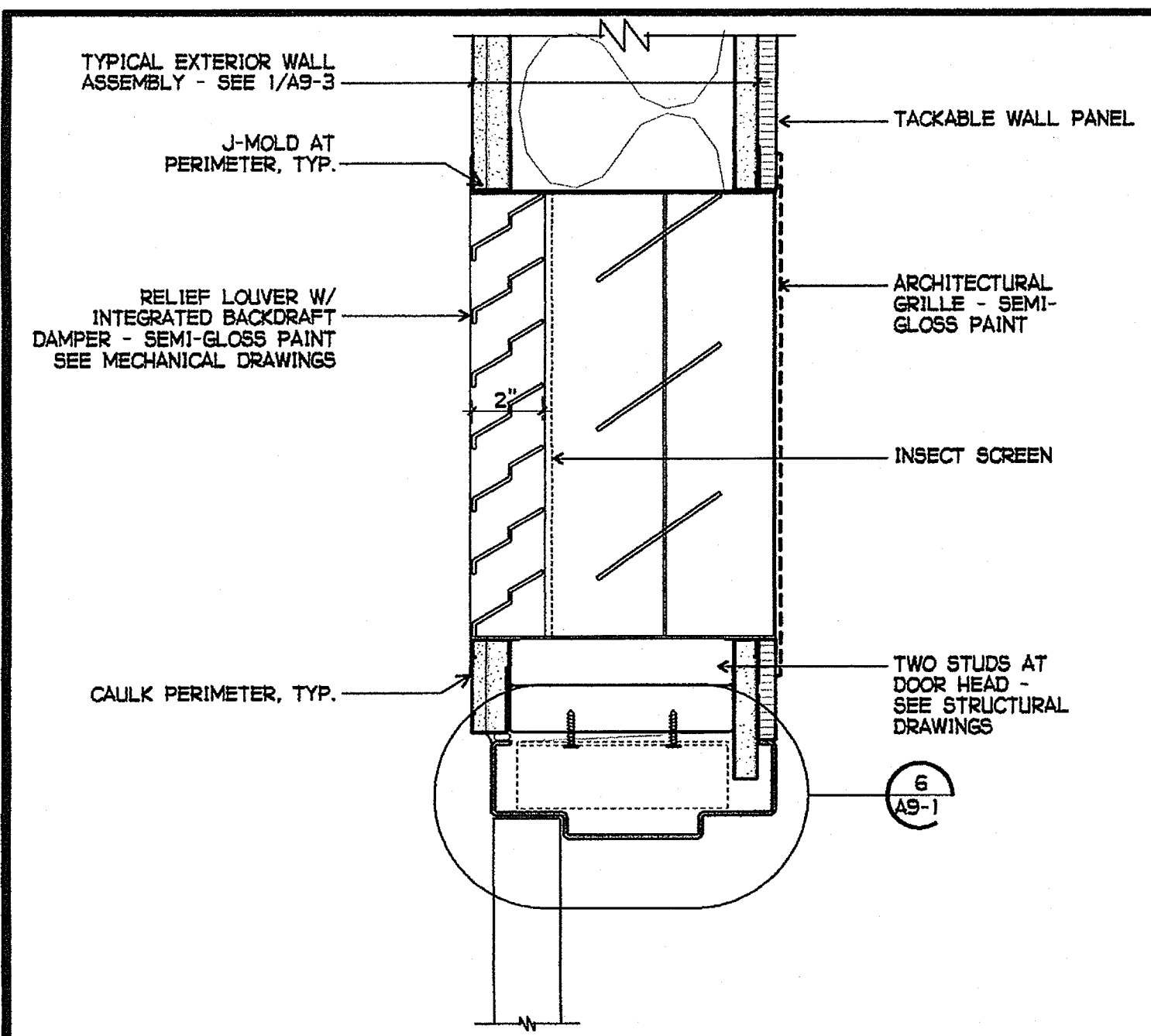
DBA IDENTIFICATION STAMP OF THE STATE ARCHITECT OFFICE OF REGULATION SERVICES
 4-106494
 AC. PG. FILED SS
 DATE MAR 2 8 2005

LICENSED ARCHITECT JOHN SCOTT GROTH
 C-26609
 4/30/2007 RENEWAL

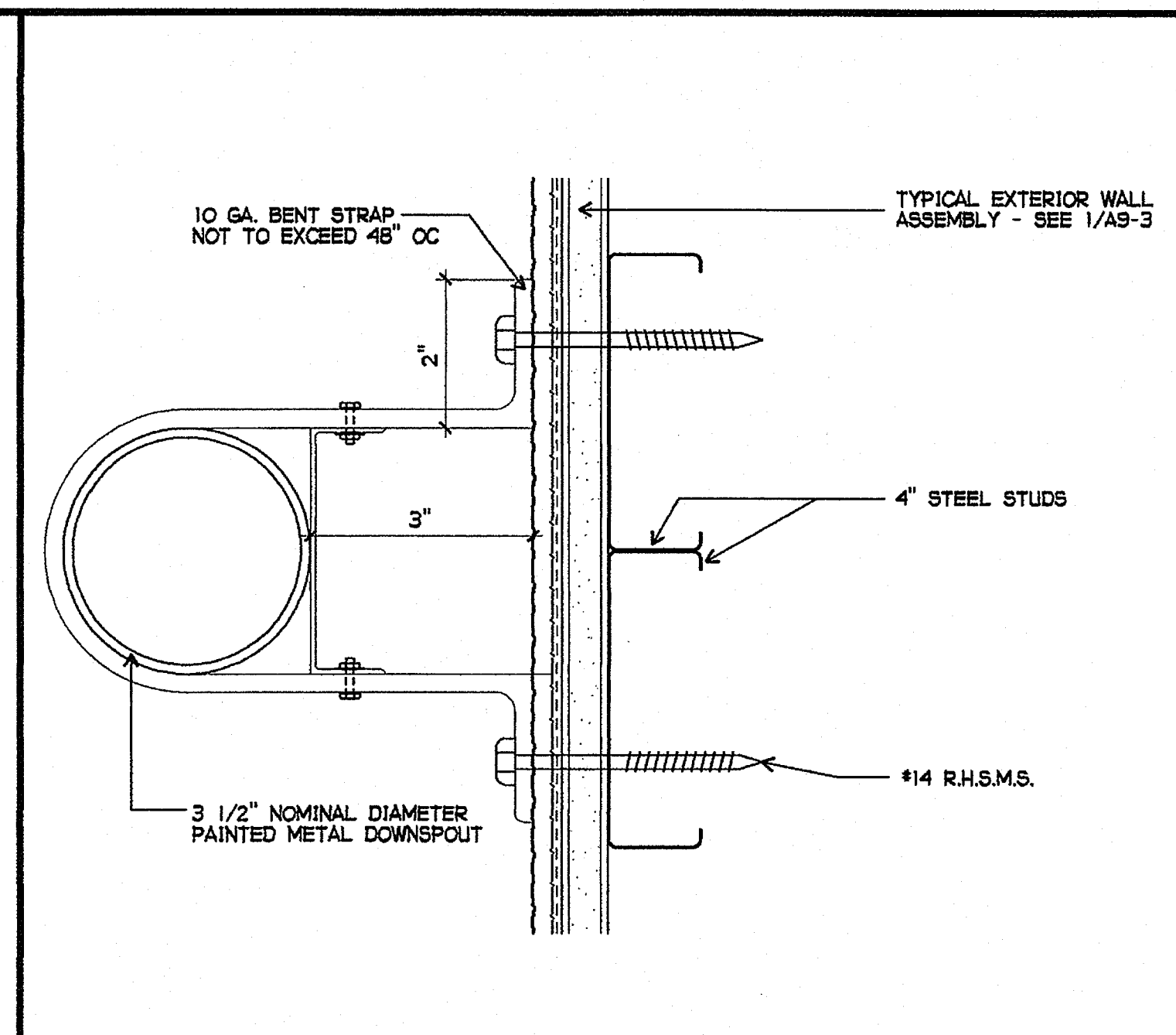
DETAILS

A9-3

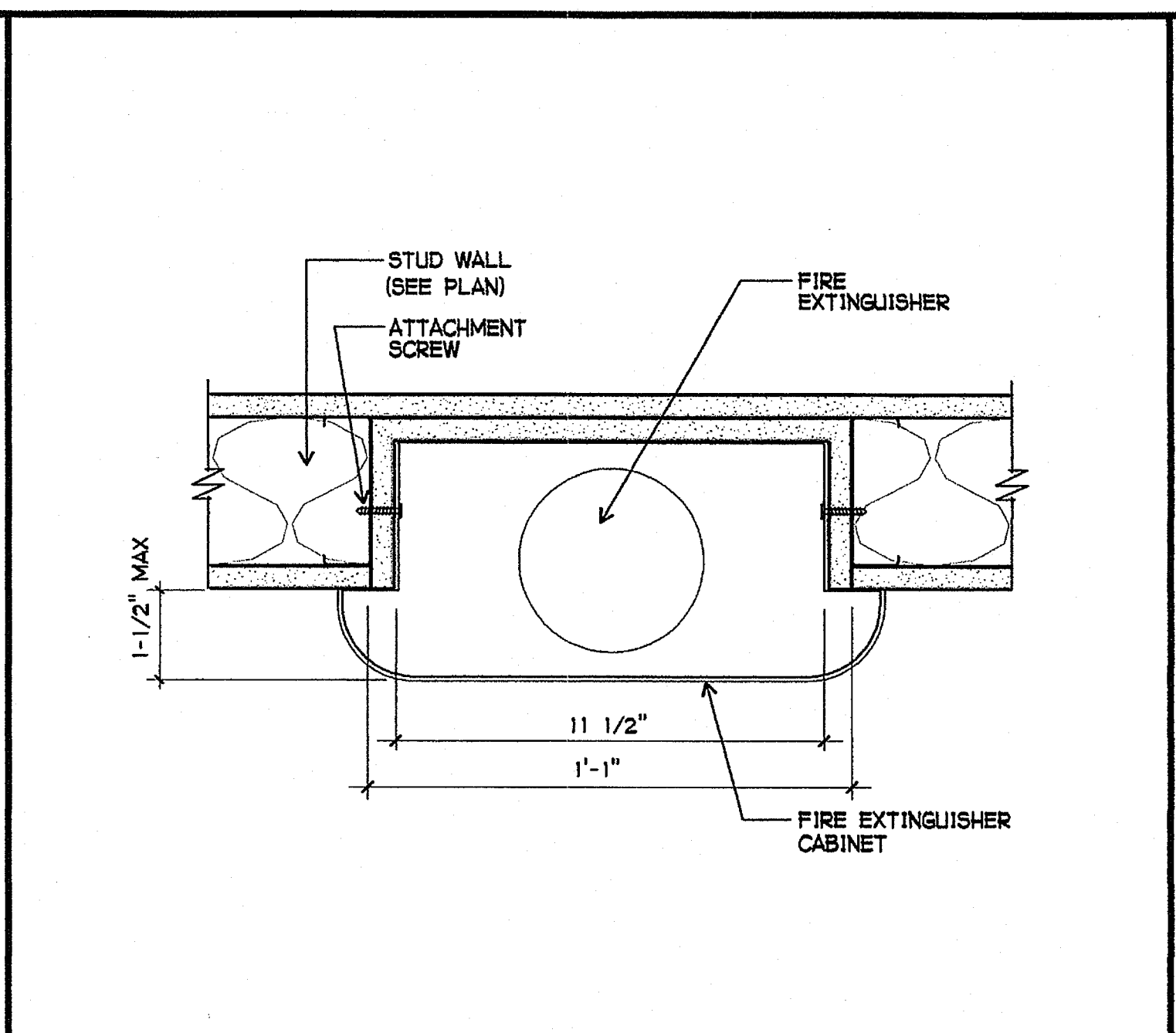
FILED: JMS NEWCONSTR DOCS\A9-3.AEC



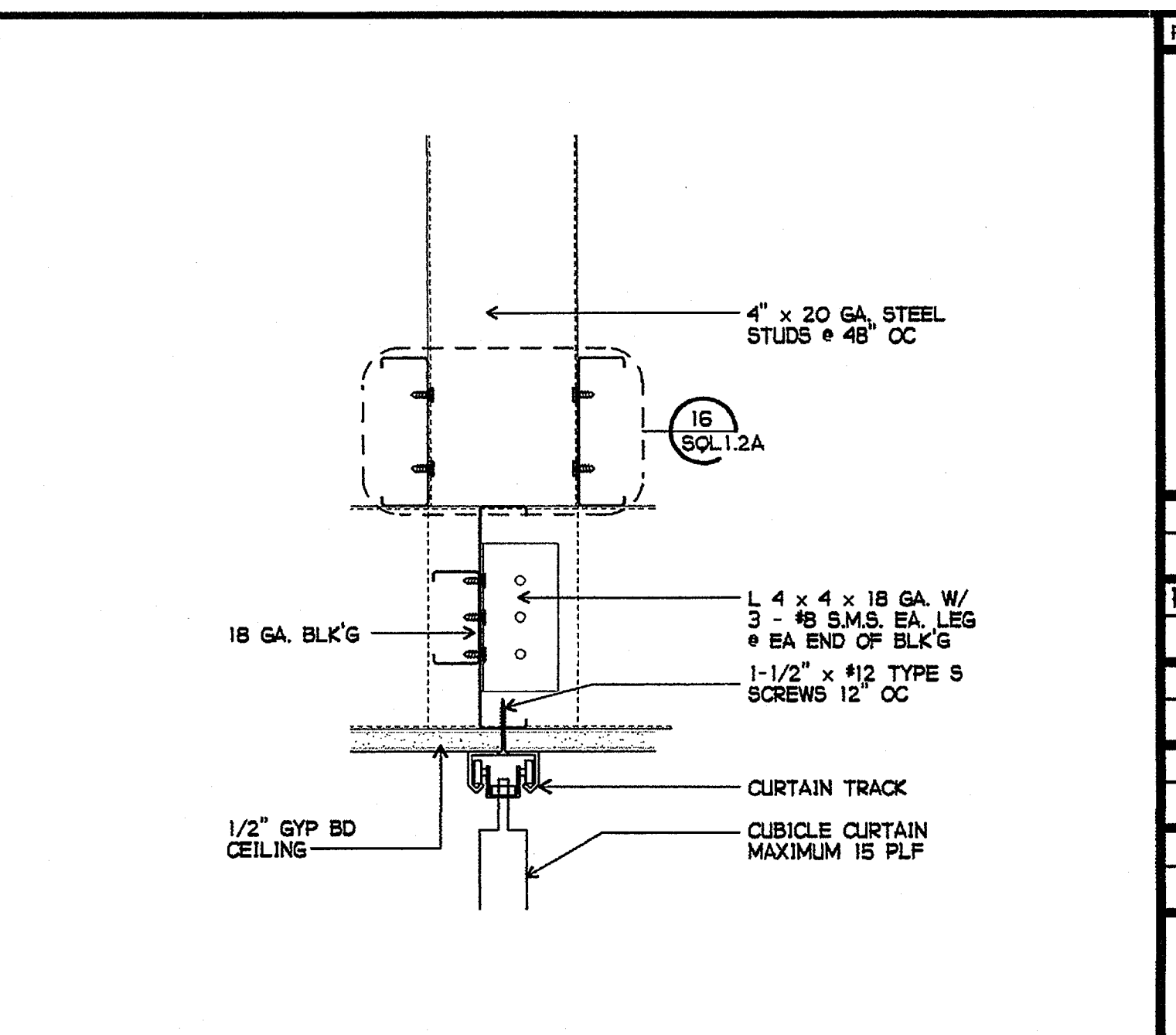
1 RELIEF LOUVER W/ INTEGRATED BACKDRAFT DAMPER AND ARCHITECTURAL GRILLE ABOVE DOOR
3" = 1'-0"



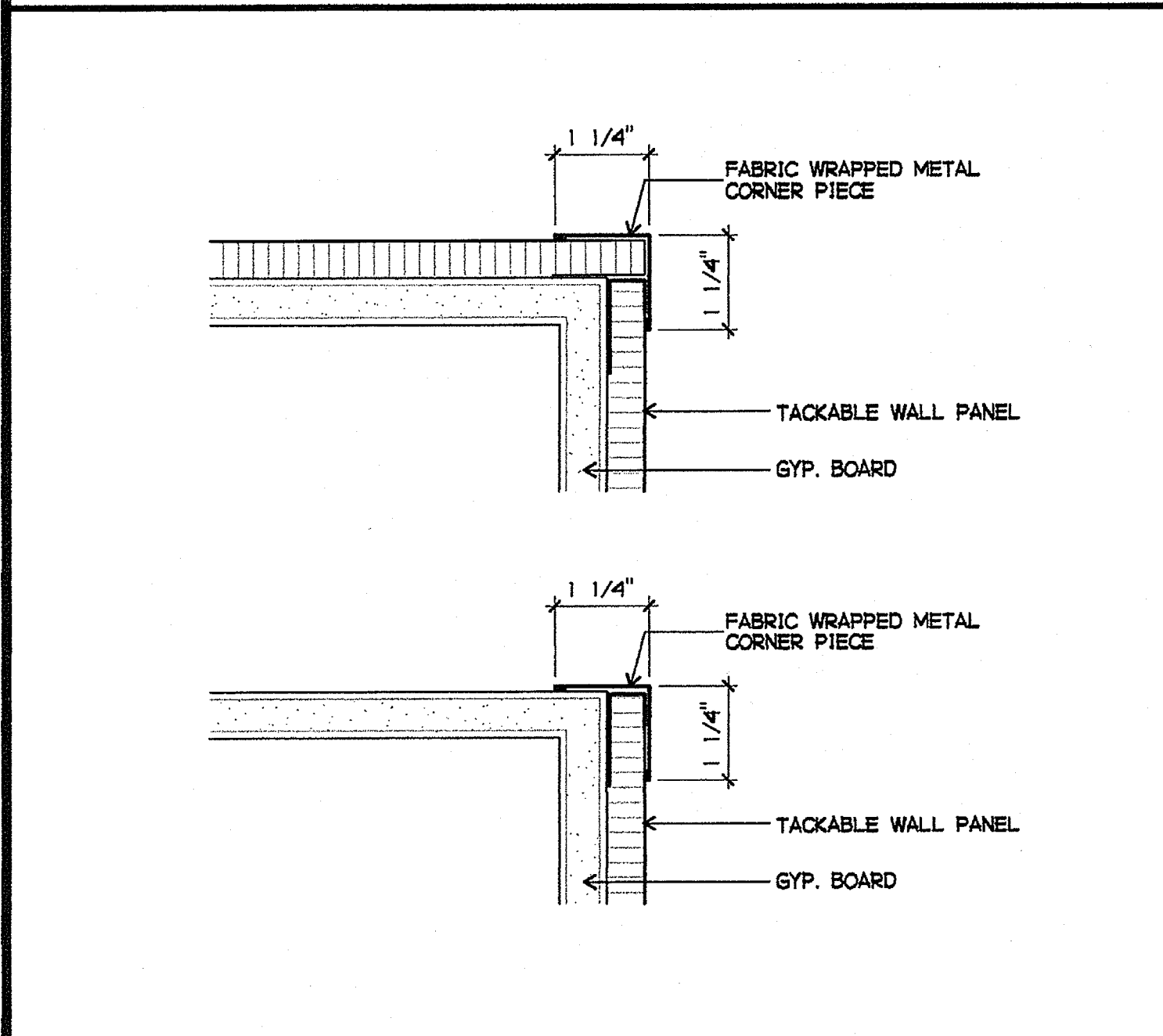
2 DOWNSPOUT
6" = 1'-0"



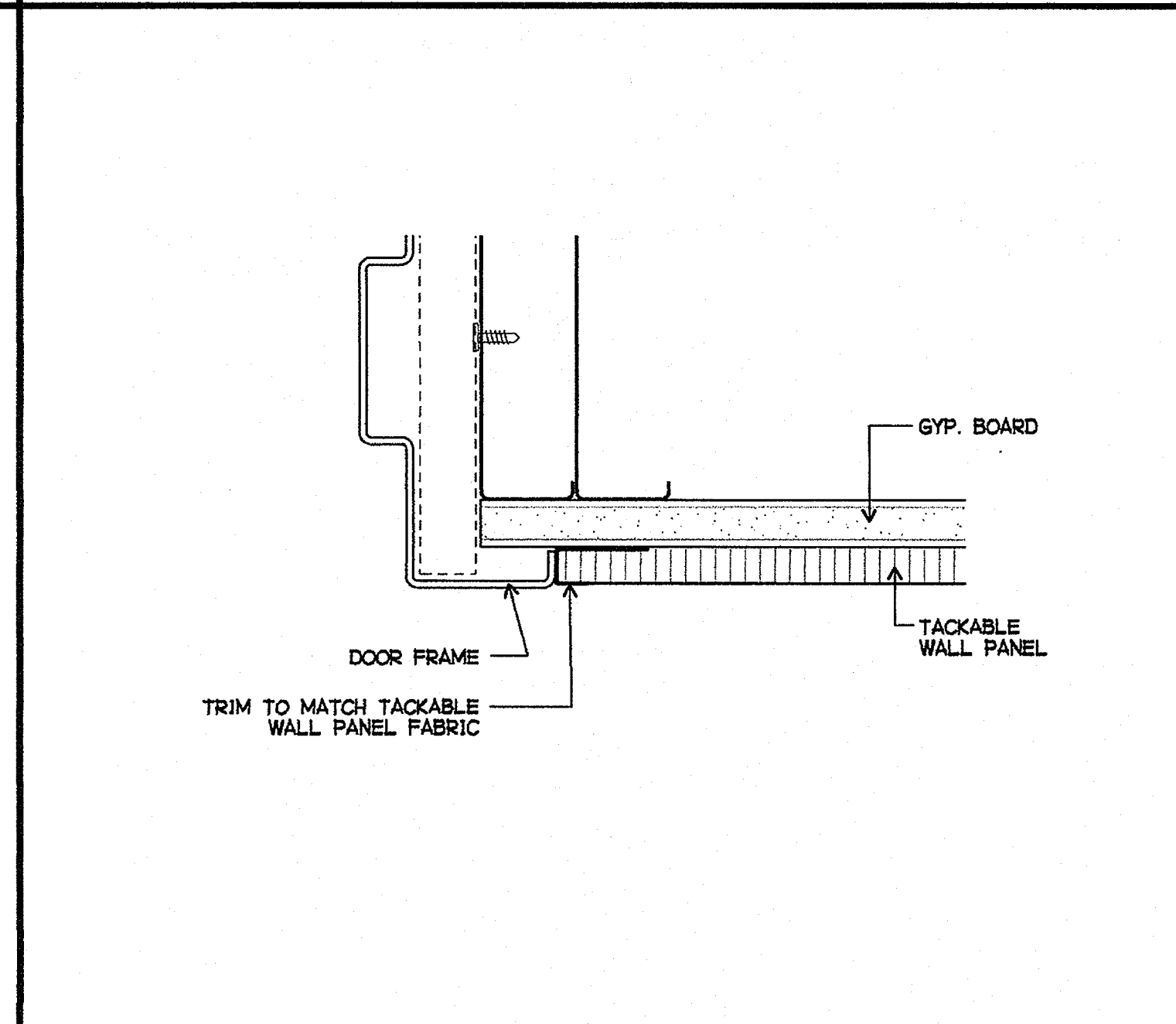
3 FIRE EXTINGUISHER AND CABINET
3" = 1'-0"



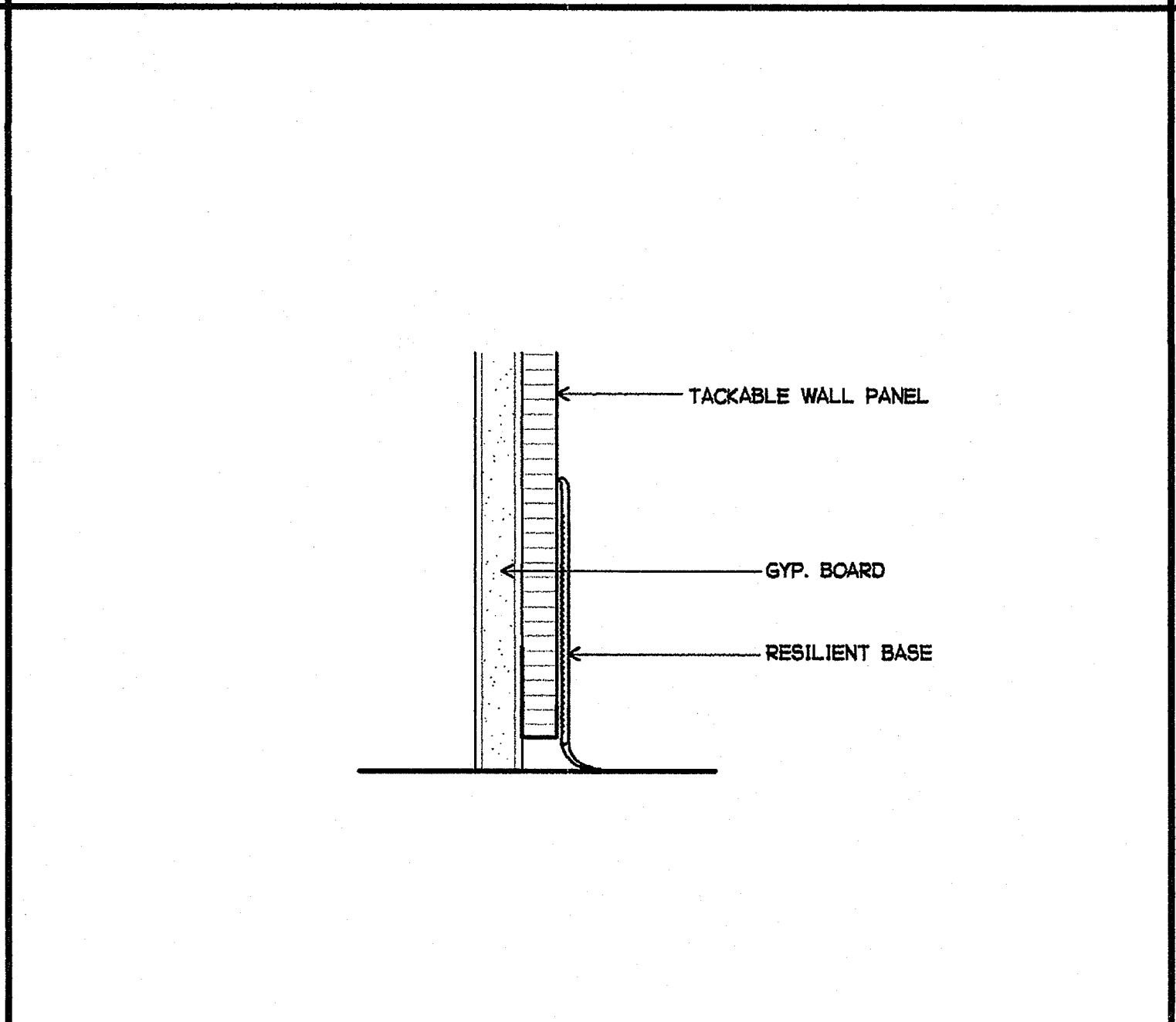
4 CURTAIN PARTITION TRACK
3" = 1'-0"



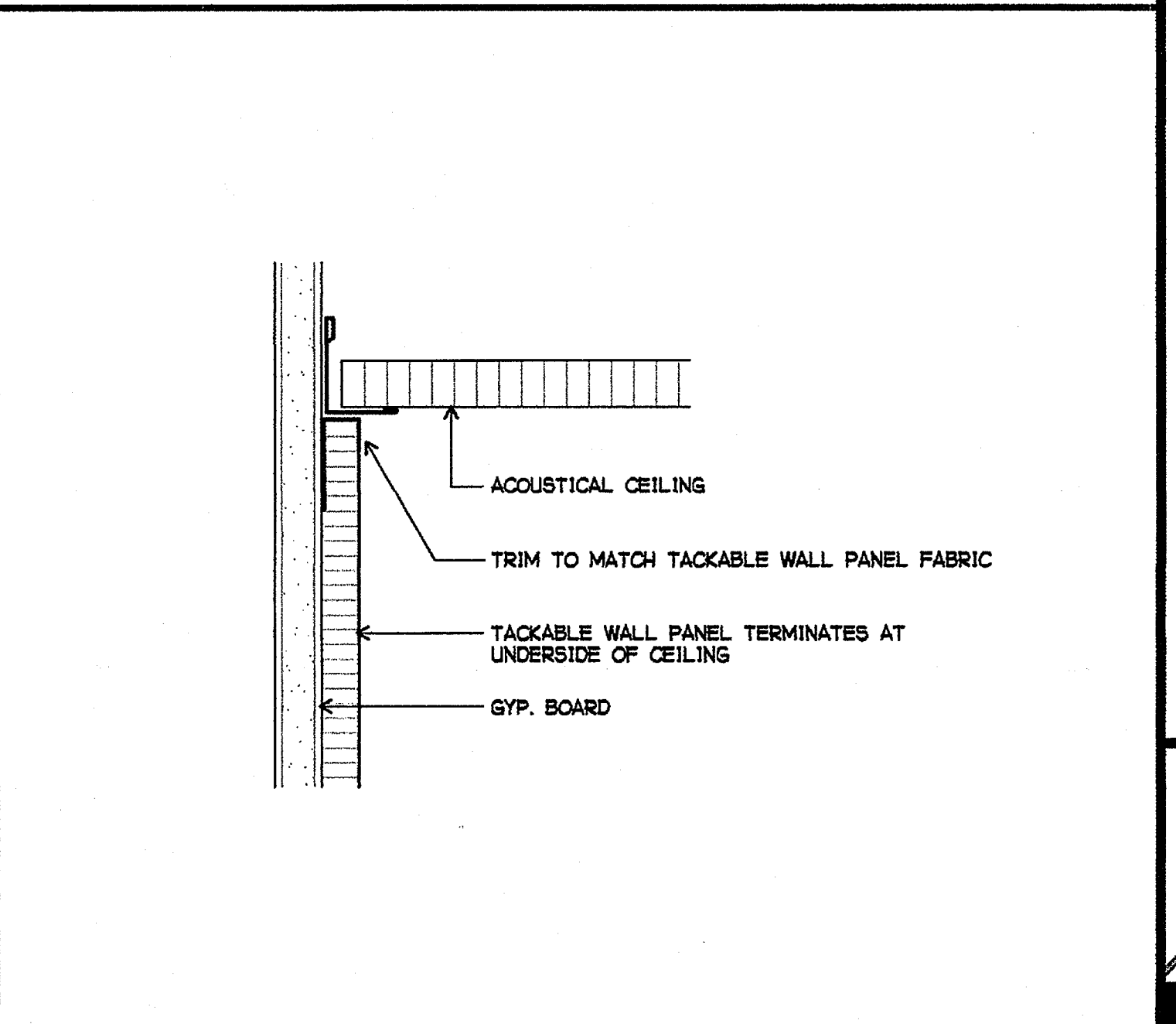
5 TACKABLE WALL PANEL - OUTSIDE CORNER
6" = 1'-0"



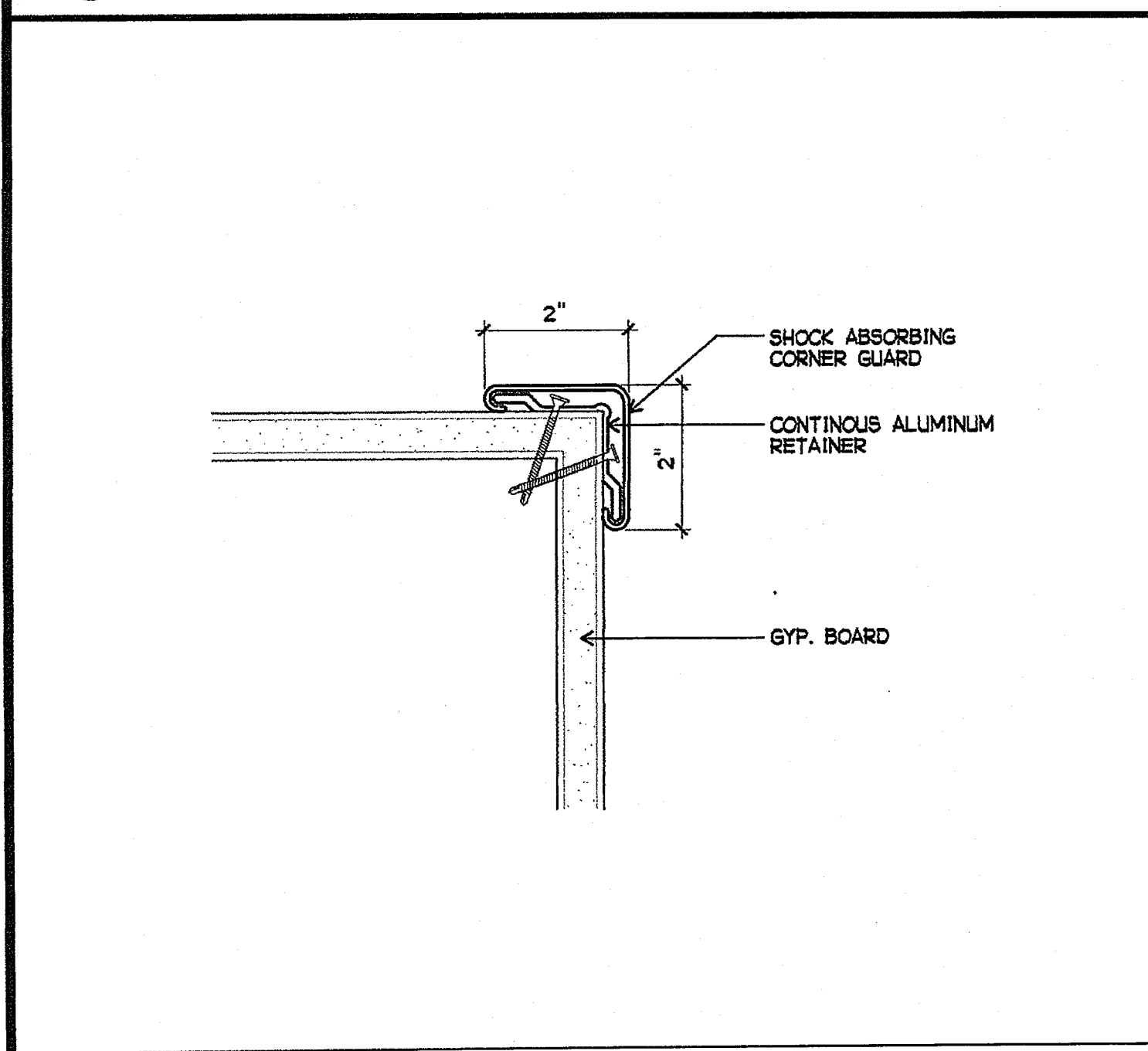
6 TACKABLE WALL PANEL - DOOR EDGE TRIM
6" = 1'-0"



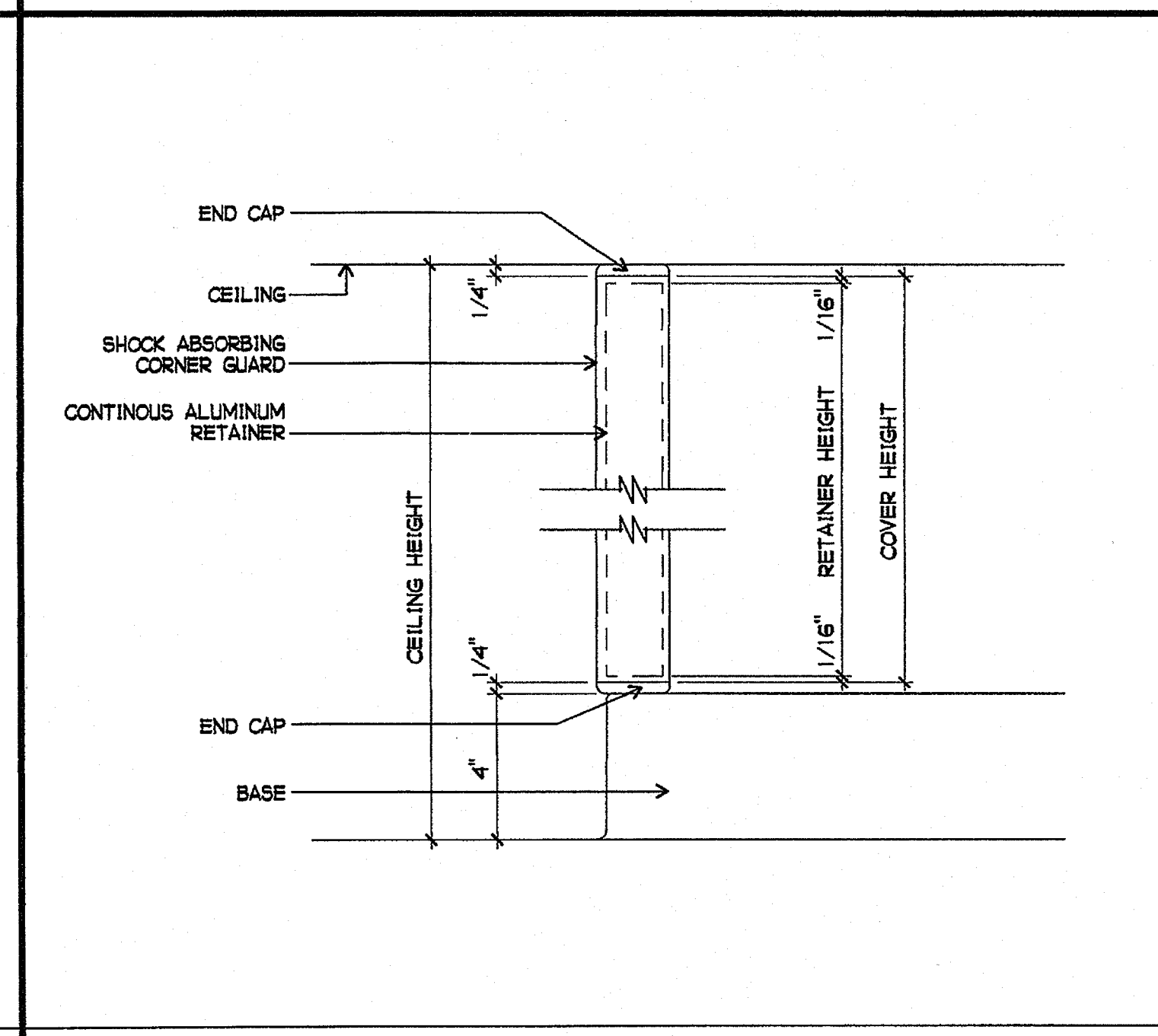
7 TACKABLE WALL PANEL - BASE
6" = 1'-0"



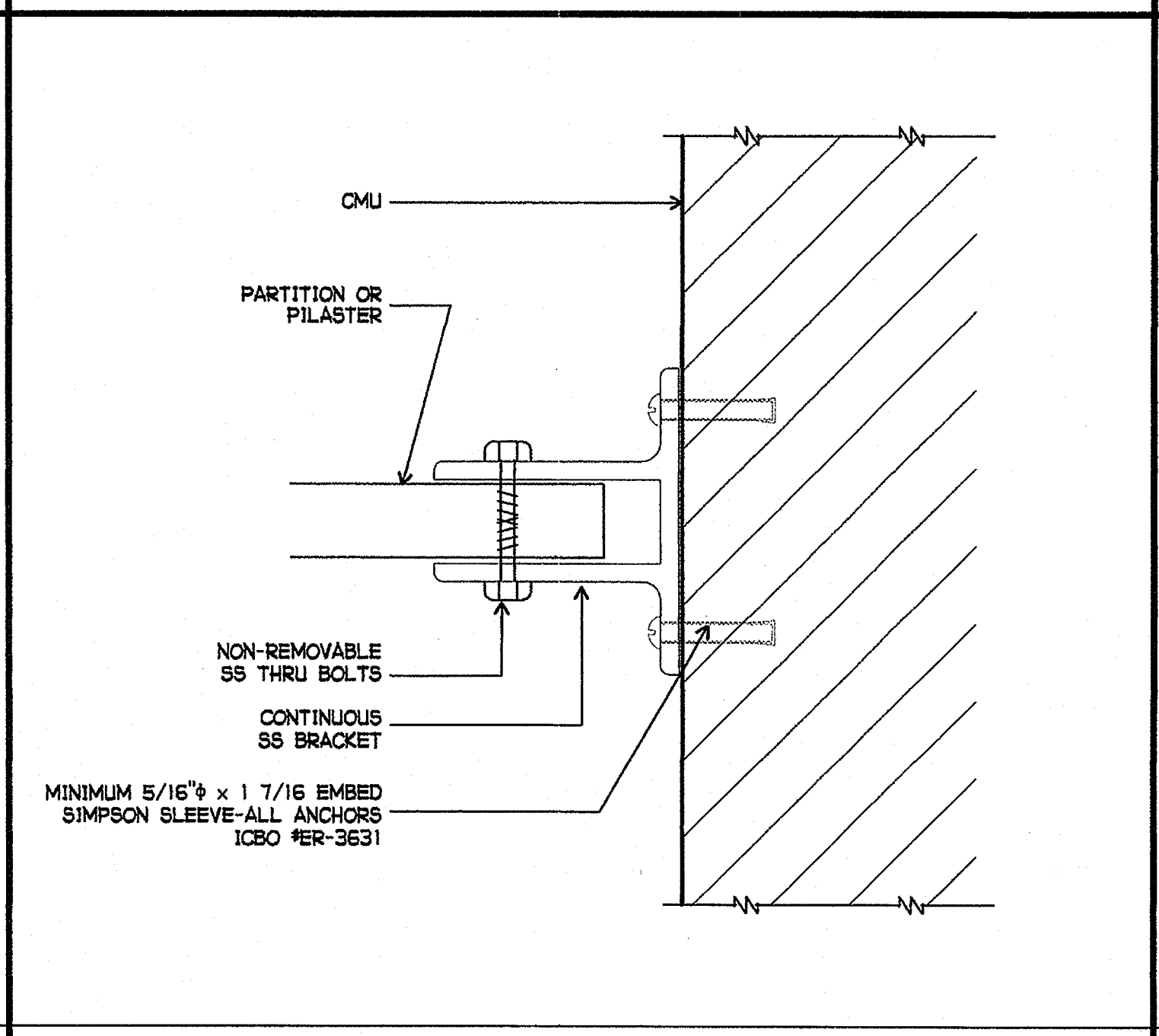
8 TACKABLE WALL PANEL - TERMINATION AT CEILING
6" = 1'-0"



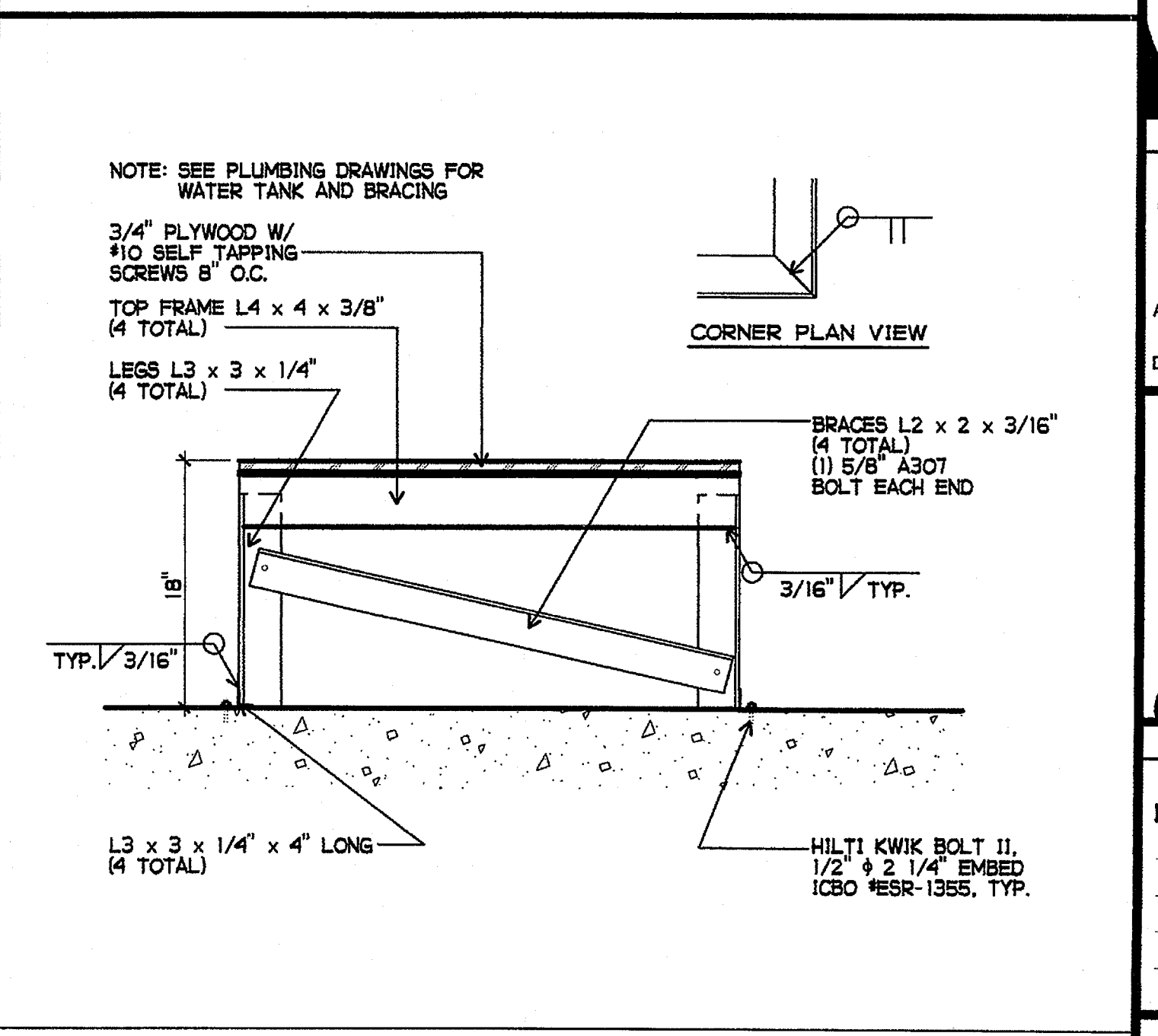
9 CORNER GUARD
6" = 1'-0"



10 CORNER GUARD BASE AND TOP
3" = 1'-0"



11 TOILET PARTITION WALL CONNECTION AT MASONRY CONSTRUCTION
6" = 1'-0"



12 WATER TANK PEDESTAL
1" = 1'-0"

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PROJECT NOS. 758-000

DATE 025

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DBA

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4-106494

AC. PLS. FLS. SS.

DATE MAR 28 2005

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4/30/2007 RENEWAL

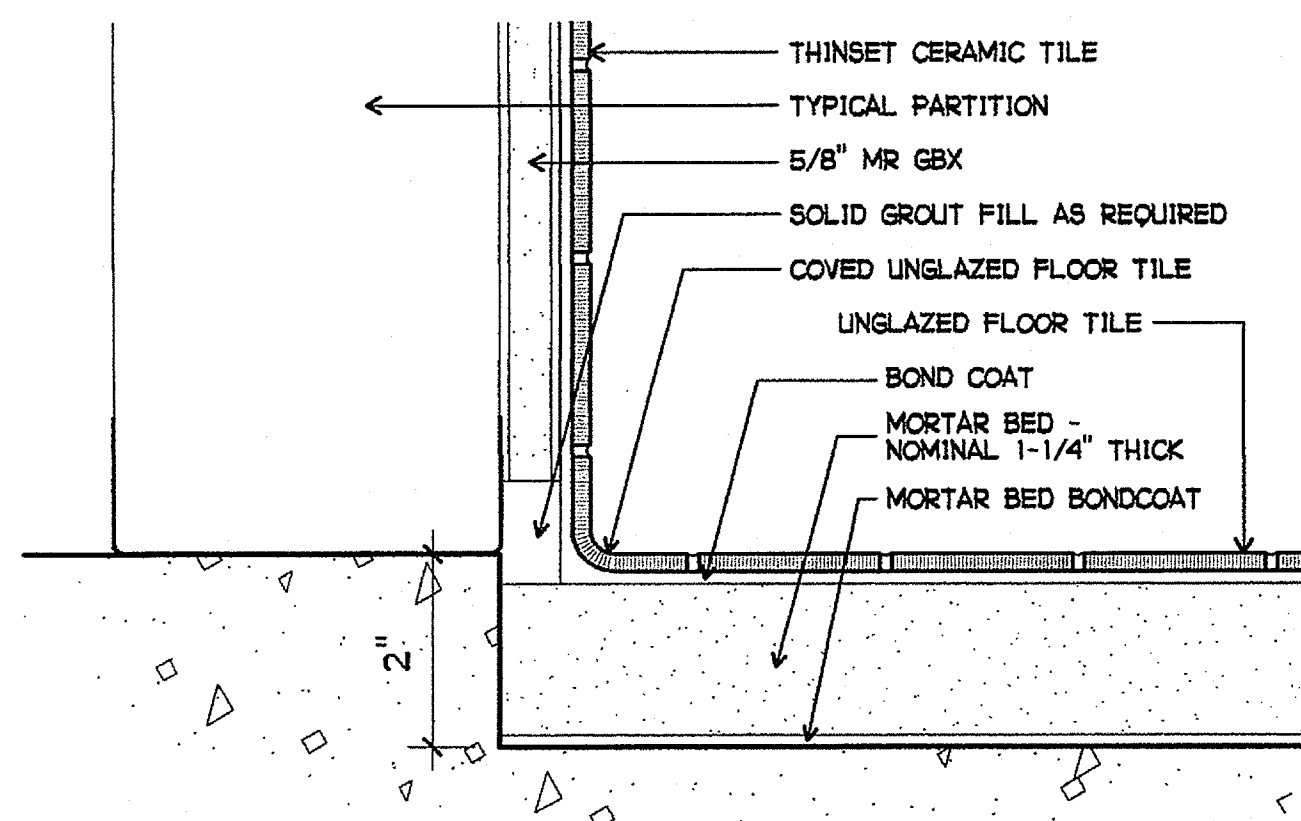
OFFICE OF CALIFORNIA ARCHITECTS

SHEET TITLE

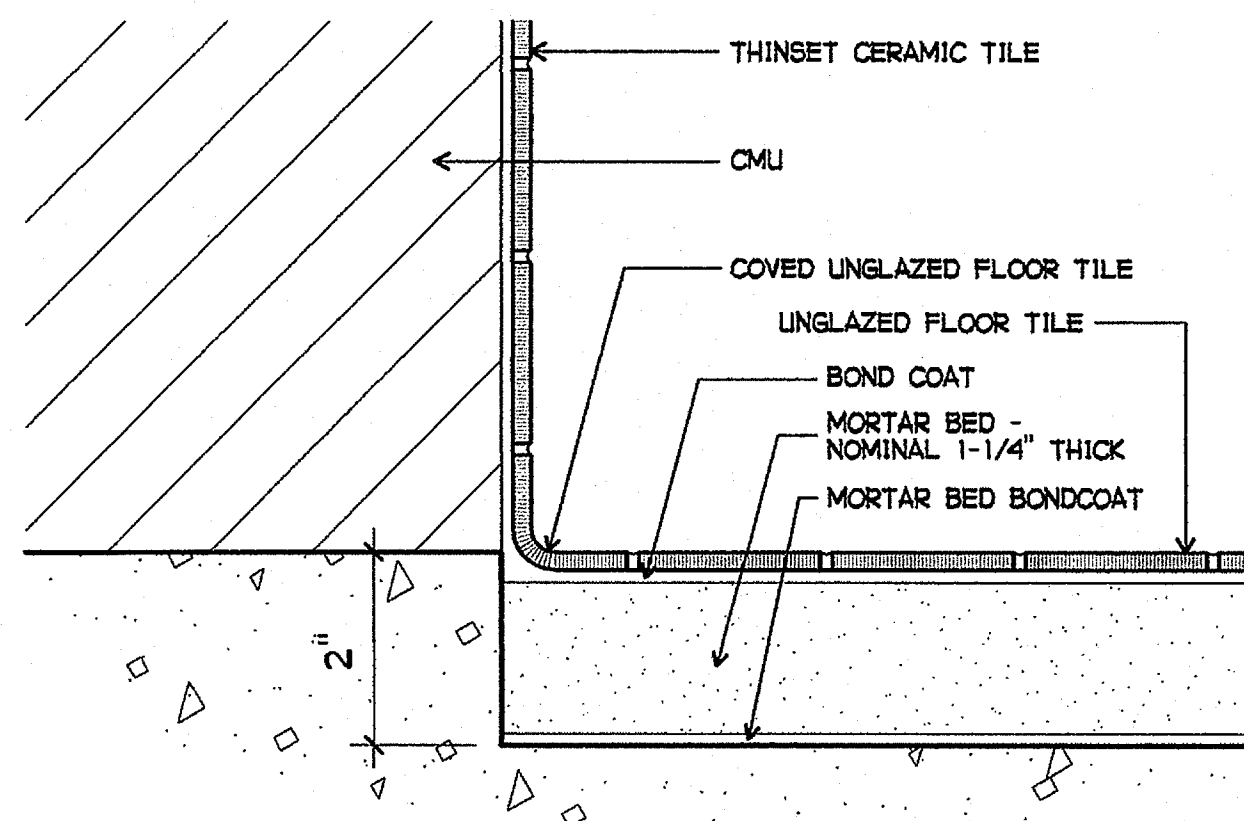
DETAILS

A9-4

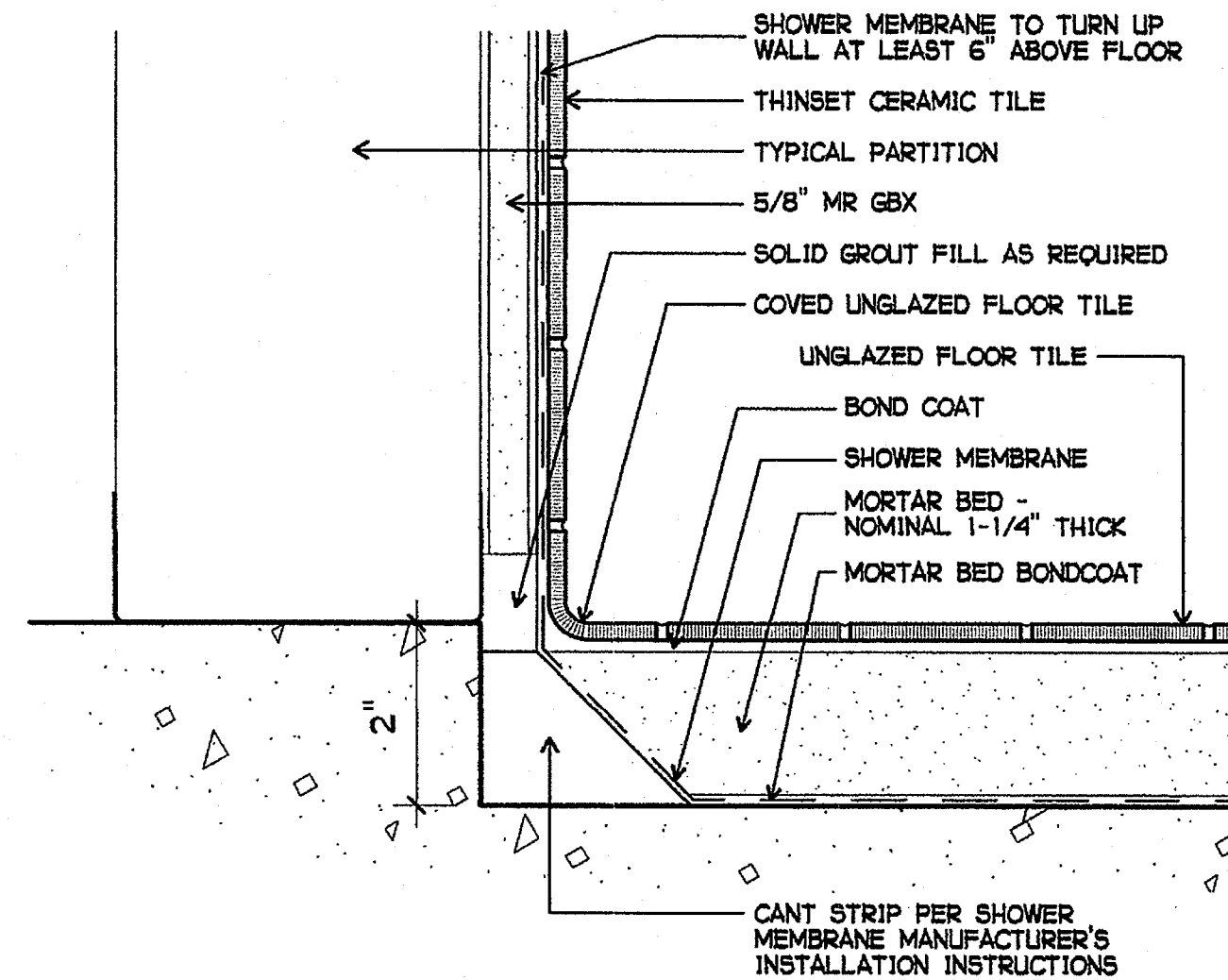
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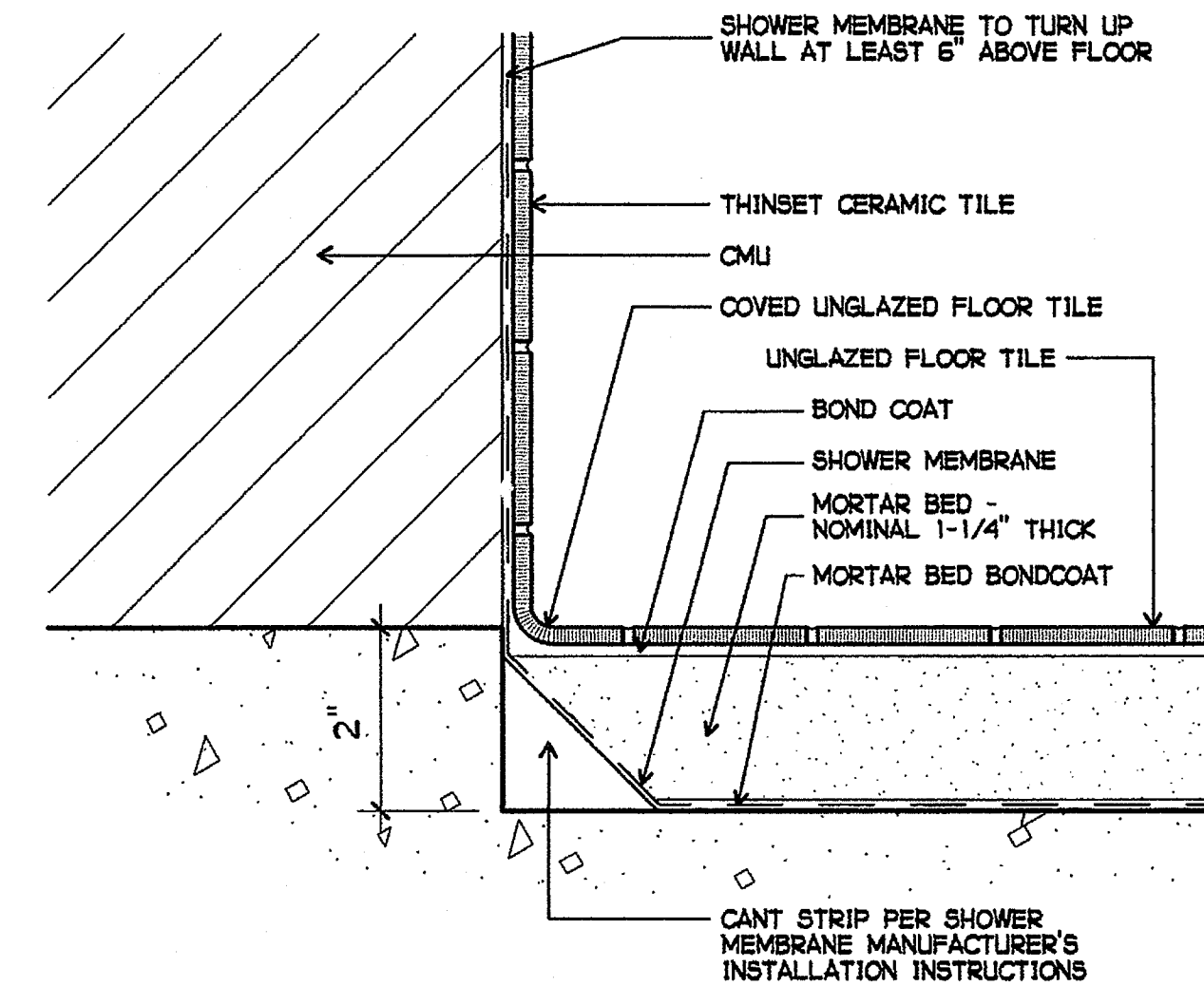
1 CERAMIC TILE BASE AND FLOOR
6" = 1'-0"



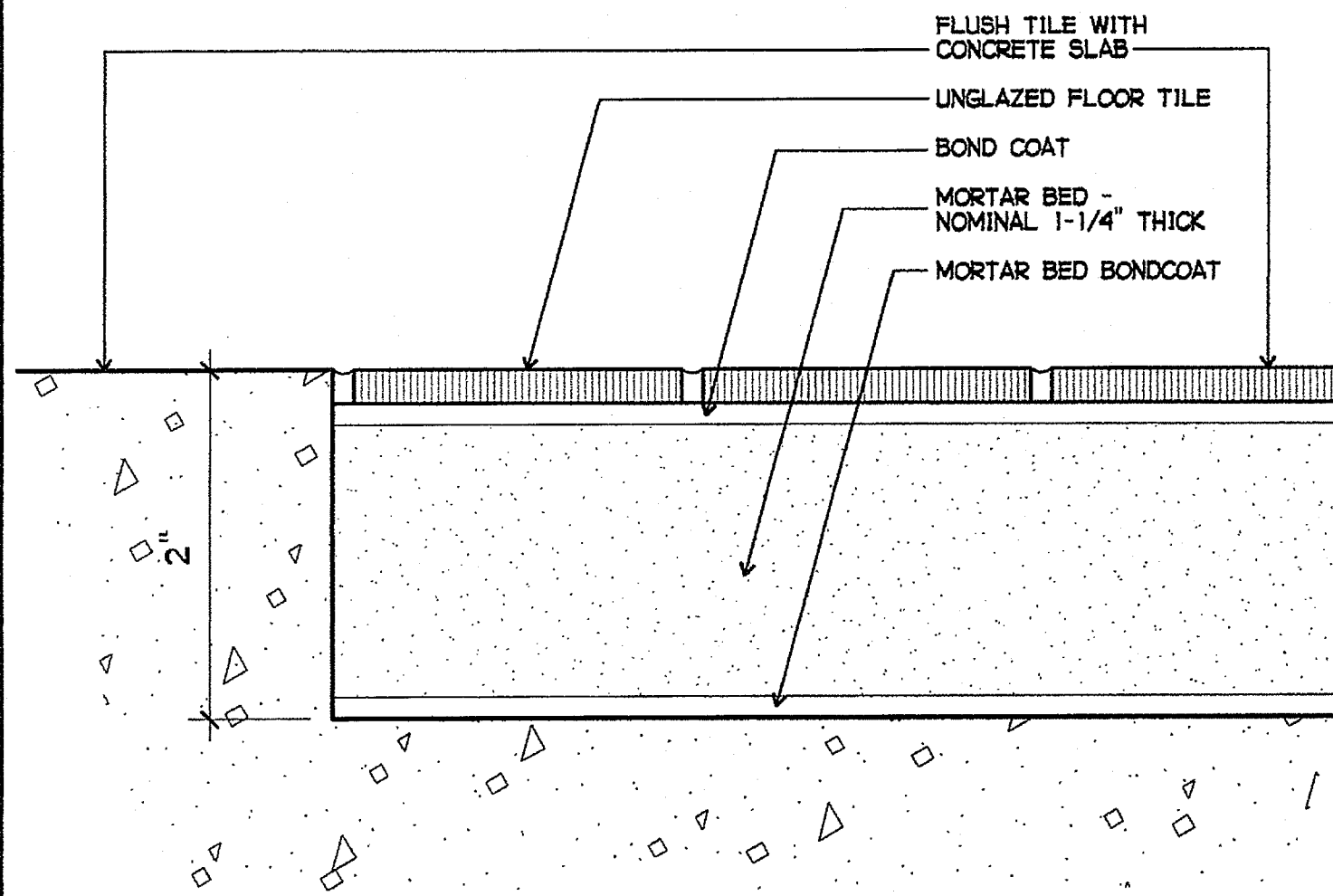
2 CERAMIC TILE BASE AND FLOOR AT MASONRY CONSTRUCTION
6" = 1'-0"



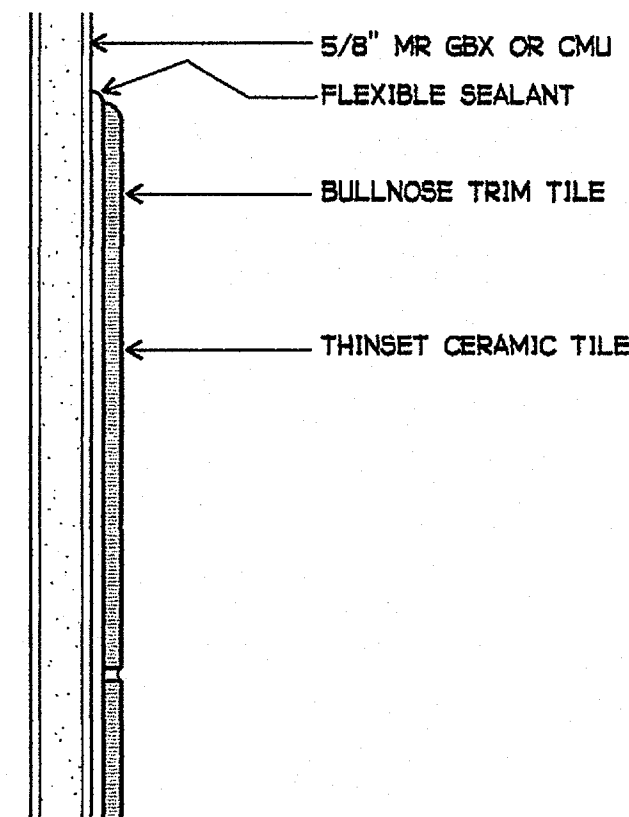
3 CERAMIC TILE BASE AND FLOOR W/ SHOWER MEMBRANE
6" = 1'-0"



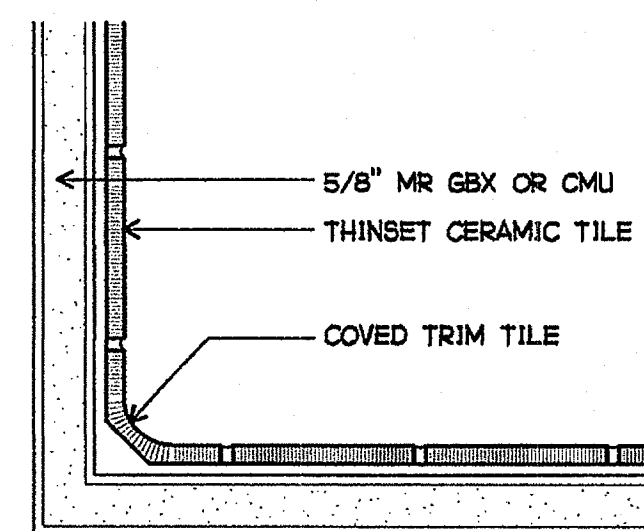
4 CERAMIC TILE BASE AND FLOOR W/ SHOWER MEMBRANE AT MASONRY CONSTRUCTION
6" = 1'-0"



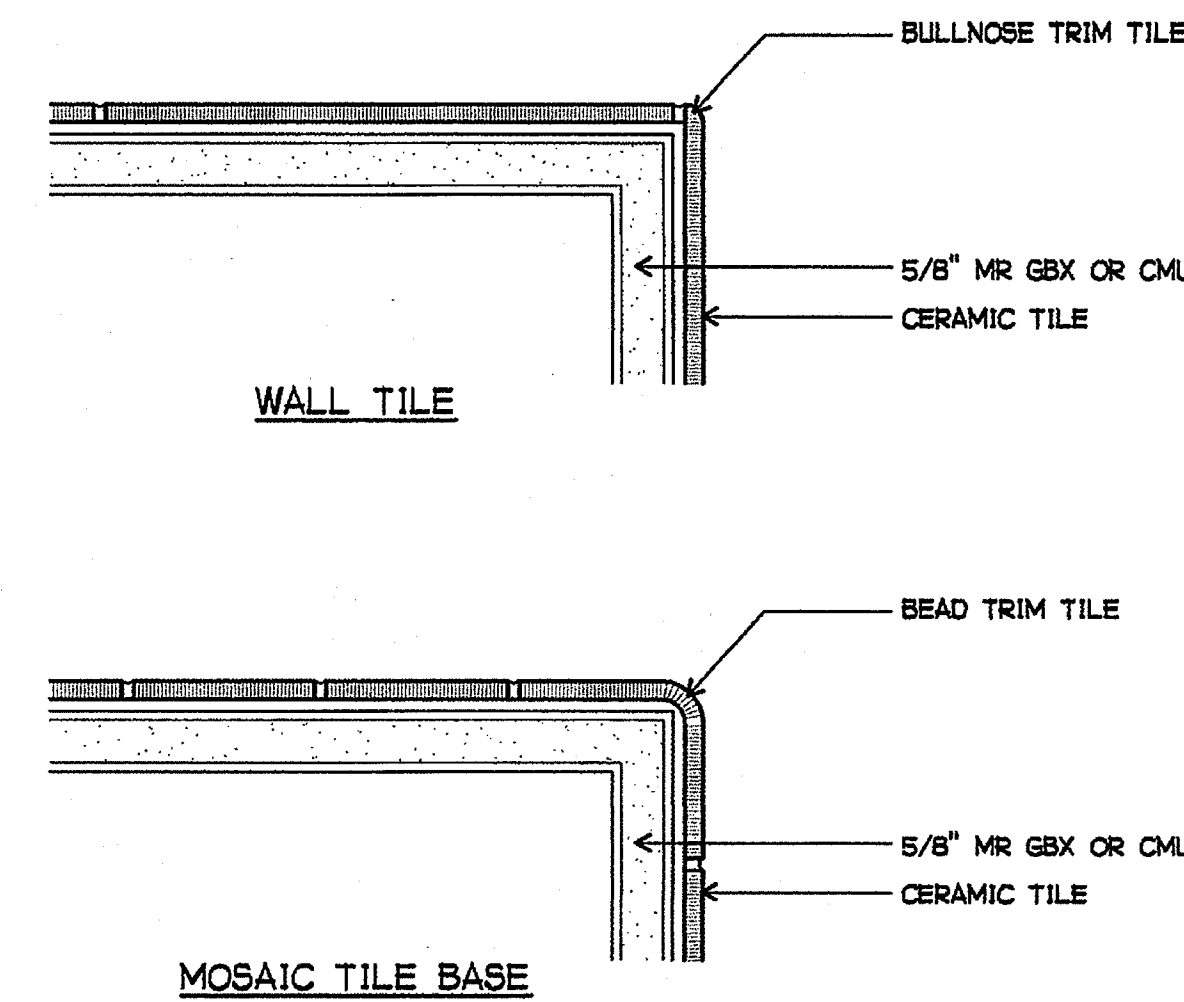
5 CERAMIC TILE TO CONCRETE FLOOR TRANSITION
FULL SCALE



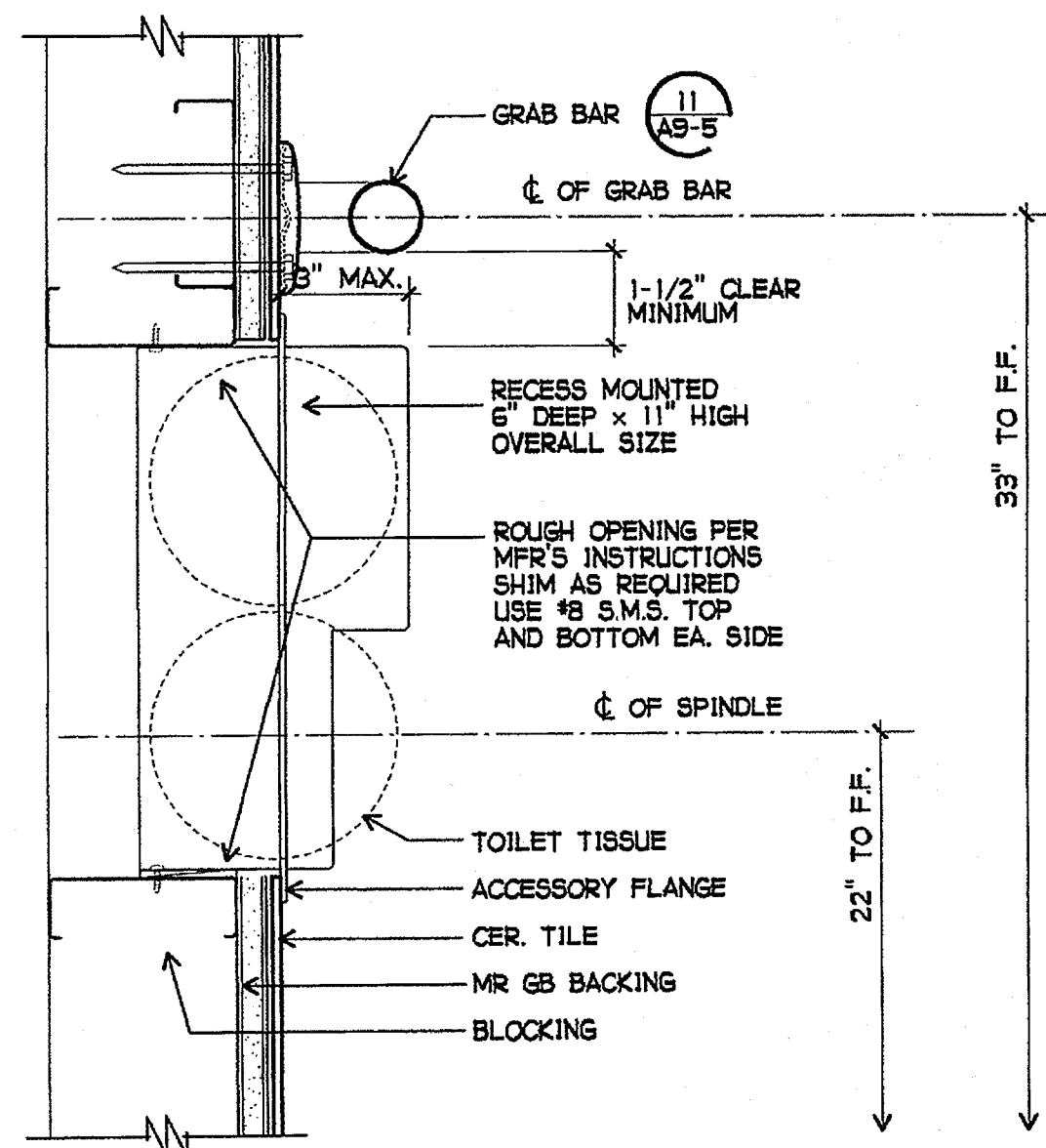
6 TOP OF CERAMIC TILE WAINGCOT
6" = 1'-0"



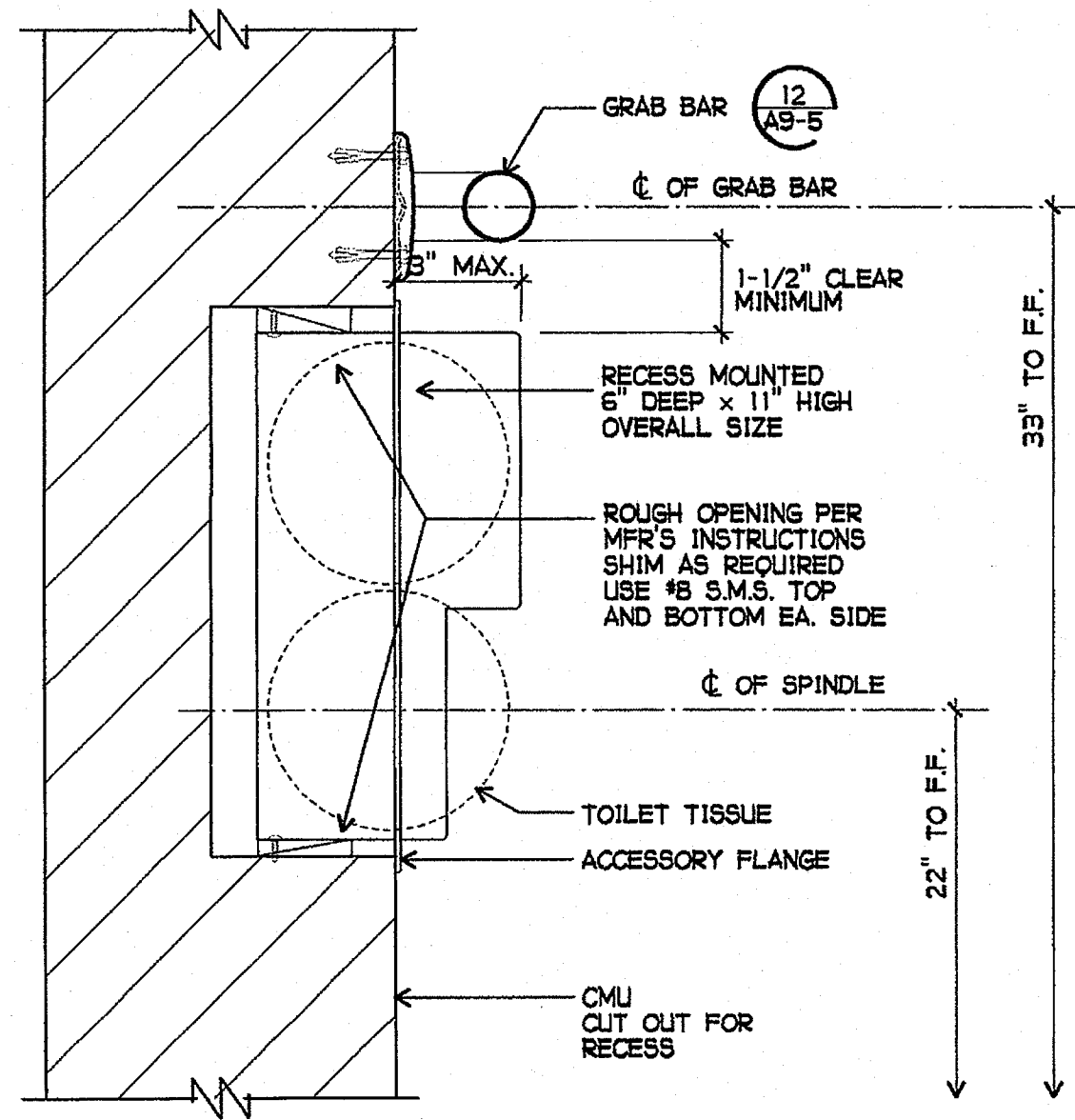
7 CERAMIC TILE INSIDE CORNER AT BASE
6" = 1'-0"



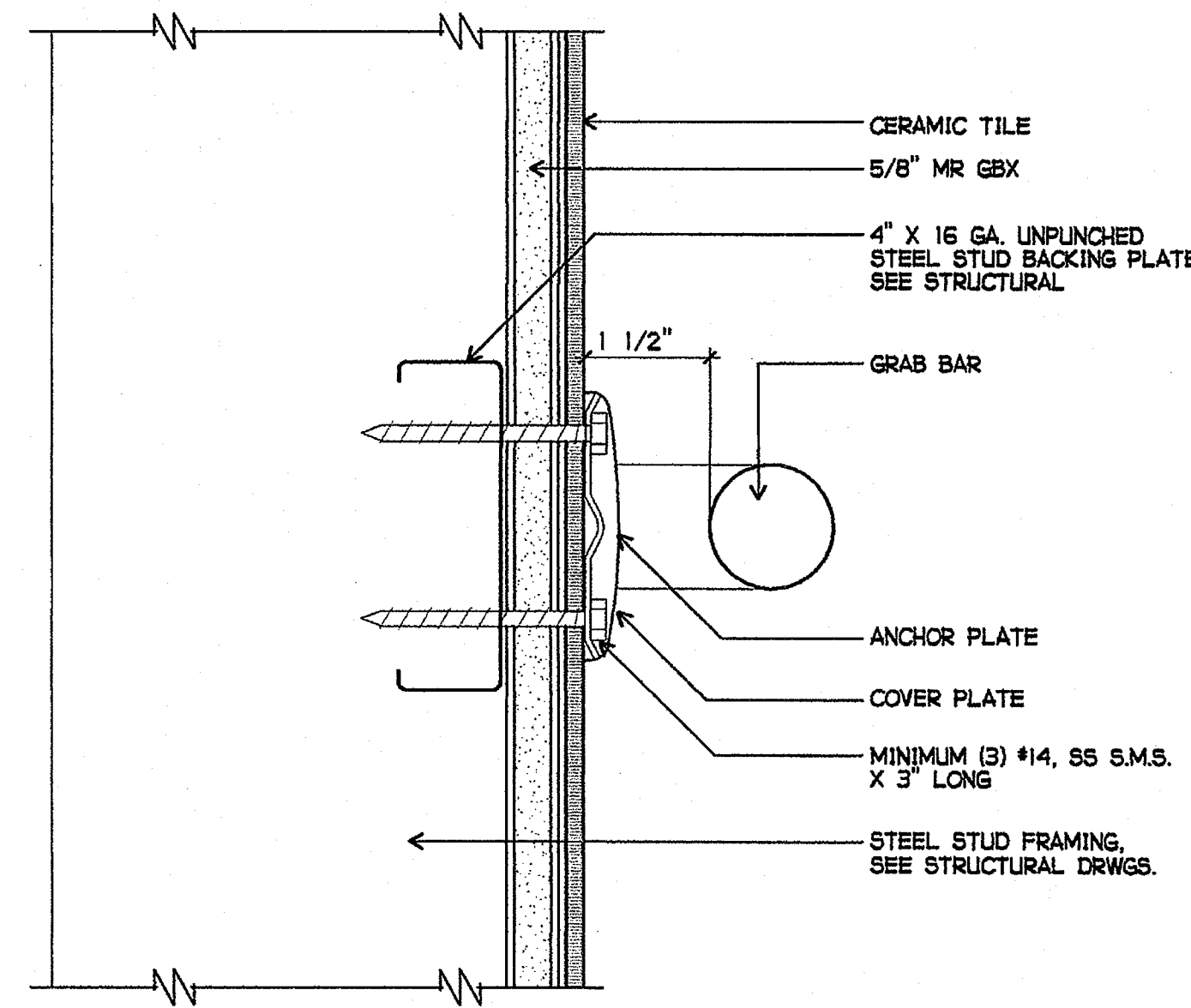
8 CERAMIC TILE OUTSIDE CORNER AT BASE AND WALL
6" = 1'-0"



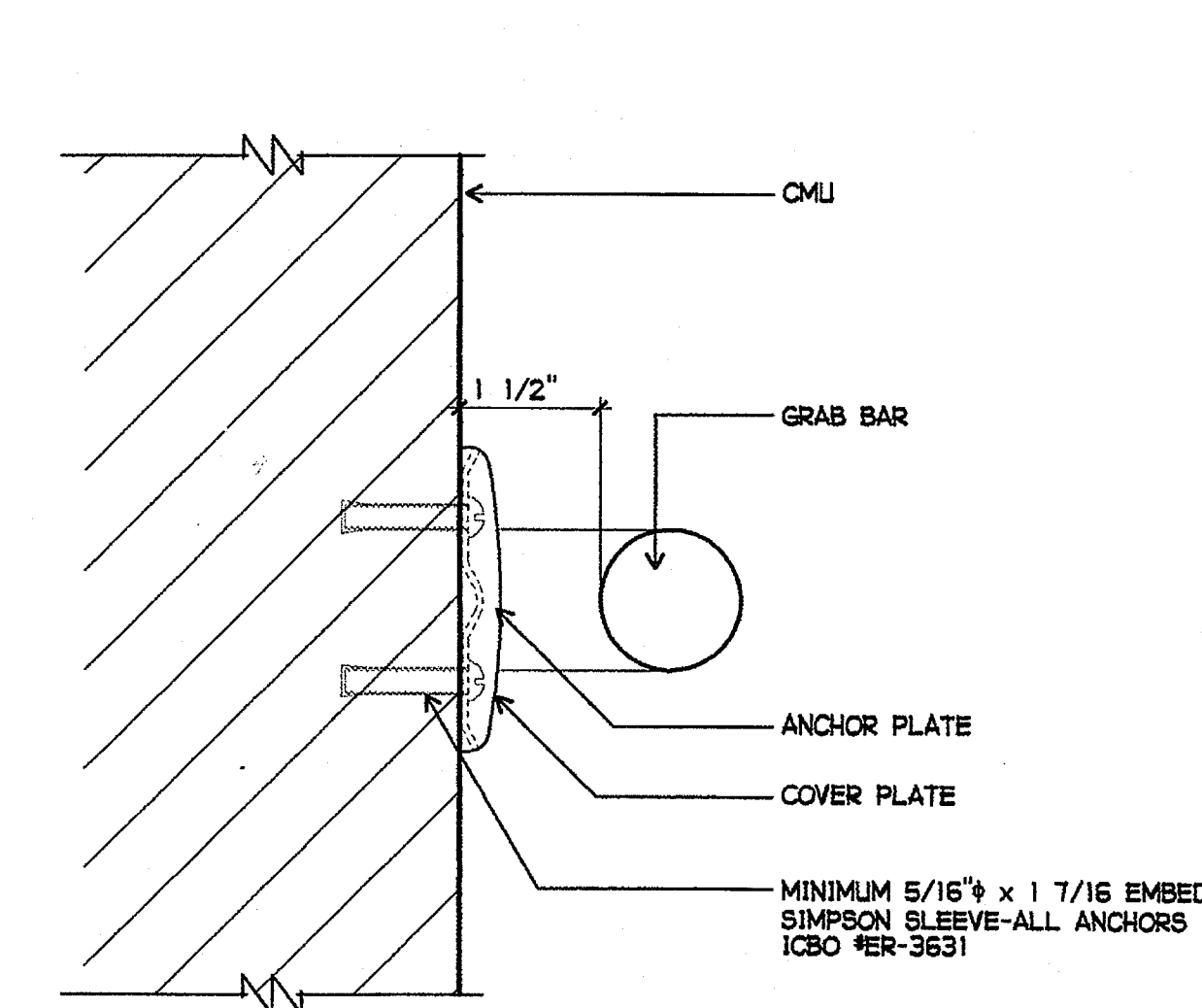
9 ACCESSIBLE TOILET TISSUE DISPENSER
3" = 1'-0"



10 ACCESSIBLE TOILET TISSUE DISPENSER AT MASONRY CONSTRUCTION
3" = 1'-0"



11 GRAB BAR ATTACHMENT
6" = 1'-0"



12 GRAB BAR ATTACHMENT AT MASONRY CONSTRUCTION
6" = 1'-0"

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DLD

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FAX 760-754-8291

SUITE 234
92054

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PROJECT NOS. 025

CUISD NO. 758-000

P. T. N. 73569-9

DATE

REVISIONS

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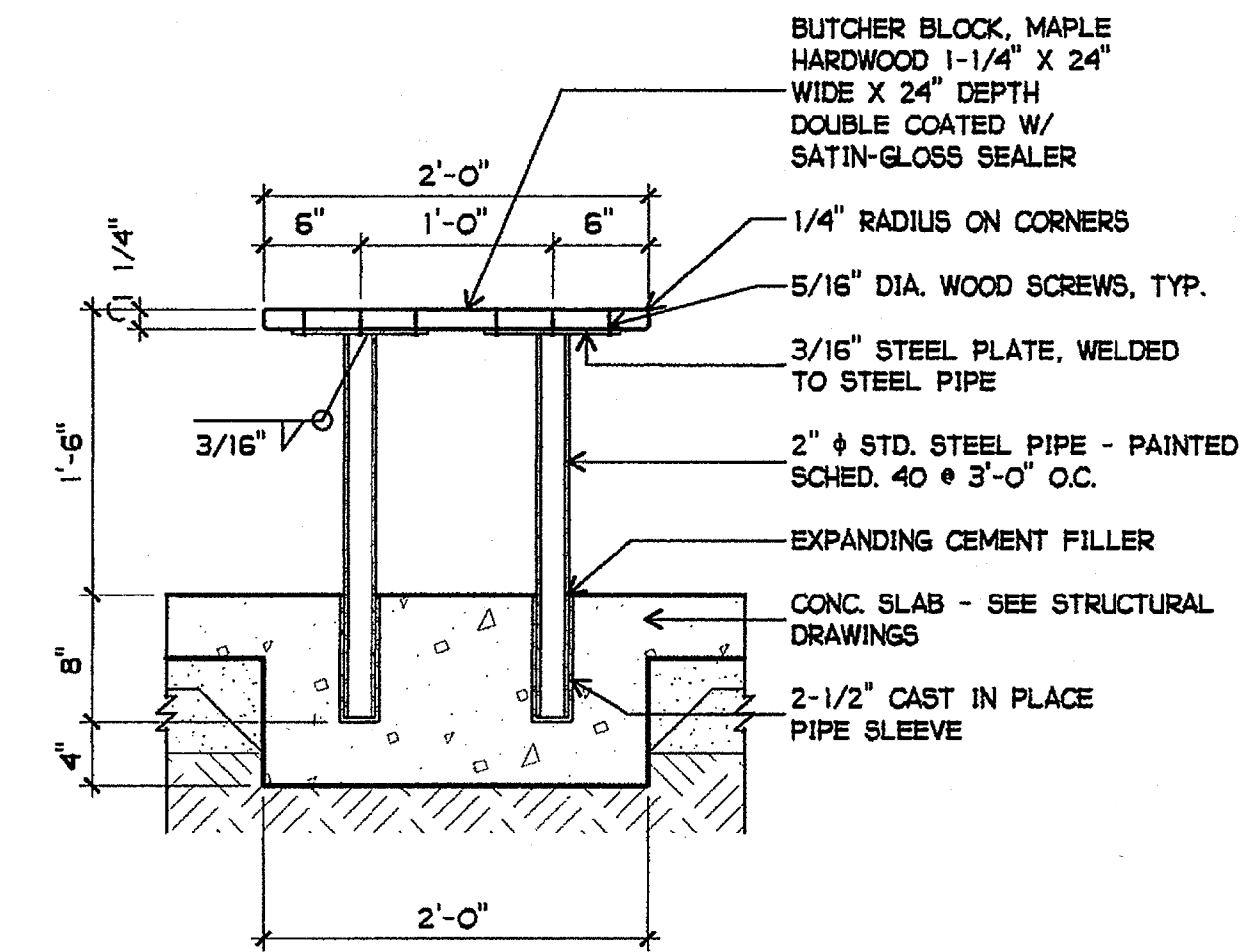
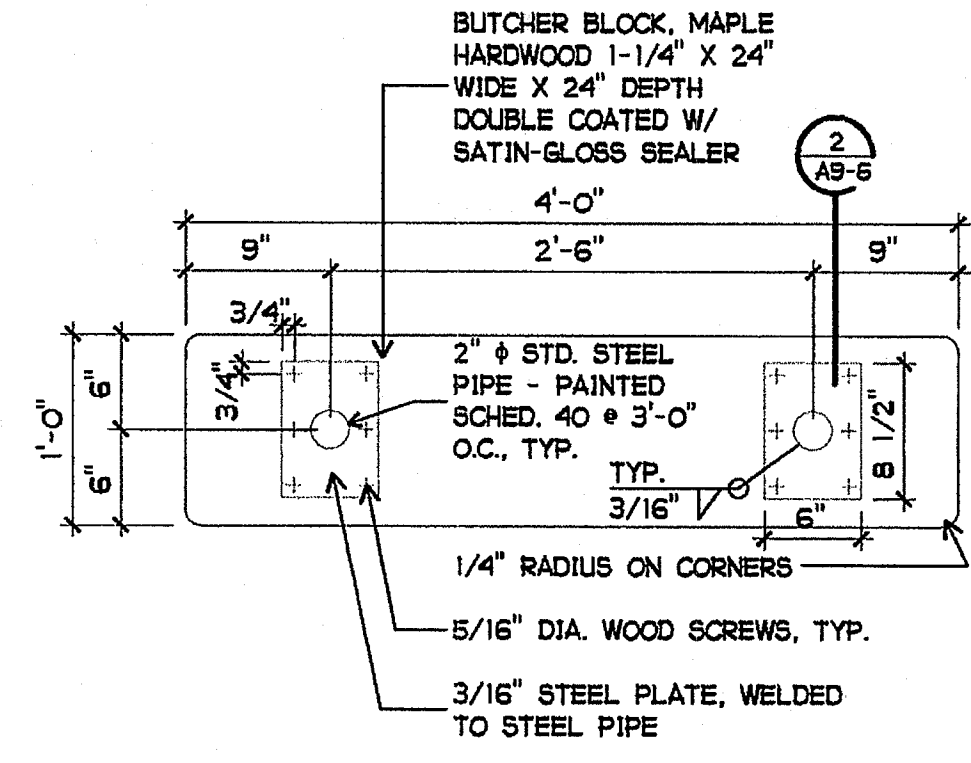
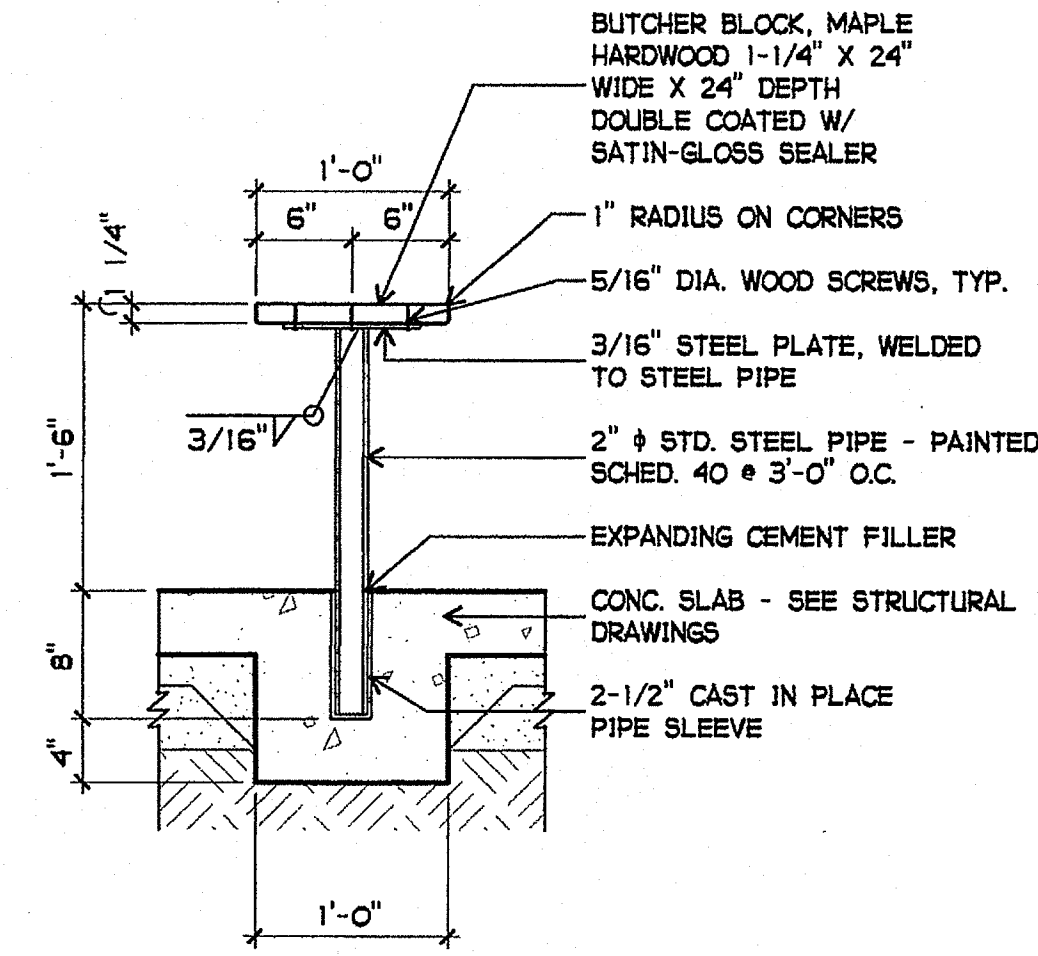
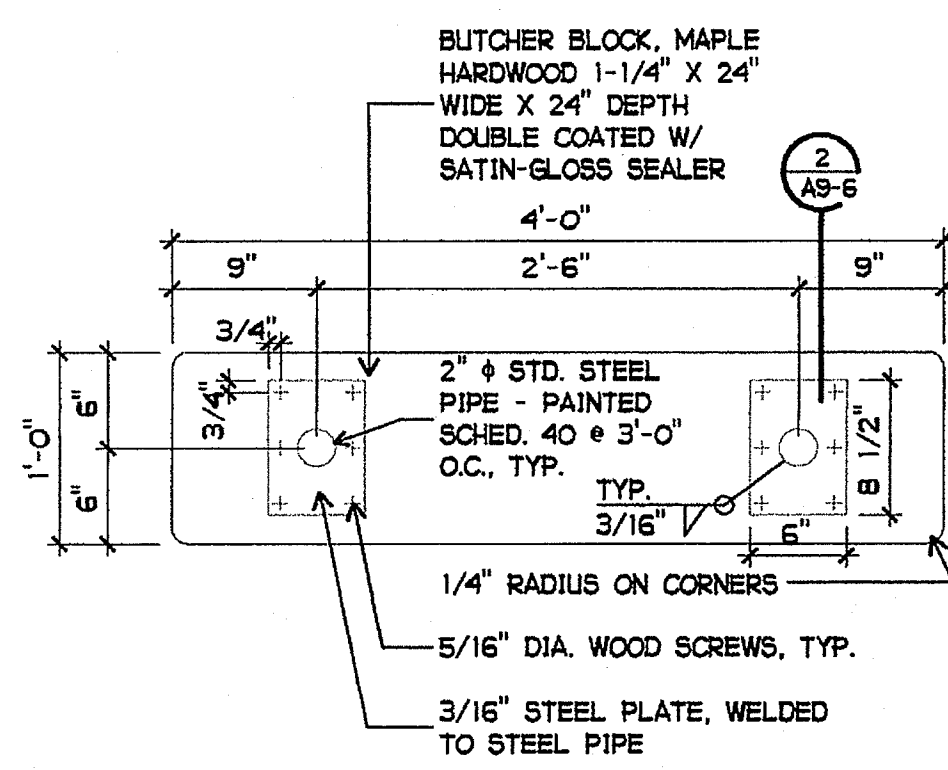
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JOHN SCOTT GROTH
C-26609
4/30/2007
RENEWAL
STATE OF CALIFORNIA

SHEET TITLE
TILE RESTROOM
DETAILS

A9-5

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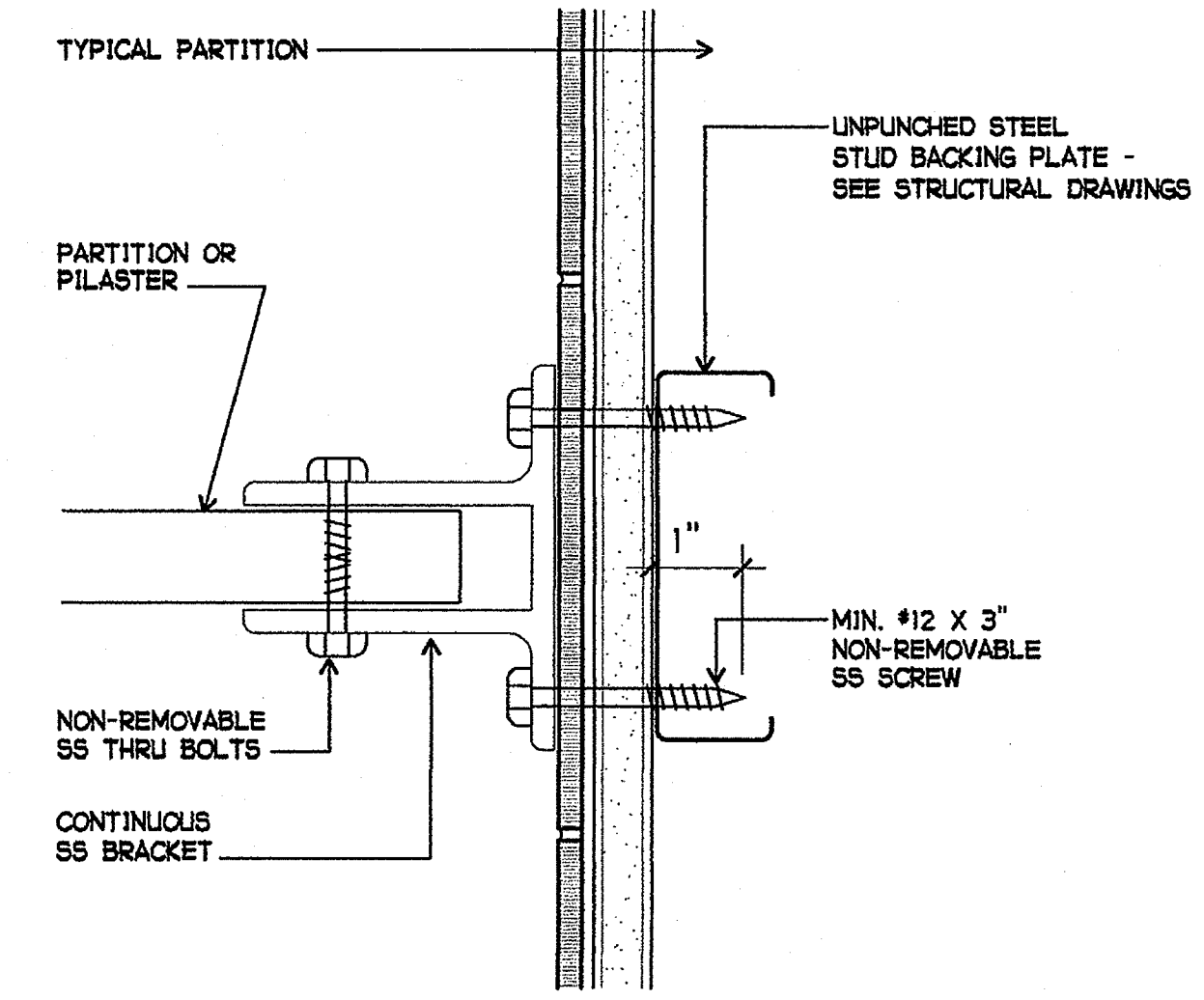
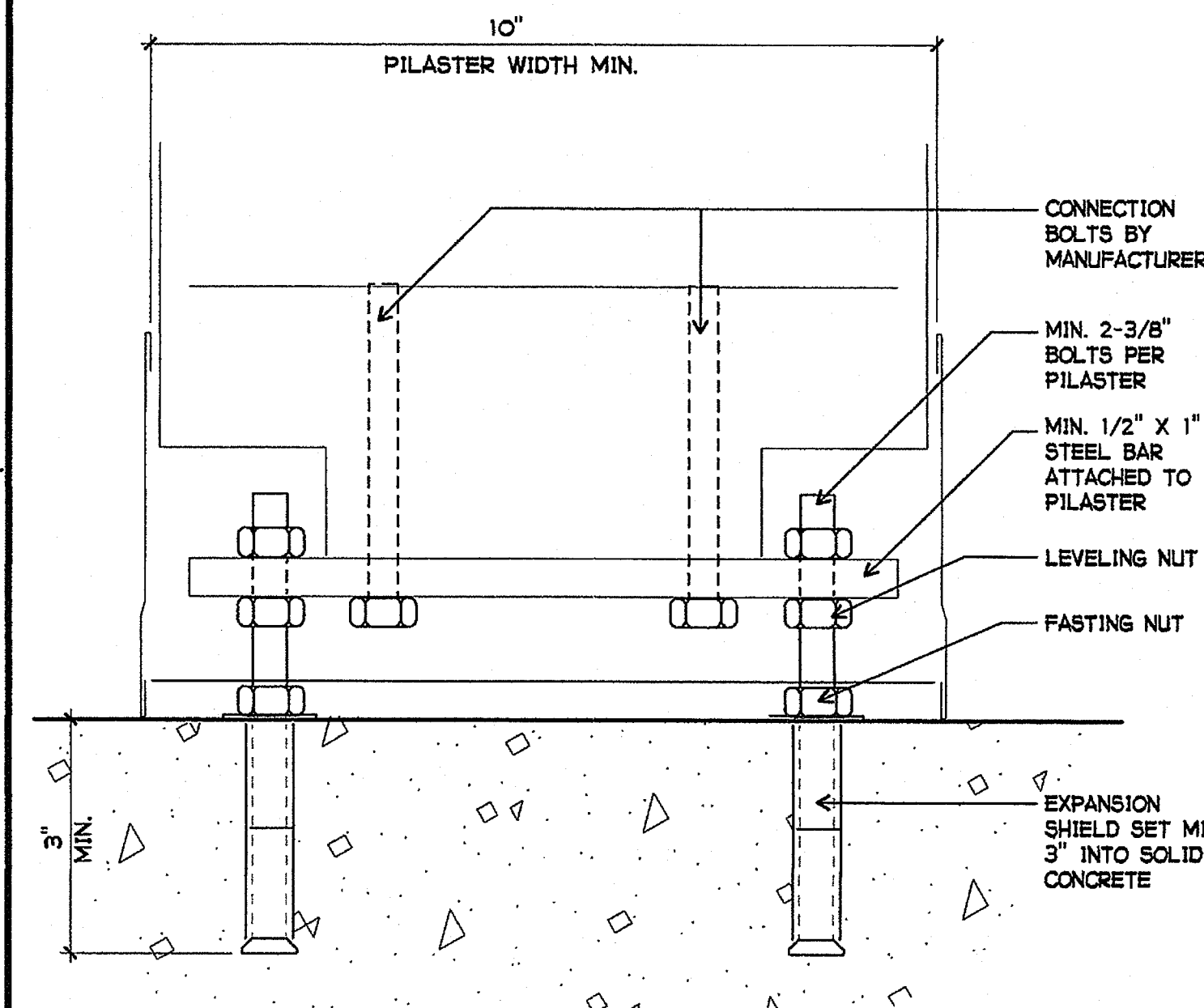
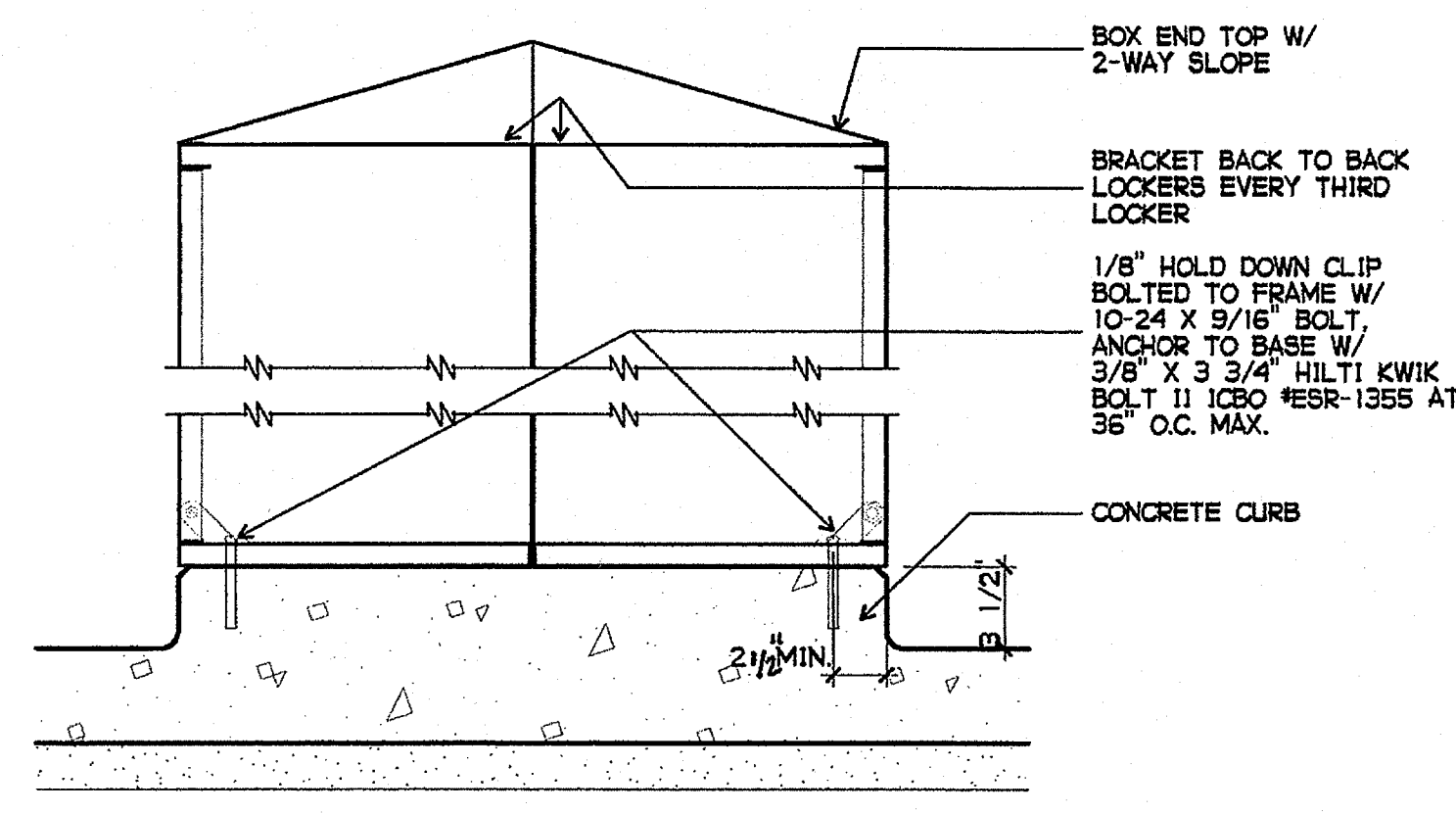
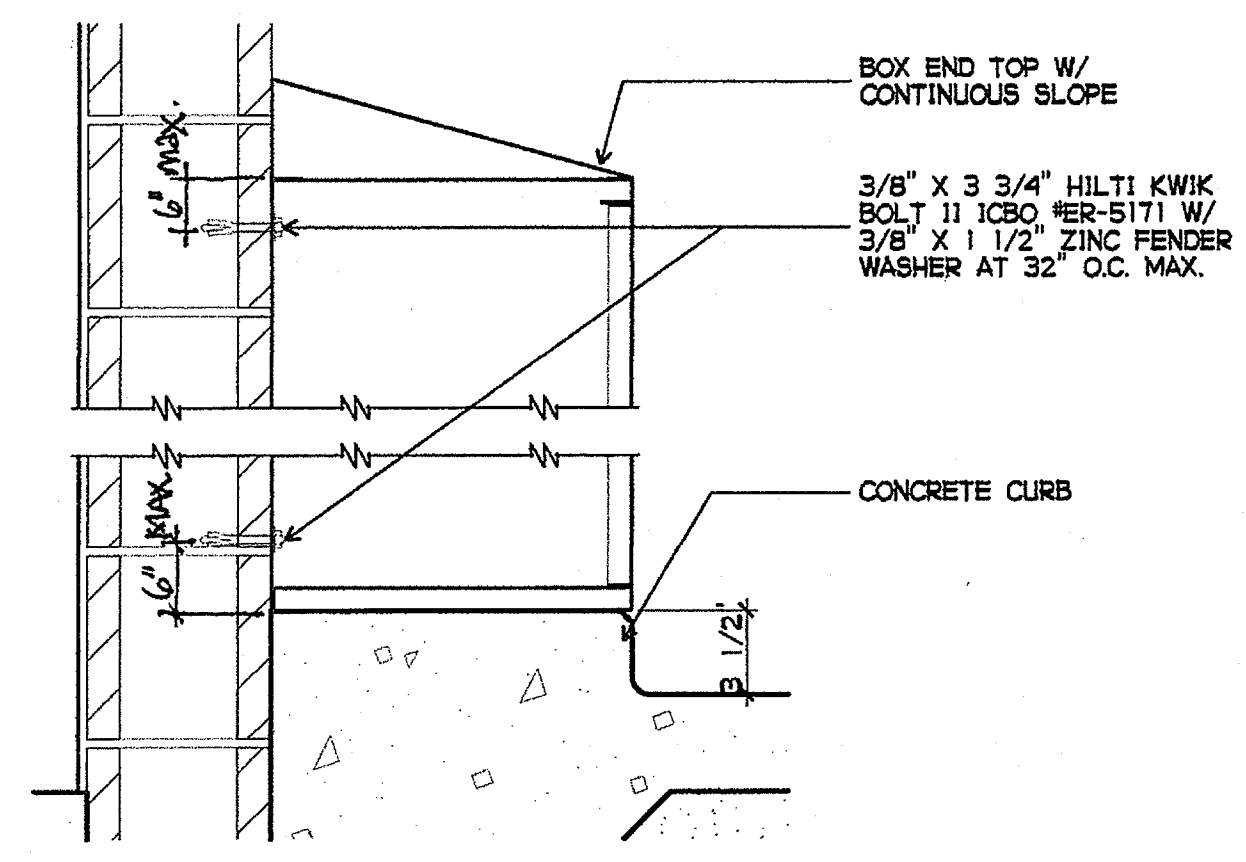


1 12' X 48' LOCKER BENCH
1" = 1'-0"

2 12' X 48' LOCKER BENCH
1" = 1'-0"

3 24' X 48' LOCKER BENCH
1" = 1'-0"

4 24' X 48' LOCKER BENCH
1" = 1'-0"

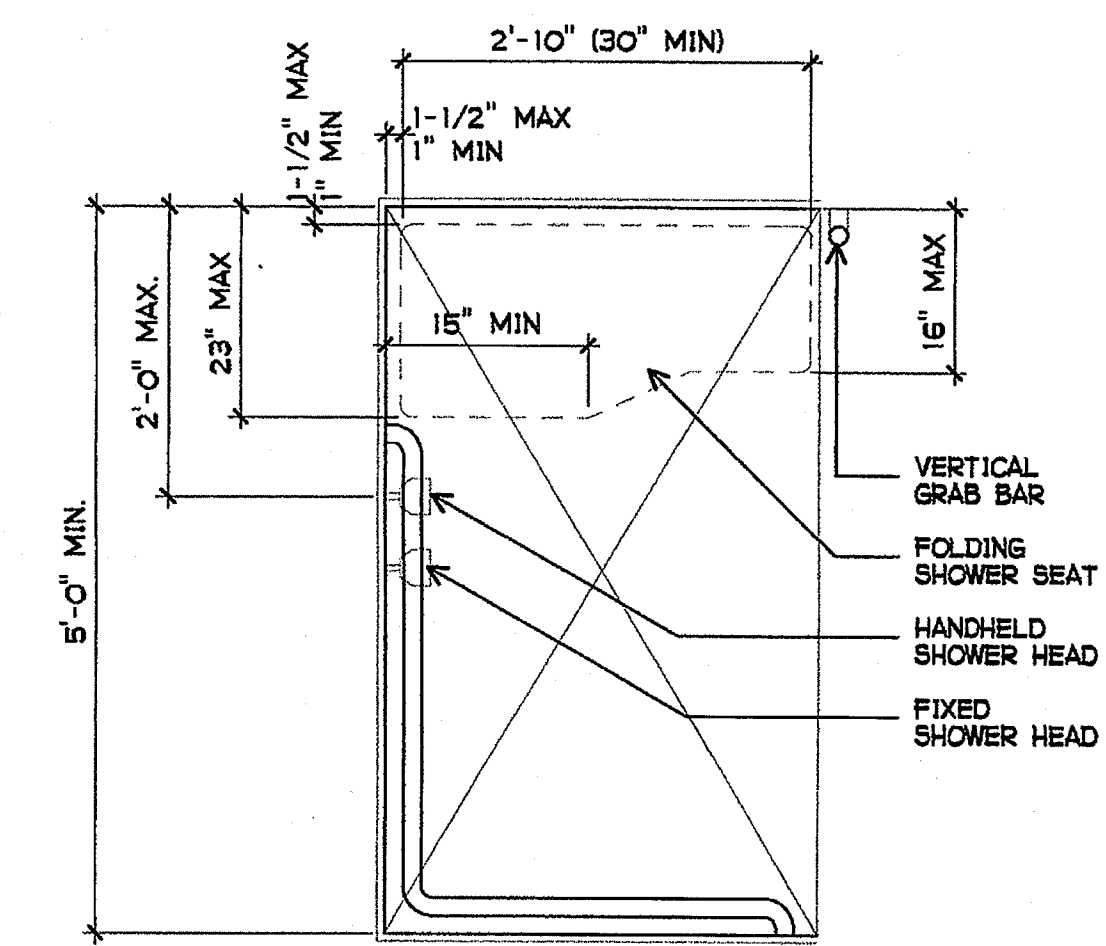
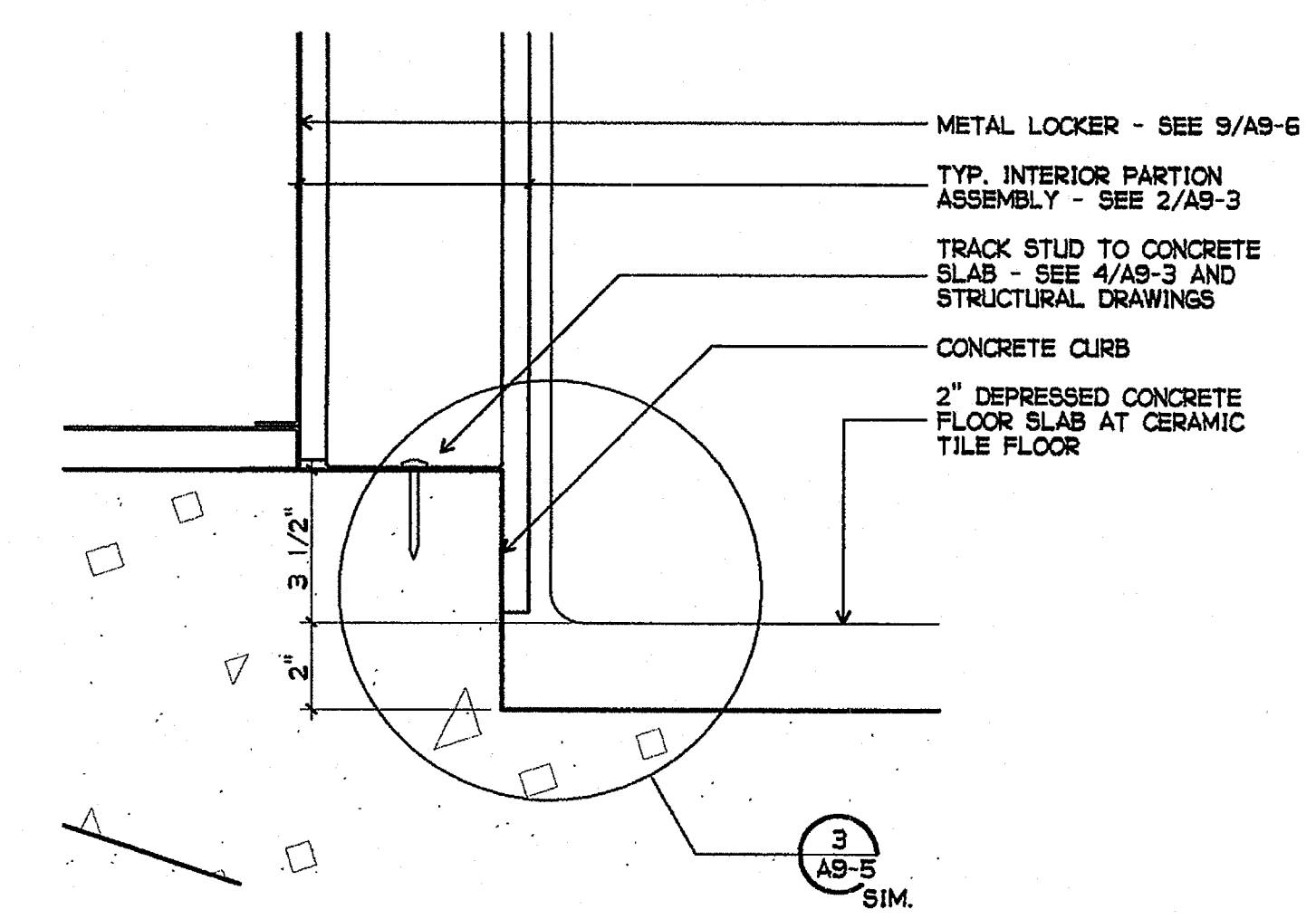
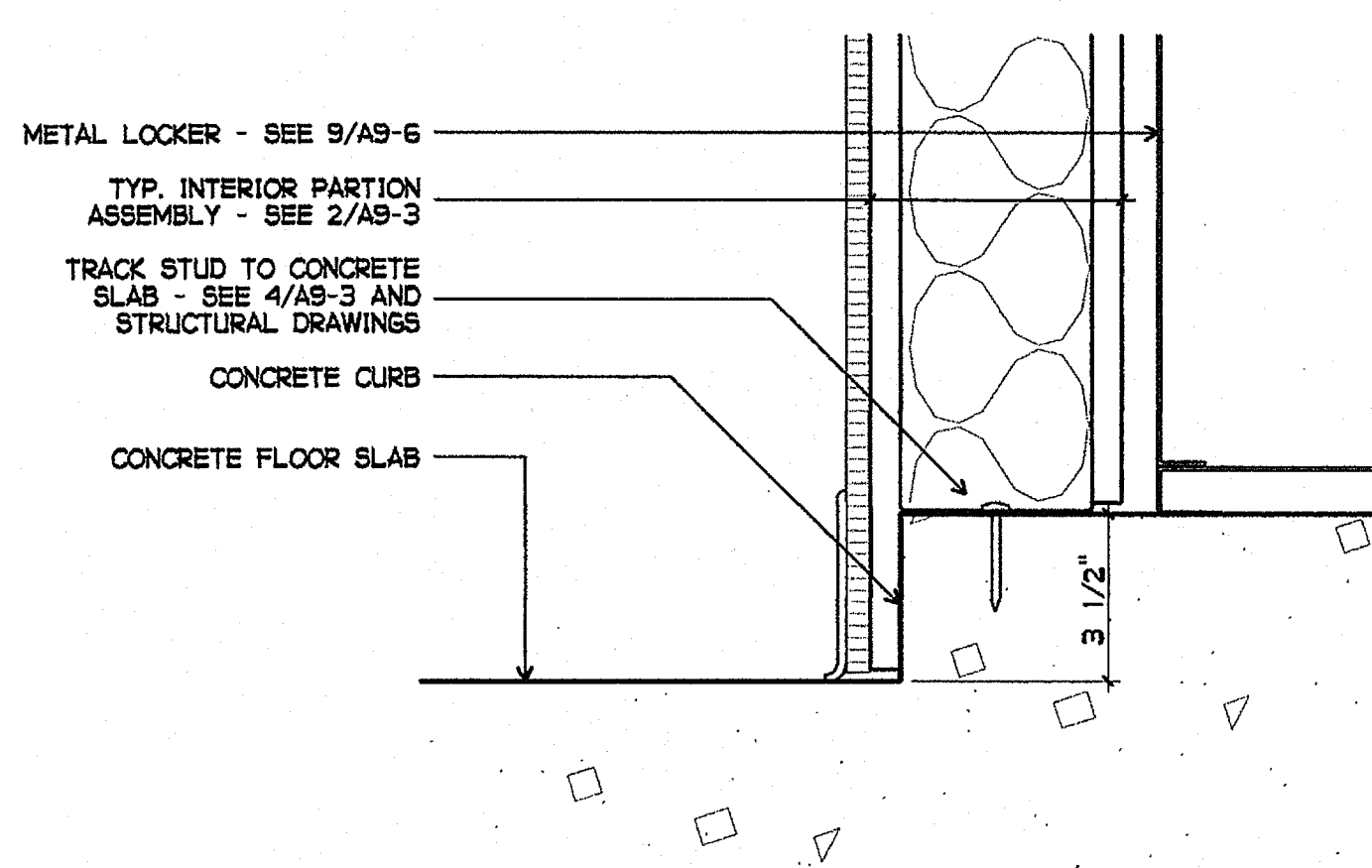
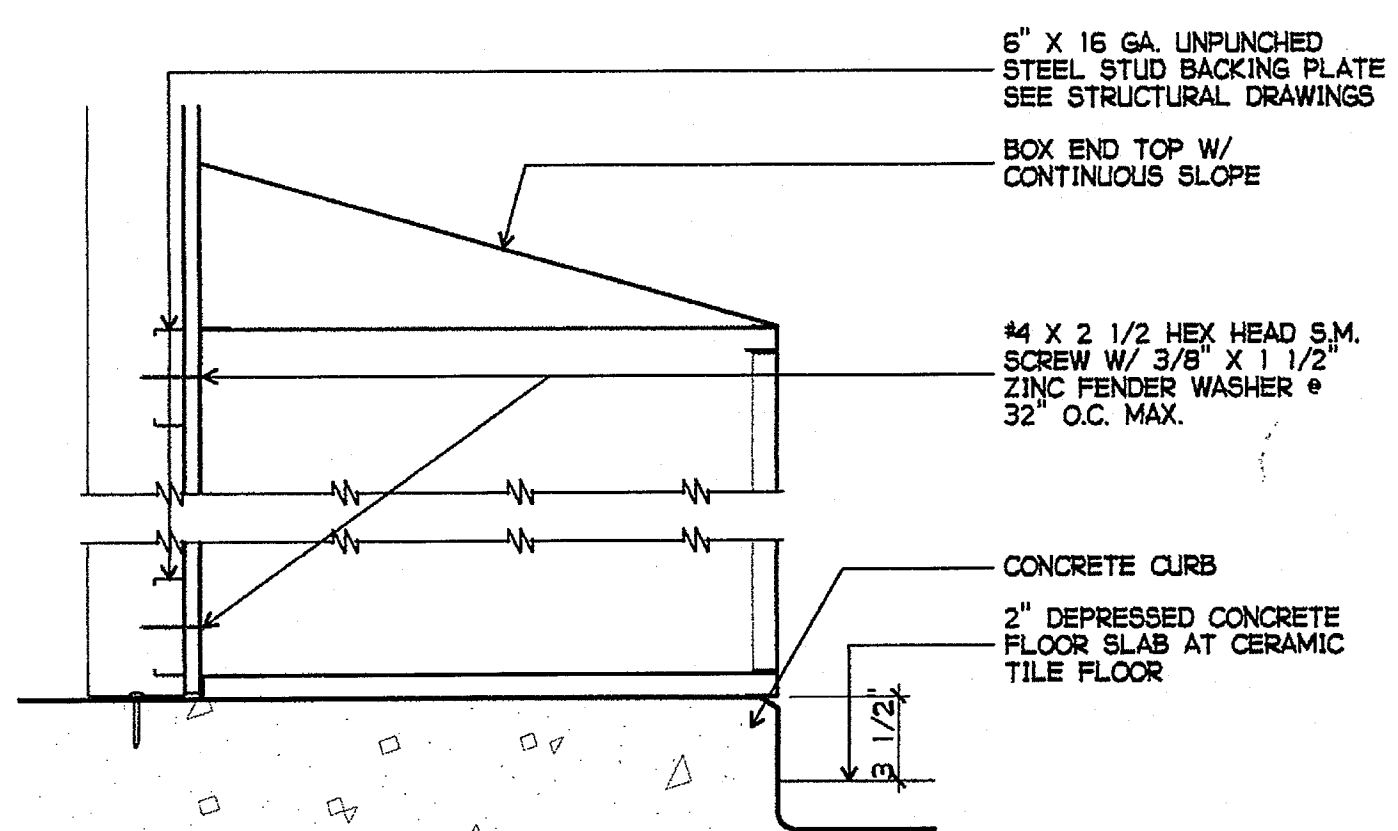


5 SINGLE ROW LOCKER AND CURB
1-1/2" = 1'-0"

6 DOUBLE ROW LOCKER AND CURB
1-1/2" = 1'-0"

7 TOILET PARTITION FLOOR CONNECTION
6" = 1'-0"

8 TOILET PARTITION WALL CONNECTION
6" = 1'-0"



9 FACULTY LOCKER AND CURB
1-1/2" = 1'-0"

10 FACULTY LOCKER CURB
3" = 1'-0"

11 FACULTY LOCKER CURB
3" = 1'-0"

12 ACCESSIBLE SHOWER PLAN
3/4" = 1'-0"

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CLUSD NO. 758-000

PROJECT NOS. 025

P. T. N. 73569-9

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4-106494

AC 16 FLN SS

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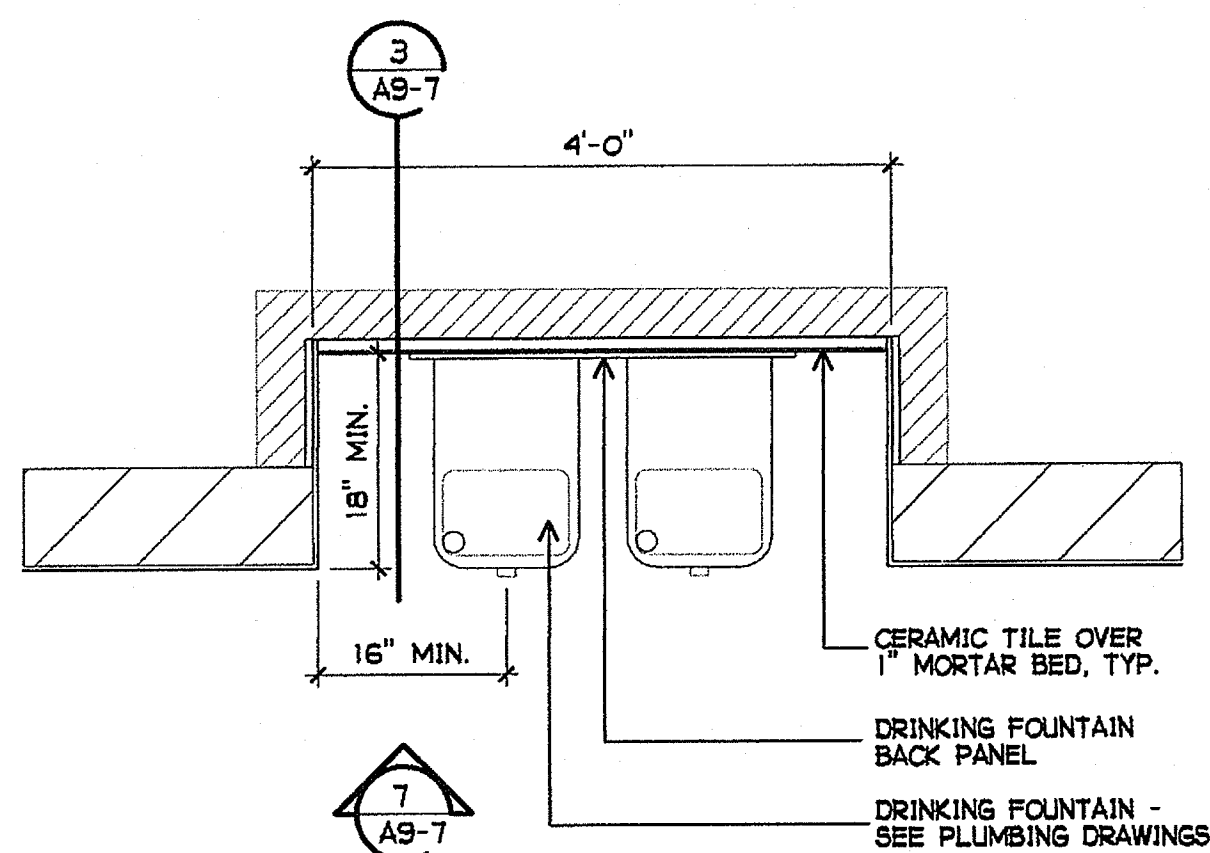
JOHN SCOTT BOB C-26609 4/30/2007 RENEWAL STATE OF CALIFORNIA

SHEET TITLE

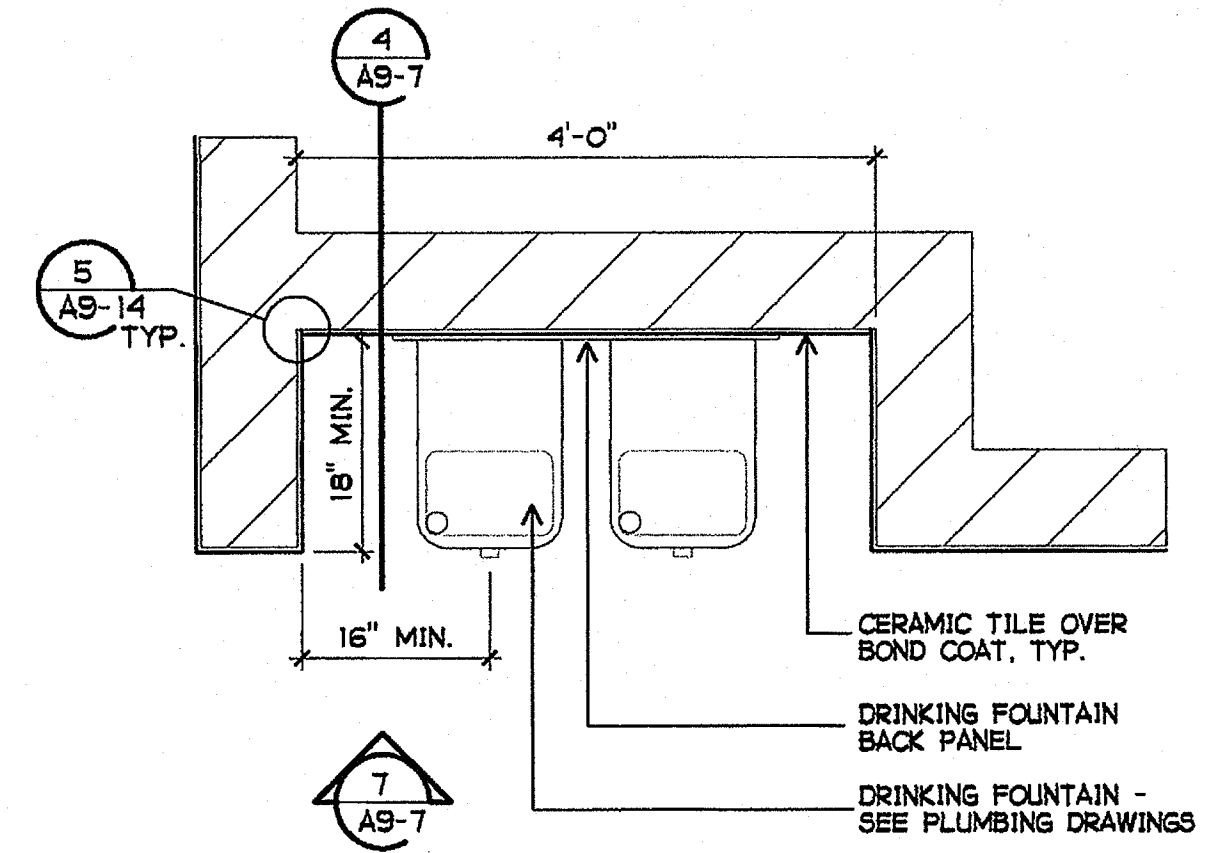
DETAILS

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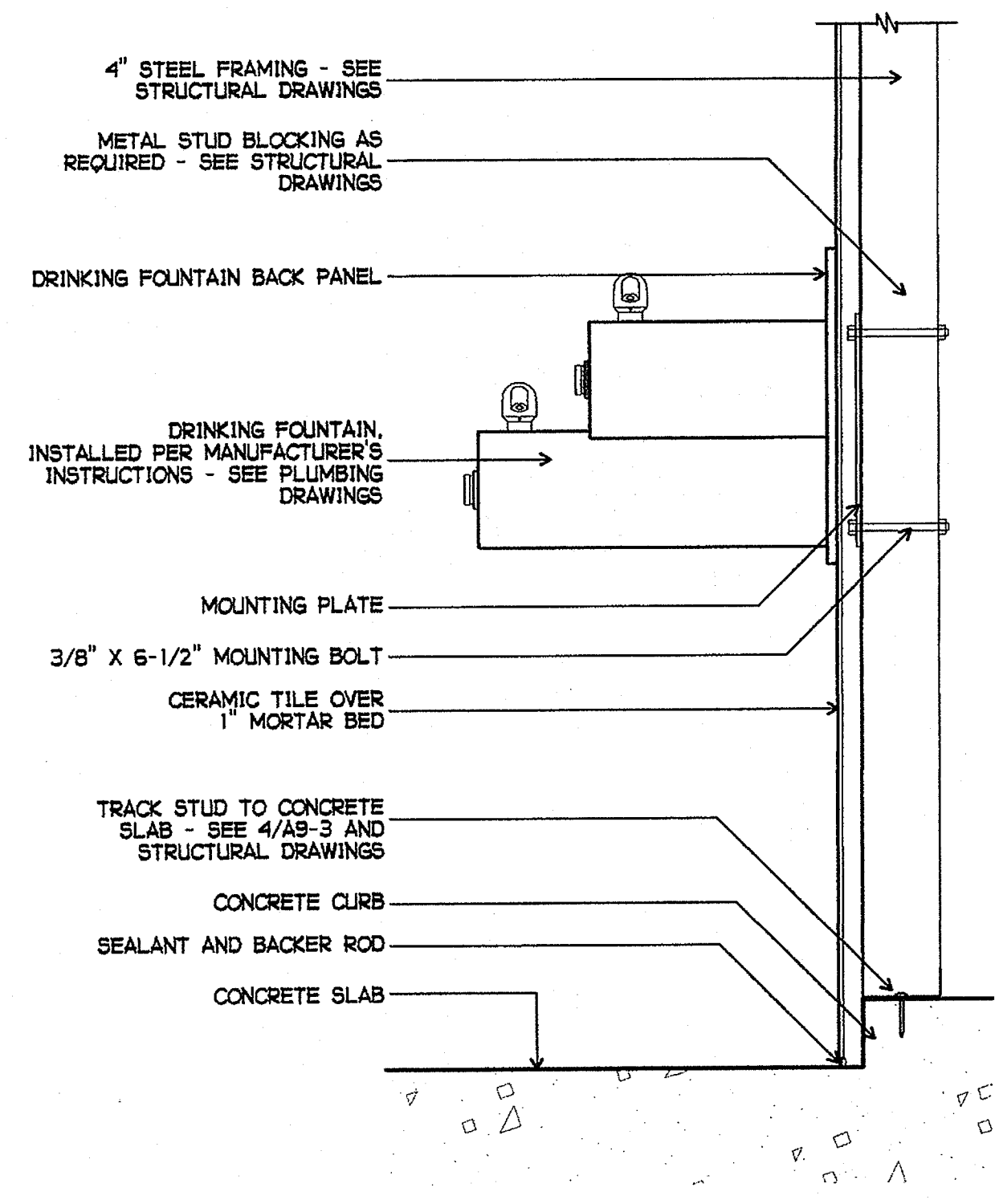
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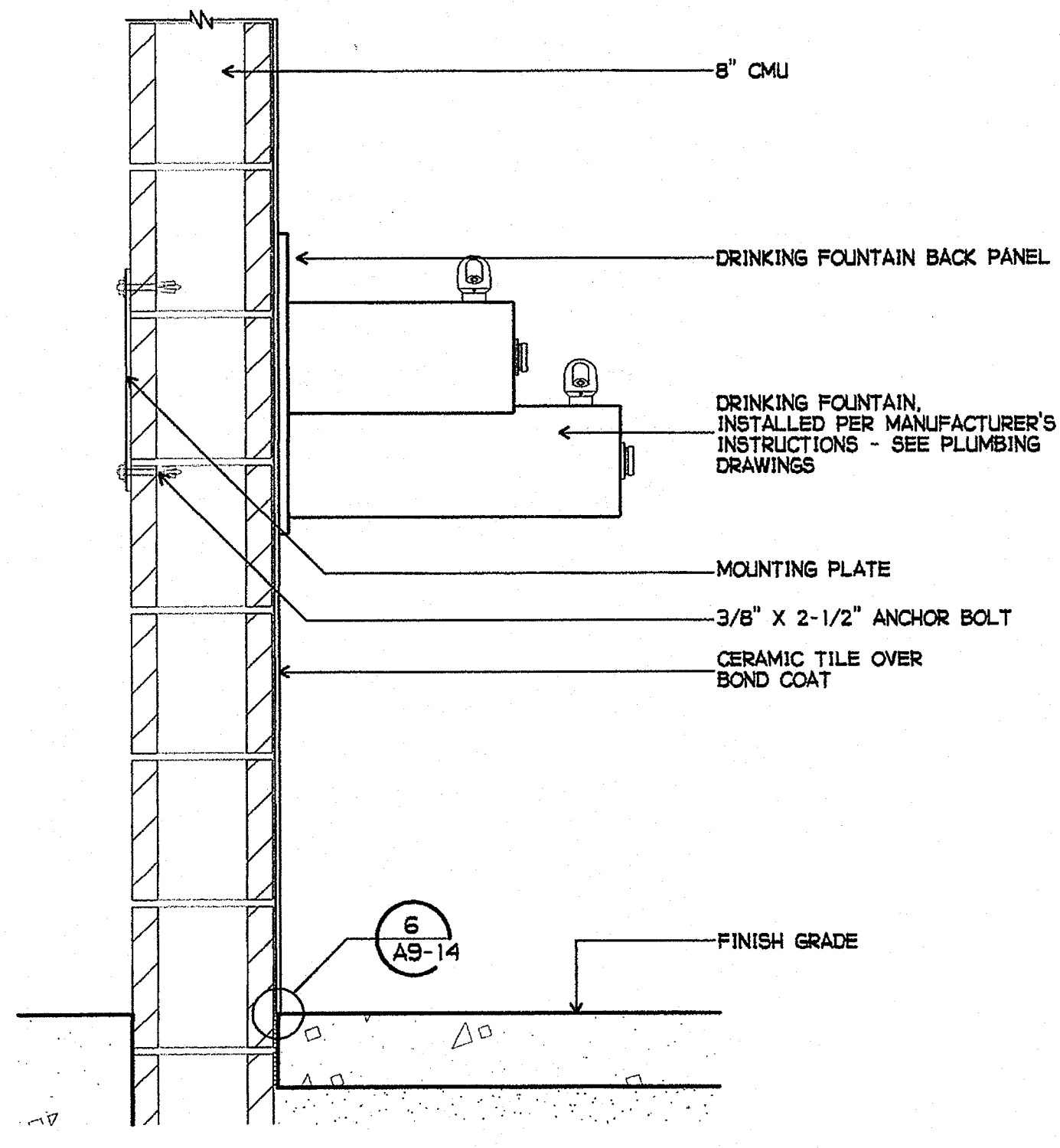
1 DRINKING FOUNTAIN
3/4" = 1'-0"



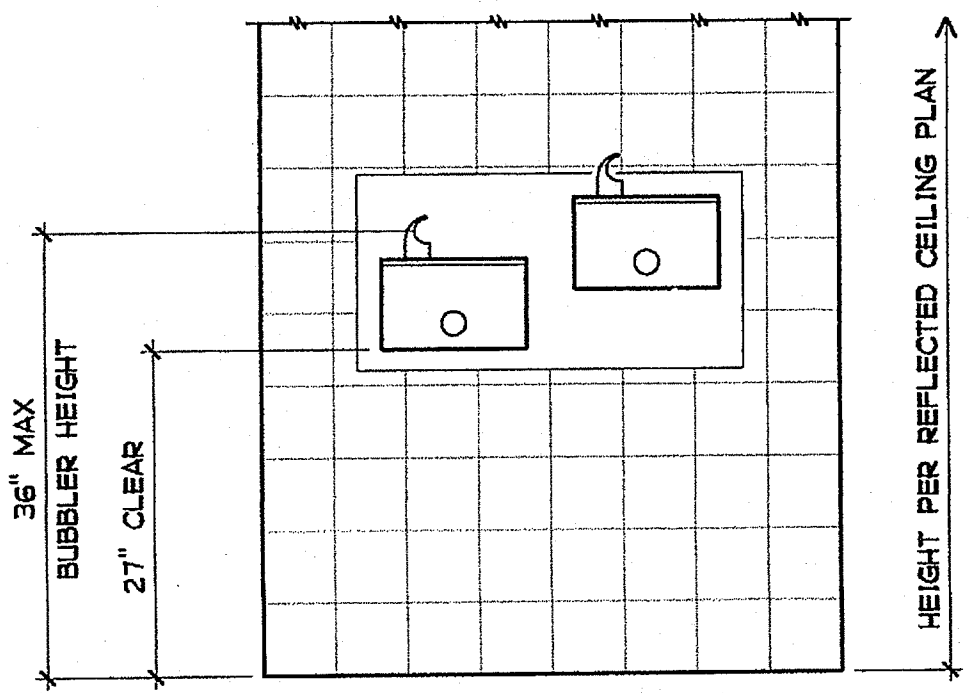
2 DRINKING FOUNTAIN
3/4" = 1'-0"



3 DRINKING FOUNTAIN
1-1/2" = 1'-0"

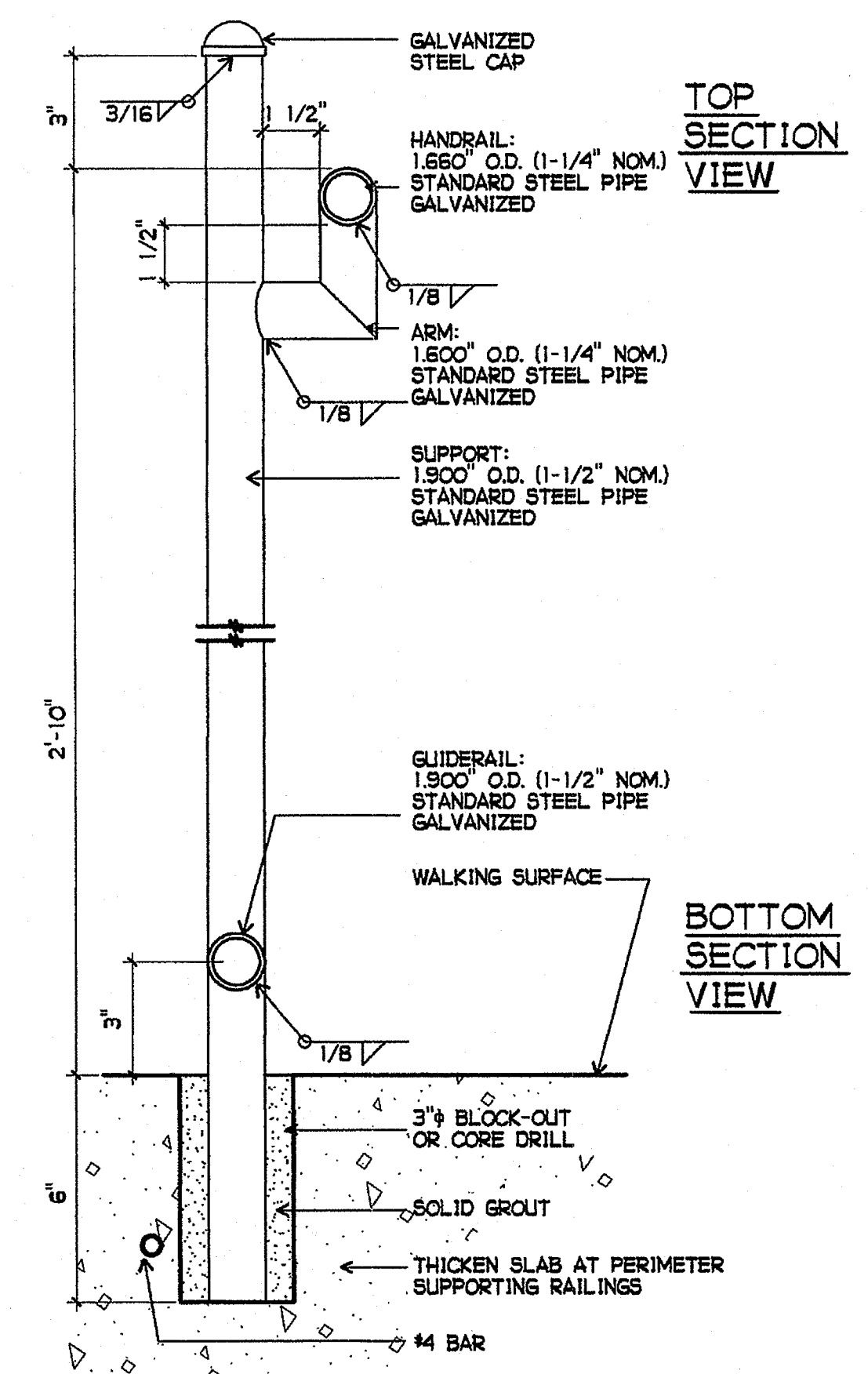


4 DRINKING FOUNTAIN
1-1/2" = 1'-0"

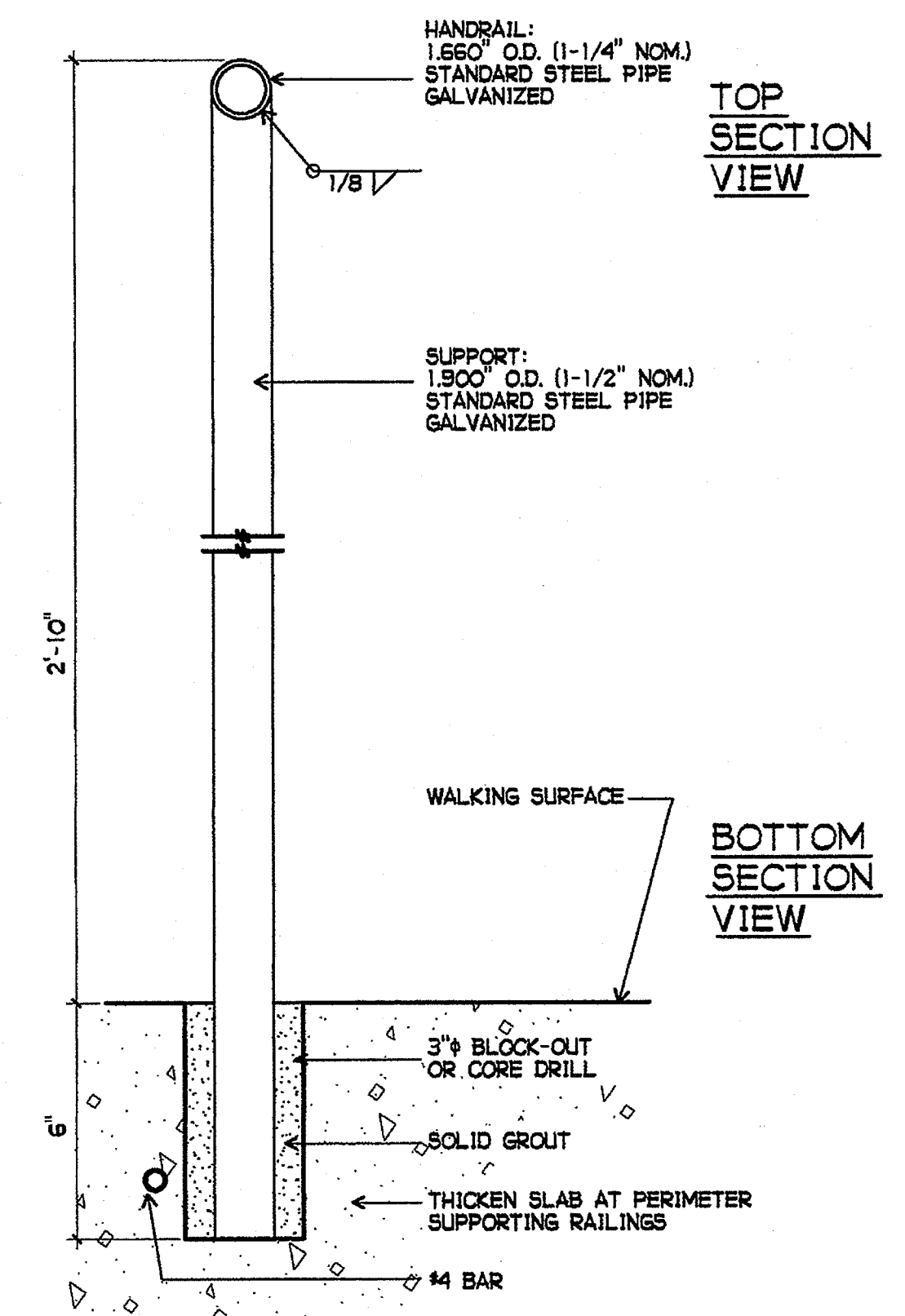


7 DRINKING FOUNTAIN
3/4" = 1'-0"

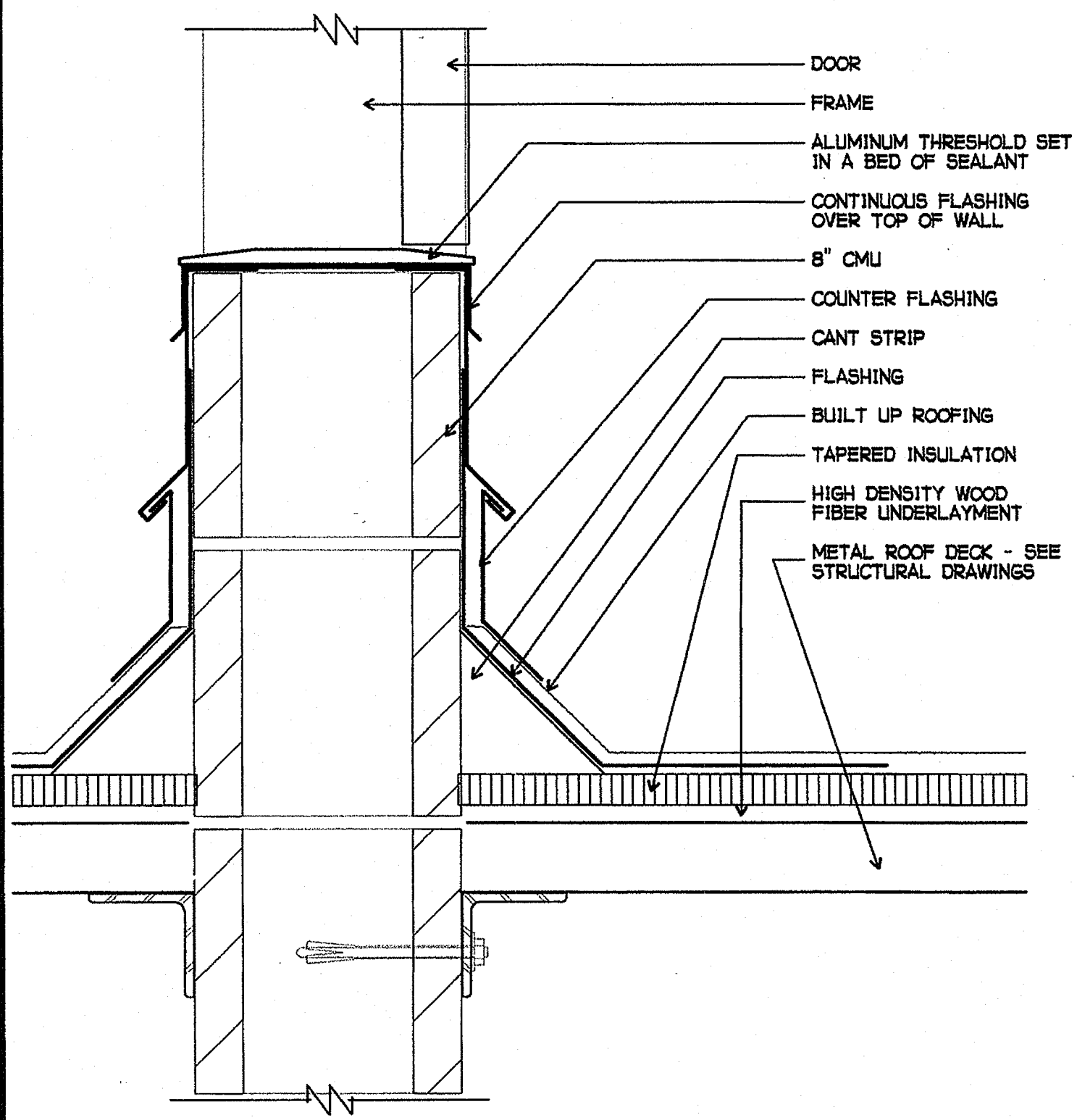
8 NOT USED



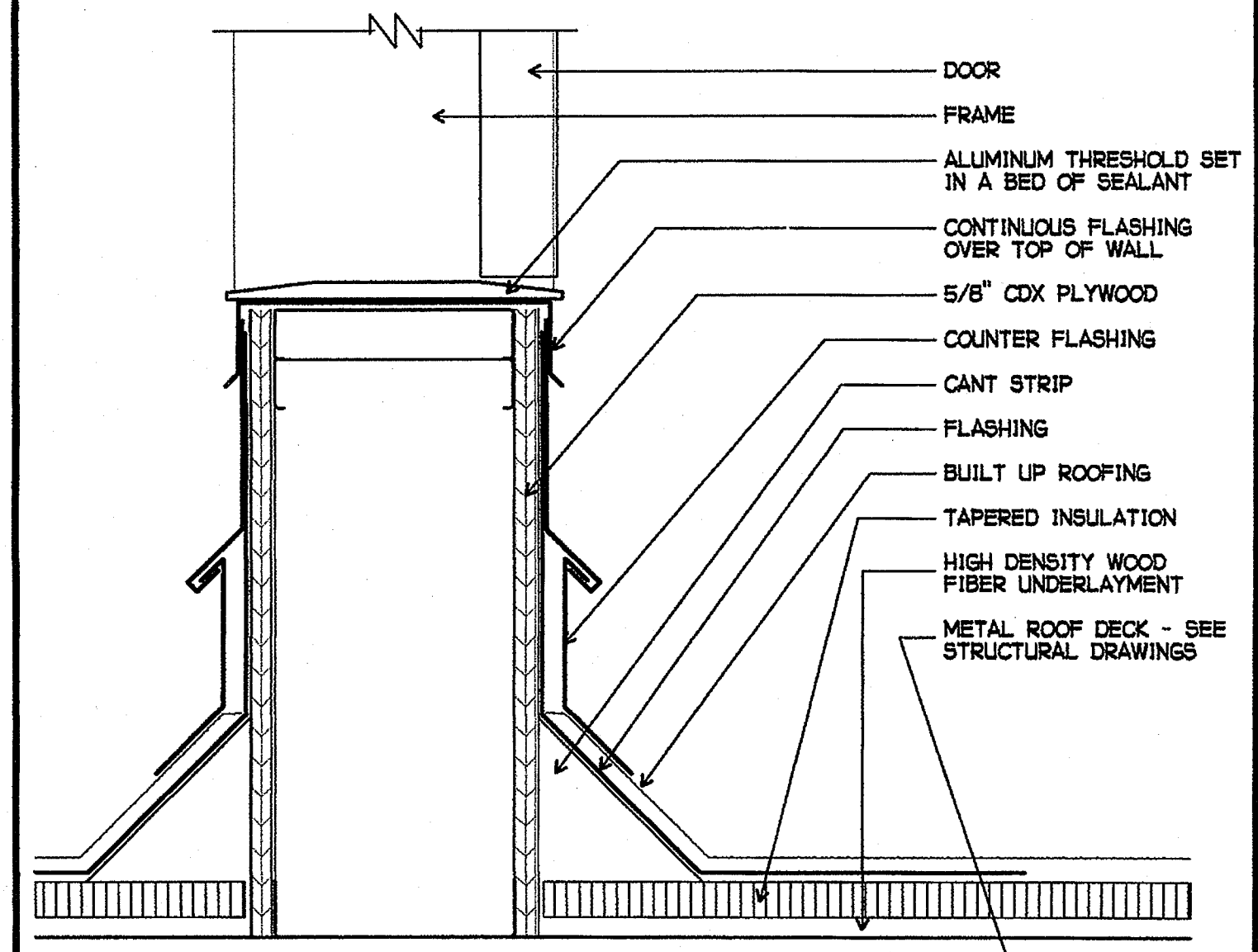
5 HANDRAIL
3" = 1'-0"



6 HANDRAIL
3" = 1'-0"



9 DOOR THRESHOLD AT ROOF
3" = 1'-0"



10 DOOR THRESHOLD AT ROOF
3" = 1'-0"

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STATE OF CALIFORNIA

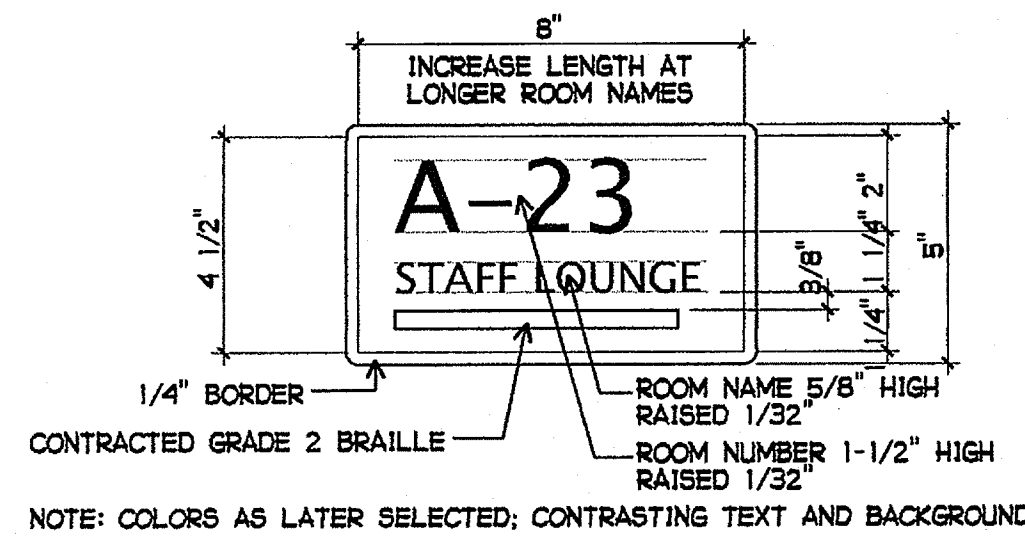
SHEET TITLE

DETAILS

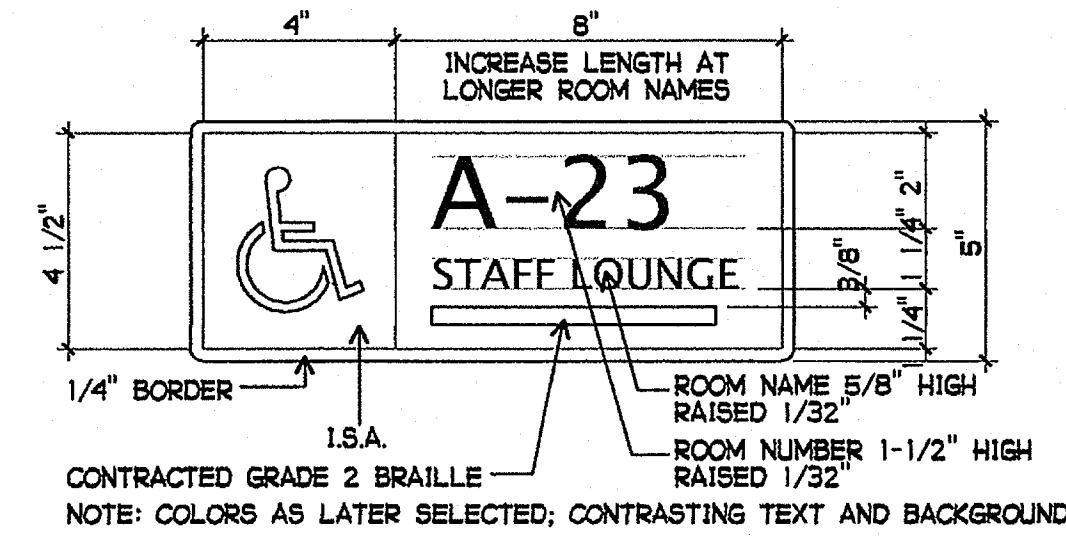
A9-7

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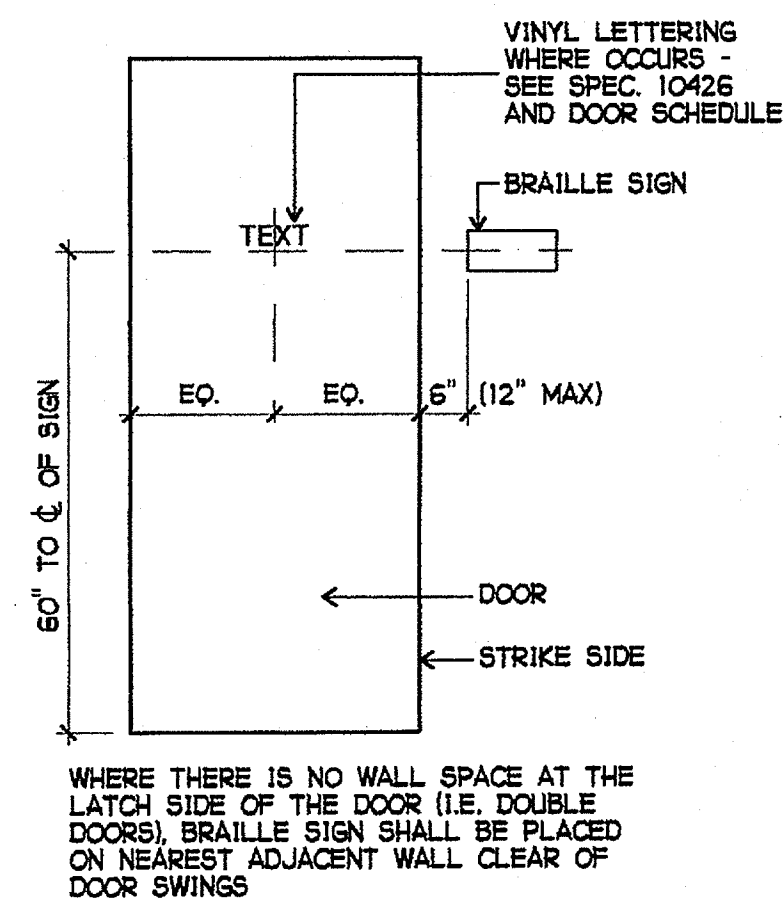
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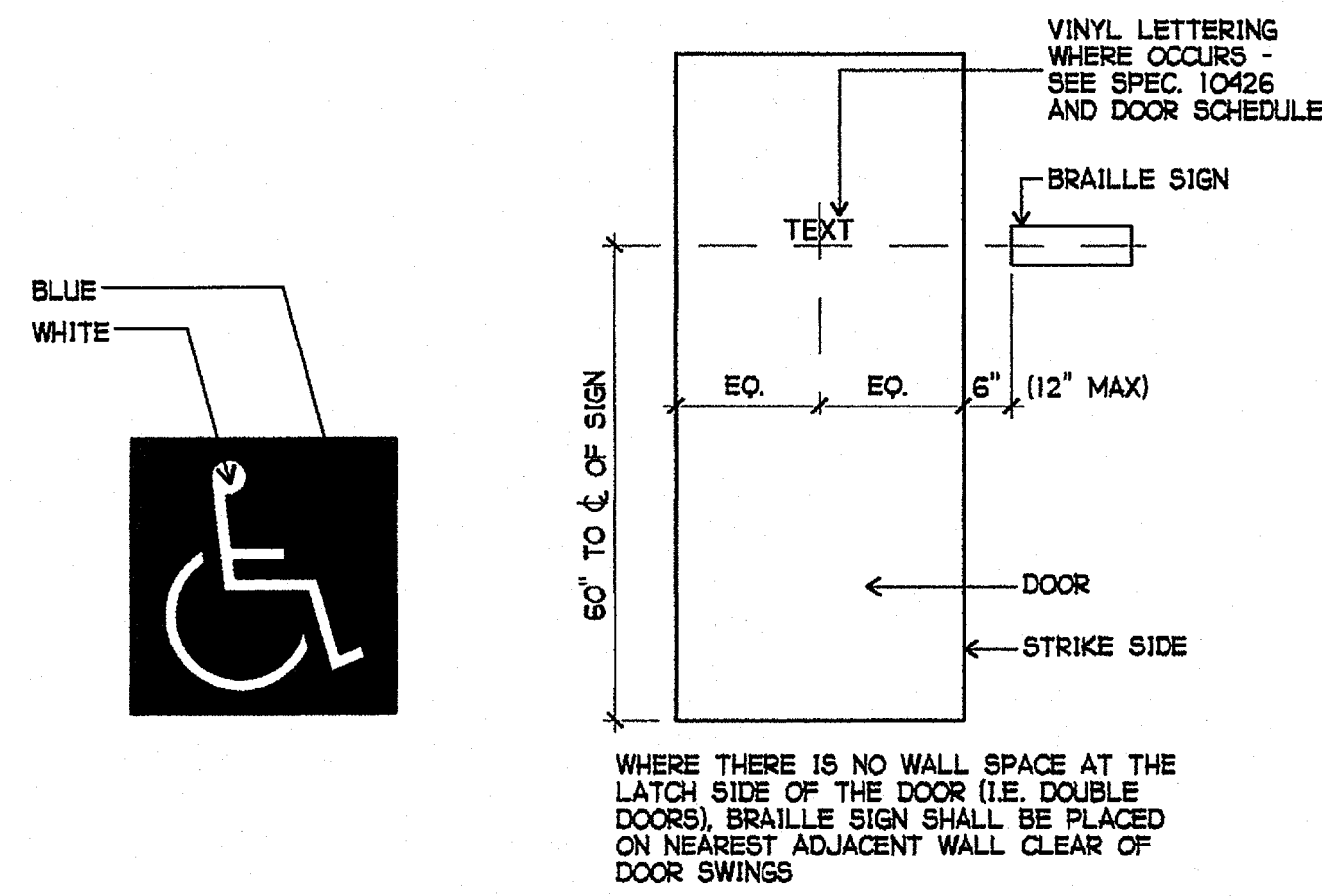
BRaille SIGN
3" = 1'-0"



BRaille SIGN WITH INTERNATIONAL SYMBOL OF ACCESSIBILITY
3" = 1'-0"



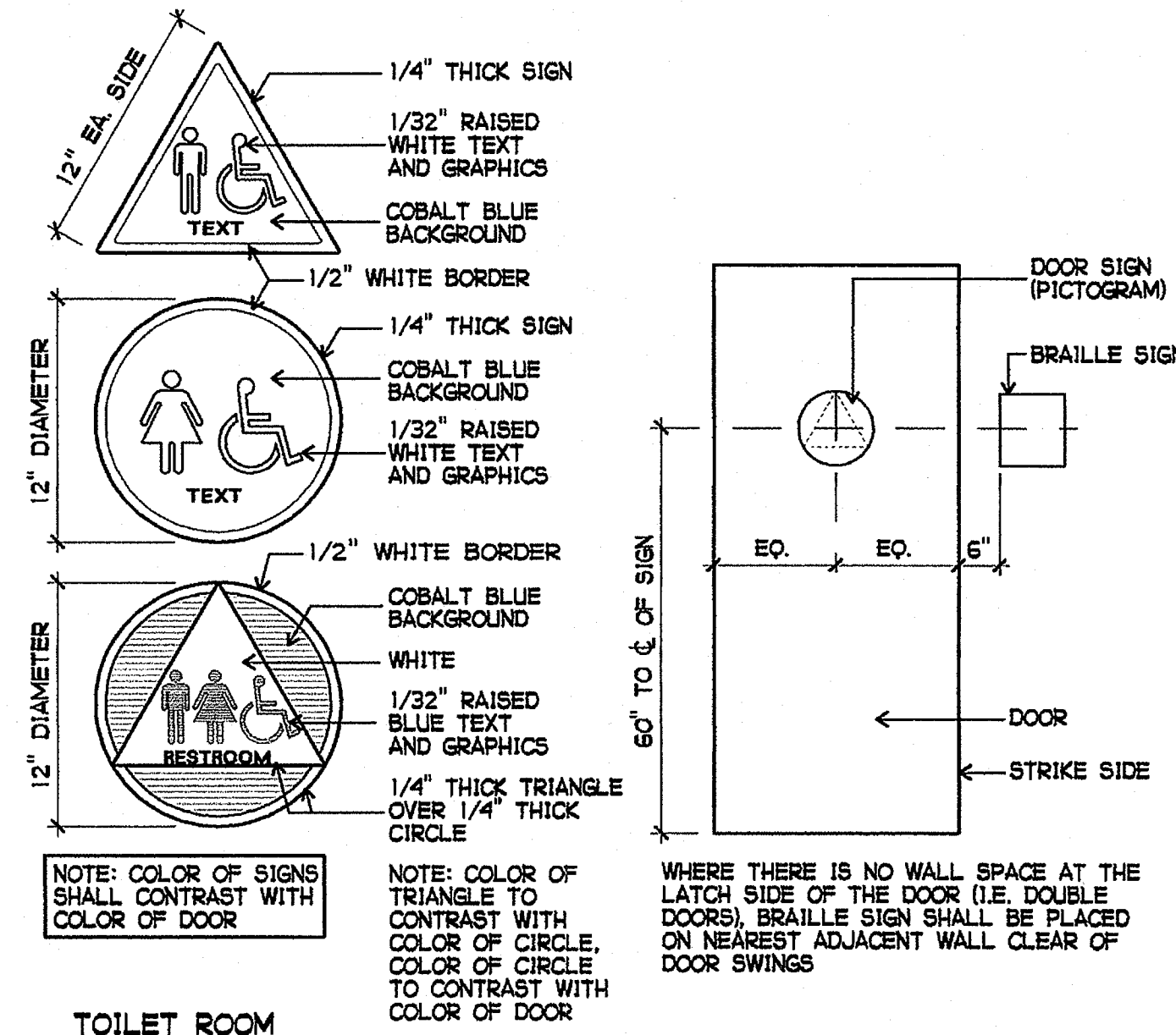
SIGN LOCATIONS
1-1/2" = 1'-0"



INTERNATIONAL SYMBOL OF ACCESSIBILITY
NOT TO SCALE

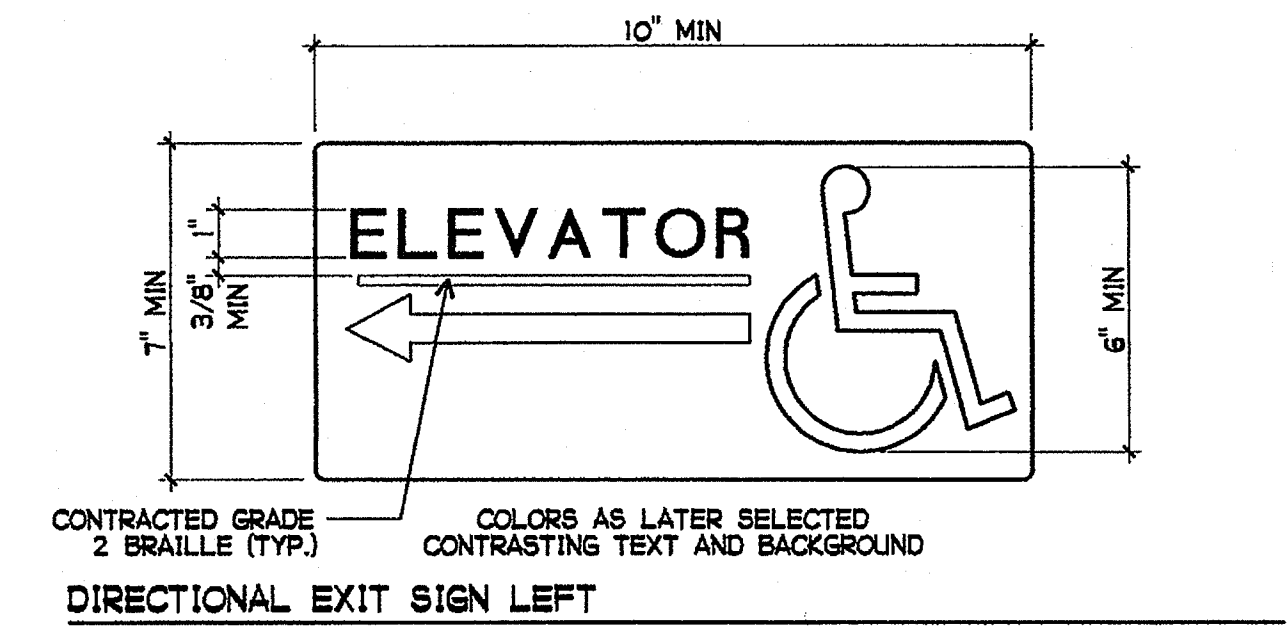
SIGN LOCATIONS
1-1/2" = 1'-0"

TOILET ROOM BRaille SIGN
3" = 1'-0"

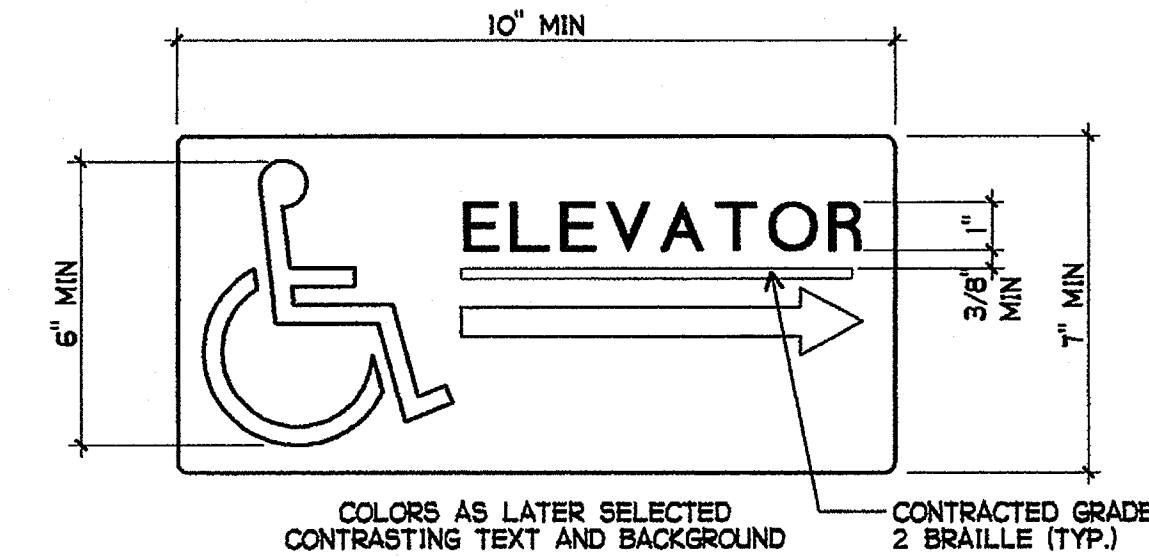


TOILET ROOM DOOR SIGNS (PICTOGRAM)
1-1/2" = 1'-0"

SIGN LOCATIONS
1-1/2" = 1'-0"

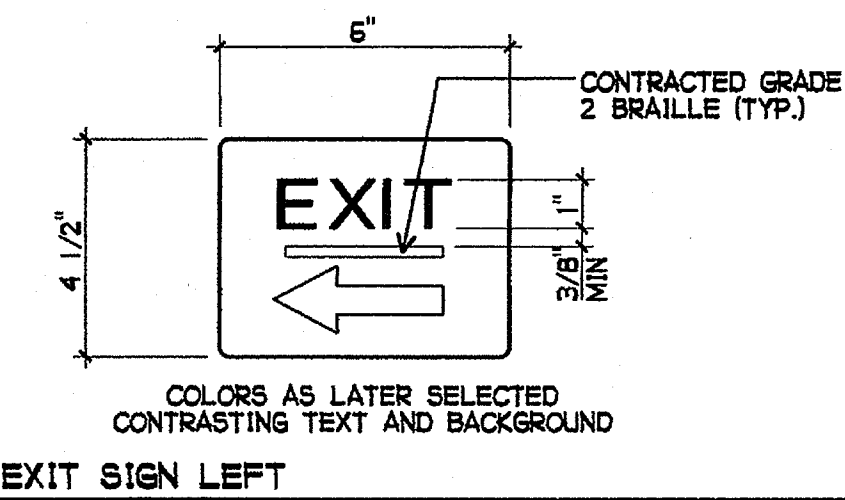


DIRECTIONAL EXIT SIGN LEFT

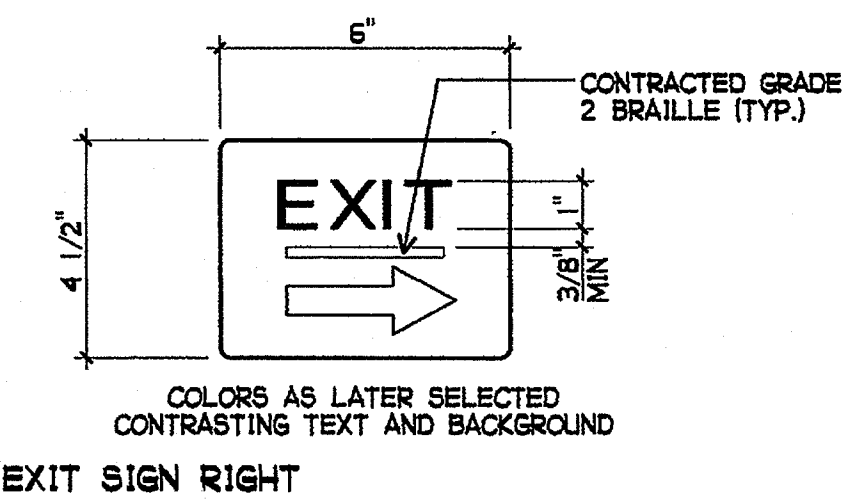


DIRECTIONAL EXIT SIGN RIGHT

4A DIRECTIONAL ELEVATOR SIGNAGE WITH I.S.A.
3" = 1'-0"



DIRECTIONAL EXIT SIGN LEFT



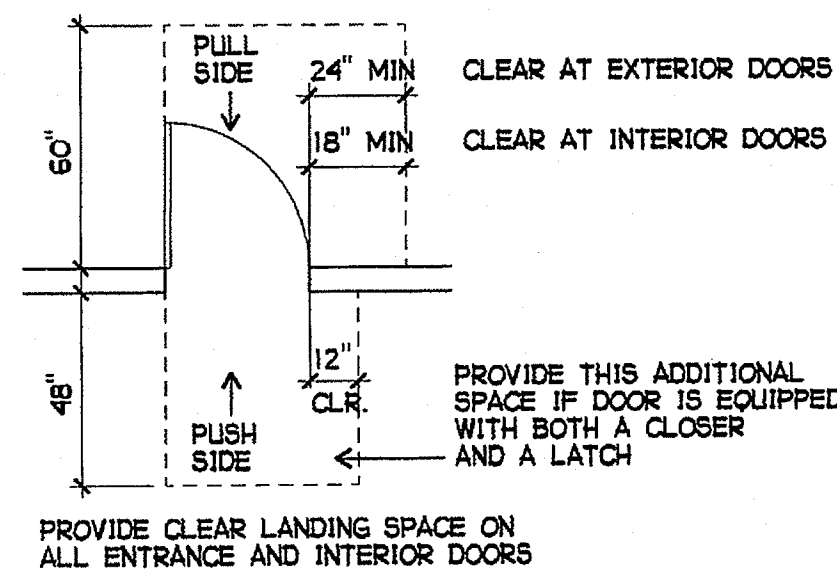
DIRECTIONAL EXIT SIGN RIGHT

1 ROOM DOOR SIGNAGE
AS NOTED

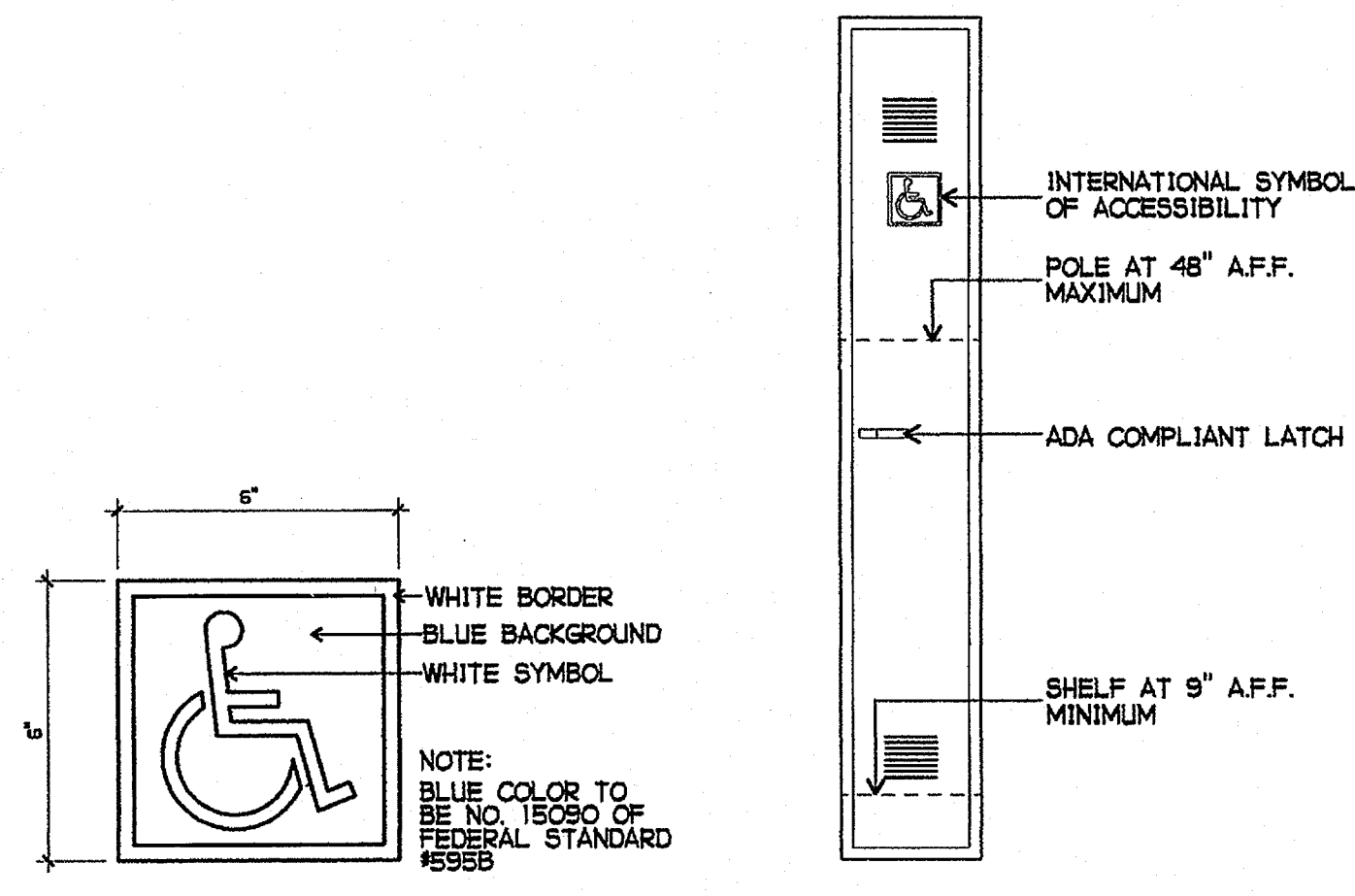
2 ROOM DOOR SIGNAGE WITH I.S.A.
AS NOTED

3 TOILET ROOM DOOR SIGNAGE
AS NOTED

4 DIRECTIONAL EXIT SIGNAGE
3" = 1'-0"



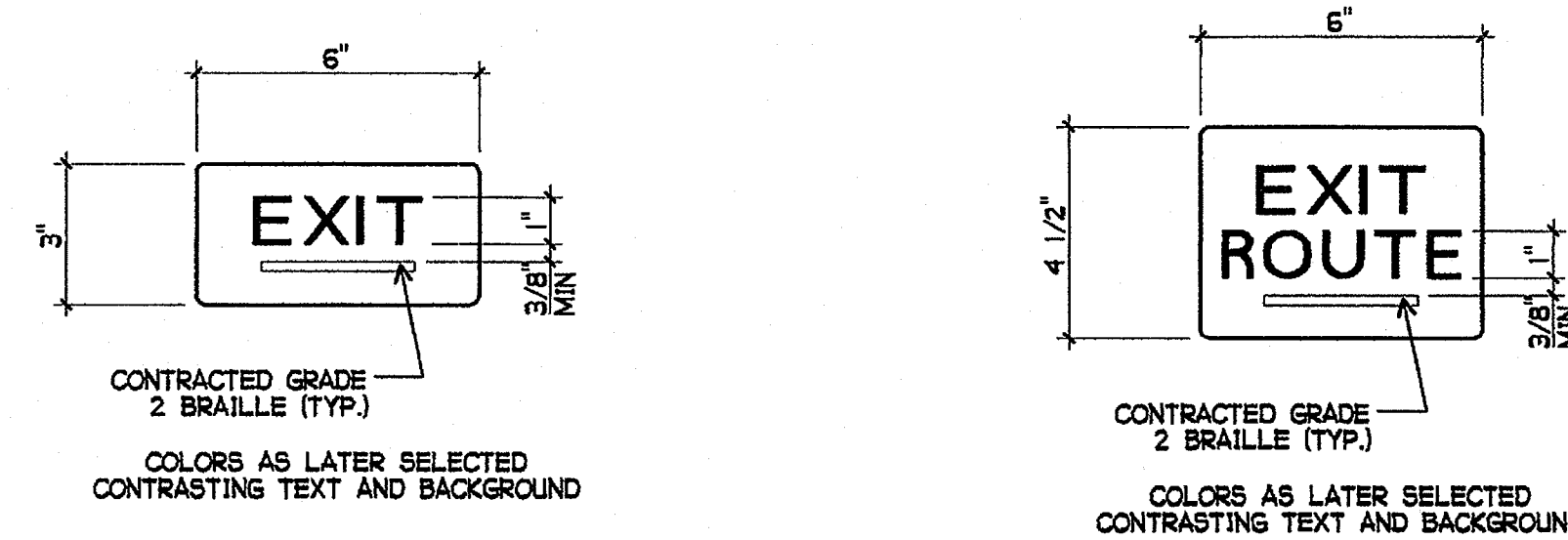
5 ACCESSIBLE DOOR CLEARANCES
1/4" = 1'-0"



INTERNATIONAL SYMBOL OF ACCESSIBILITY
NOT TO SCALE

ADA COMPLIANT LOCKER DOOR
3/4" = 1'-0"

6 ACCESSIBLE LOCKER SIGNAGE
AS NOTED



7 EXIT SIGNAGE
3" = 1'-0"

PLOTTED 3/18/2005 12:14 PM
GROTH ARCHITECTS, INC.
AS THESE DESIGNS, SPECIFICATIONS, AND CONTRACT DOCUMENTS ARE BEING PREPARED FOR THE PROJECT, THE ARCHITECT'S OFFICE SHALL BE KEPT ADVISED OF ANY CHANGES TO THE PROJECT. THERE SHALL BE NO ADDITIONS, DELETIONS, OR CHANGES TO THE PROJECT WITHOUT THE WRITTEN CONSENT OF GROTH ARCHITECTS, INC.

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CLUSD NO.
758-000
PROJECT NOS.
025
P. T. N.
73569-9
DATE
REVISIONS

JEFFERSON MS NEW CONSTRUCTION
823 ACACIA STREET
OCEANSIDE, CA 92054
OCEANSIDE UNIFIED S.D.

space art
function time

GROTH ARCHITECTS, INC.
3355 MISSION AVE. SUITE 234
OCEANSIDE, CALIFORNIA 92054
PHONE 760-754-8191
FAX 760-754-8291

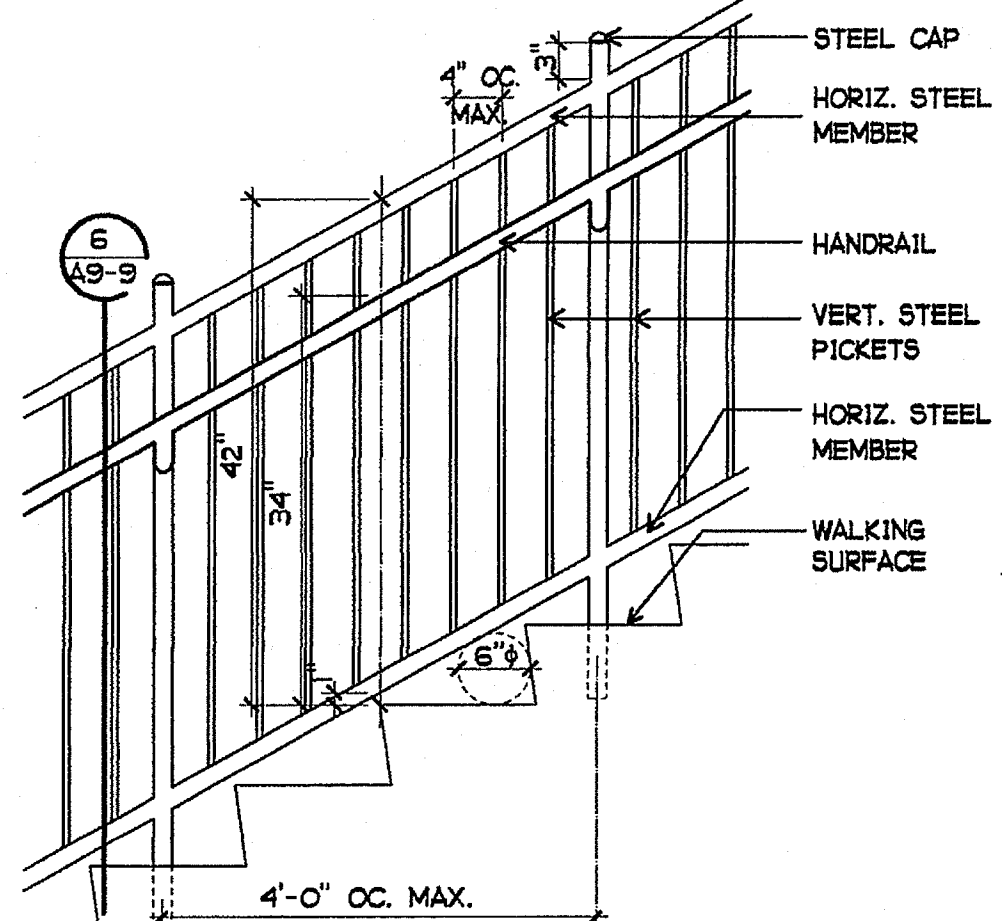
DSA
IDENTIFICATION STAMP
DIV. OF THE STATE ARCHITECT
OFFICE OF REGULATION SERVICES
4-106494
AC 1/16 FLA. SS 2
DATE MAR 28 2005

LICENSURE ARCHITECT
JOHN SCOTT GROTH
C-26609
4/30/2007 RENEWAL
STATE OF CALIFORNIA

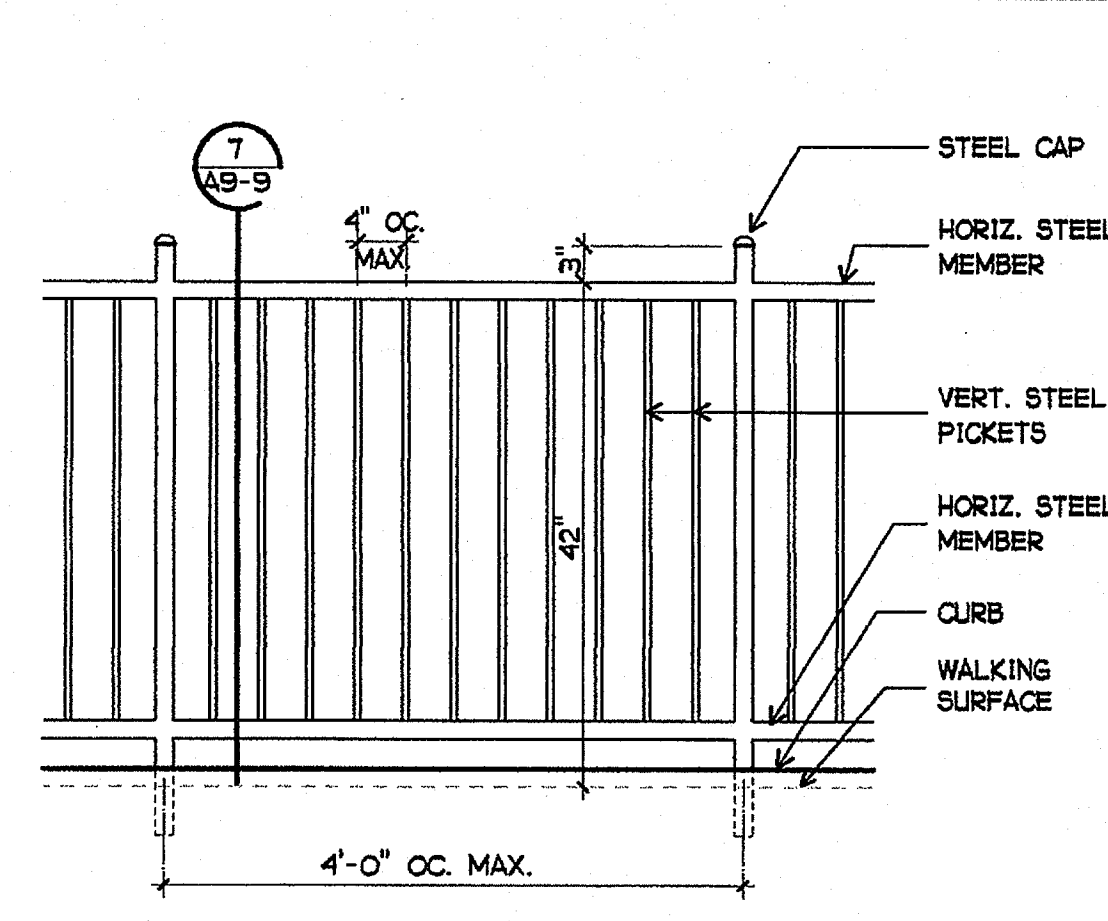
SHEET TITLE

SIGNAGE DETAILS

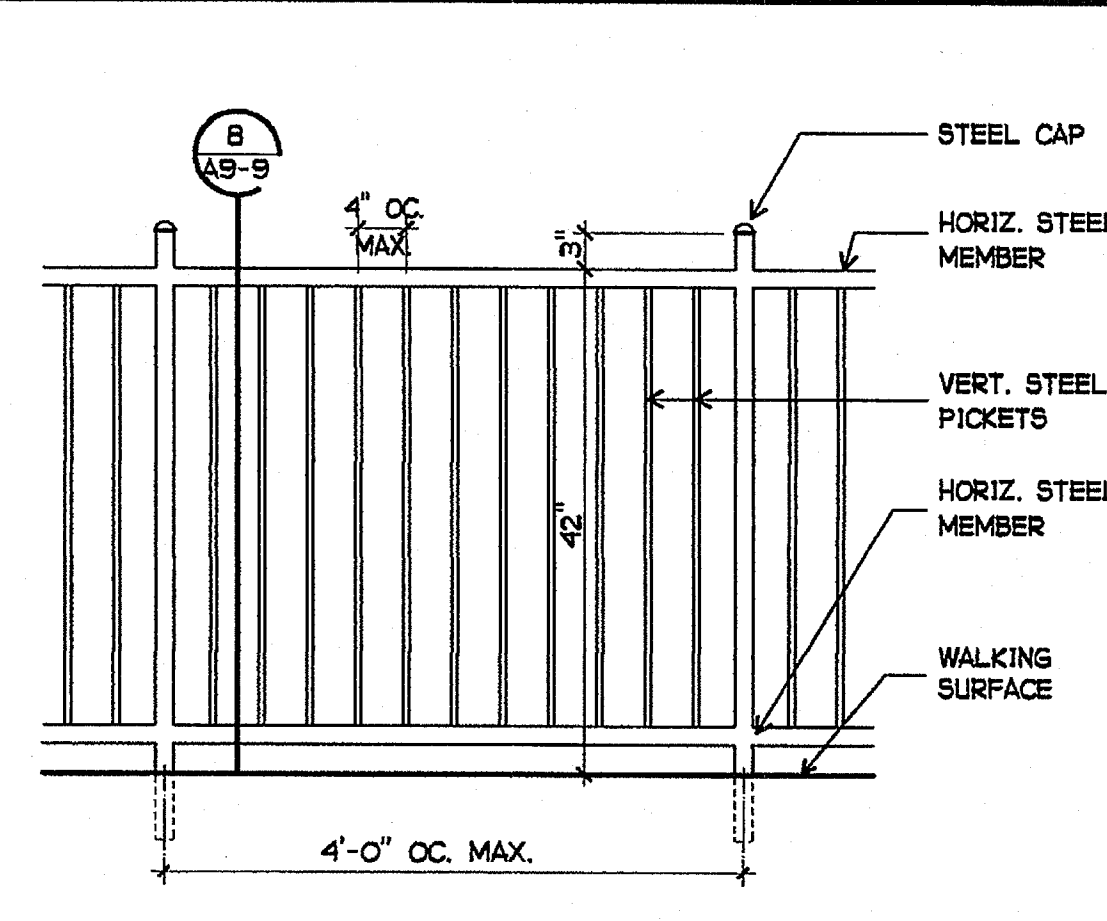
A9-8



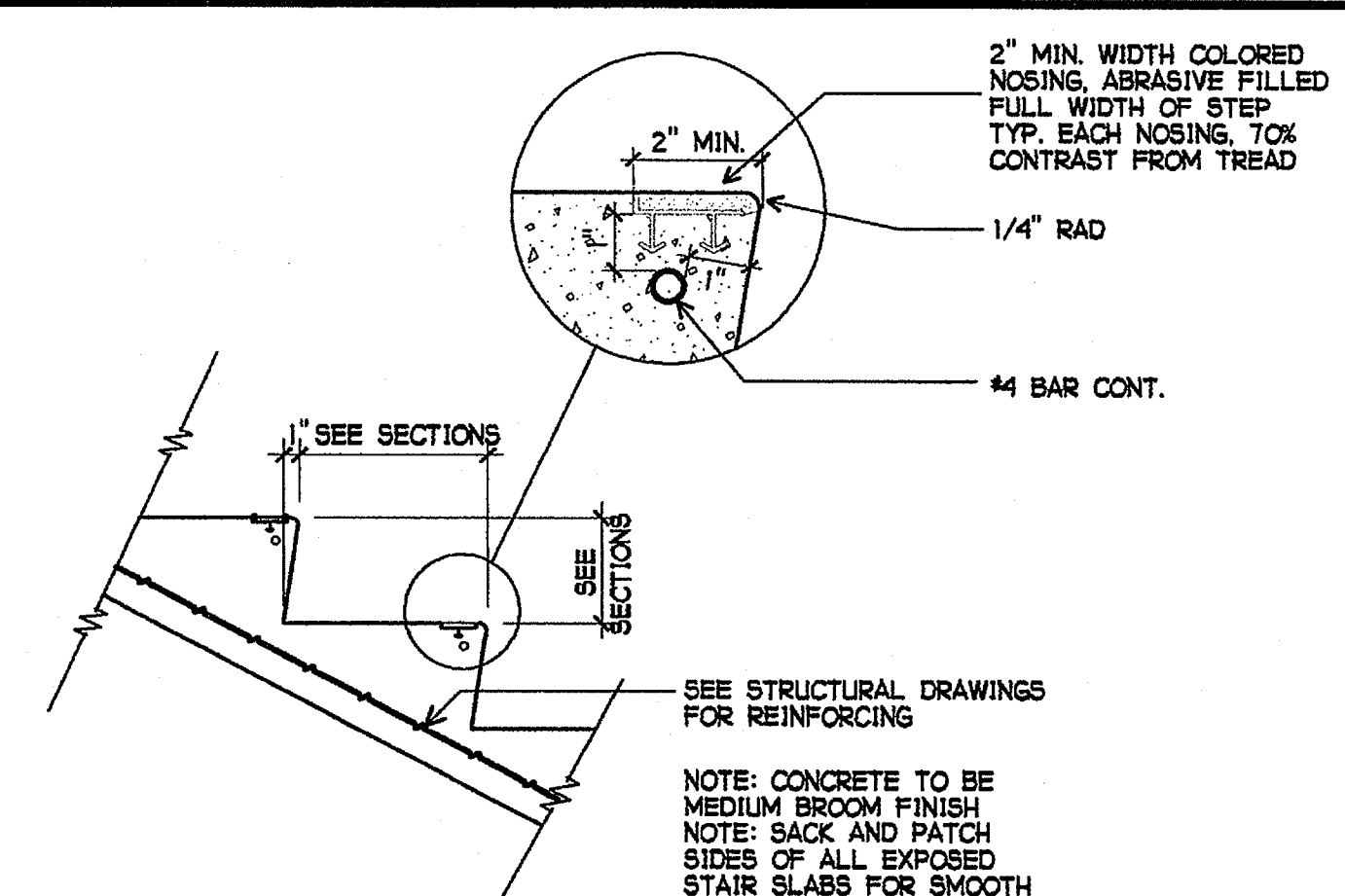
1 GUARDRAIL AND HANDRAIL AT STAIR
3" = 1'-0"



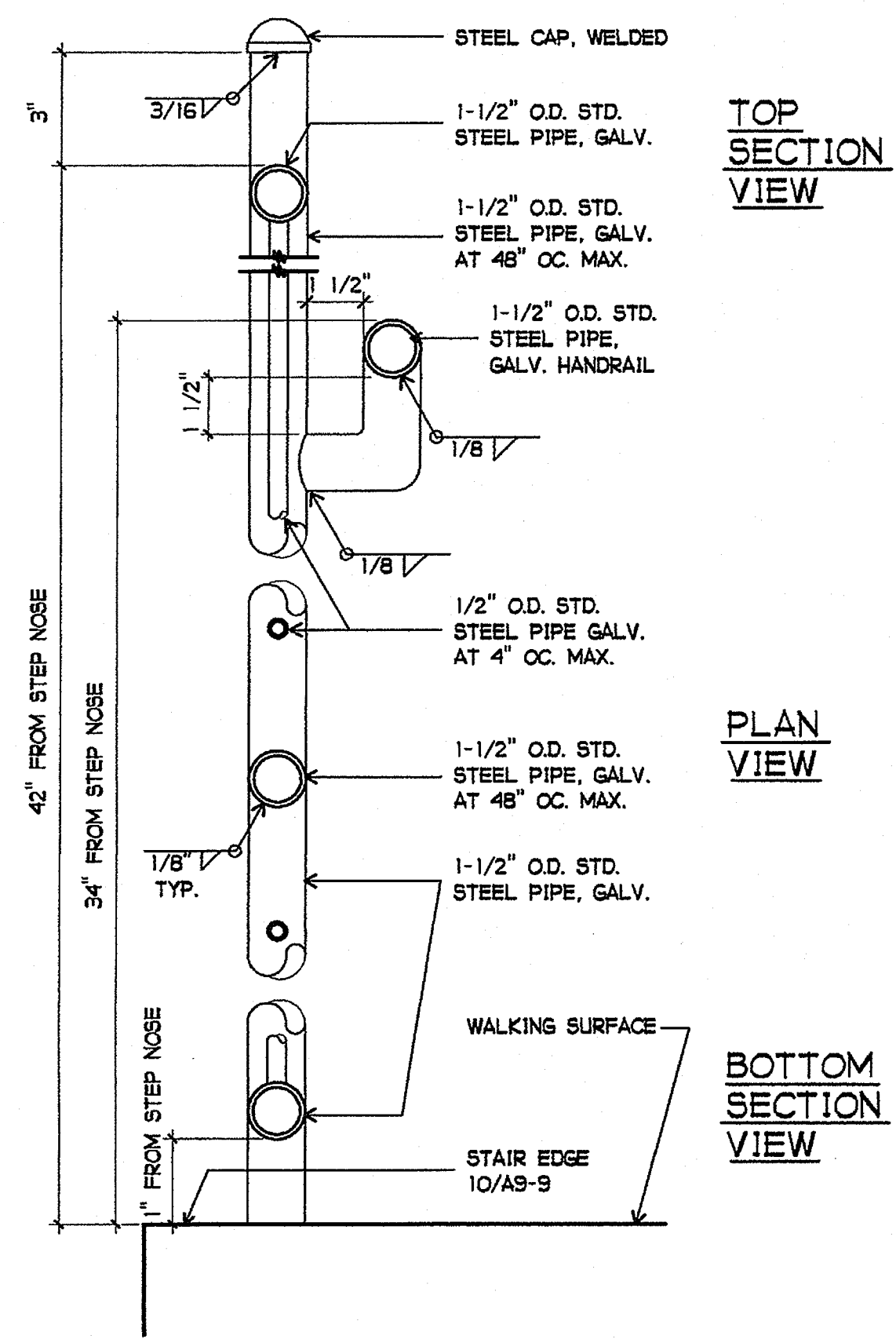
2 GUARDRAIL AT LANDING
3" = 1'-0"



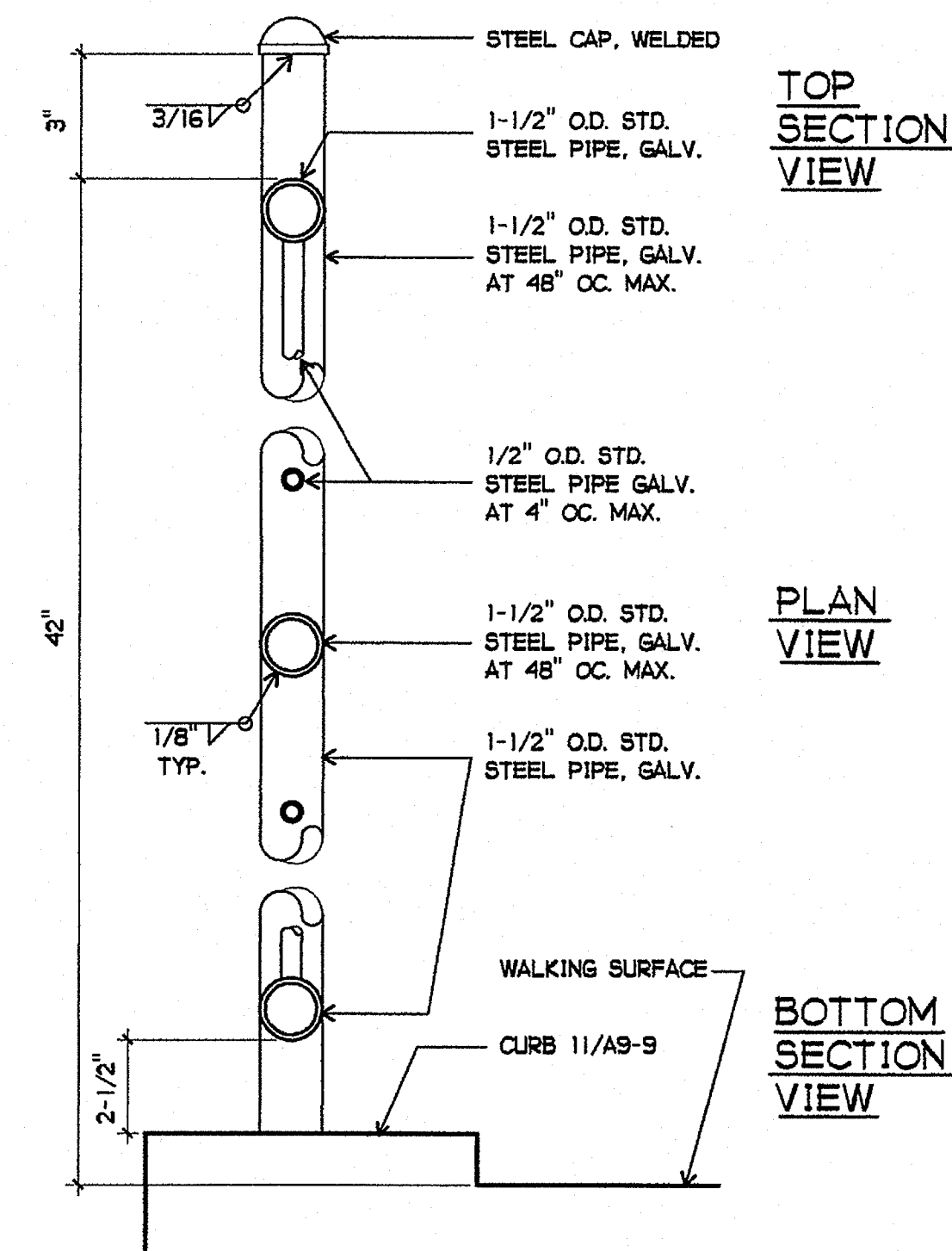
3 GUARDRAIL AT INTERMEDIATE LANDING
3" = 1'-0"



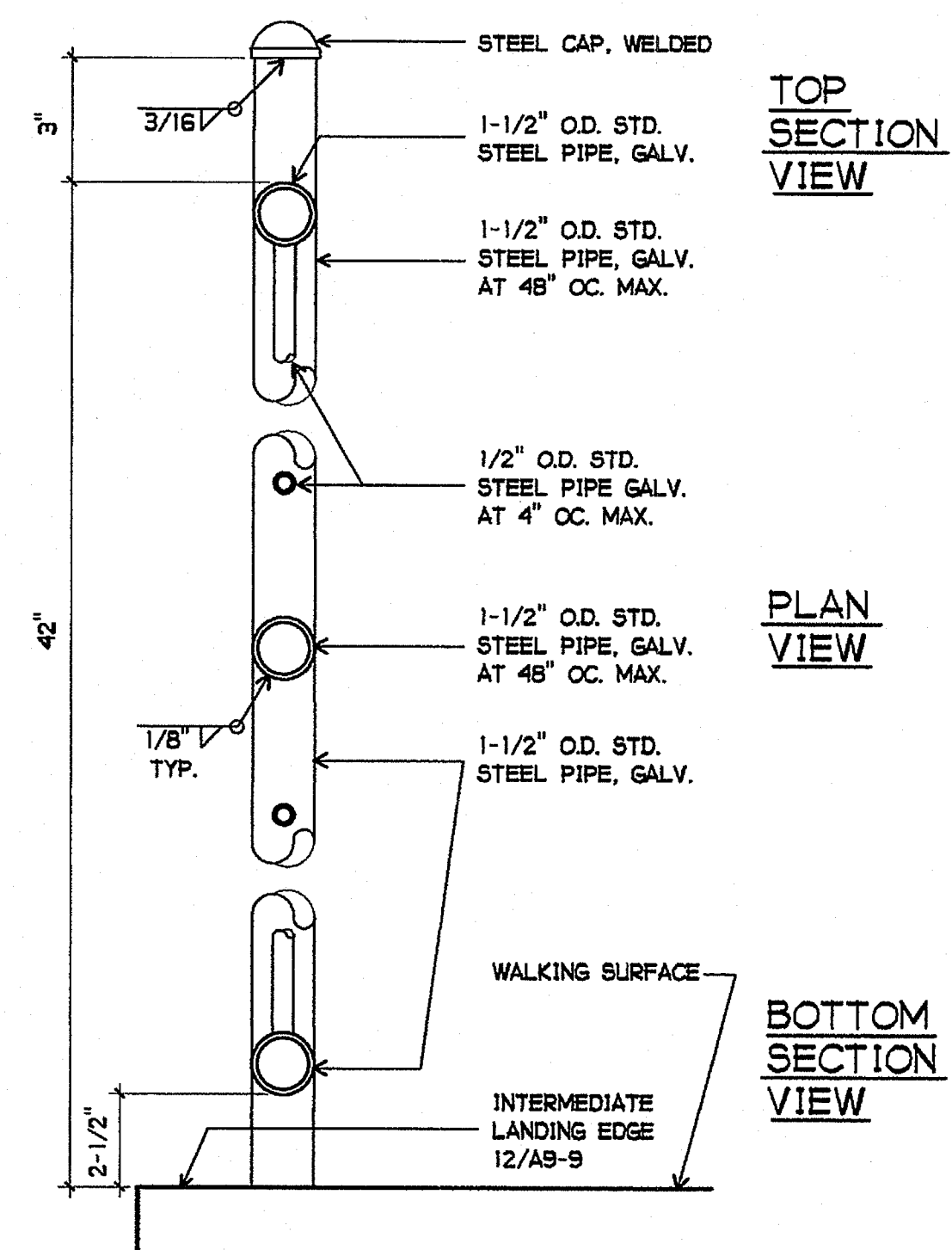
4 STAIR STEPS
1" = 1'-0"



6 GUARDRAIL AND HANDRAIL AT STAIR
3" = 1'-0"



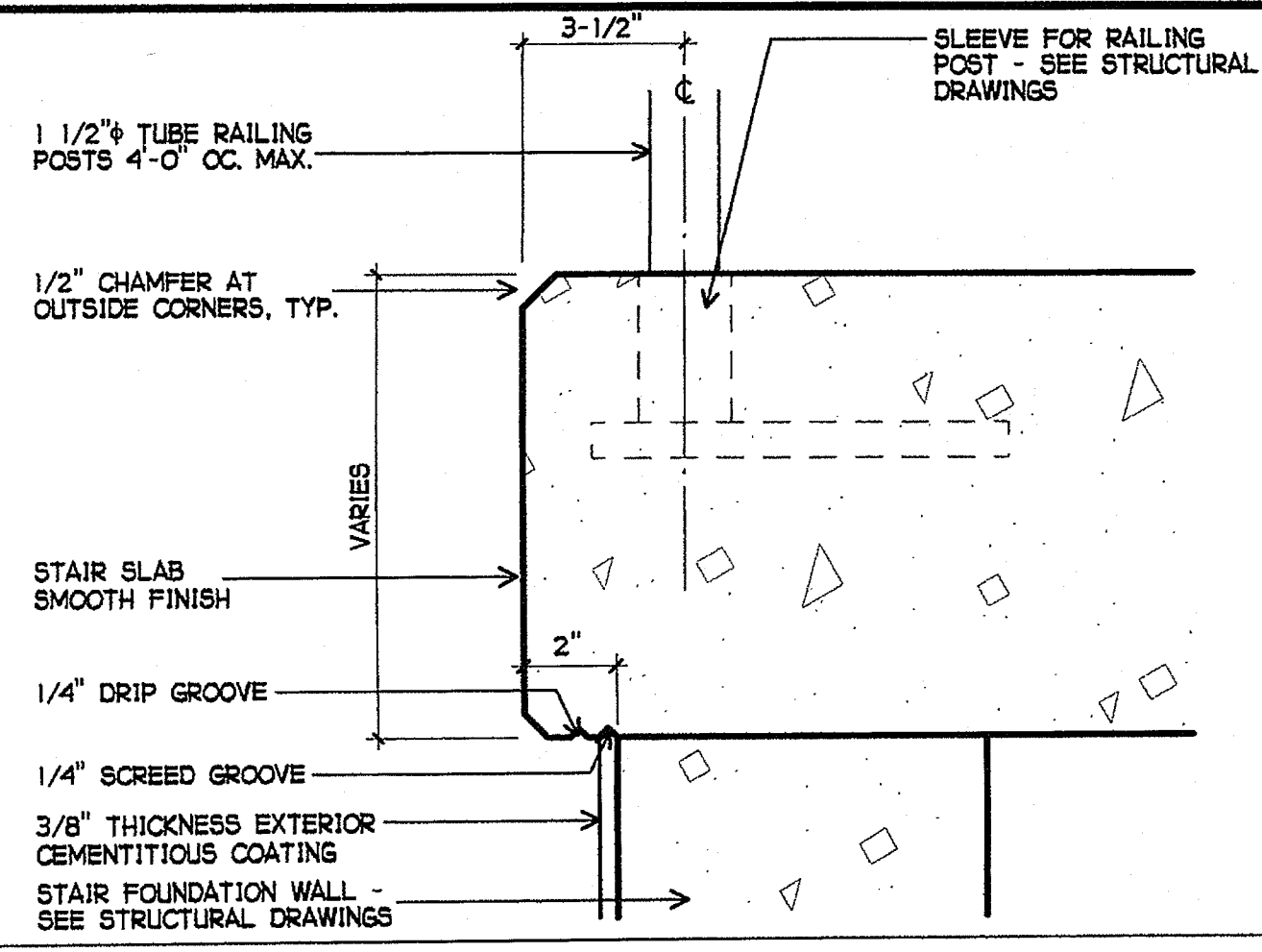
7 GUARDRAIL AT LANDING
3" = 1'-0"



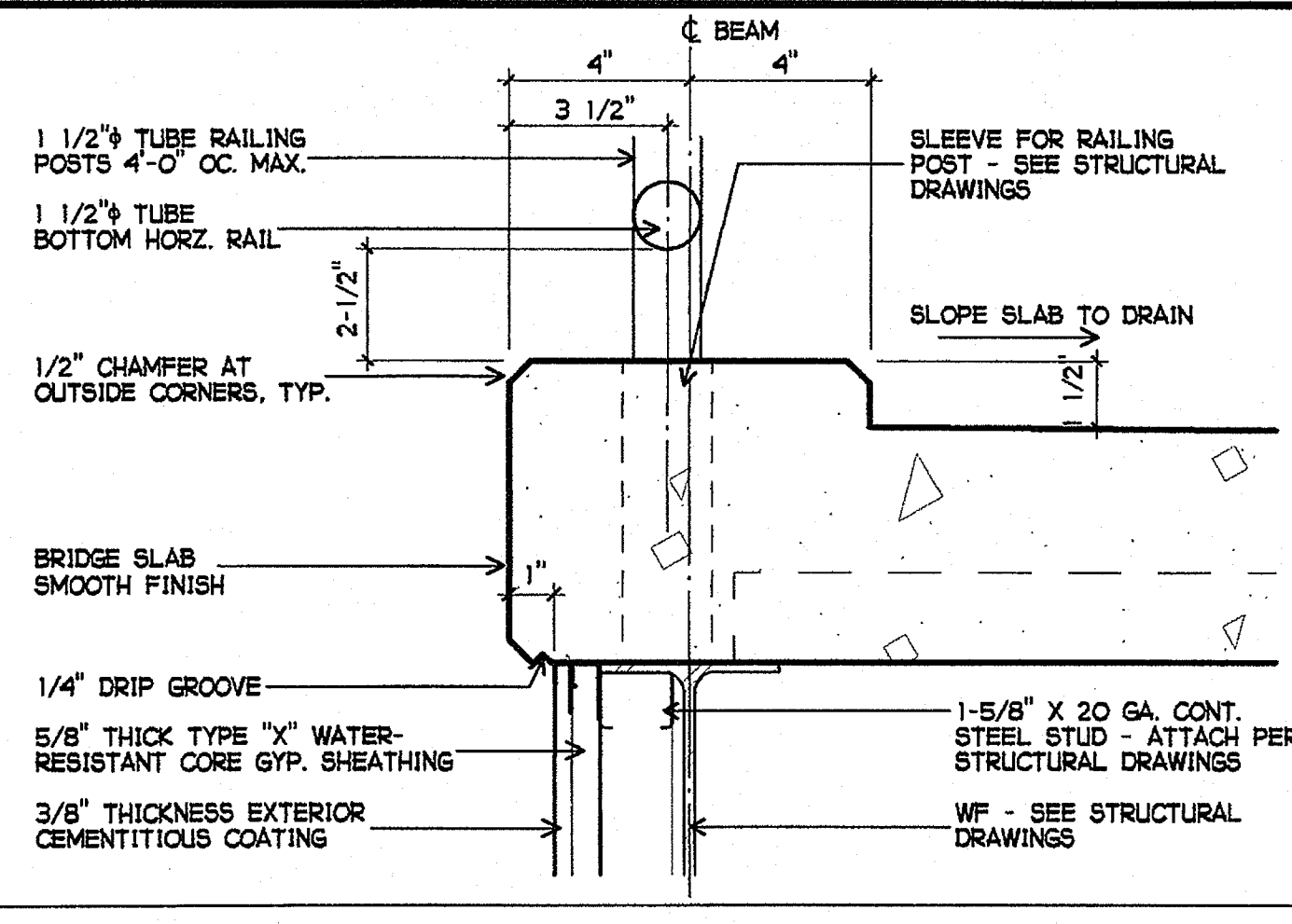
8 GUARDRAIL AT INTERMEDIATE LANDING
3" = 1'-0"

5 NOT USED

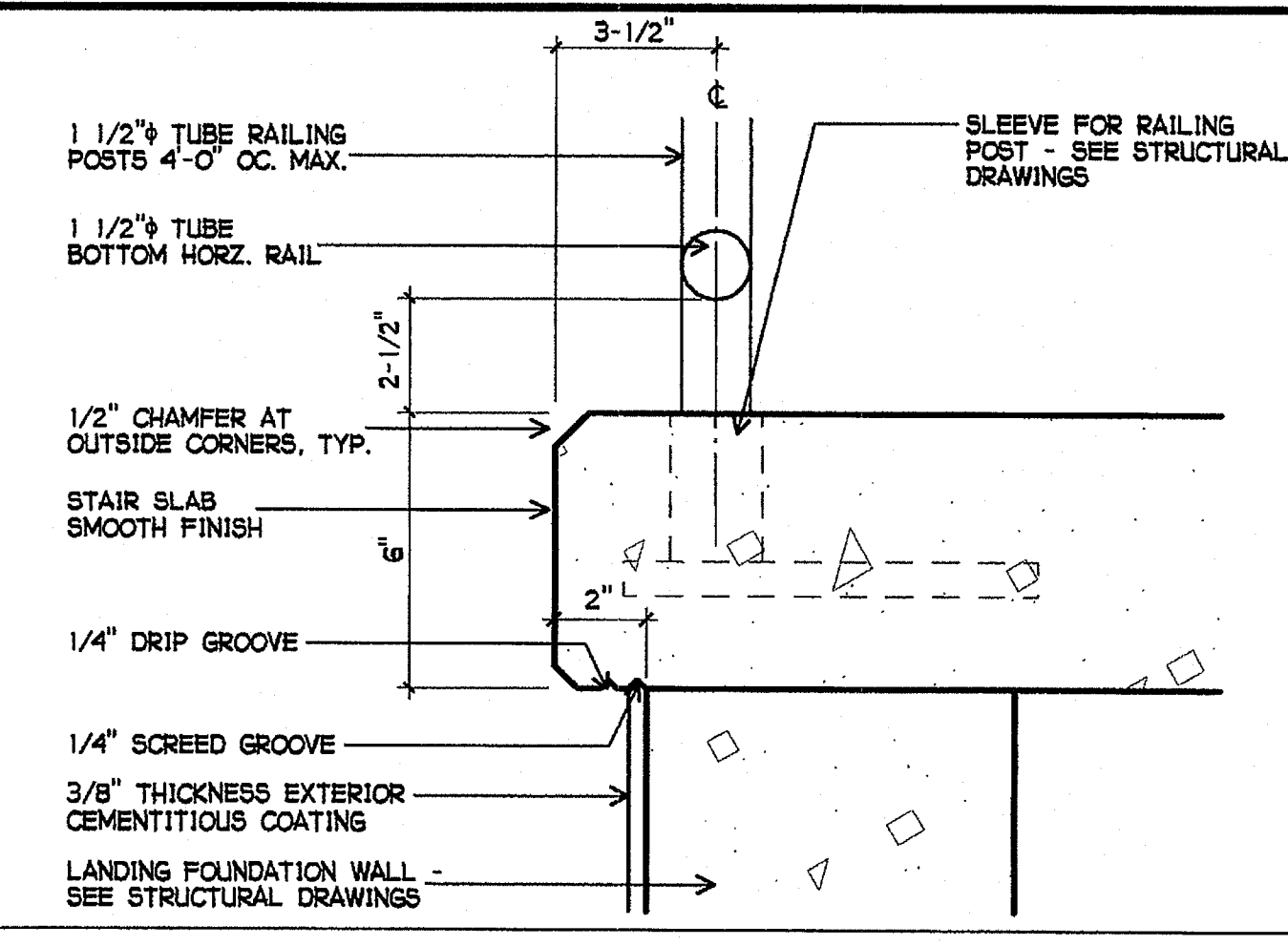
9 NOT USED



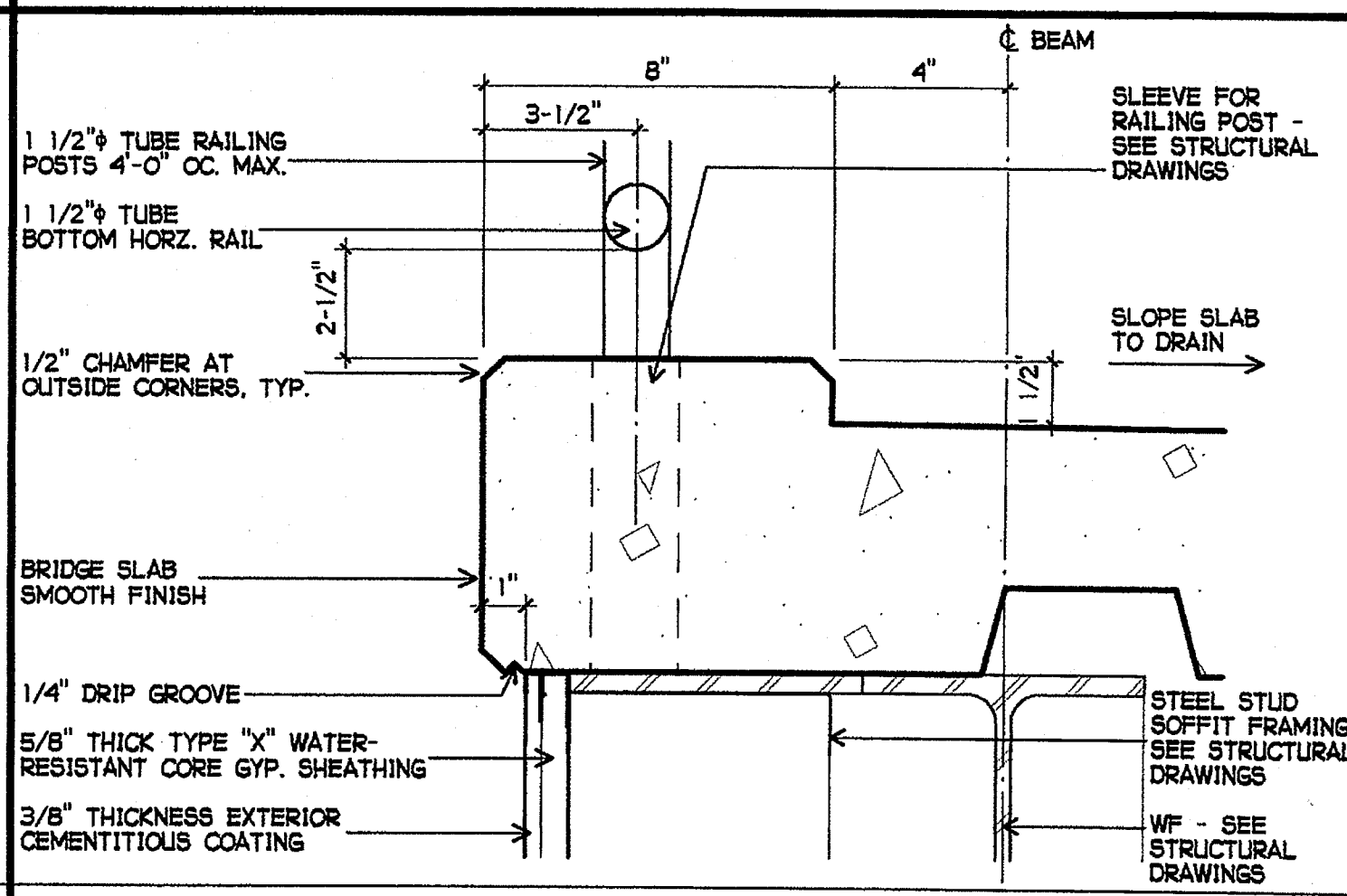
10 STAIR EDGE
3" = 1'-0"



11 LANDING CURB
3" = 1'-0"



12 INTERMEDIATE LANDING EDGE
3" = 1'-0"



13 BRIDGE CURB
3" = 1'-0"

PLOTTED 3/18/2005 12:13 PM
DLD
GROTH ARCHITECTS, INC.
CLSD NO. 758-000
PROJECT NOS. 025
P. T. N. 73569-9
DATE

REVISIONS
JEFFERSON MS NEW CONSTRUCTION
823 ACACIA STREET
OCEANSIDE, CA 92054
OCEANSIDE UNIFIED S.D.

space art
function time
GROTH ARCHITECTS, INC.
3355 MISSION AVE. SUITE 234
OCEANSIDE, CALIFORNIA 92054
PHONE 760-754-8191
FAX 760-754-8291

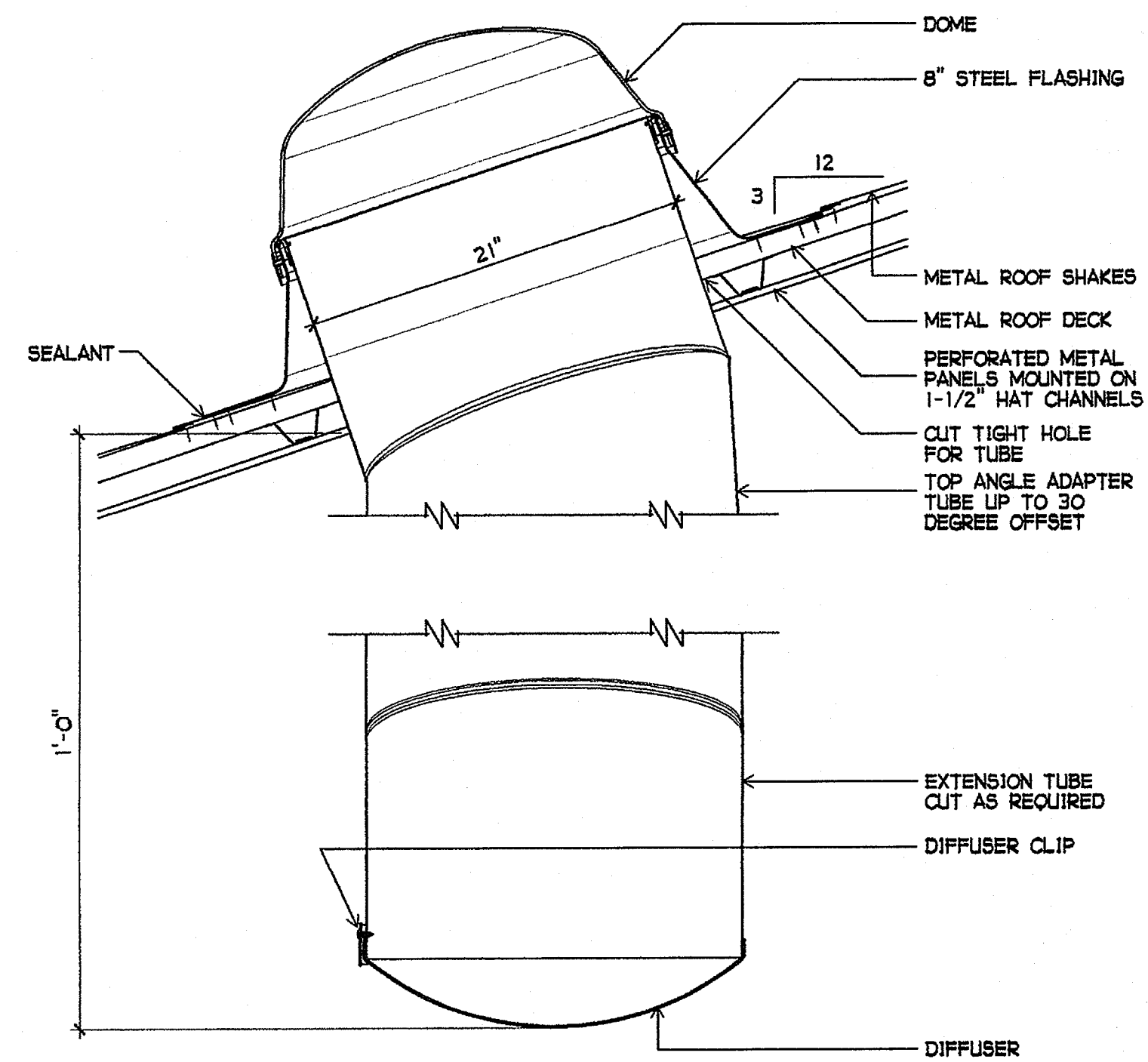
DSA
IDENTIFICATION STAMP
DIV. OF THE STATE ARCHITECT
OFFICE OF REGULATION SERVICES
4-106494
AC RB FLSX SS
DATE MAR 28 2005

REGISTERED ARCHITECT
JOHN SCOTT BRADY
C-26609
4/30/2007 RENEWAL
STATE OF CALIFORNIA

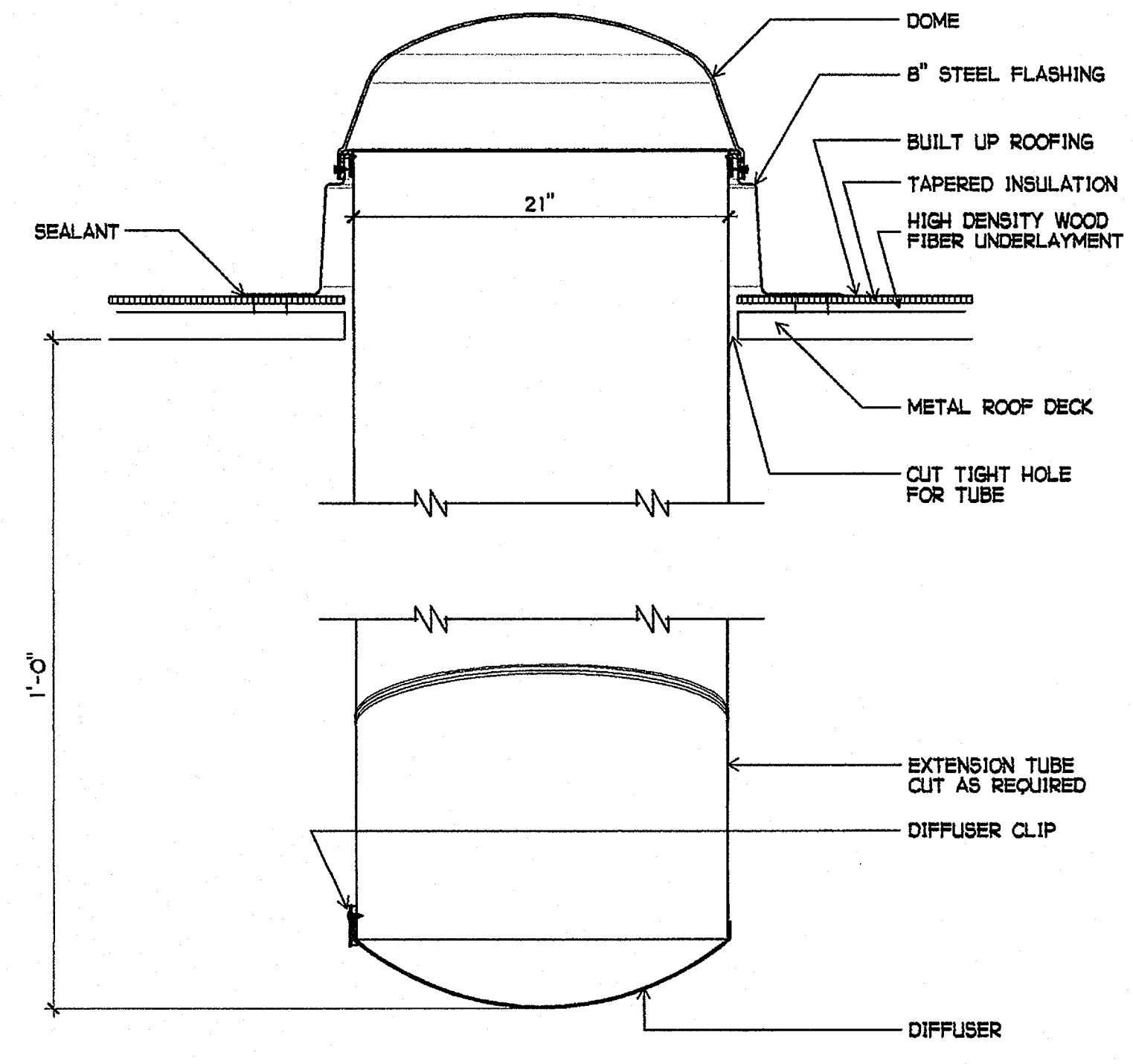
SHEET TITLE
STAIR DETAILS

A9-9

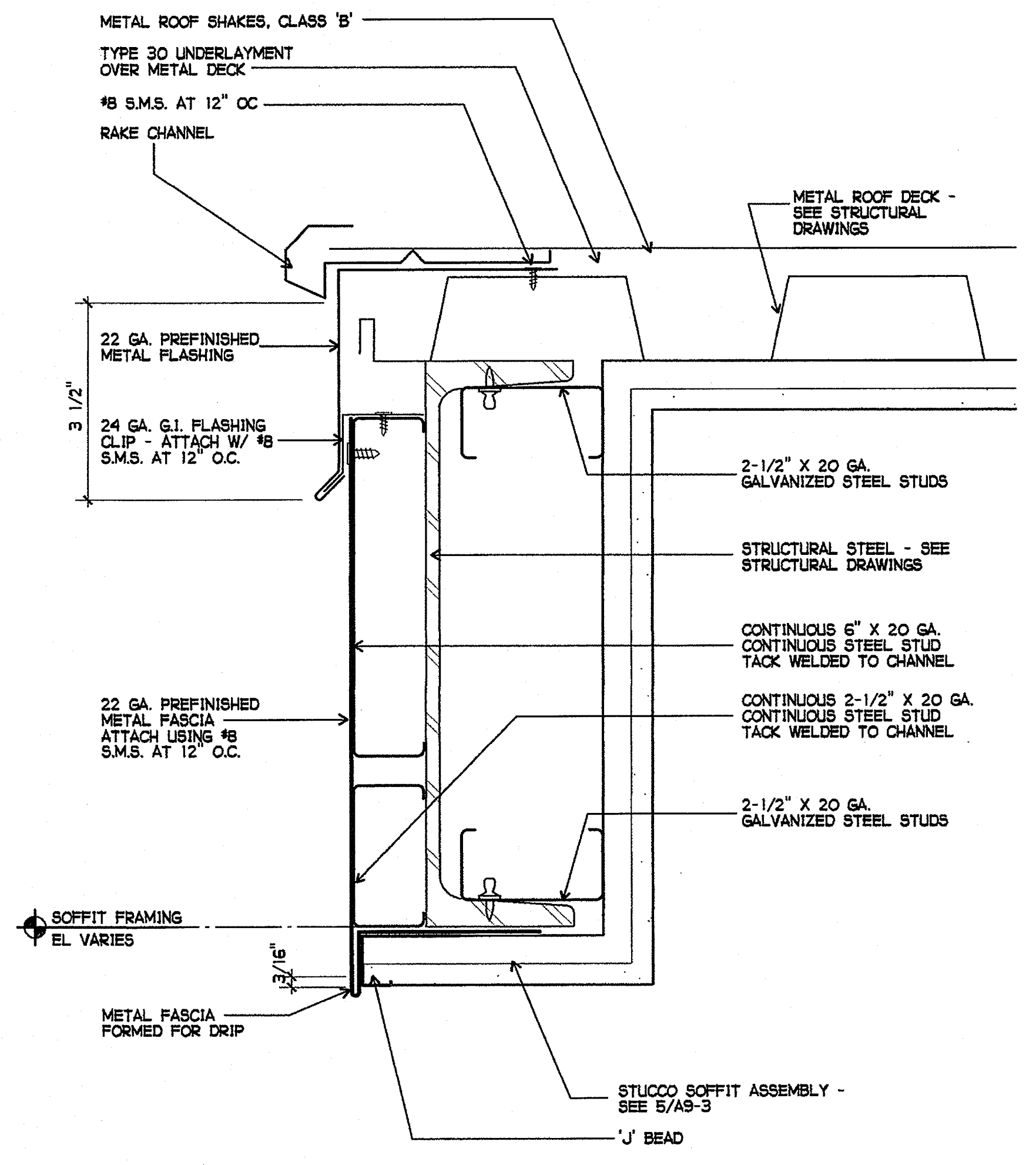
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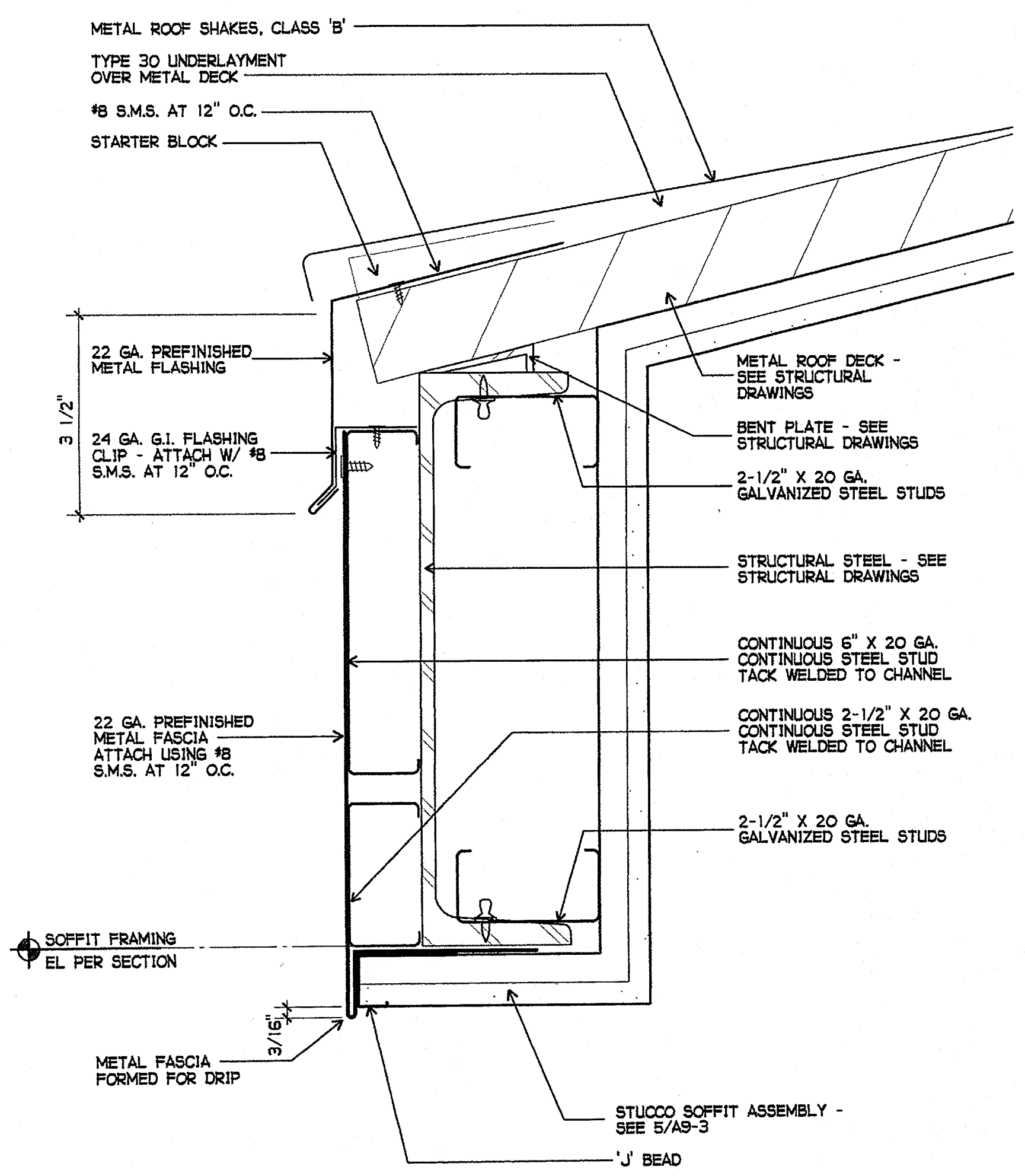
1 21" TUBULAR SKYLIGHT AT METAL SHAKE ROOFING/OPEN CEILING
1-1/2" = 1'-0"



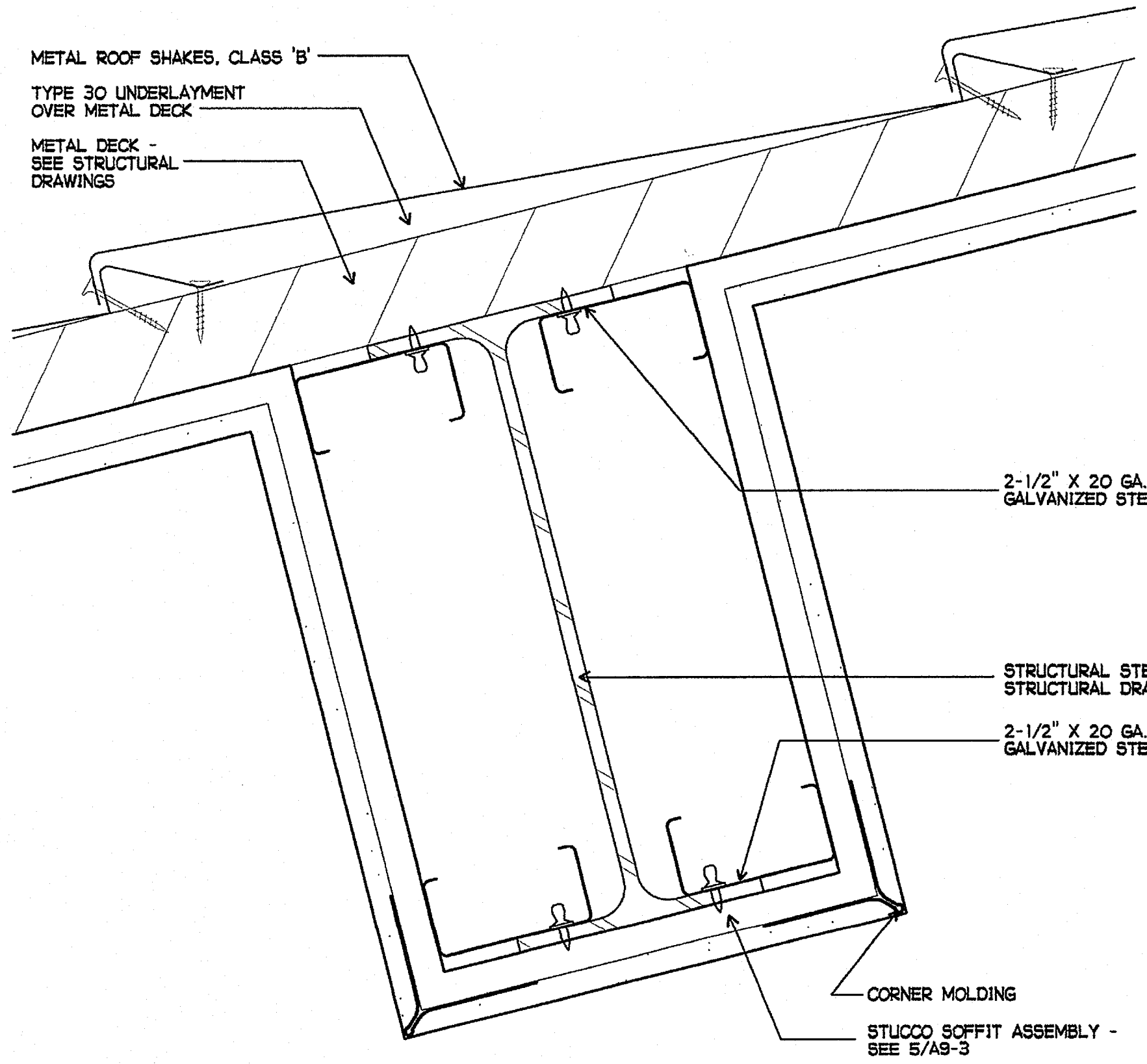
2 21" TUBULAR SKYLIGHT AT BUILT-UP ROOF/OPEN CEILING
1-1/2" = 1'-0"



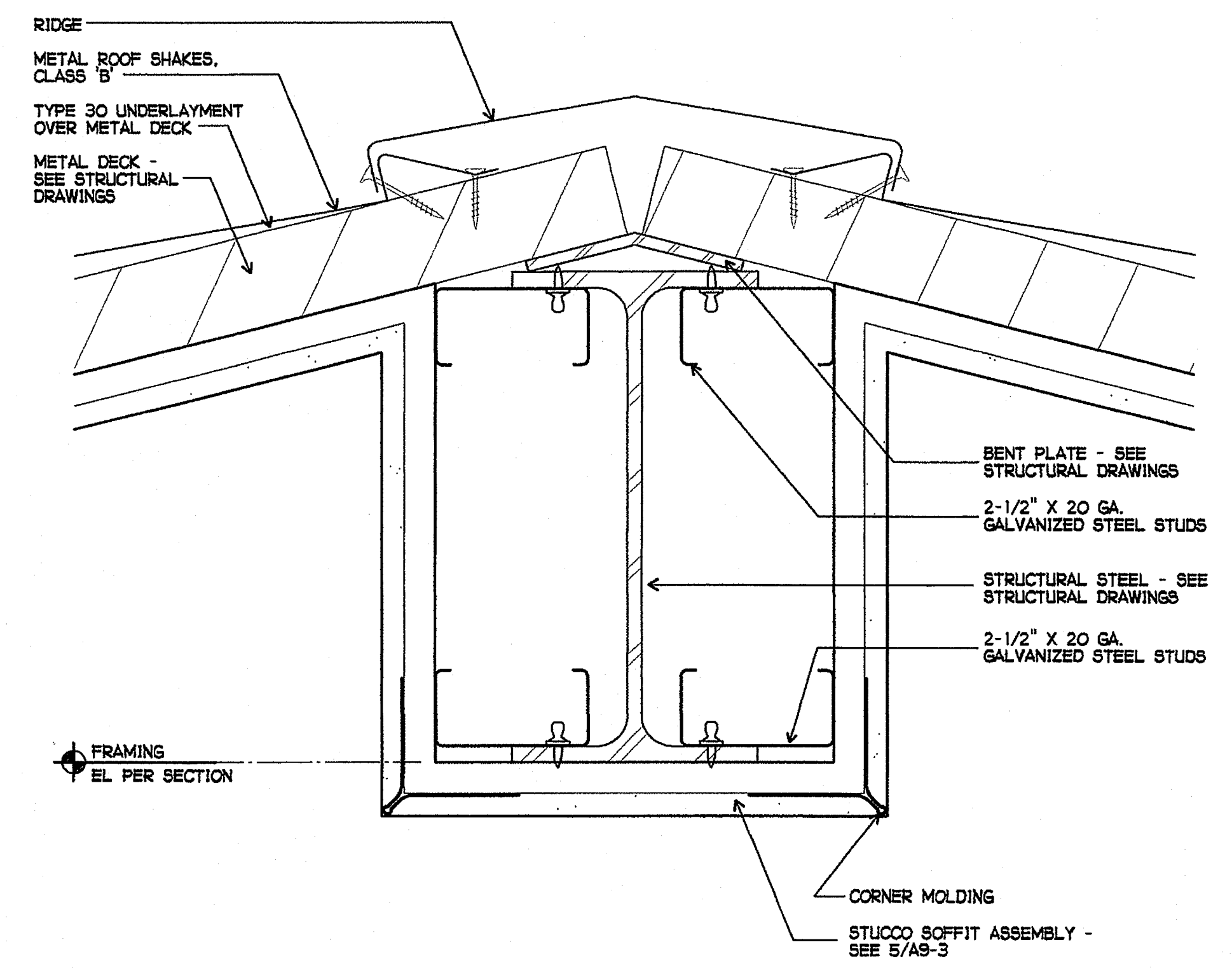
3 ROOF RAKE
6" = 1'-0"



4 ROOF EAVE
6" = 1'-0"



5 ROOF BEAM
6" = 1'-0"



6 ROOF RIDGE
6" = 1'-0"

GROTH ARCHITECTS, INC.
 823 ACACIA STREET
 OCEANSIDE, CA 92054
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 758-000
 PROJECT NOS. 025
 P. T. N. 73569-9
 DATE
 REVISIONS

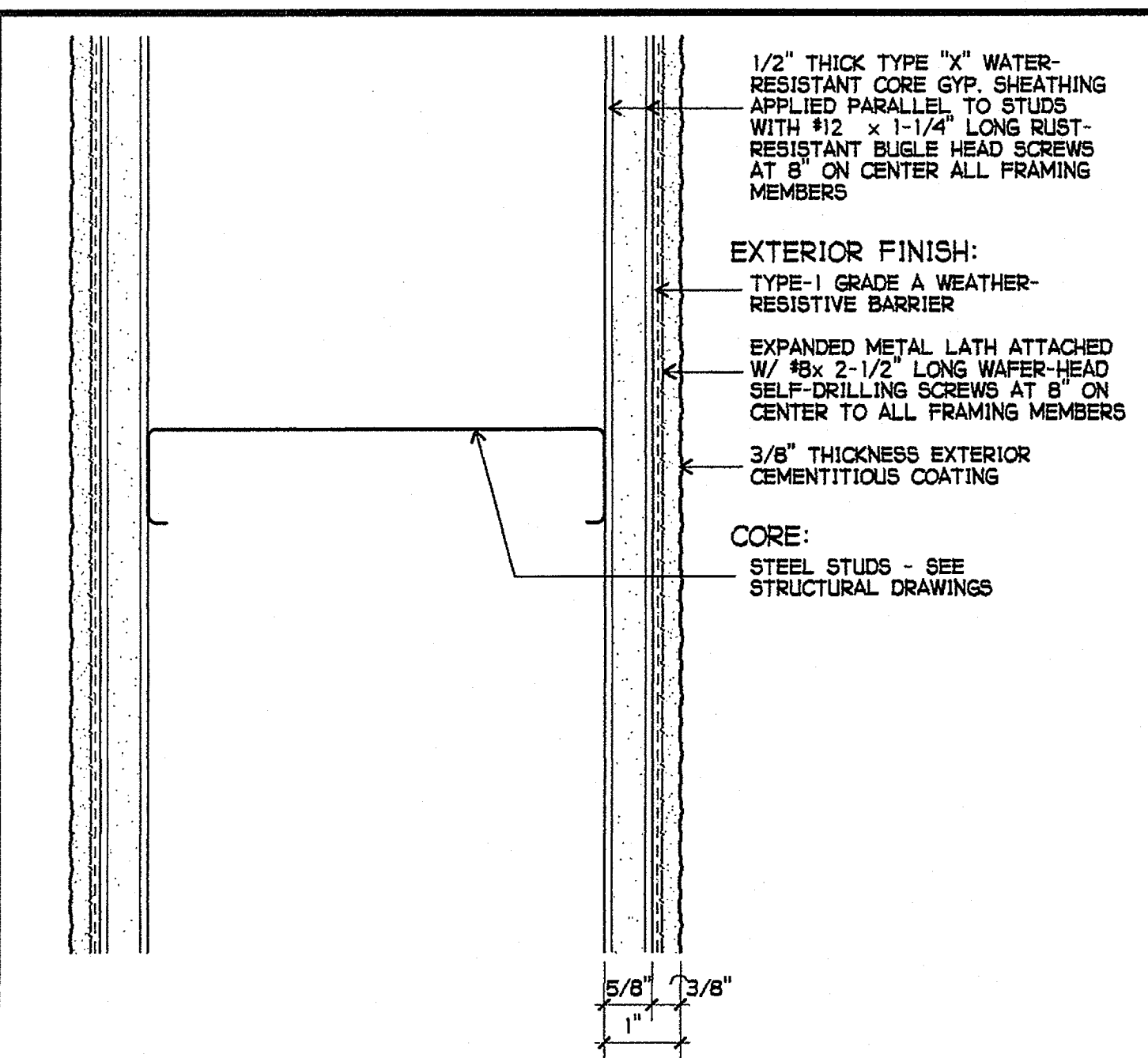
GROTH ARCHITECTS, INC.
 3355 MISSION AVE. SUITE 234
 OCEANSIDE, CALIFORNIA 92054
 PHONE 760-754-8191
 FAX 760-754-8291

DSA
 IDENTIFICATION STAMP
 DIV. OF THE STATE ARCHITECT
 OFFICE OF REGULATION SERVICES
 4-106494
 AC. *PL* FL. *SK* SS. *SS*
 DATE MAR 28 2005

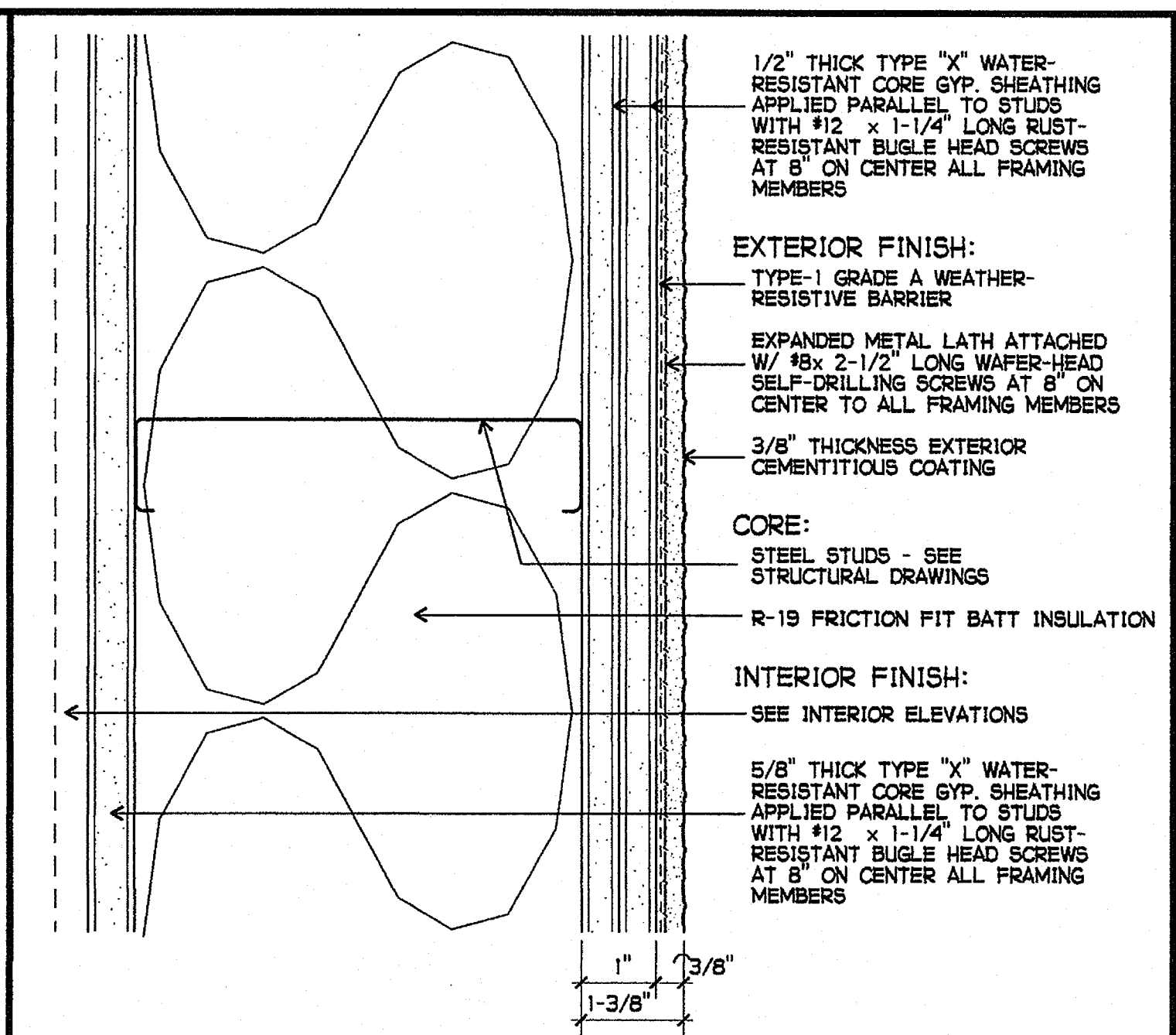
LICENSED ARCHITECT
 JOHN SCOTT GROTH
 C-26609
 4/30/2007
 RENEWAL
 STATE OF CALIFORNIA

DETAILS
 A9-10

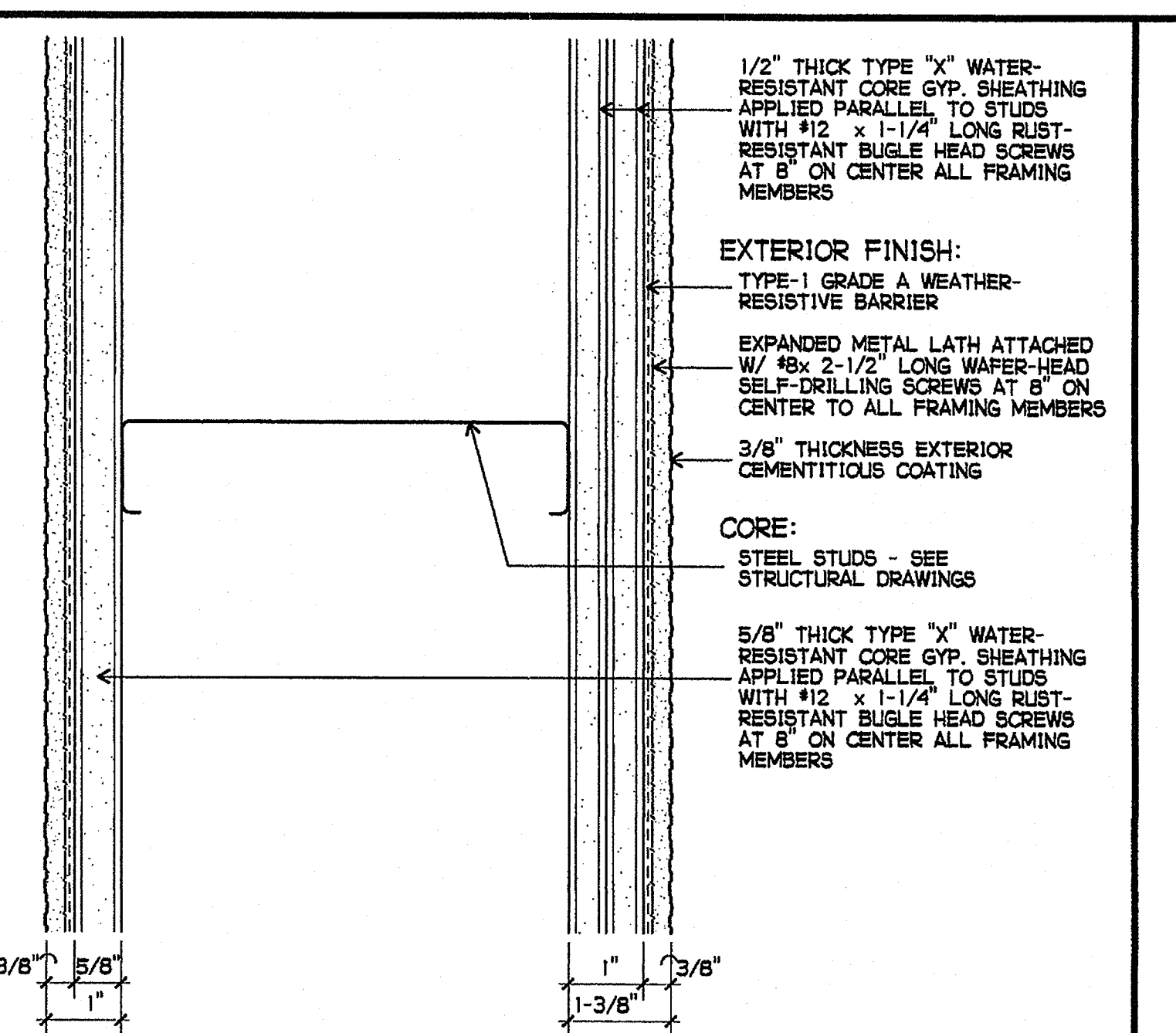
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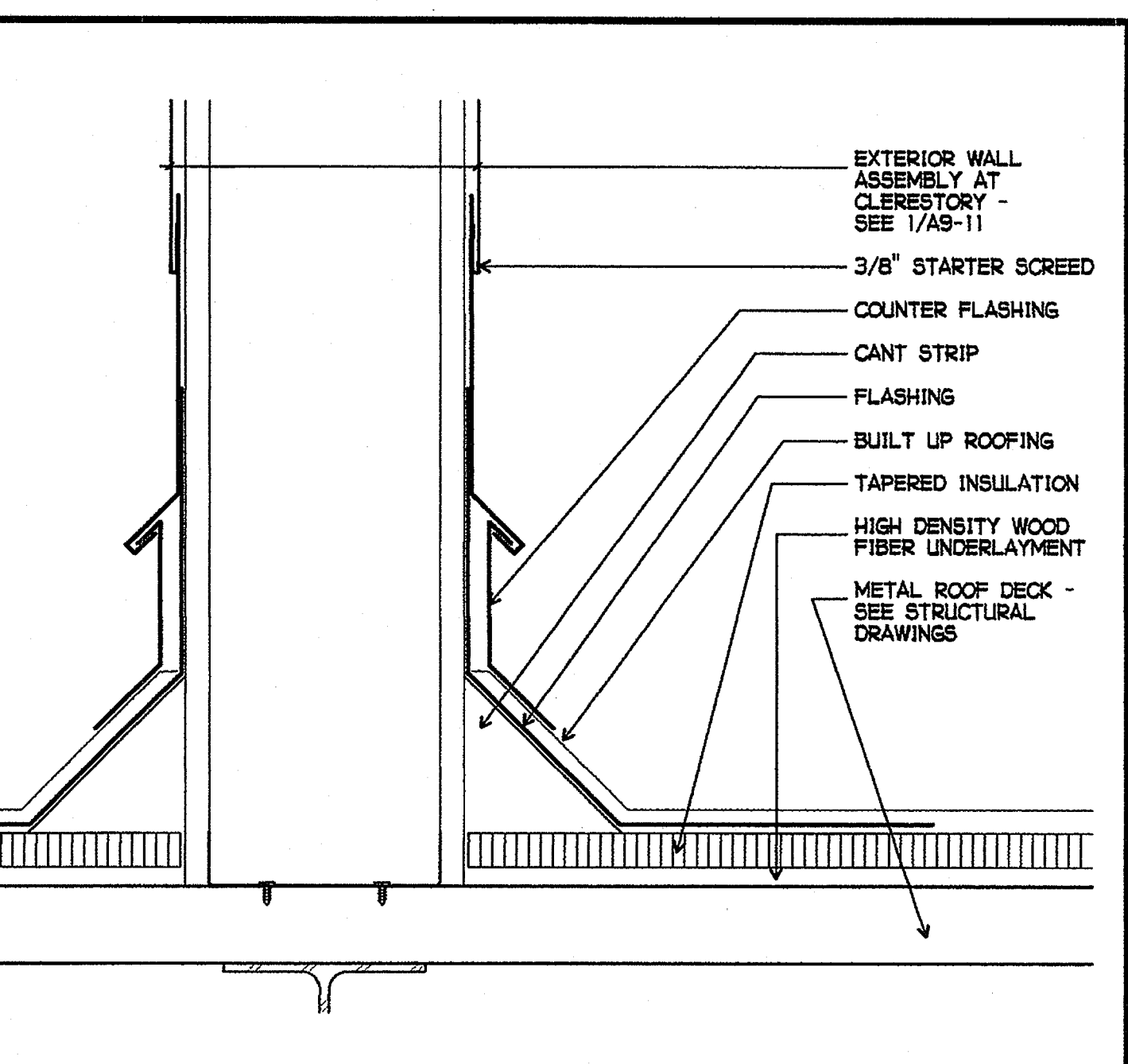
1 EXTERIOR WALL ASSEMBLY AT CLERESTORY
6" = 1'-0"



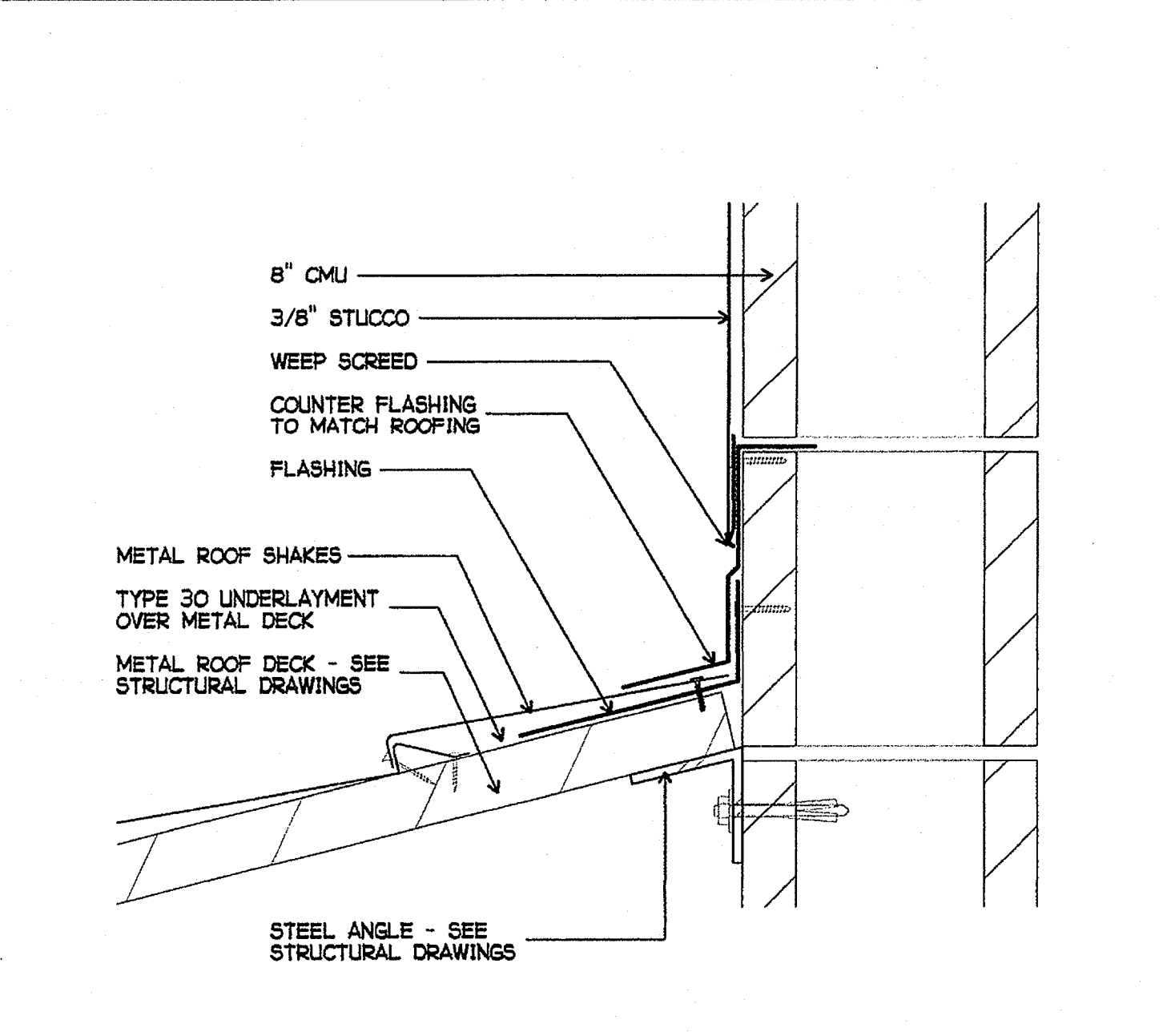
2 EXTERIOR WALL ASSEMBLY AT CLERESTORY
6" = 1'-0"



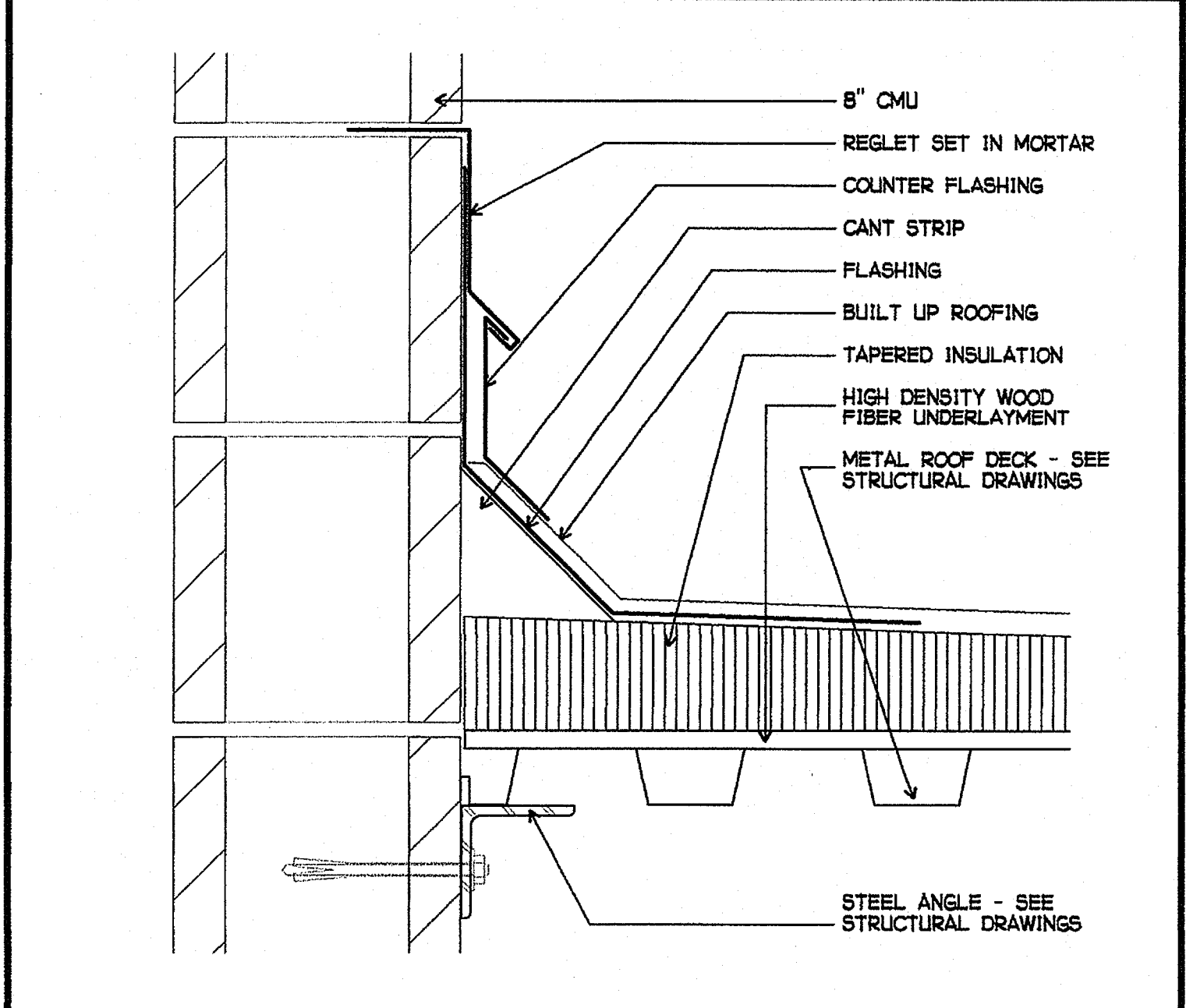
3 EXTERIOR WALL ASSEMBLY AT CLERESTORY
6" = 1'-0"



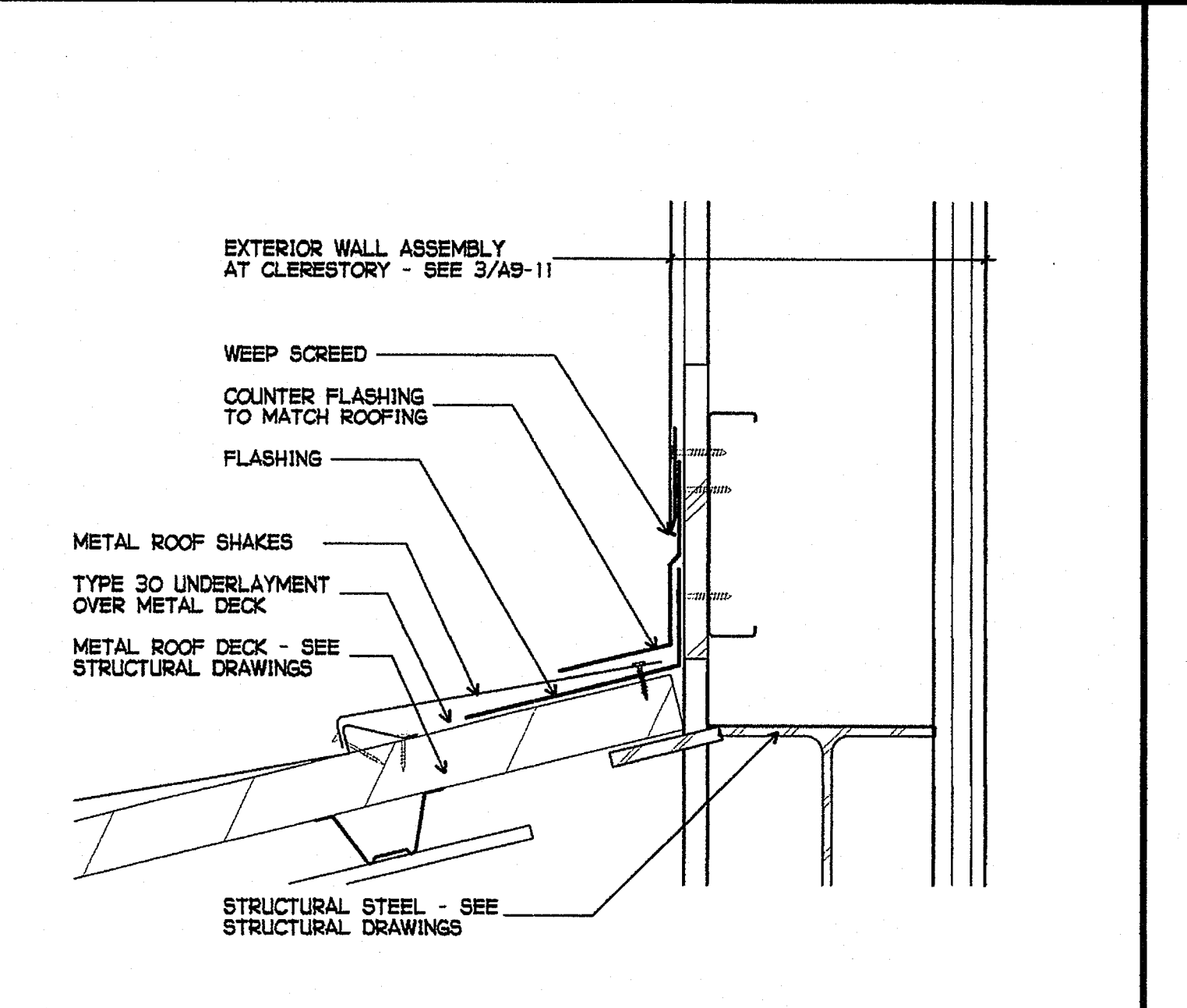
4 FLASHING AT CLERESTORY
3" = 1'-0"



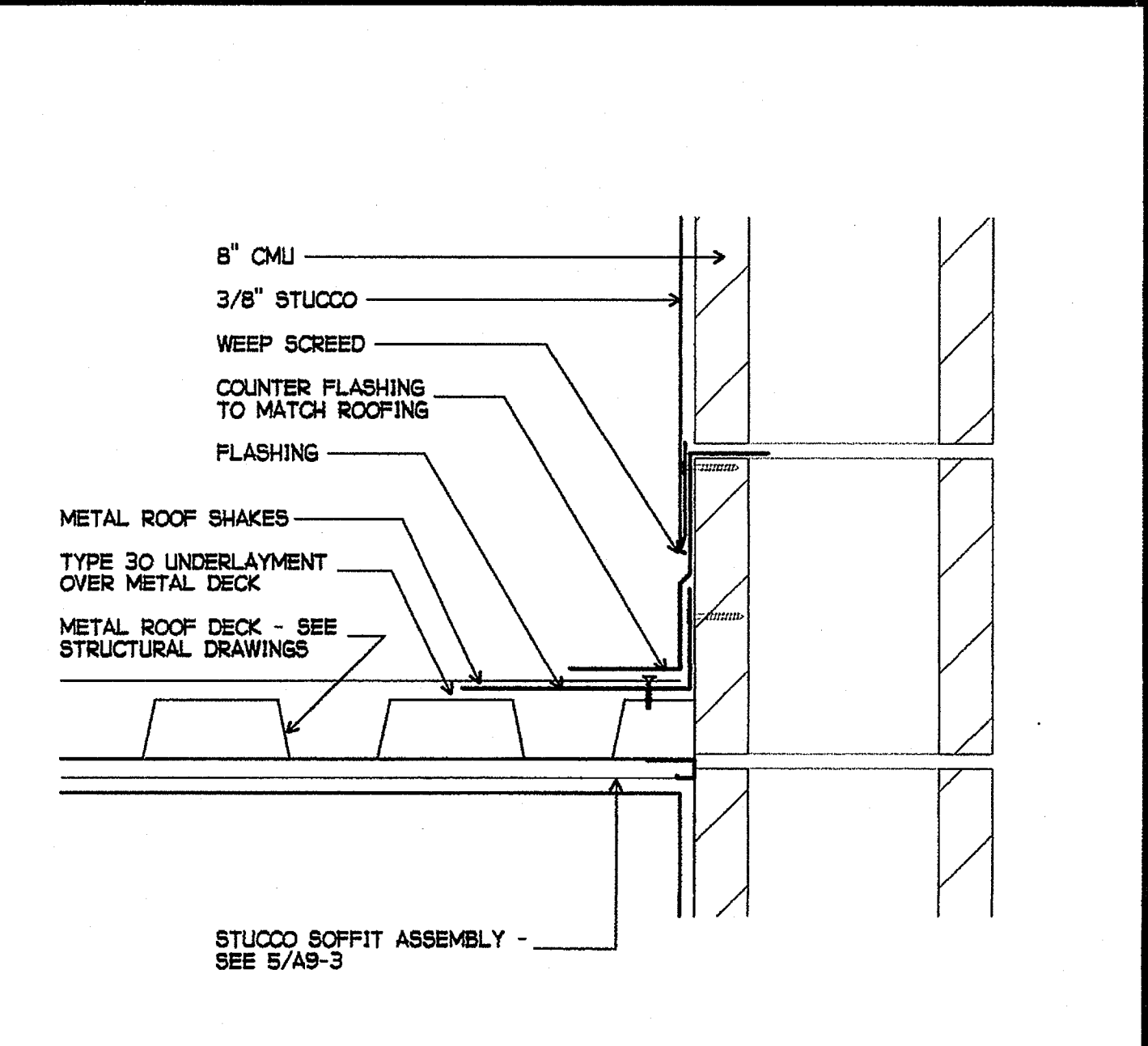
5 FLASHING AT CLERESTORY
3" = 1'-0"



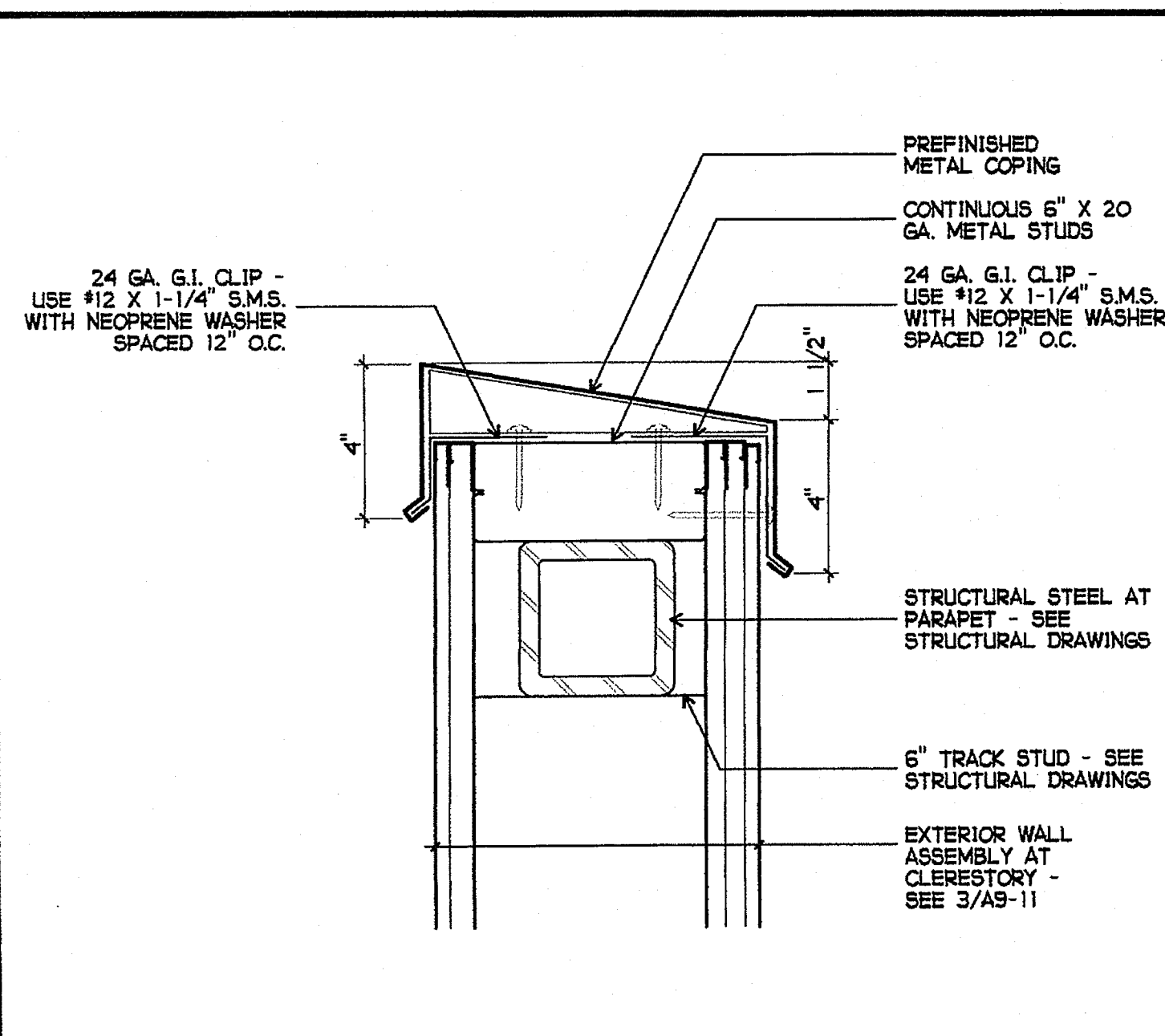
6 FLASHING AT CLERESTORY
3" = 1'-0"



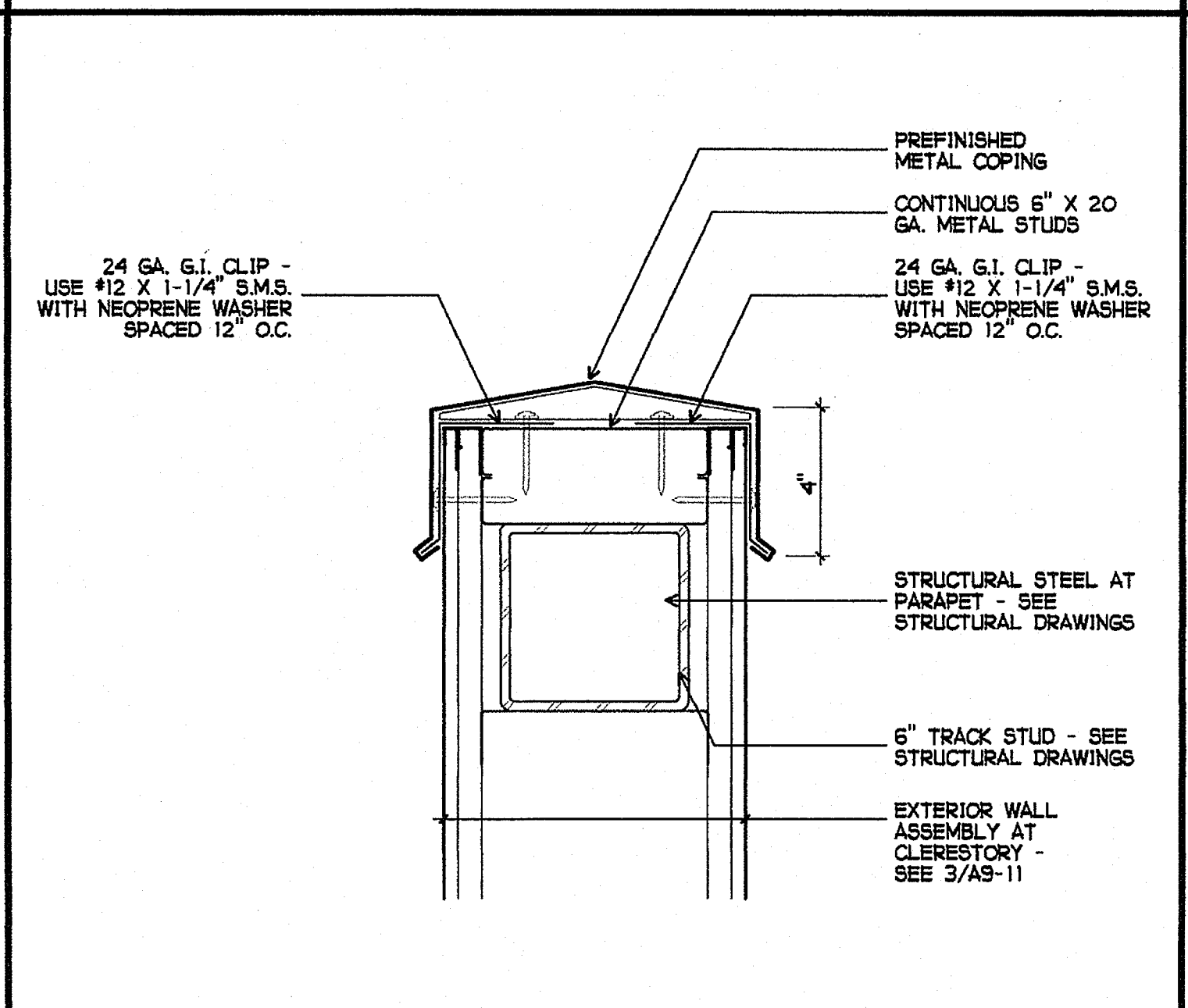
7 FLASHING AT CLERESTORY
3" = 1'-0"



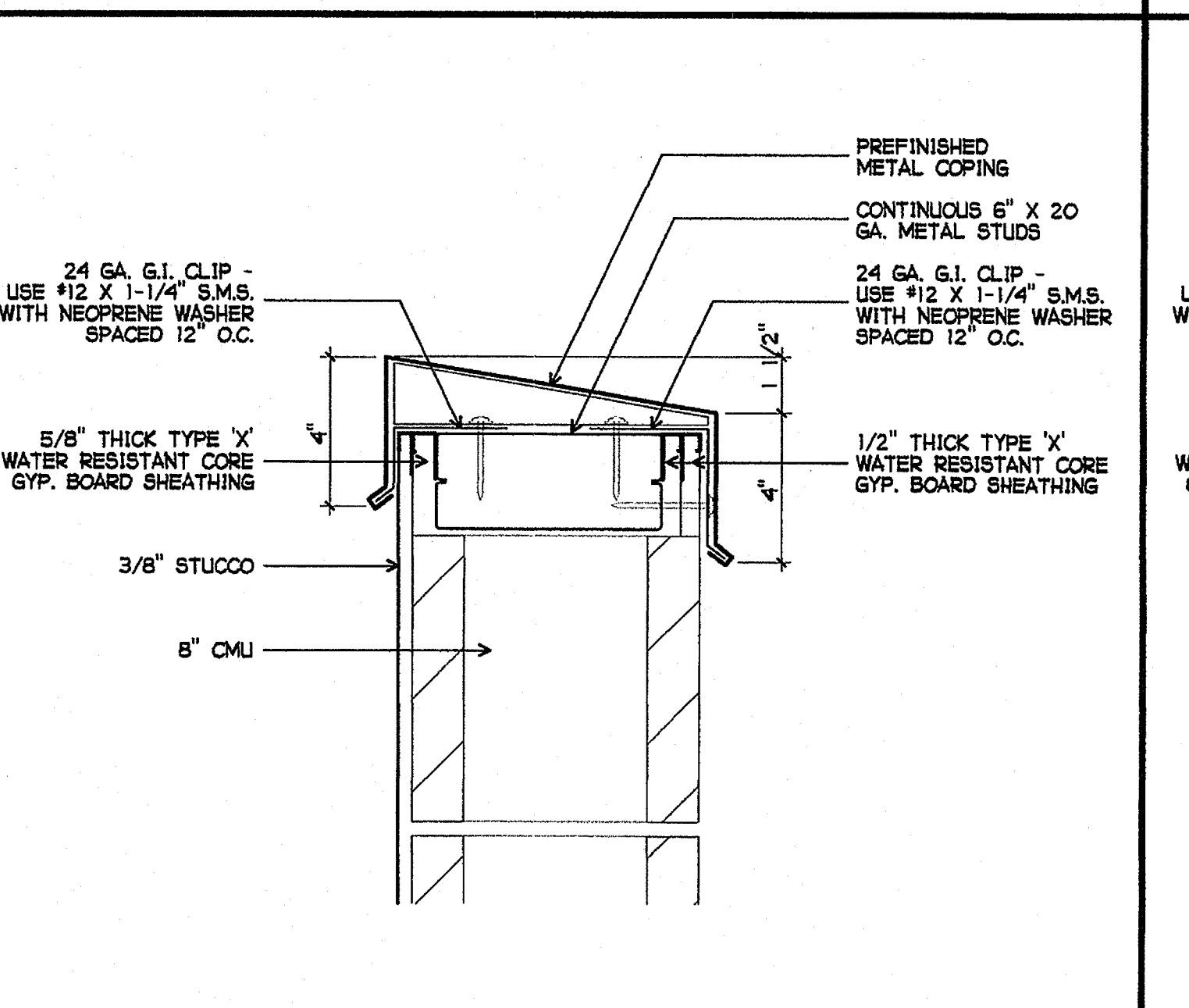
8 FLASHING AT CLERESTORY
3" = 1'-0"



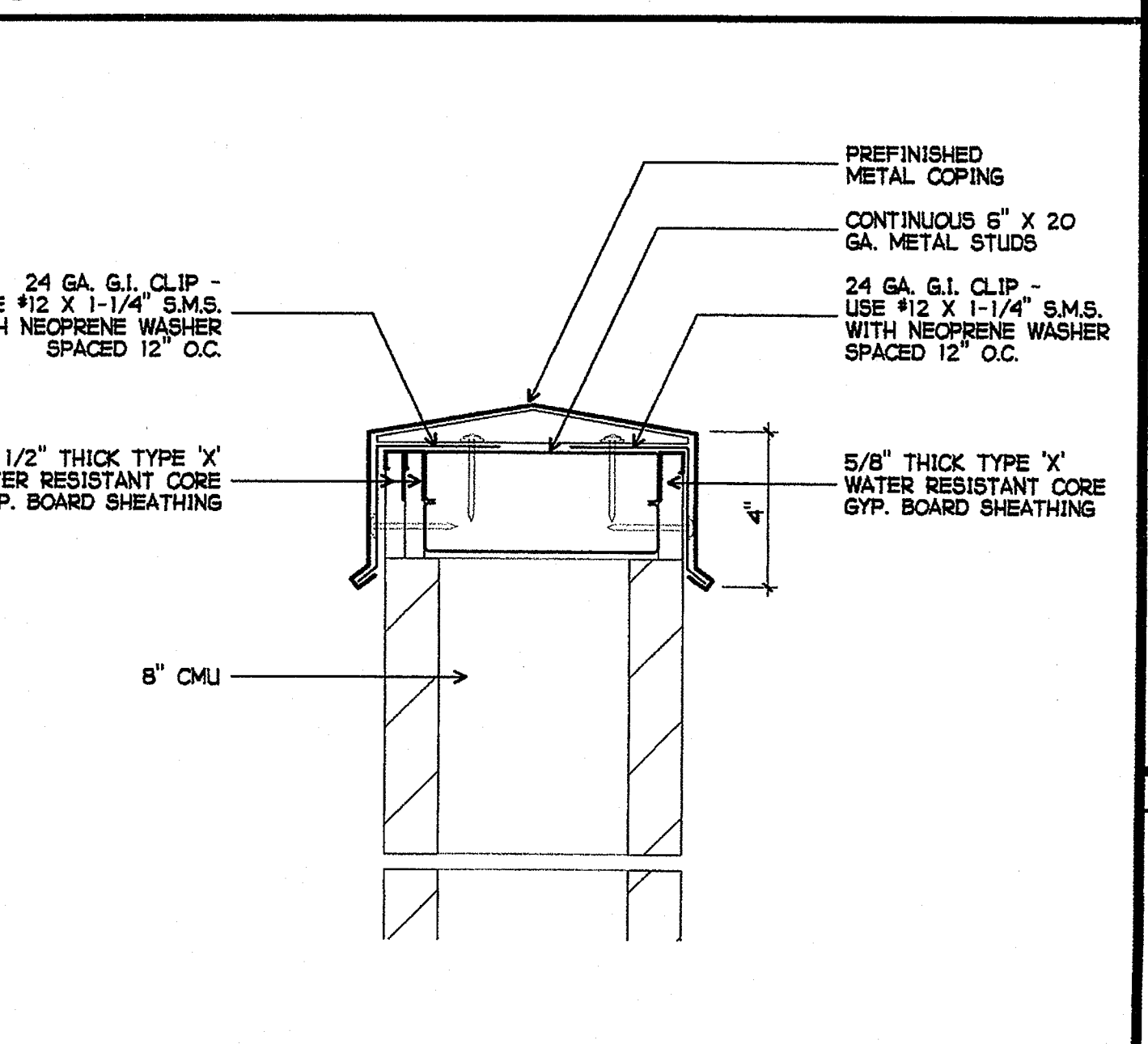
9 COPING AT CLERESTORY PARAPET
3" = 1'-0"



10 COPING AT CLERESTORY PARAPET
3" = 1'-0"



11 COPING AT CLERESTORY PARAPET
3" = 1'-0"



12 COPING AT CLERESTORY PARAPET
3" = 1'-0"

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GROTH ARCHITECTS, INC.
823 ACACIA STREET
OCEANSIDE, CA 92054
OCEANSIDE UNIFIED S.D.

JEFFERSON MS NEW CONSTRUCTION

CLSD NO. 758-000
PROJECT NOS. 025
P. T. N. 73569-9
DATE
REVISIONS

3355 MISSION AVE. SUITE 234
OCEANSIDE, CALIFORNIA 92054

PHONE 760-754-8191
FAX 760-754-8291

space art
function time

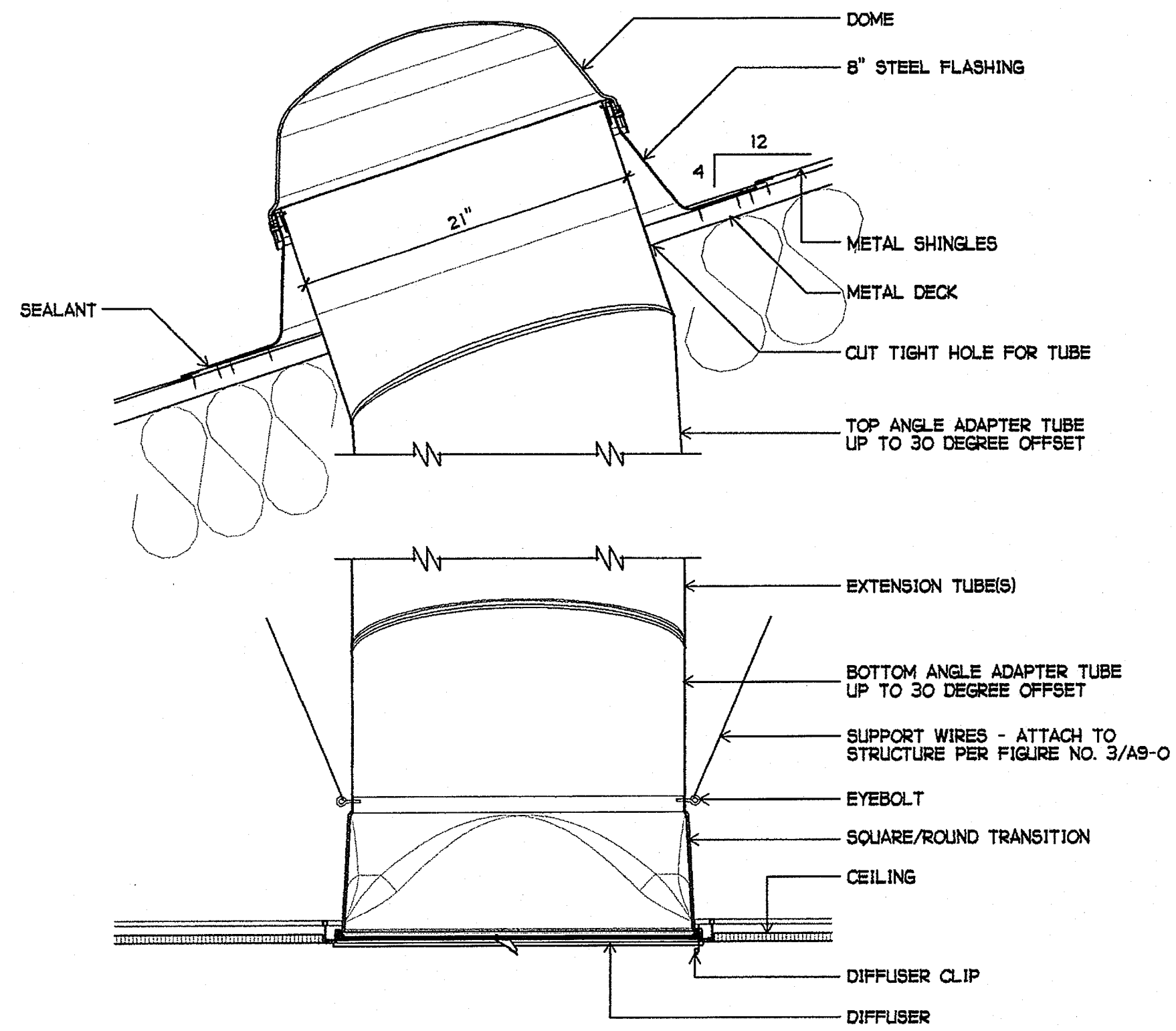
DSA
IDENTIFICATION STAMP
DIV. OF THE STATE ARCHITECT
OFFICE OF REGULATION SERVICES
4-106494
AC PLB FLJ SS
DATE: MAR 28 2005

REGISTERED ARCHITECT
JOHN SCOTT GROTH
C-26609
4/30/2007 RENEWAL
STATE OF CALIFORNIA

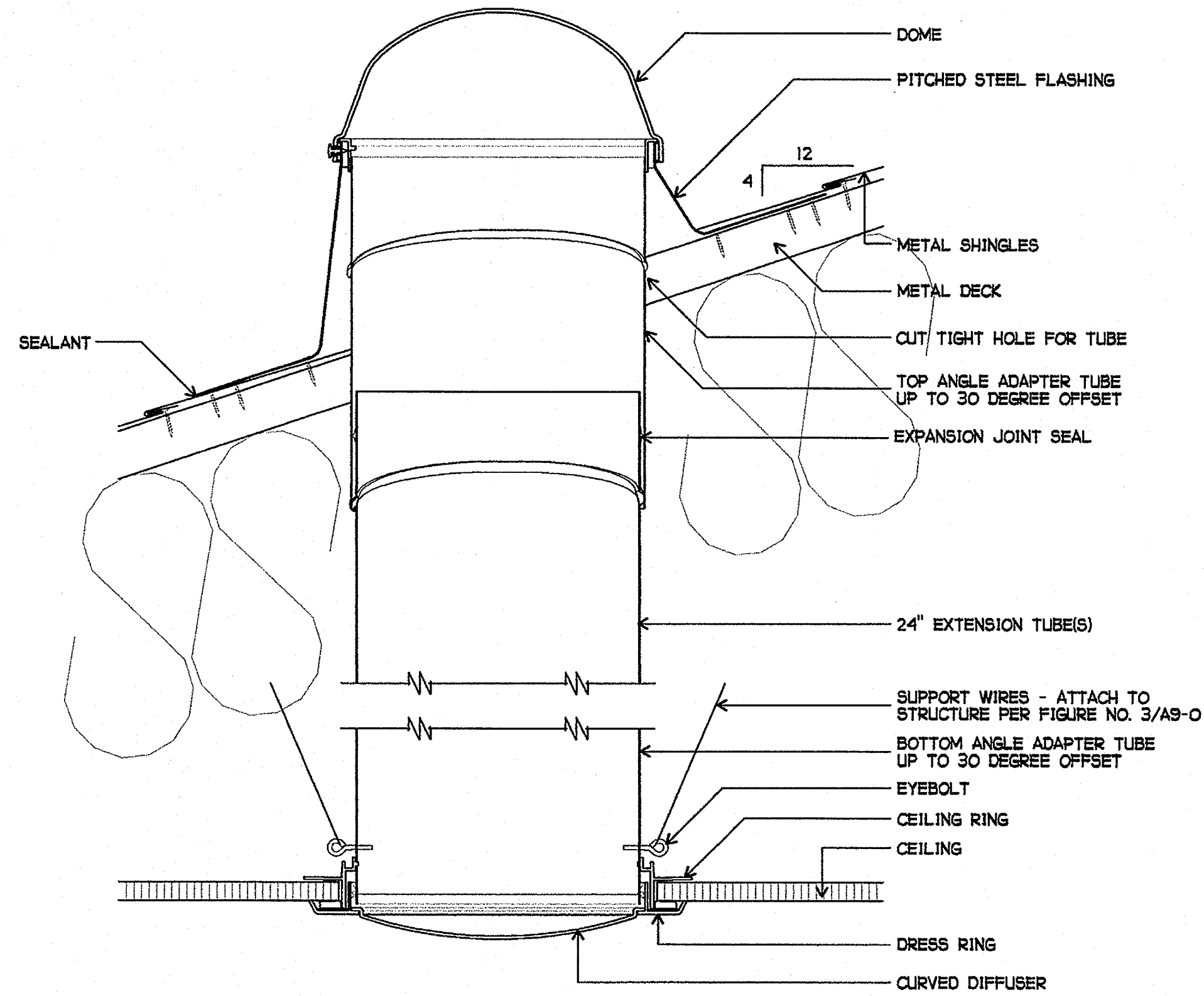
SHEET TITLE
DETAILS

A9-11

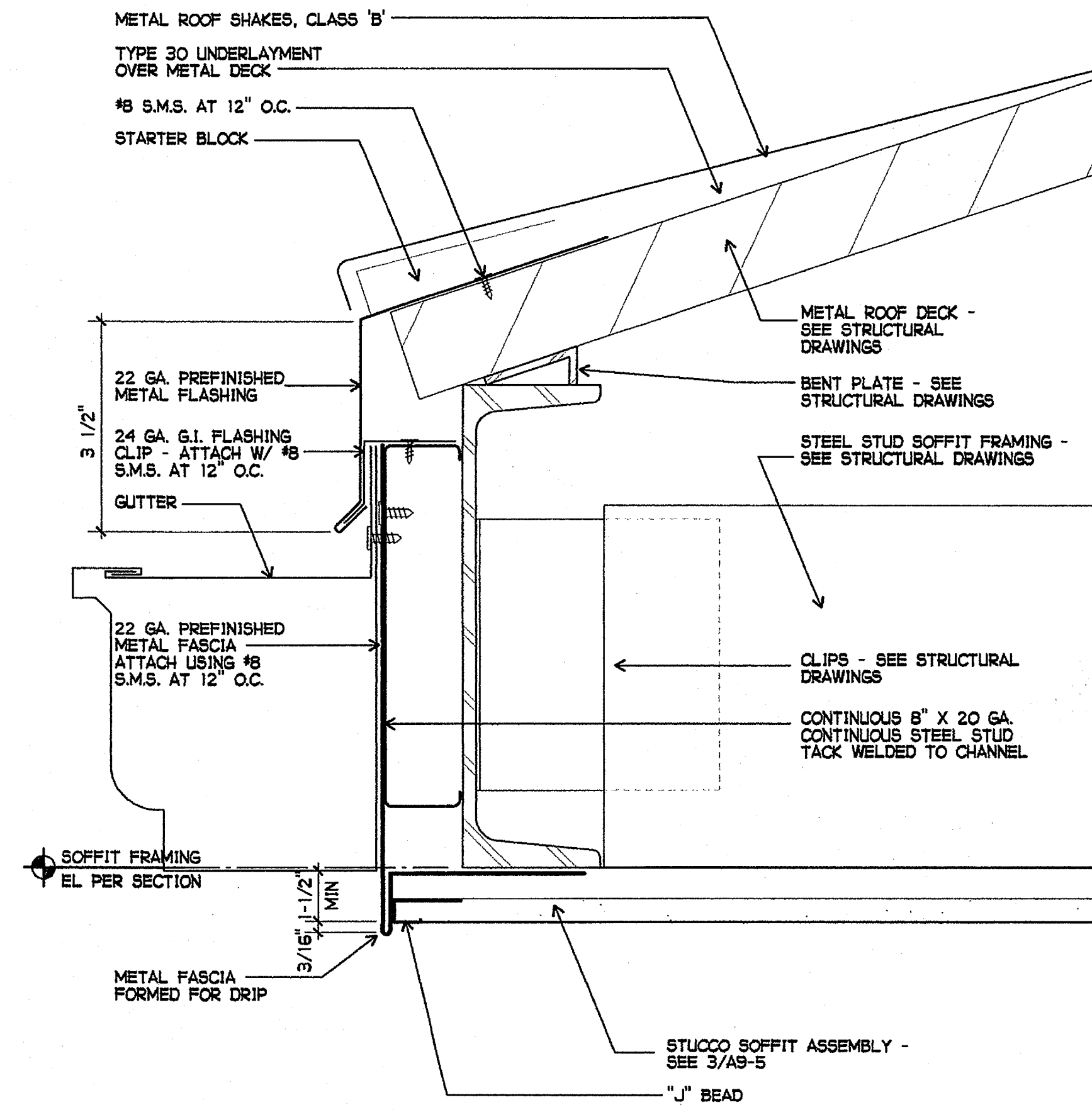
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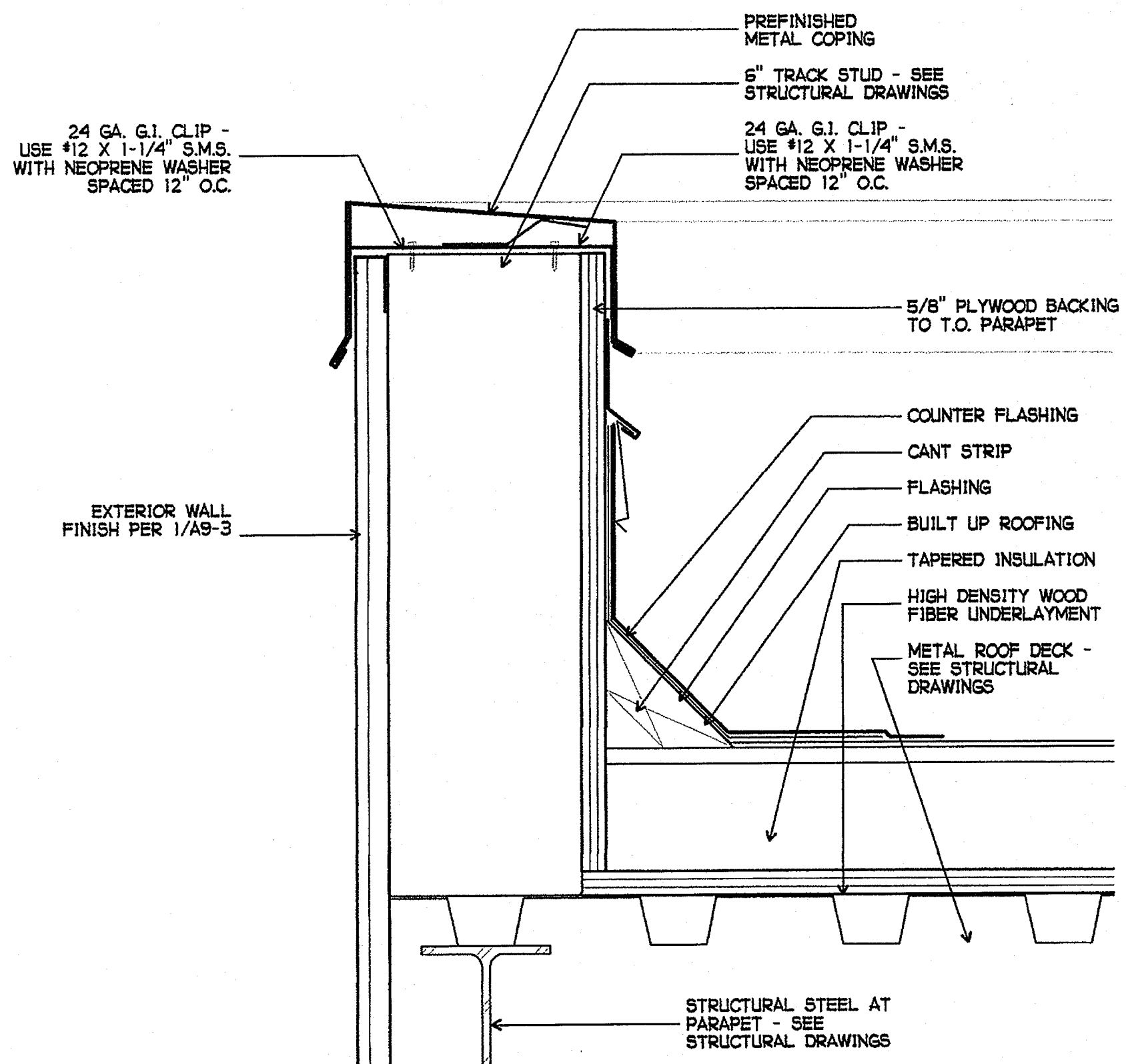
1 21" TUBULAR SKYLIGHT AT METAL SHAKE ROOFING/ACOUSTICAL CEILING
1-1/2" = 1'-0"



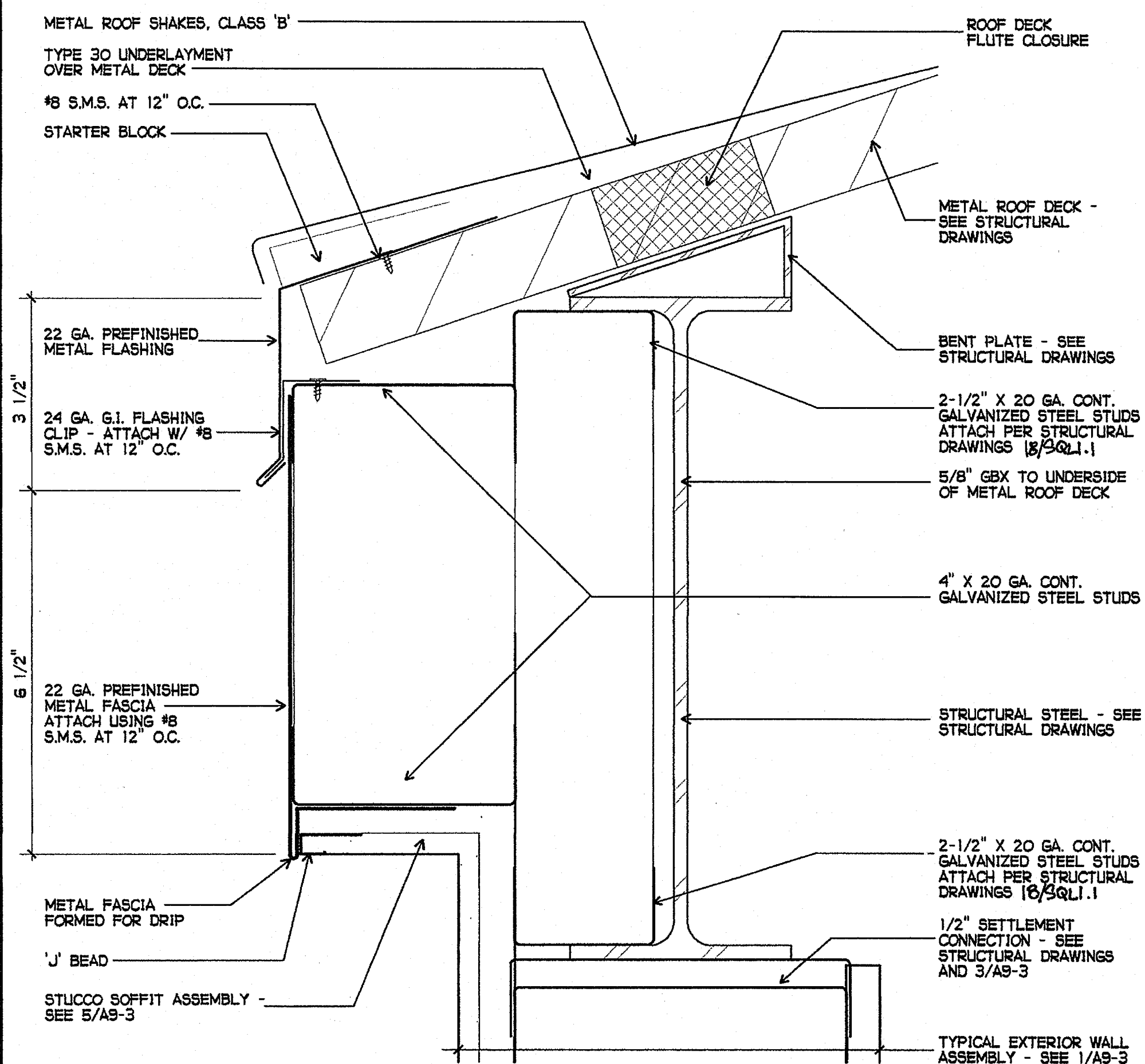
2 10" TUBULAR SKYLIGHT AT METAL SHAKE ROOFING/ACOUSTICAL CEILING
3" = 1'-0"



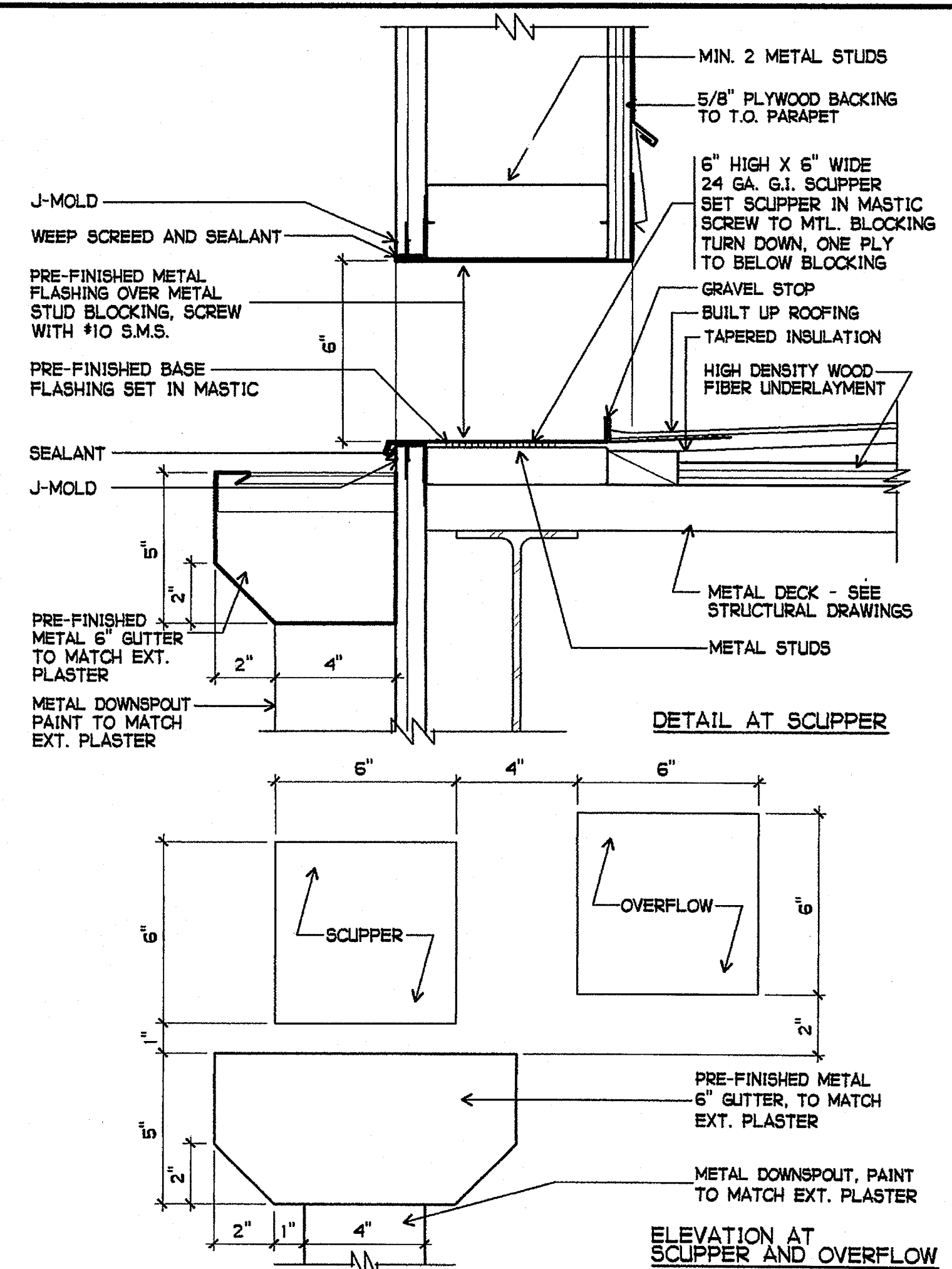
3 ROOF EAVE
6" = 1'-0"



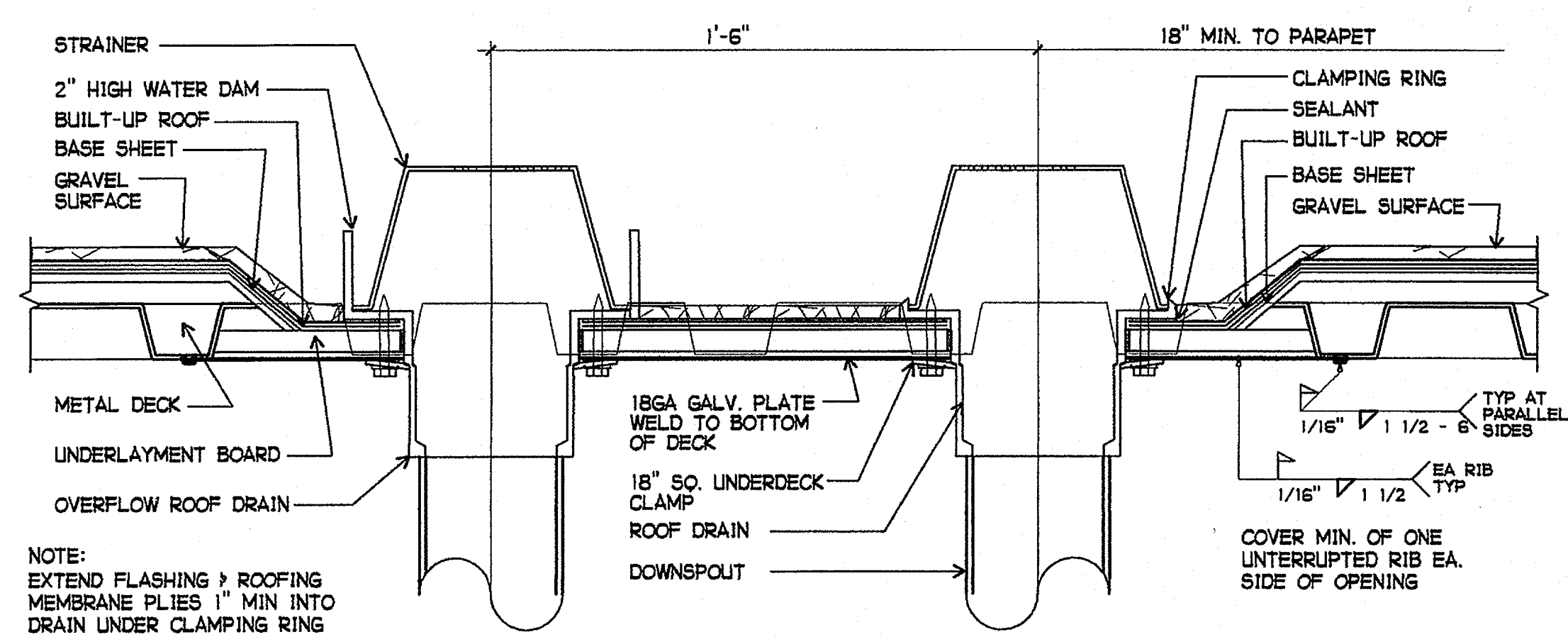
4 PARAPET AT ELEVATOR ROOF
3" = 1'-0"



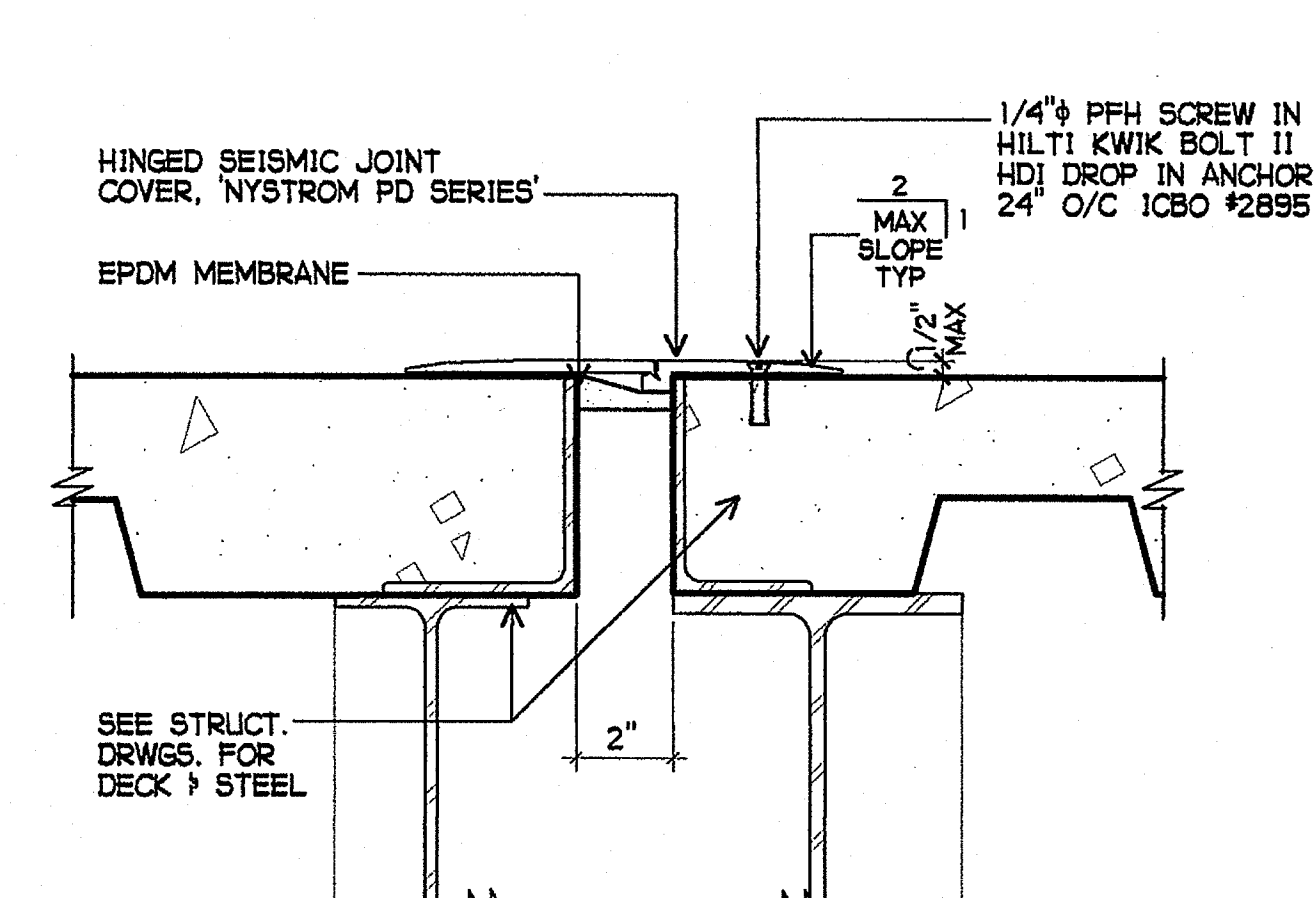
5 EAVE AT ELEVATOR PUMP ROOM
6" = 1'-0"



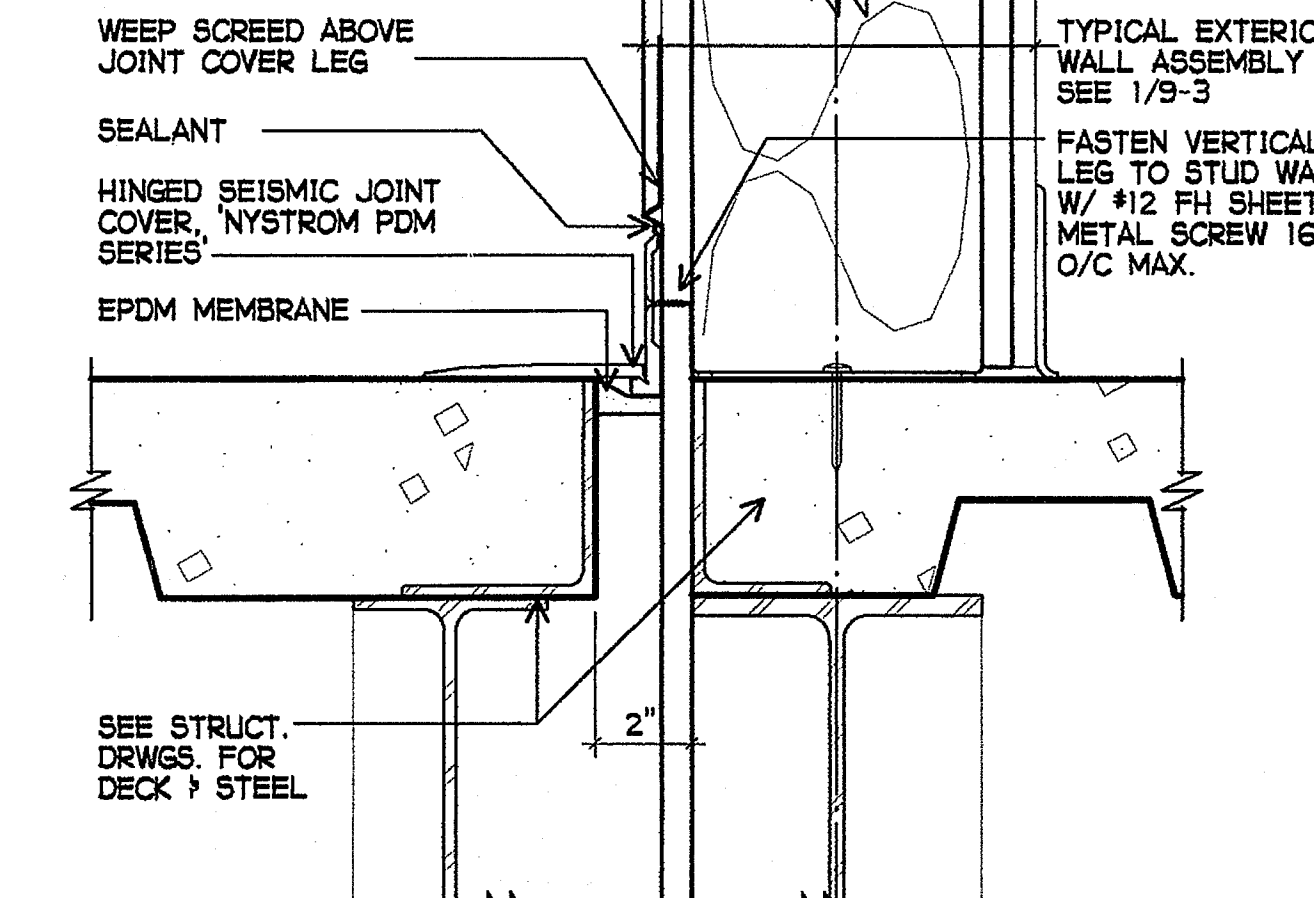
6 SCUPPER/DOWNSPOUT AT ELEVATOR ROOF
3" = 1'-0"



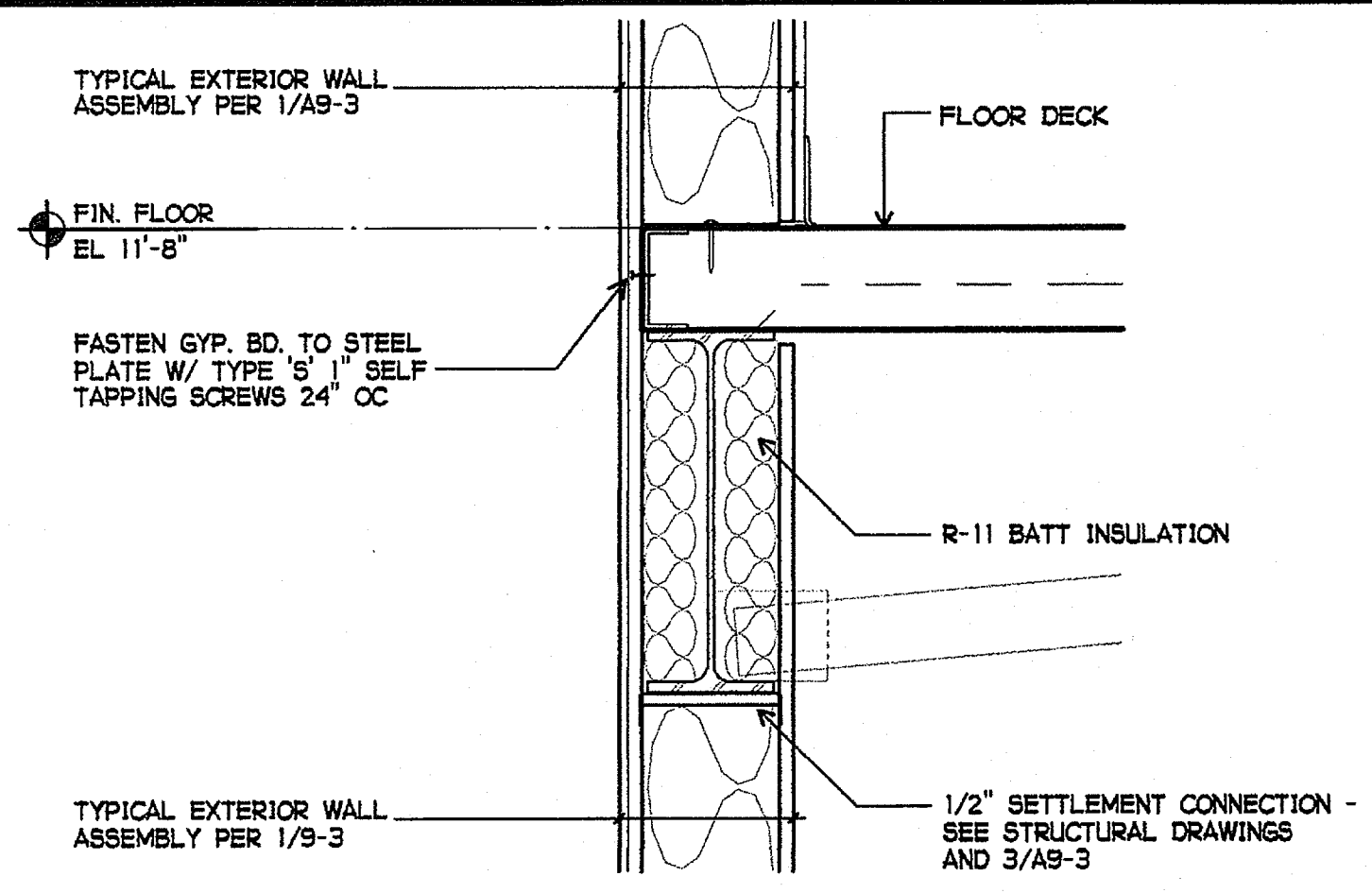
1 ROOF AND OVERFLOW DRAIN
NOT TO SCALE



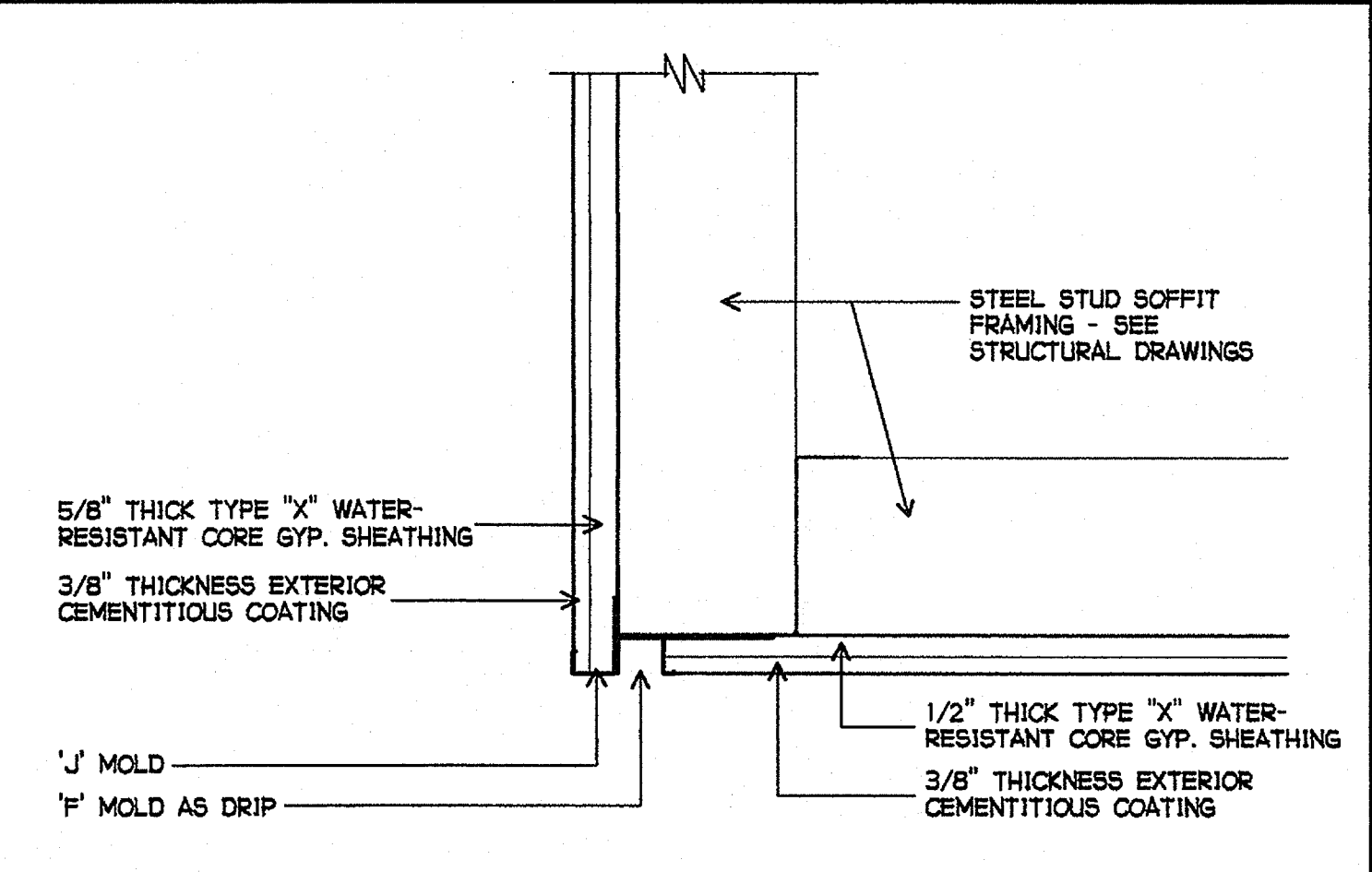
2 EXPANSION JOINT FLOOR TO FLOOR
3\"/>



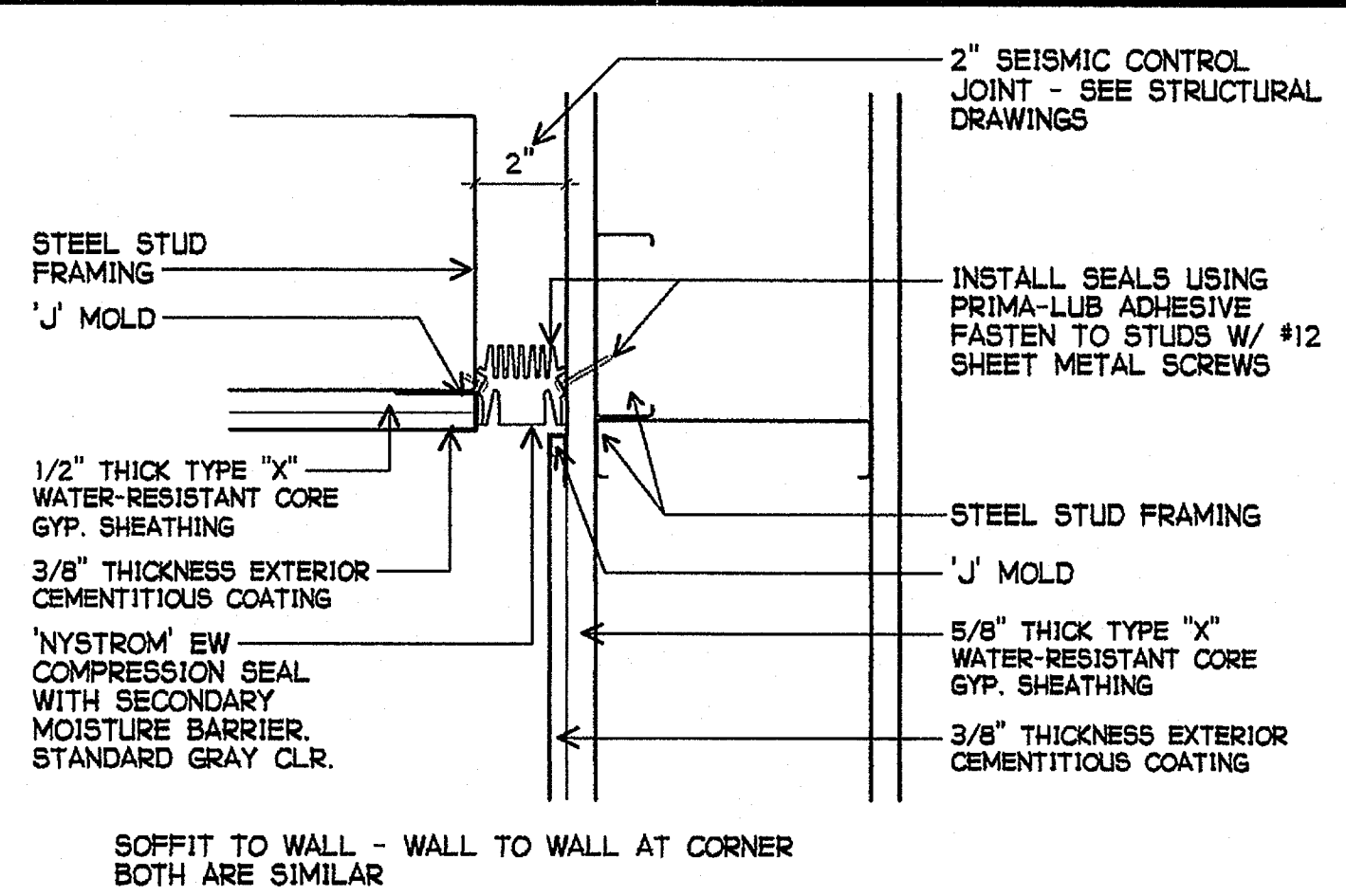
3 EXPANSION JOINT FLOOR TO WALL
3\"/>



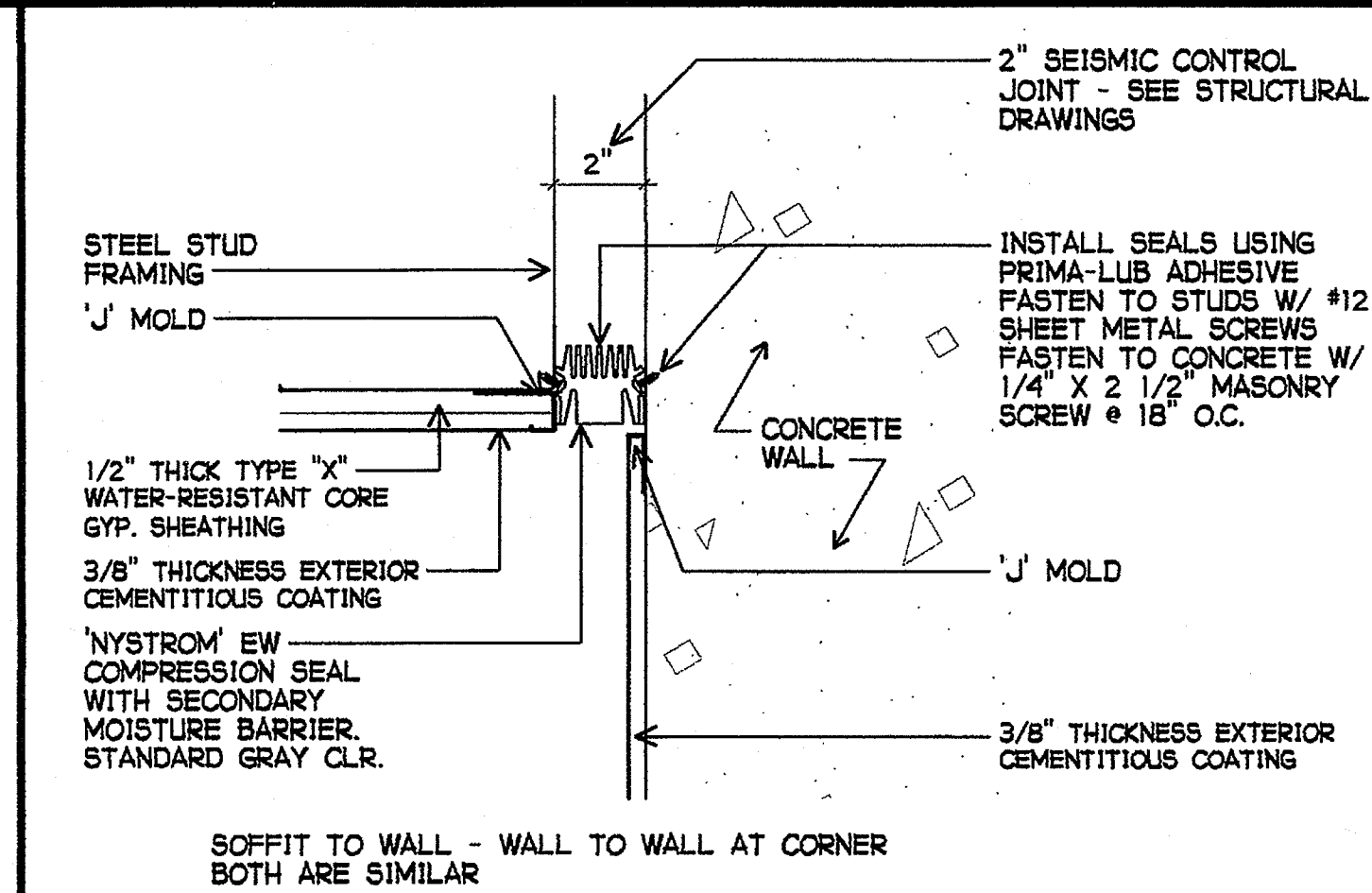
4 EXTERIOR WALL AT BEAM
1-1/2\"/>



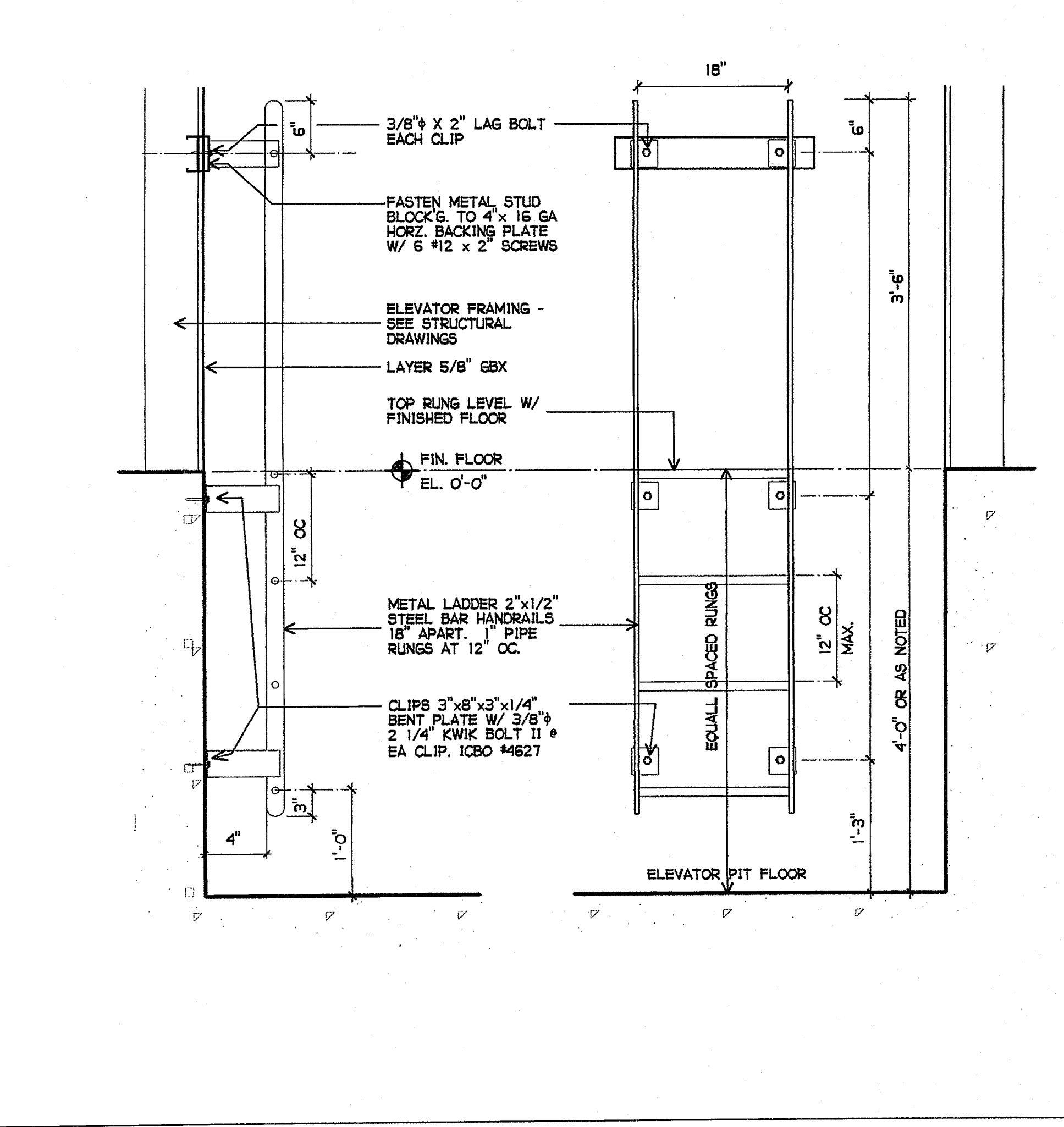
5 SOFFIT FASCIA DRIP
1-1/2\"/>



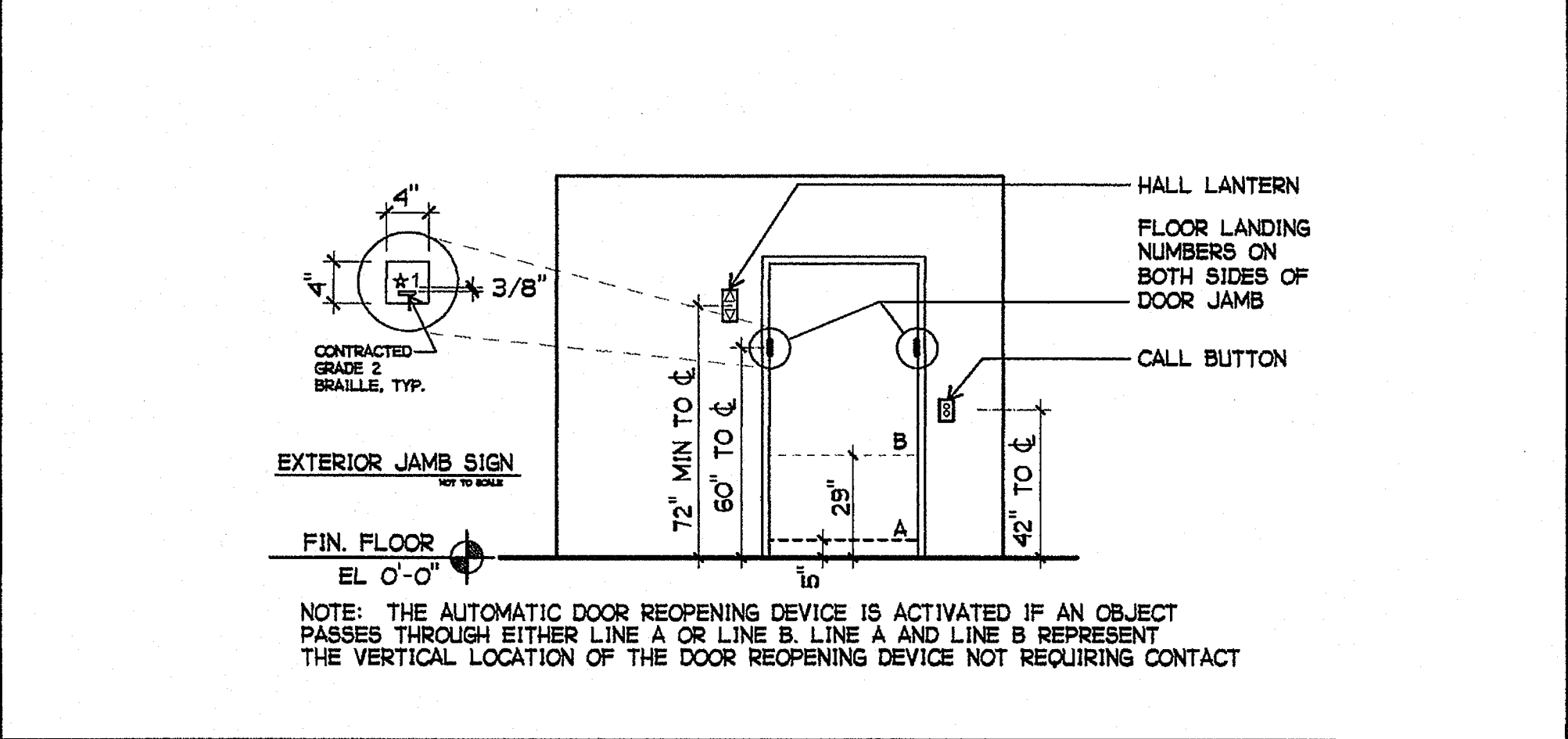
6 SEISMIC CONTROL AT STUCCO FINISH
3\"/>



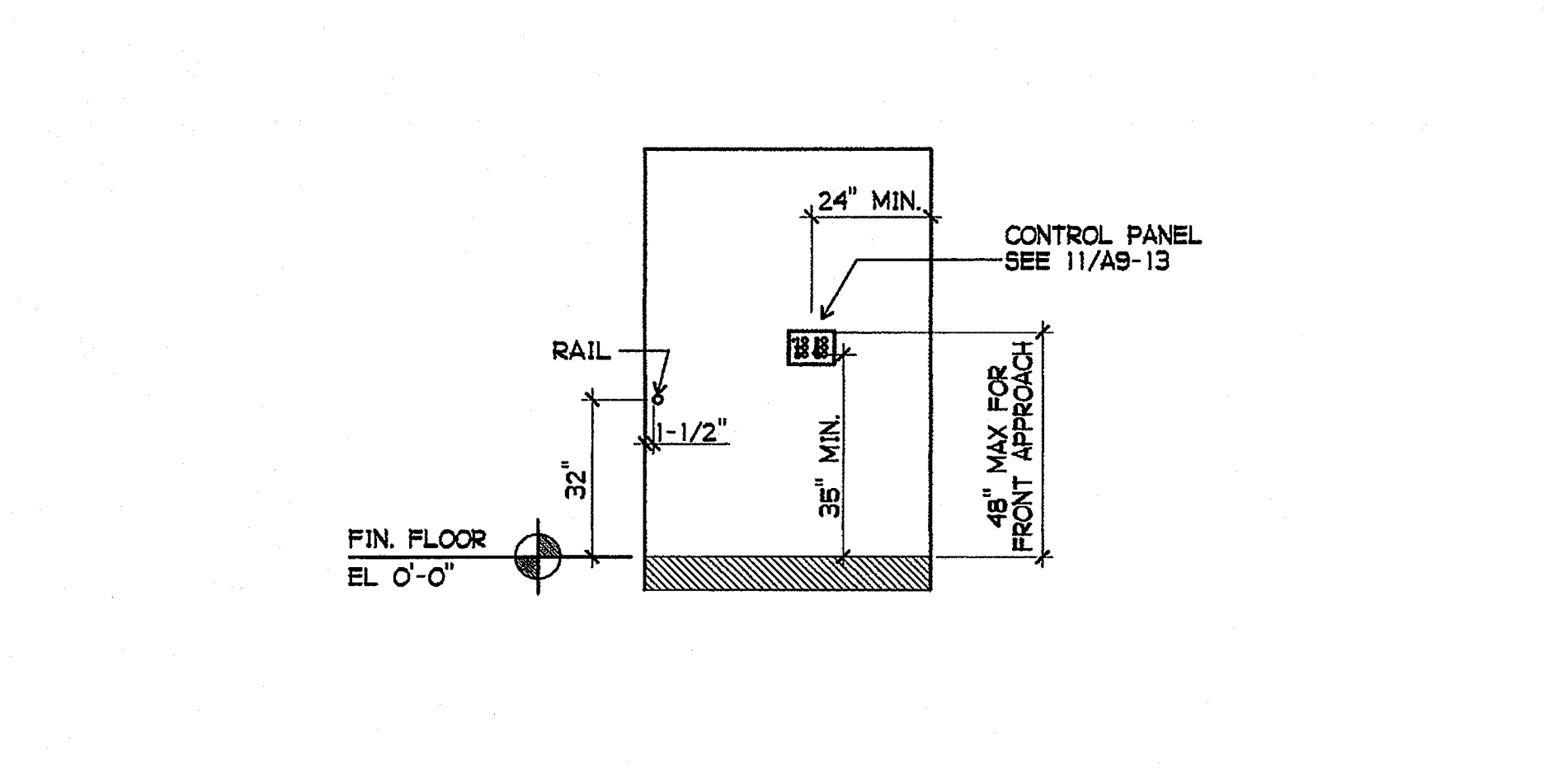
7 SEISMIC CONTROL AT STUCCO FINISH AT CONCRETE WALL
3\"/>



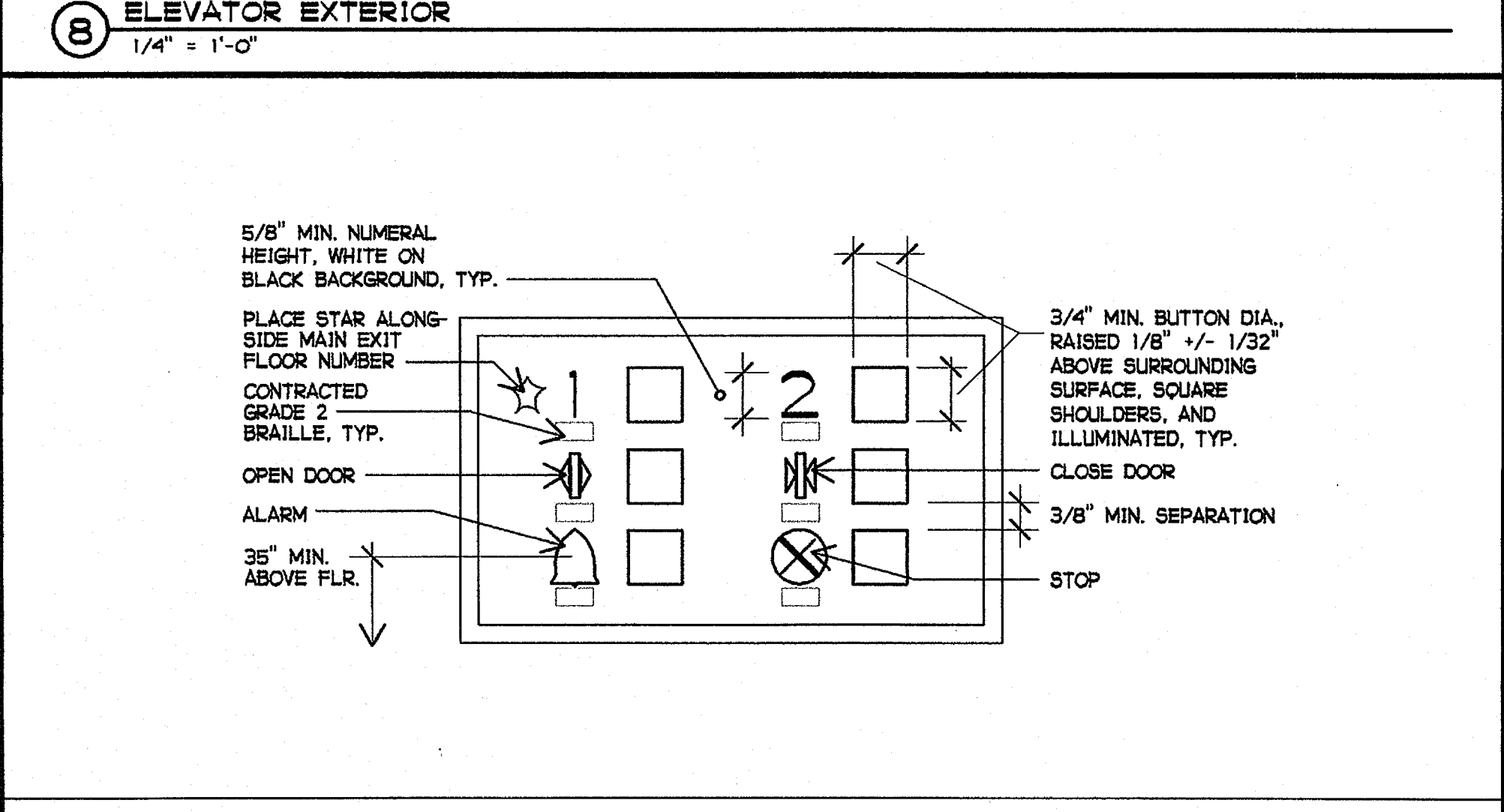
10 ELEVATOR PIT - LADDER AND SUMP
1\"/>



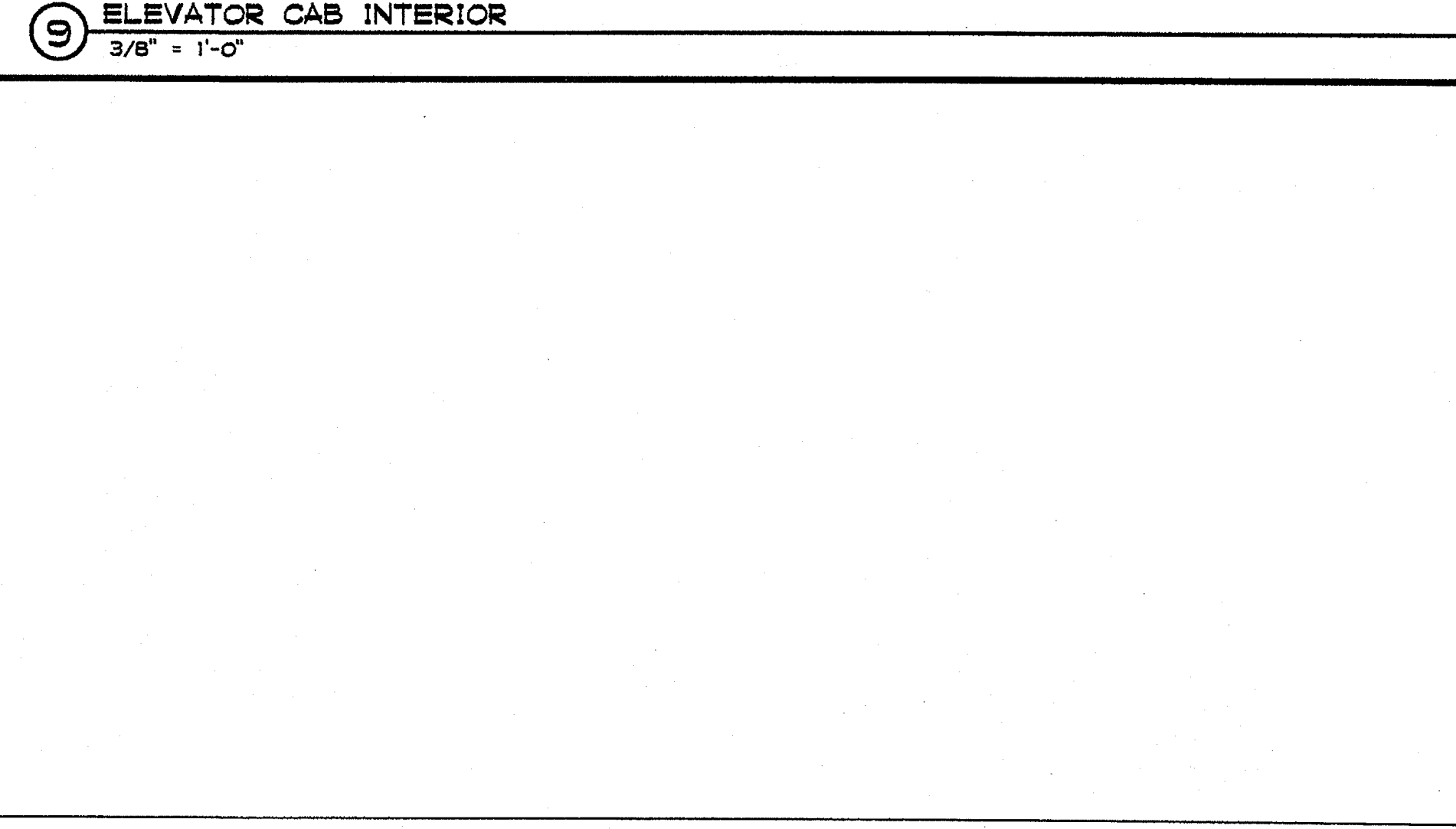
8 ELEVATOR EXTERIOR
1/4\"/>



9 ELEVATOR CAB INTERIOR
3/8\"/>



11 ELEVATOR CONTROL PANEL
NOT TO SCALE



12 NOT USED

PLOTTED 3/18/2005 12:07 PM

GROTH ARCHITECTS, INC. 823 ACACIA STREET OCEANSIDE, CA 92054 OCEANSIDE UNIFIED S.D.

JEFFERSON MS NEW CONSTRUCTION

PROJECT NOS. 758-000

DATE

REVISIONS

DSA IDENTIFICATION STAMP DIV. OF THE STATE ARCHITECT OFFICE OF REGULATION SERVICES 4-106494 AC PLS SS DATE MAR 28 2005

LICENSURE ARCHITECT JOHN SCOTT GROTH C-26609 4/30/2007 RENEWAL STATE OF CALIFORNIA

SHEET TITLE

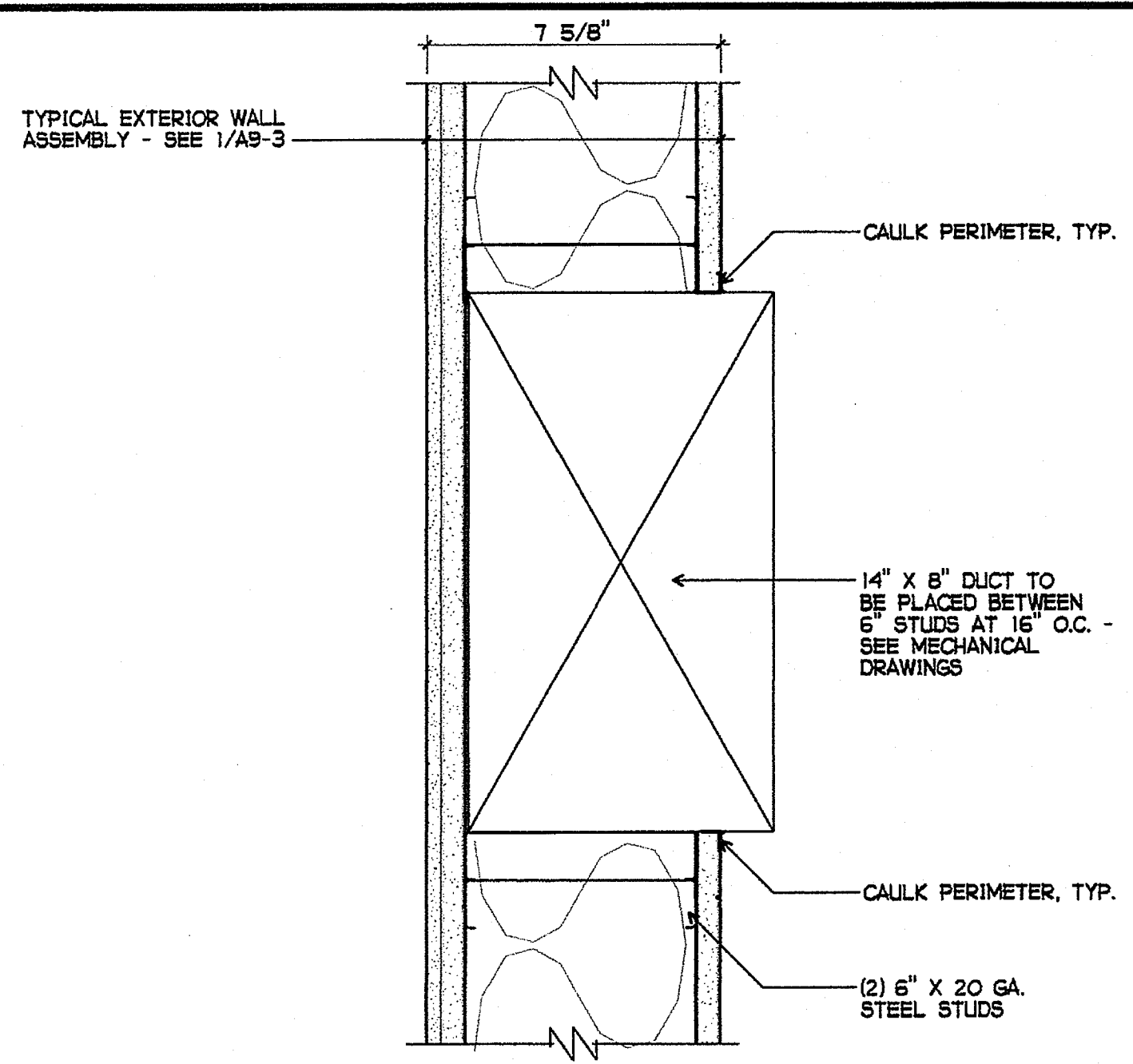
DETAILS

A9-13

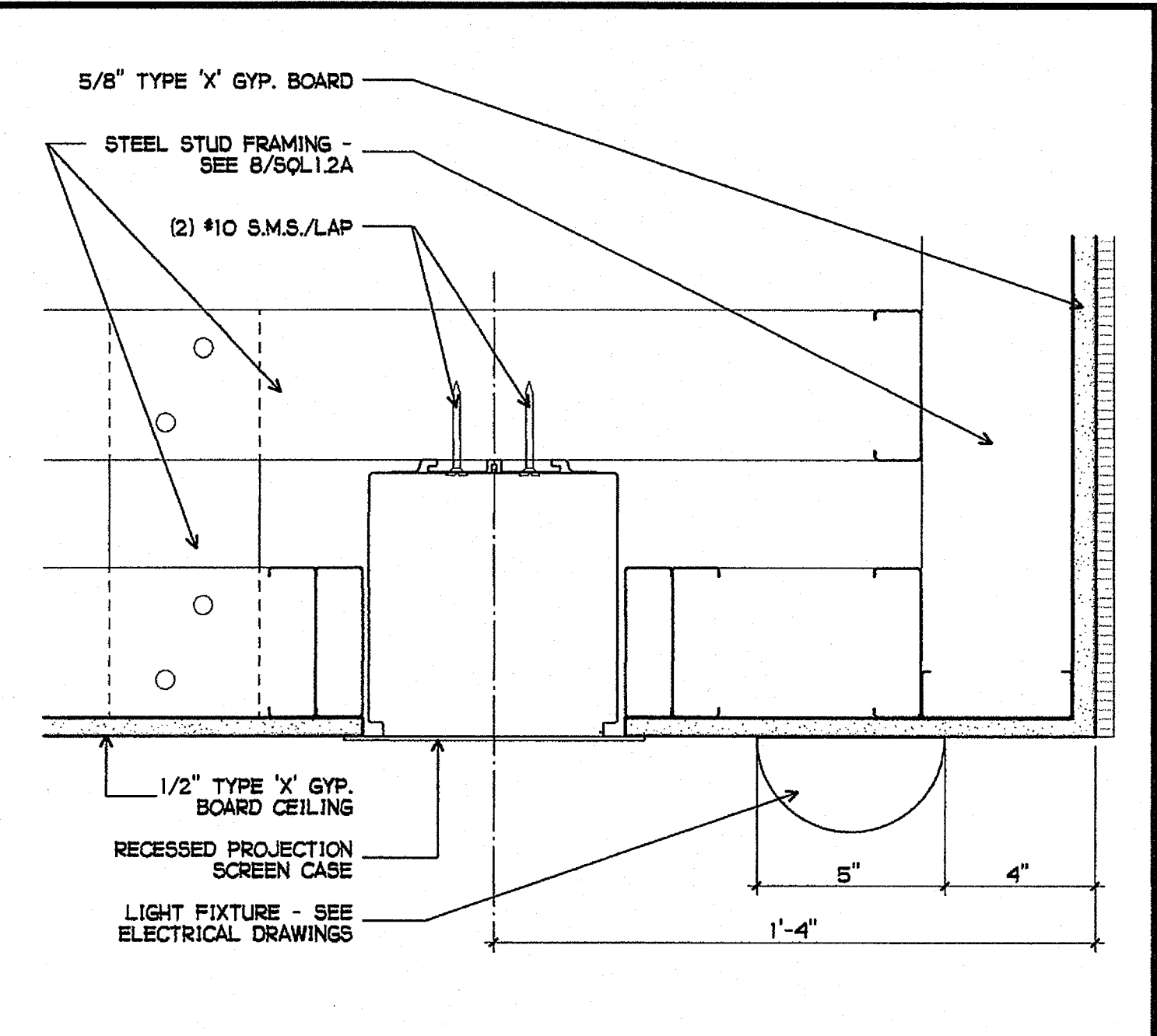
PHONE 760-754-8191 FAX 760-754-8291

SUITE 234 3355 MISSION AVE. OCEANSIDE, CALIFORNIA 92054

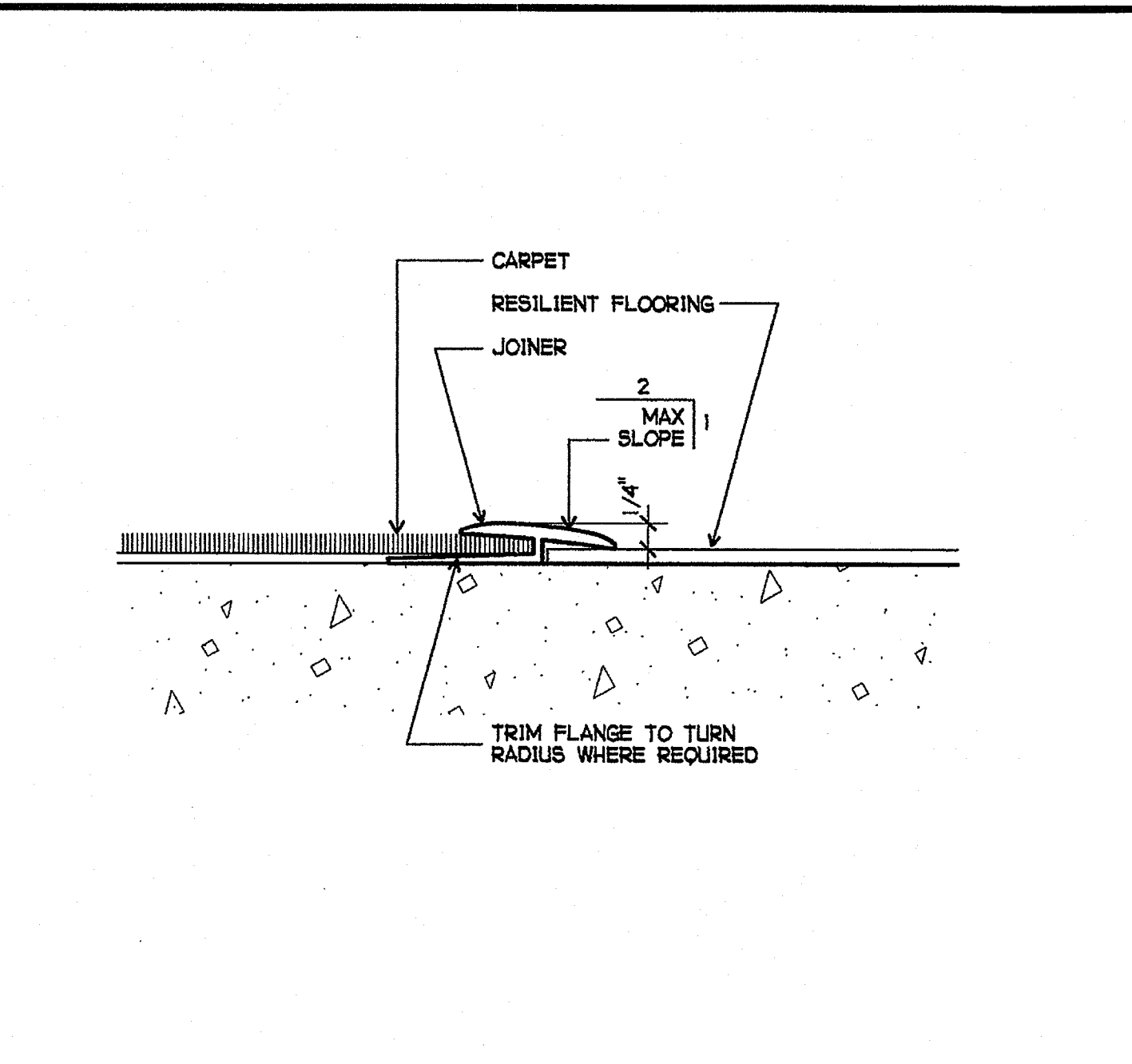
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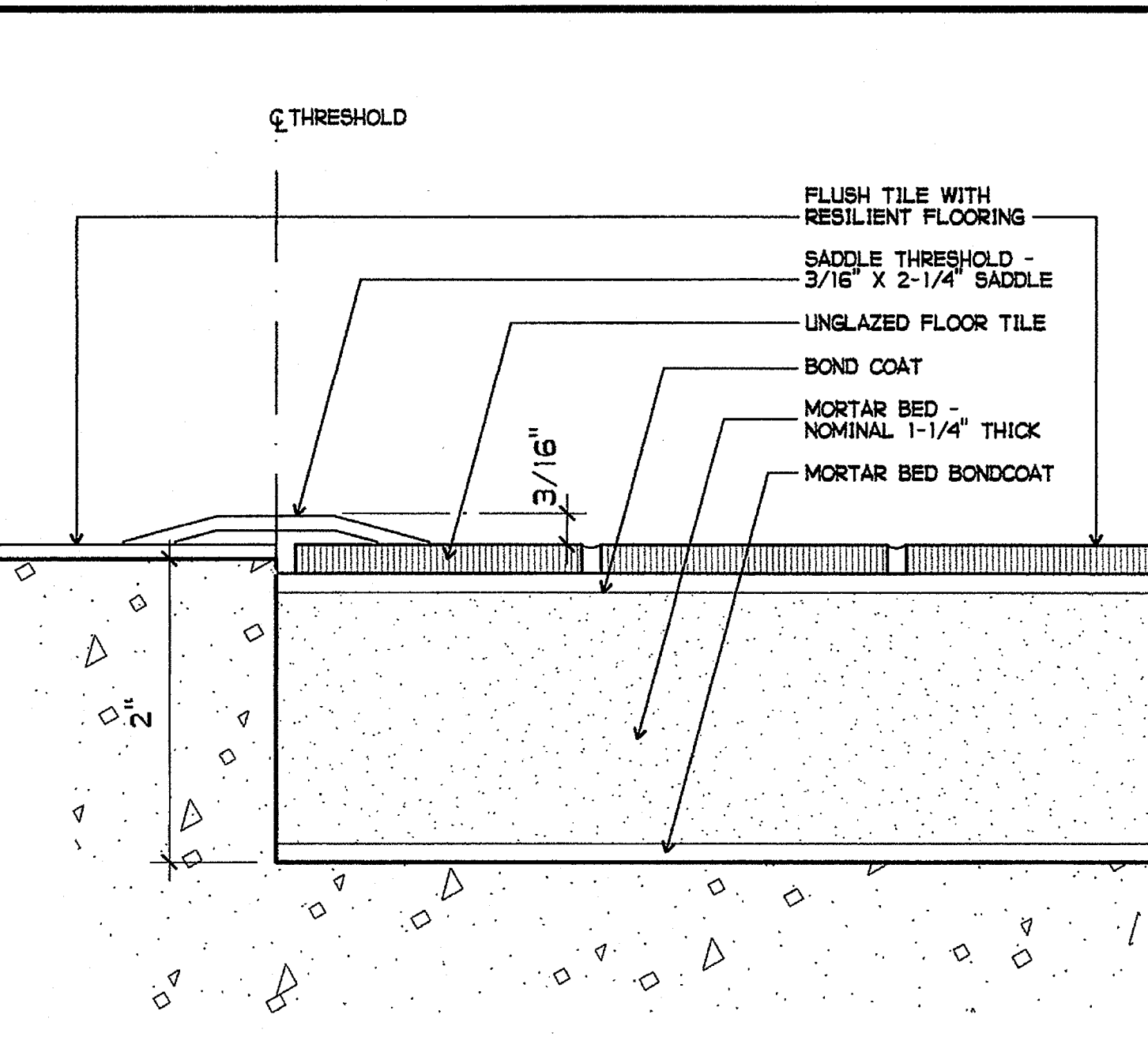
① DUCT PLACEMENT AT MECHANICAL ROOM
3" = 1'-0"



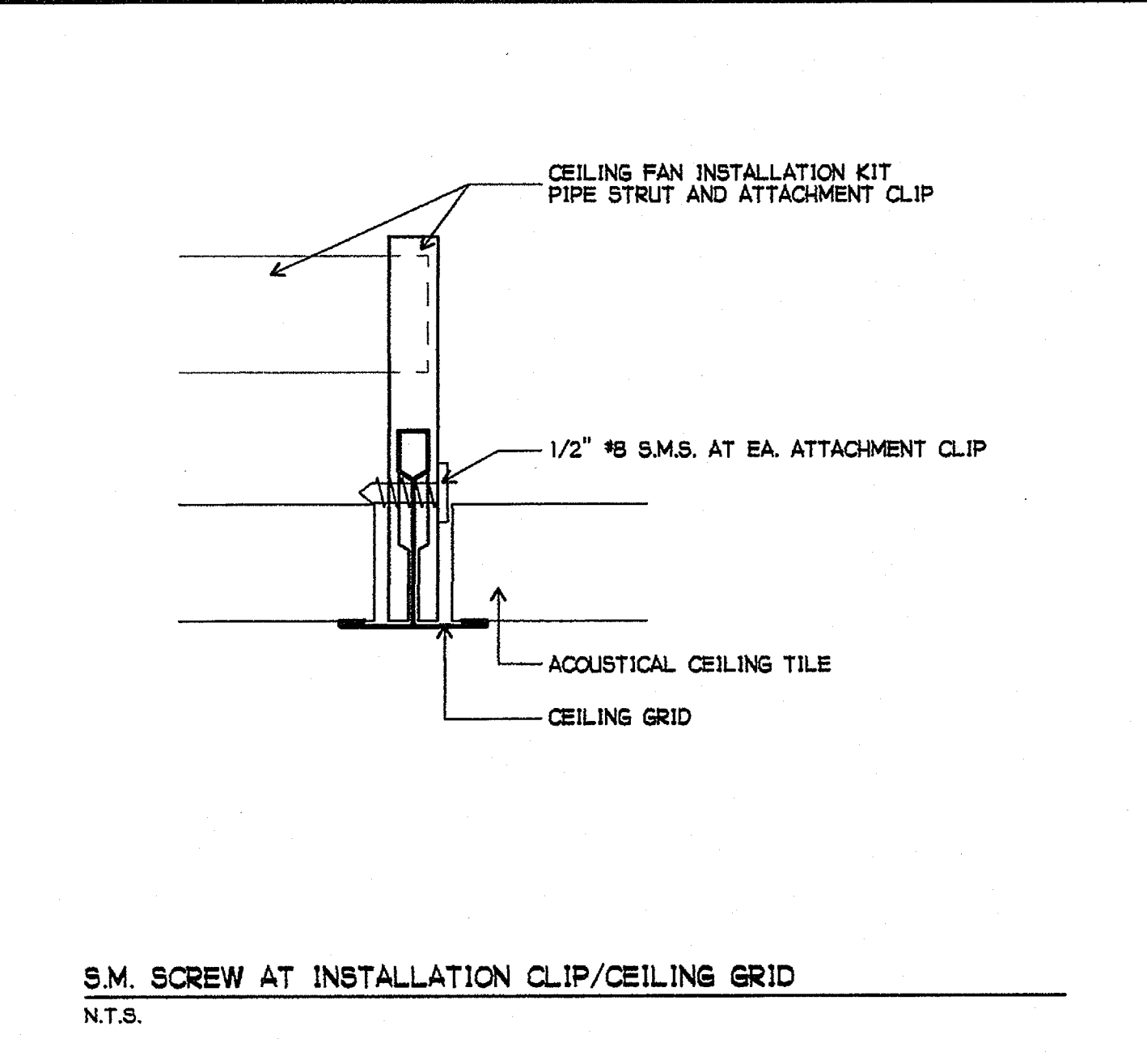
② RECESSED PROJECTION SCREEN/LIGHT FIXTURE AT SOFFIT
3" = 1'-0"



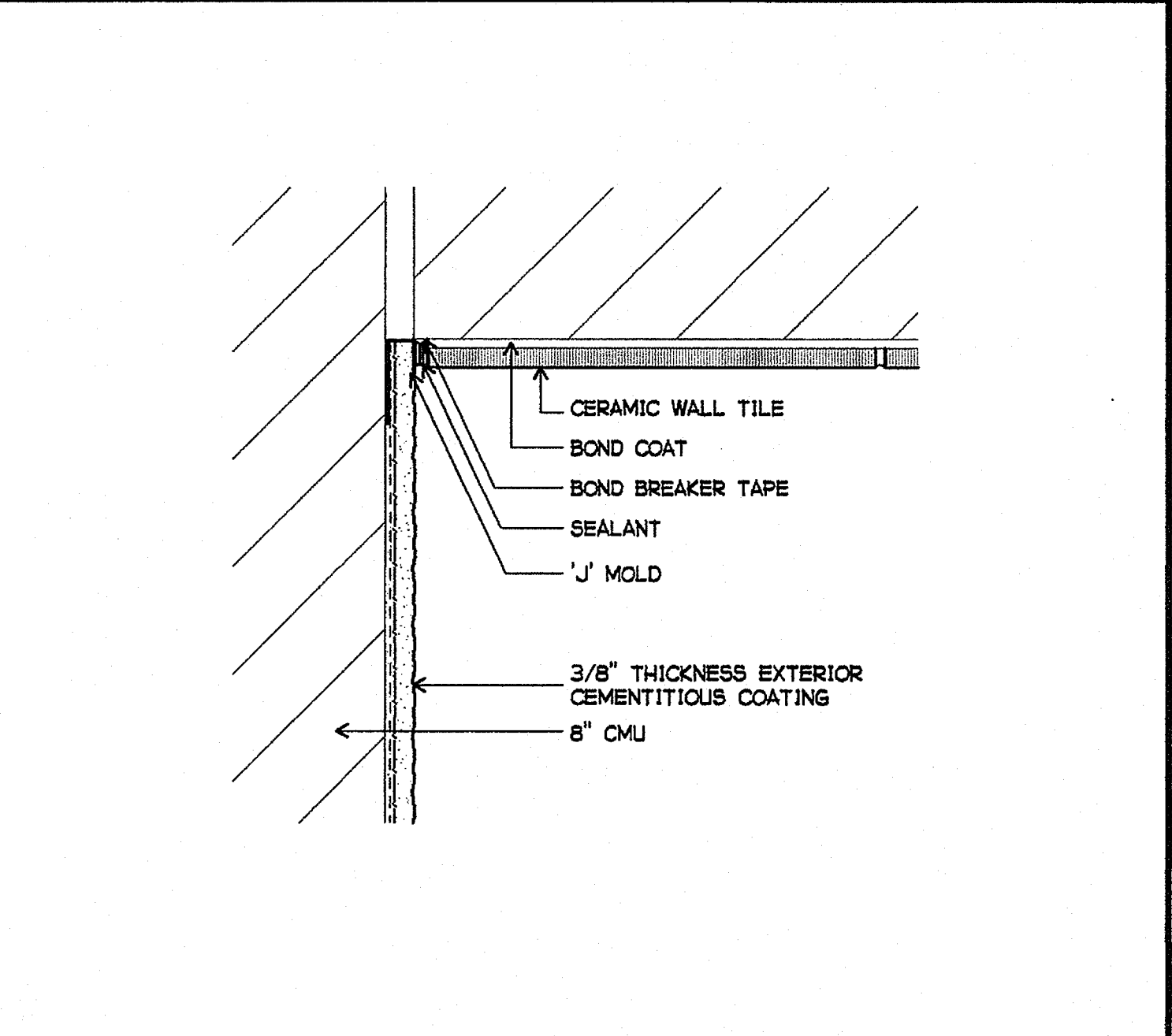
③ RESILIENT FLOORING/CARPET JOINER
FULL SCALE



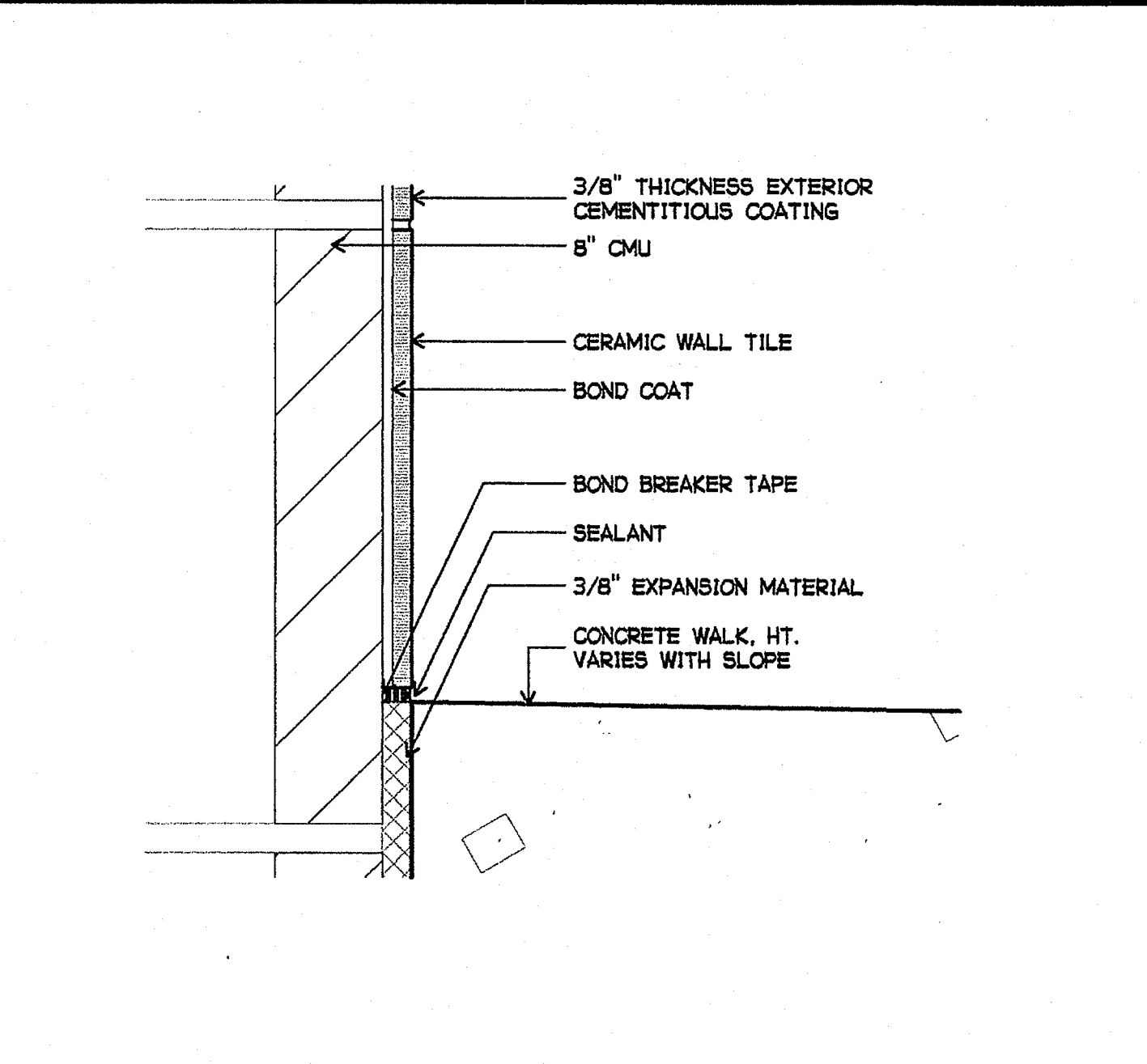
④ CERAMIC TILE TO RESILIENT FLOOR TRANSITION
FULL SCALE



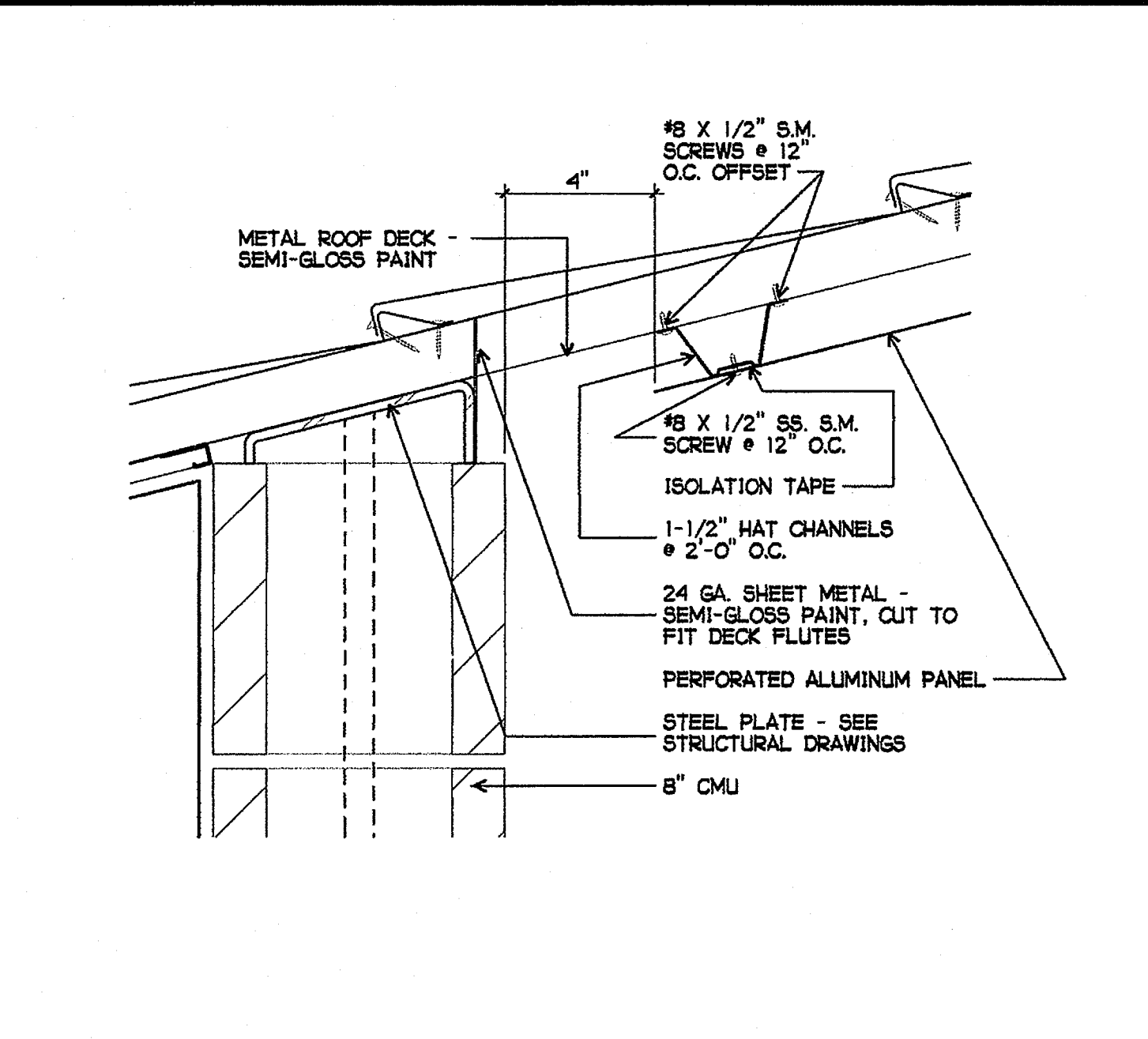
S.M. SCREW AT INSTALLATION CLIP/CEILING GRID
N.T.S.



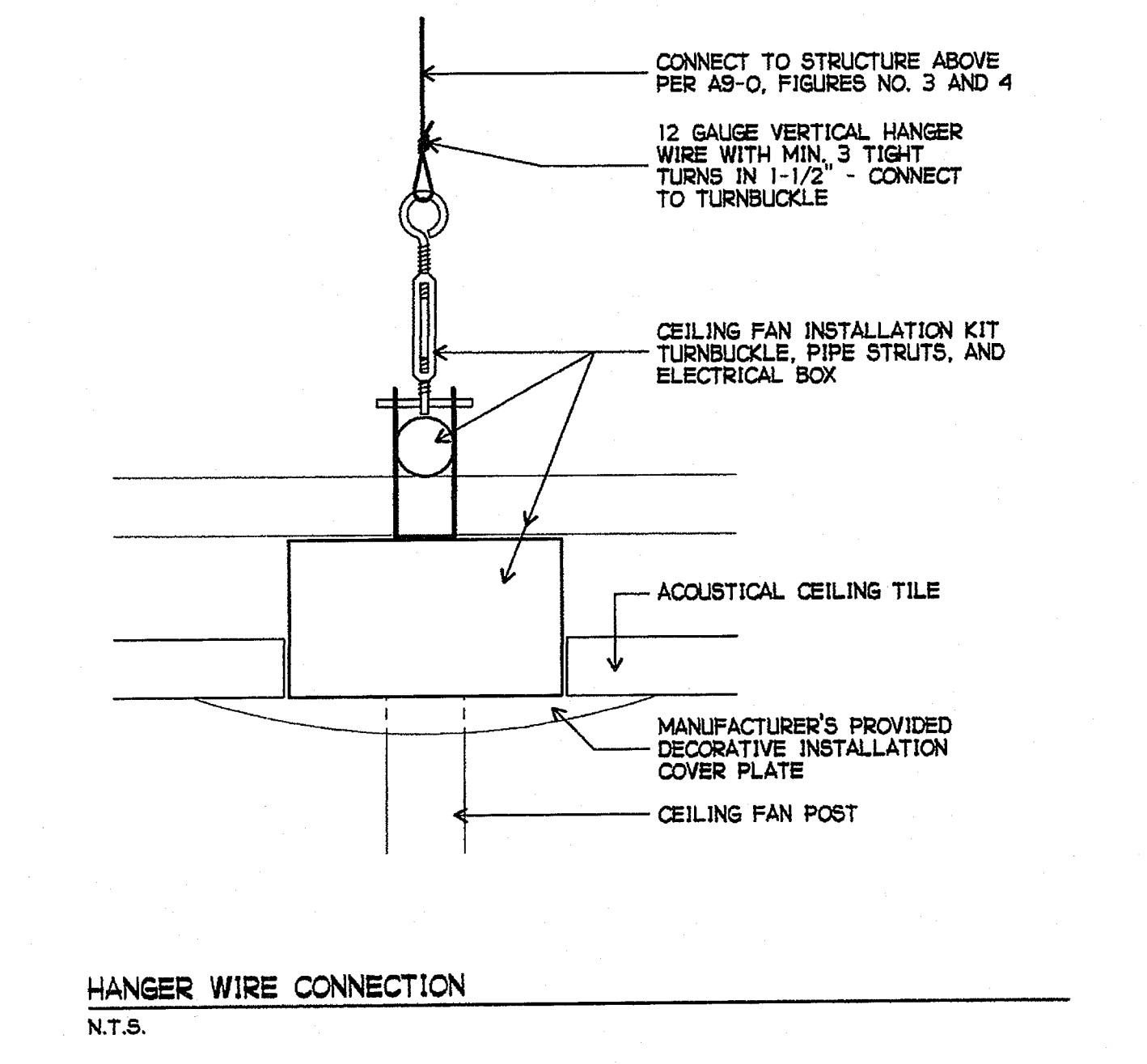
⑤ CERAMIC TILE WAJNSCOT INSIDE CORNER AT DRINKING FOUNTAIN
6" = 1'-0"



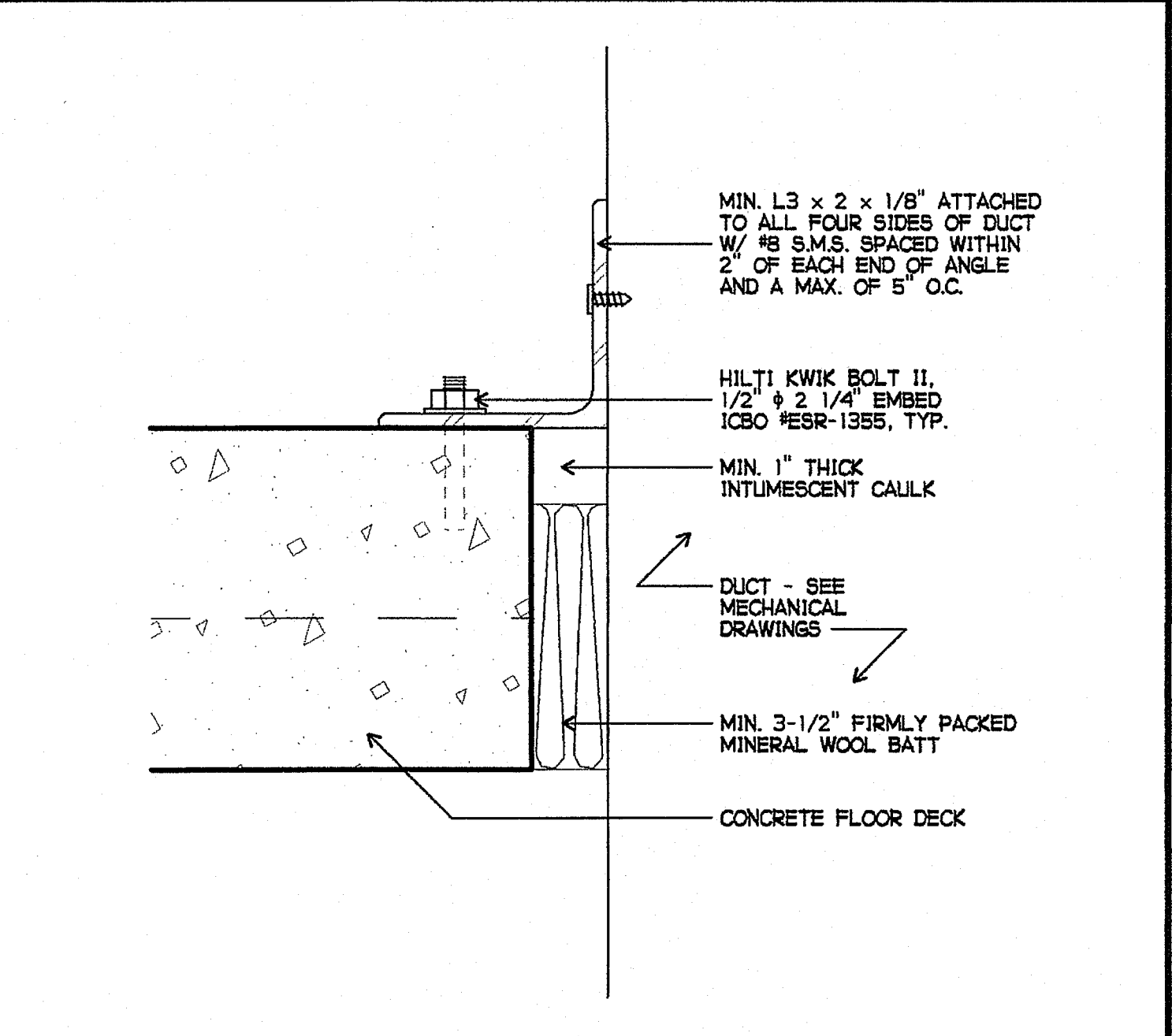
⑥ CERAMIC TILE WAJNSCOT BASE AT DRINKING FOUNTAIN
6" = 1'-0"



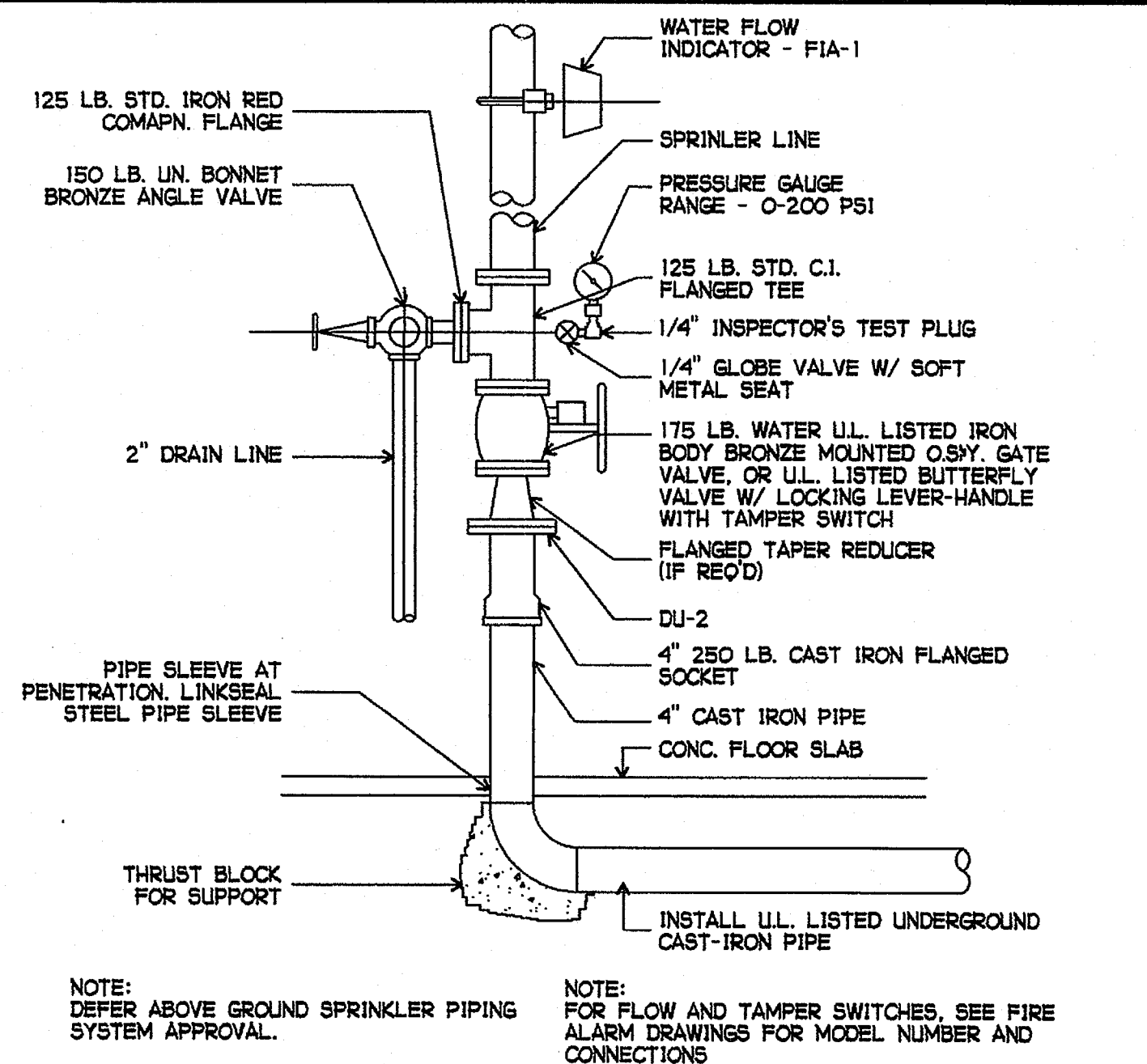
⑦ PERFORATED ALUMINUM PANELS AT OPEN CEILING
3" = 1'-0"



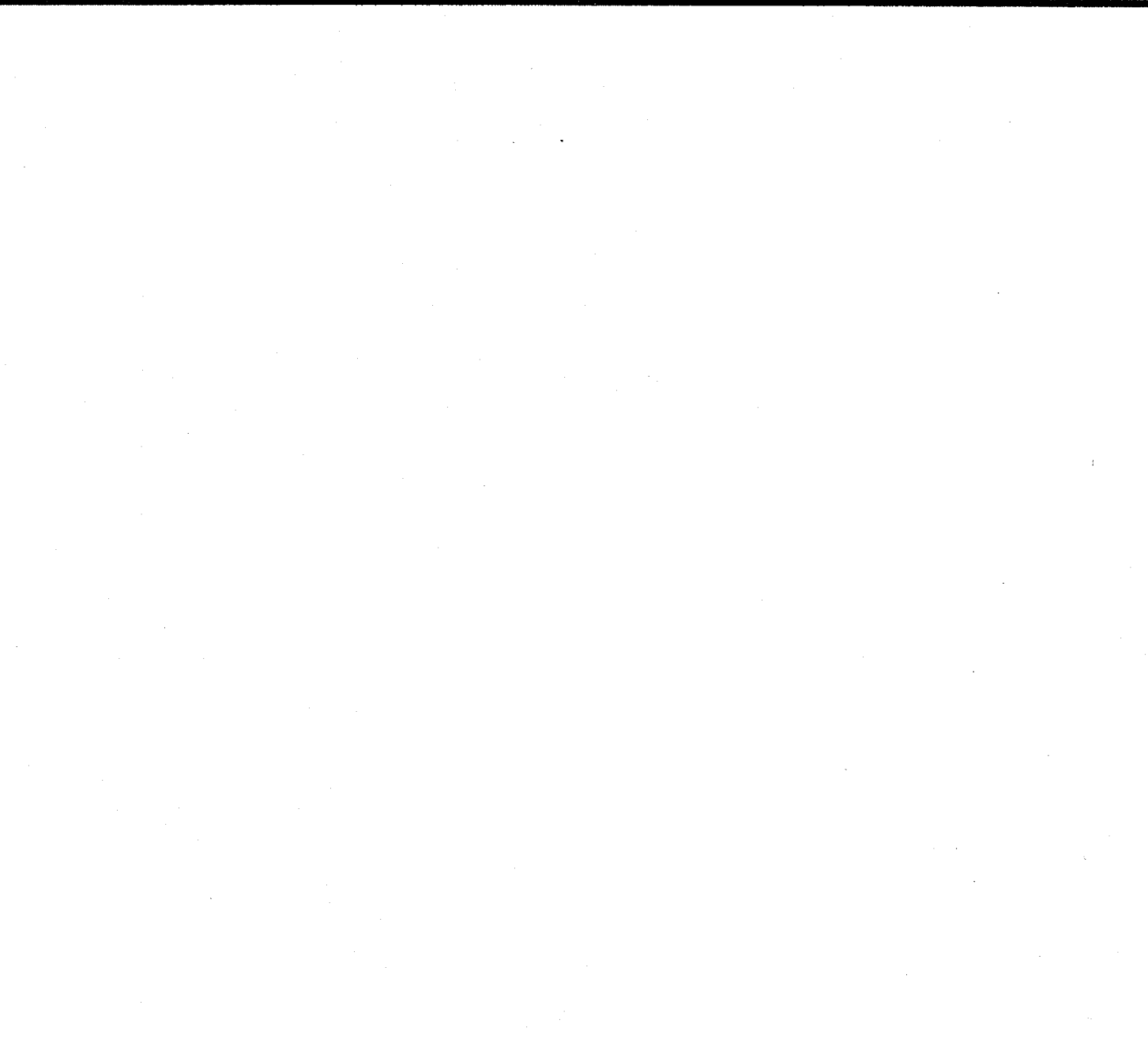
⑧ CEILING FAN
N.T.S.



⑨ FIRESTOPPING, UL SYSTEM NO. G-AJ-7042, 3-HR F RATING
6" = 1'-0"



⑩ FIRE SPRINKLER INSTALLATION
NO SCALE



⑪ NOT USED

PLOTTED 3/21/2005 3:09 PM

GROTH ARCHITECTS, INC. 823 ACACIA STREET OCEANSIDE, CA 92054

PHONE 760-754-8191 FAX 760-754-8291

CUSD NO. 758-000

PROJECT NOS. 025

P. T. N. 73569-9

DATE

REVISIONS

JEFFERSON MS NEW CONSTRUCTION

space time function art

GROTH ARCHITECTS, INC. 3355 MISSION AVE. SUITE 234 OCEANSIDE, CALIFORNIA 92054

DSA IDENTIFICATION STAMP DIV. OF THE STATE ARCHITECT OFFICE OF REGULATION SERVICES 4-106494

AC [Signature] FLS [Signature] SS [Signature]

DATE: MAR 28 2005

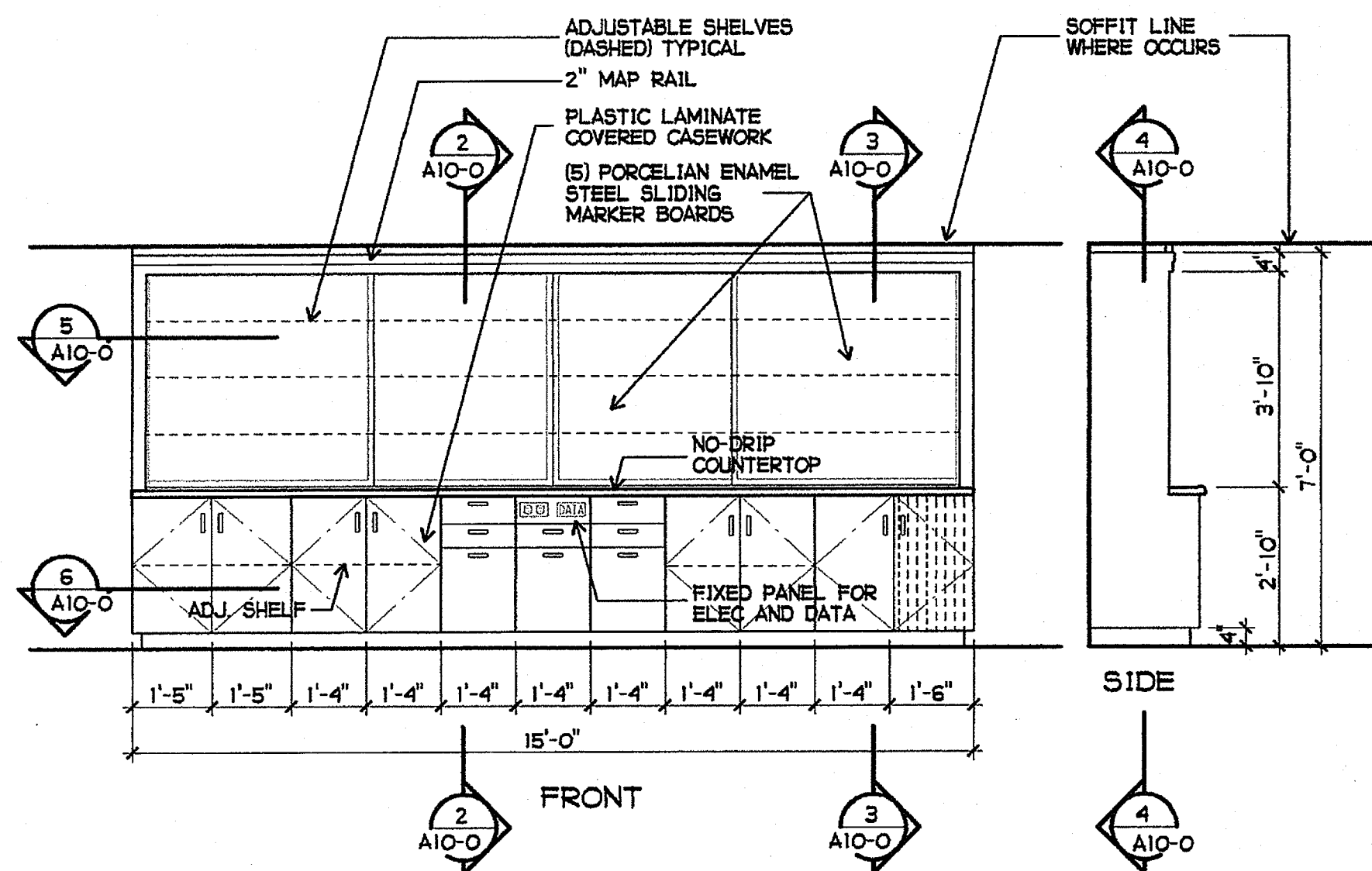
REGISTERED ARCHITECT JOHN SCOTT BERTH C-26609 4/30/2007 RENEWAL STATE OF CALIFORNIA

SHEET TITLE

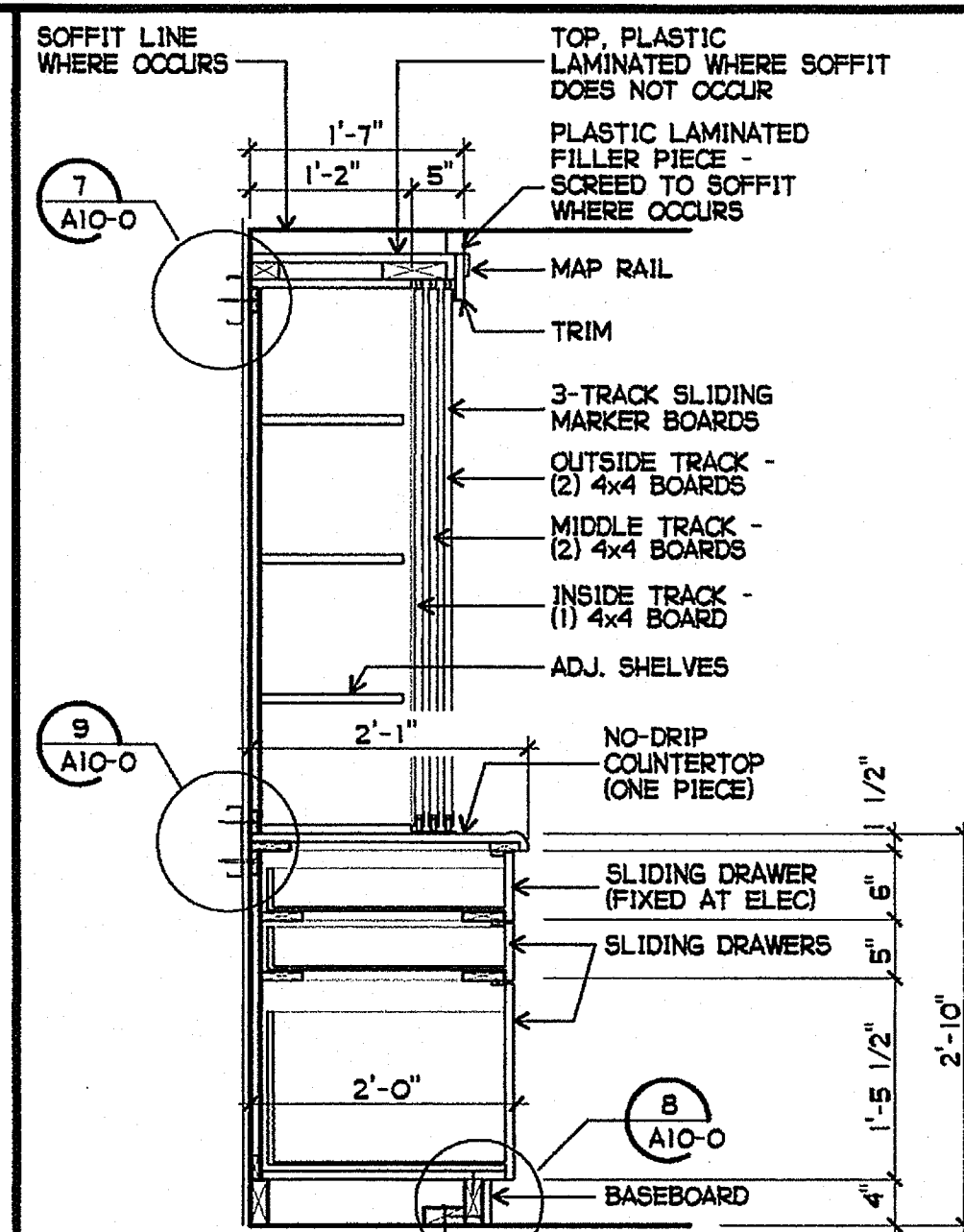
DETAILS

A9-14

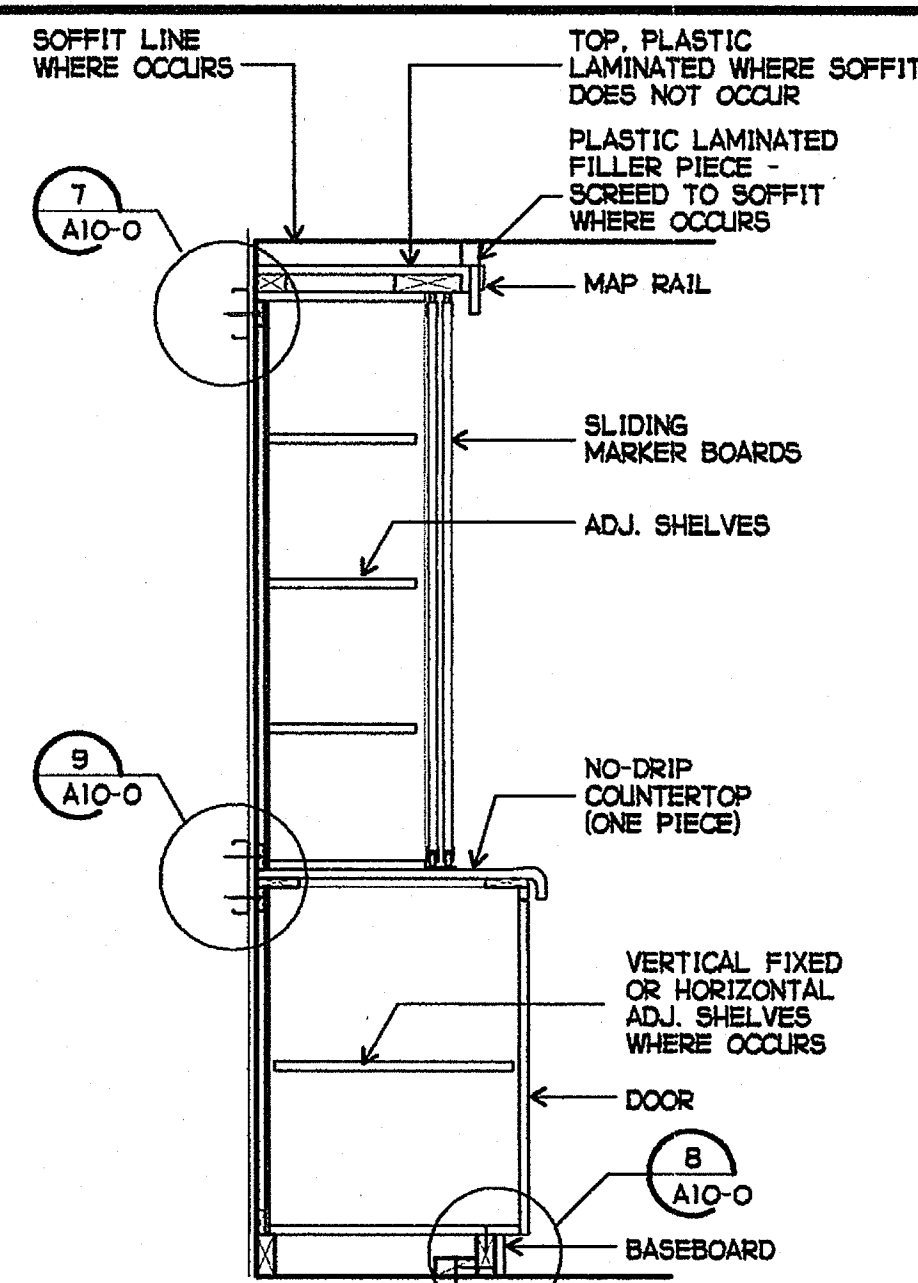
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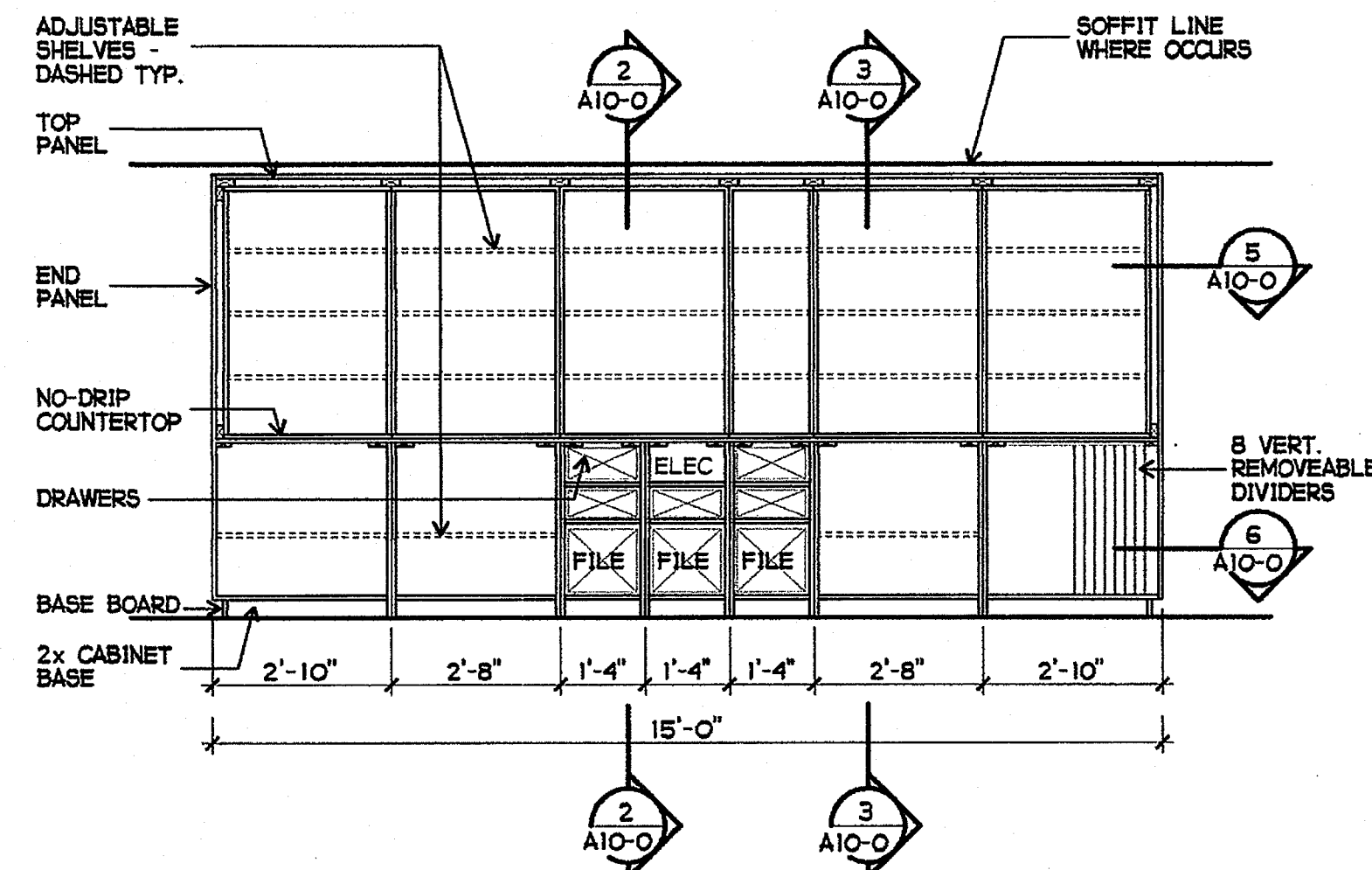
1 MODULAR TEACHING UNIT ELEVATION
3/8" = 1'-0"



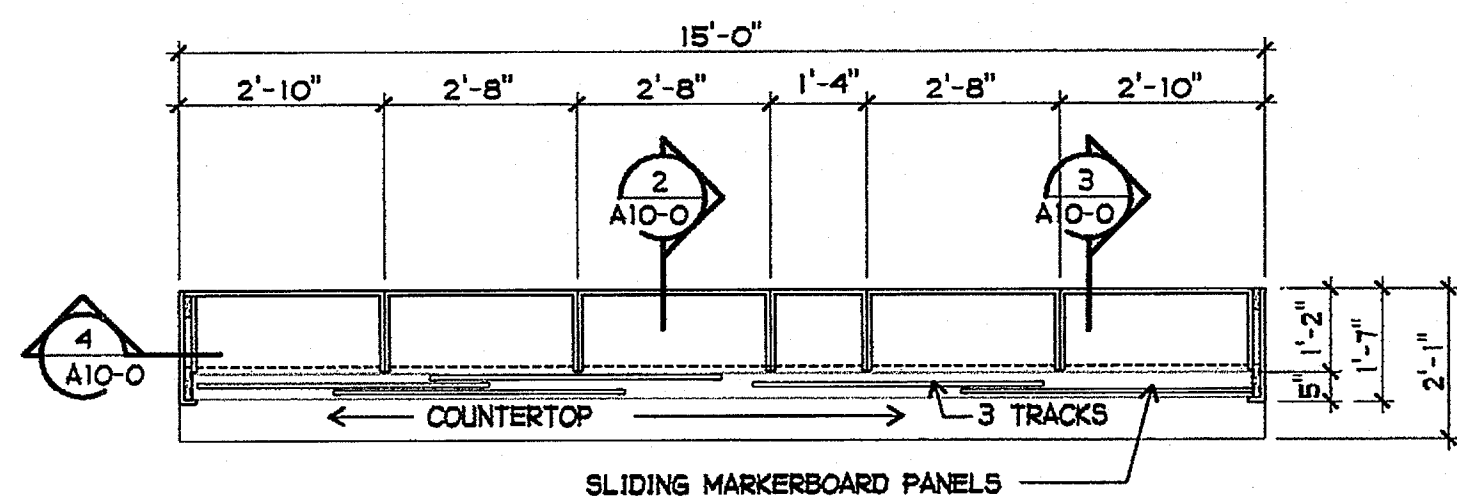
2 TEACHING UNIT SECTION
3/4" = 1'-0"



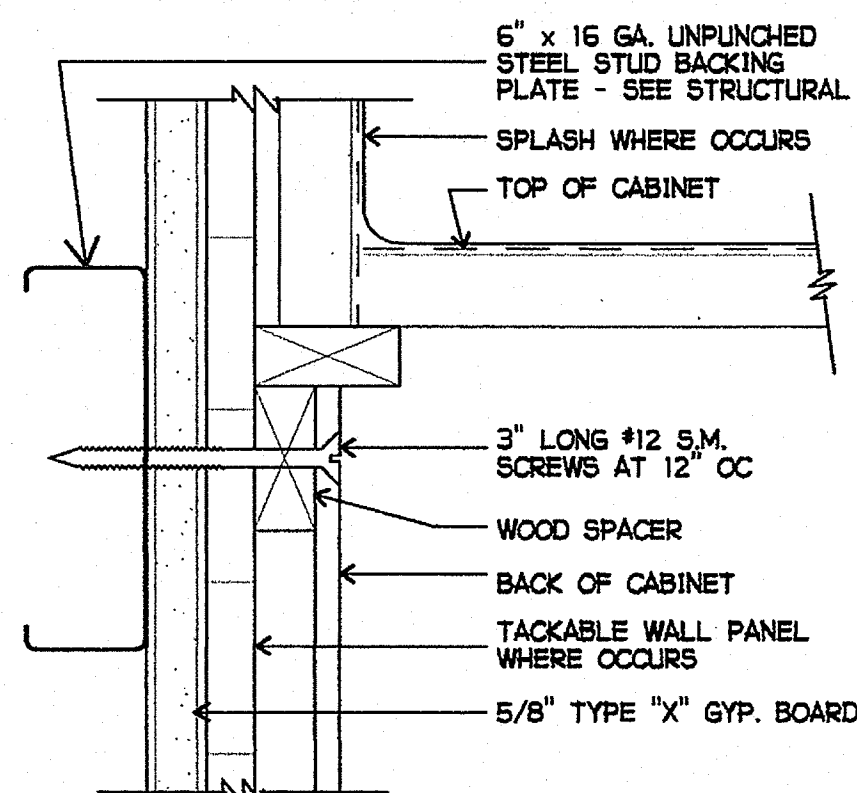
3 TEACHING UNIT SECTION
3/4" = 1'-0"



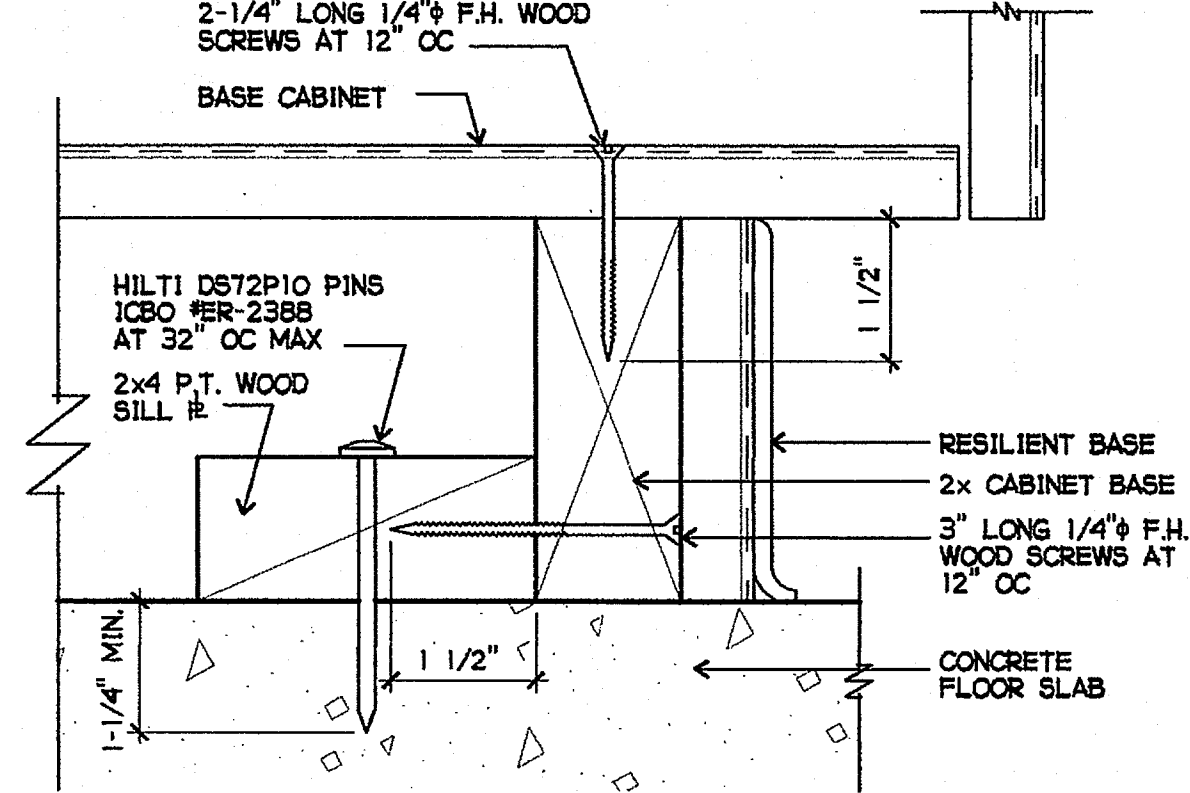
4 MODULAR TEACHING UNIT ELEVATION
3/8" = 1'-0"



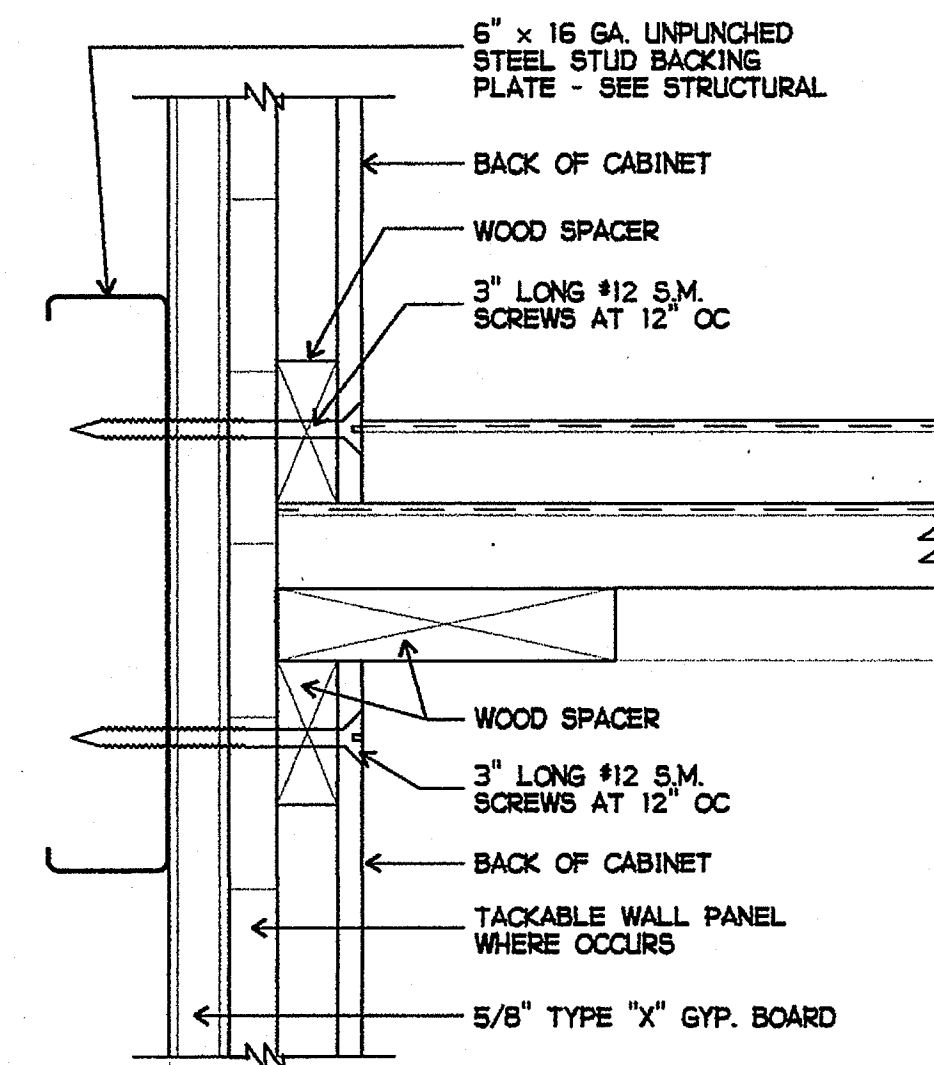
5 TEACHING UNIT PLAN SECTION
3/8" = 1'-0"



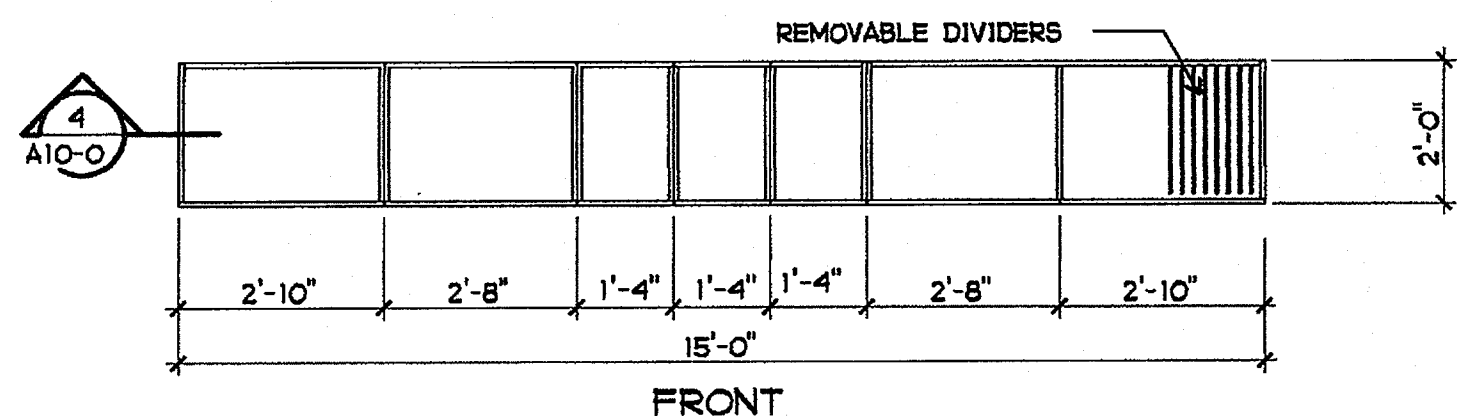
7 CABINET WALL ANCHOR
6" = 1'-0"



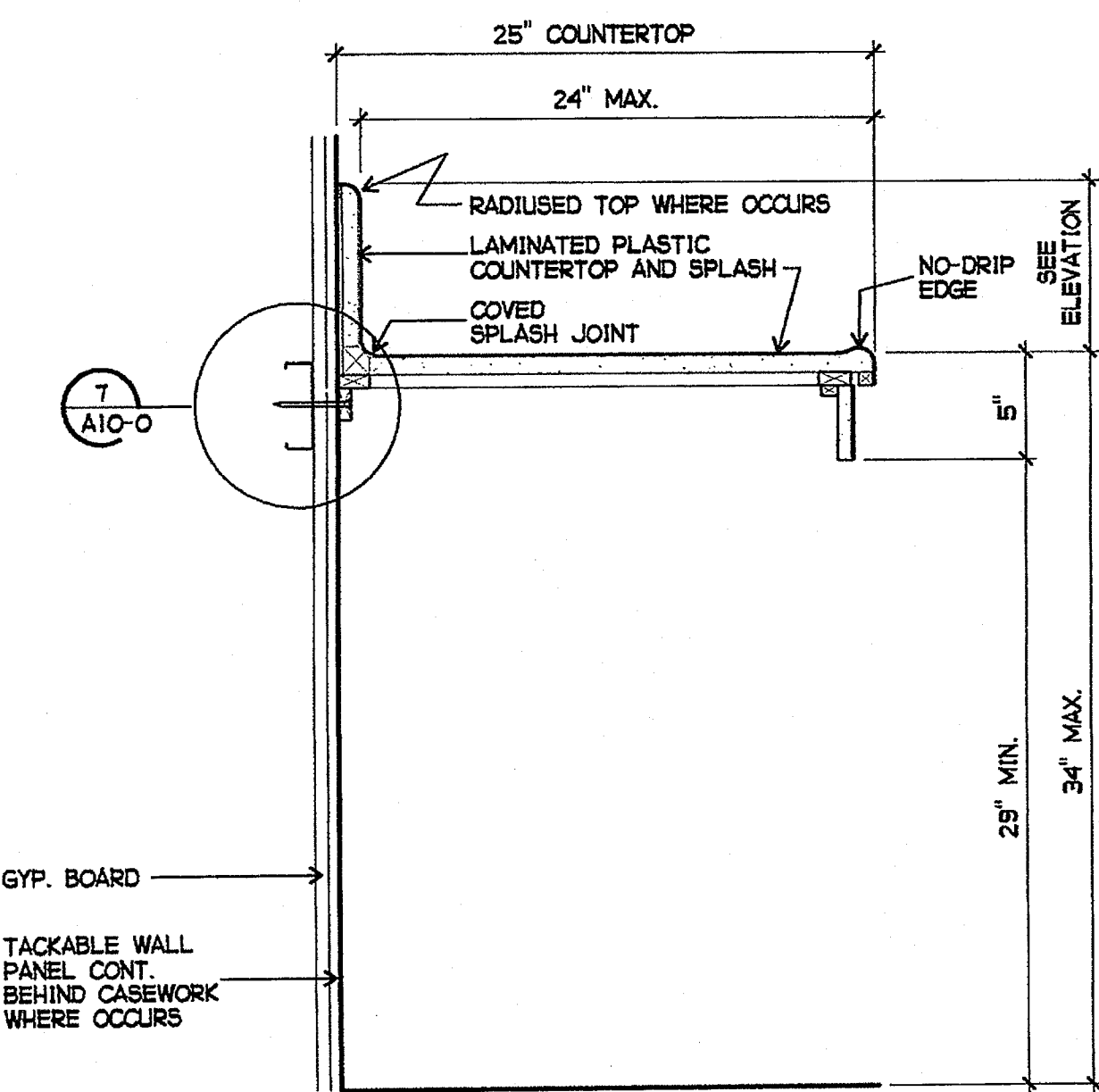
8 BASE CABINET FLOOR ANCHOR
6" = 1'-0"



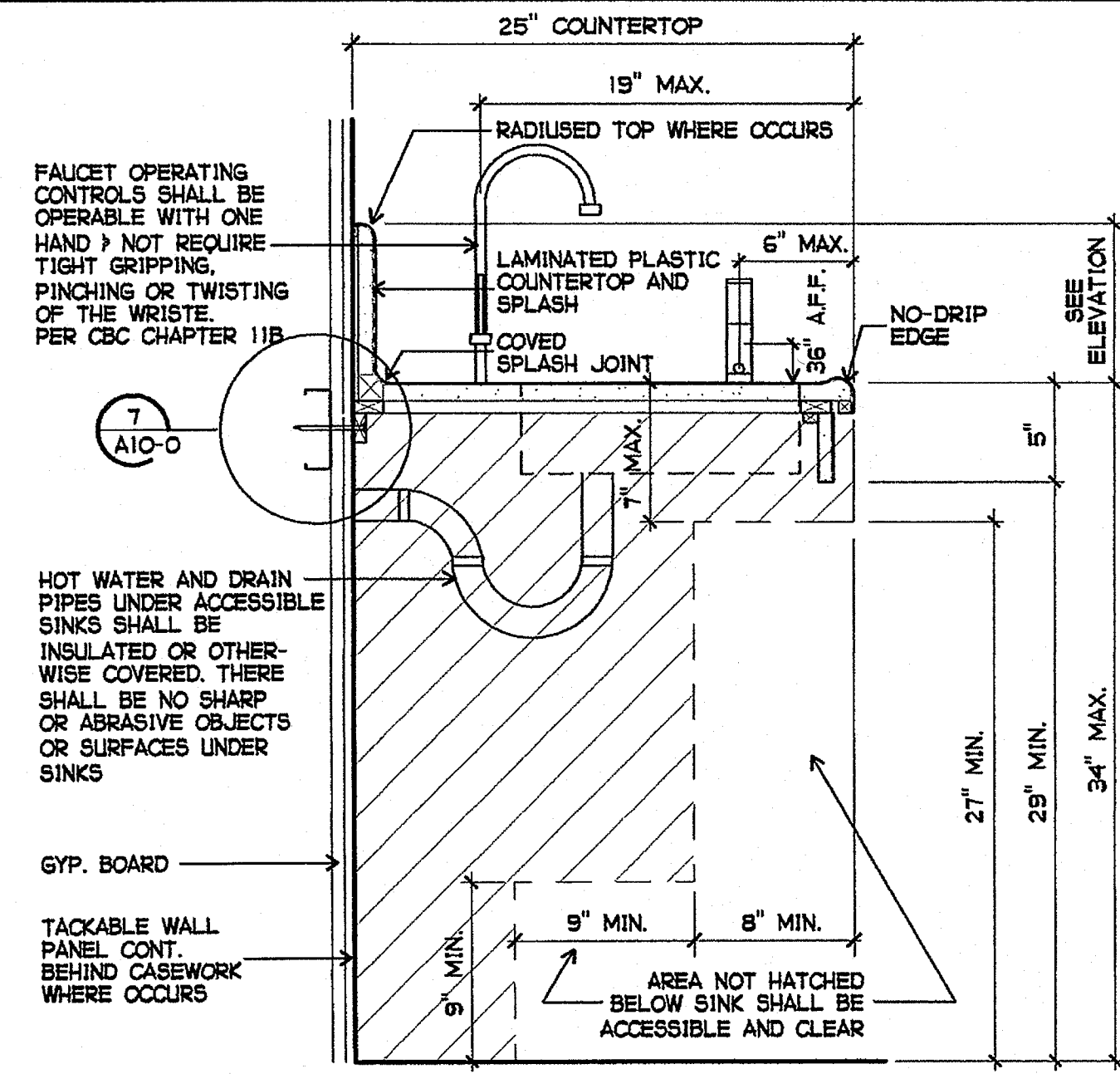
9 CABINET WALL ANCHOR
6" = 1'-0"



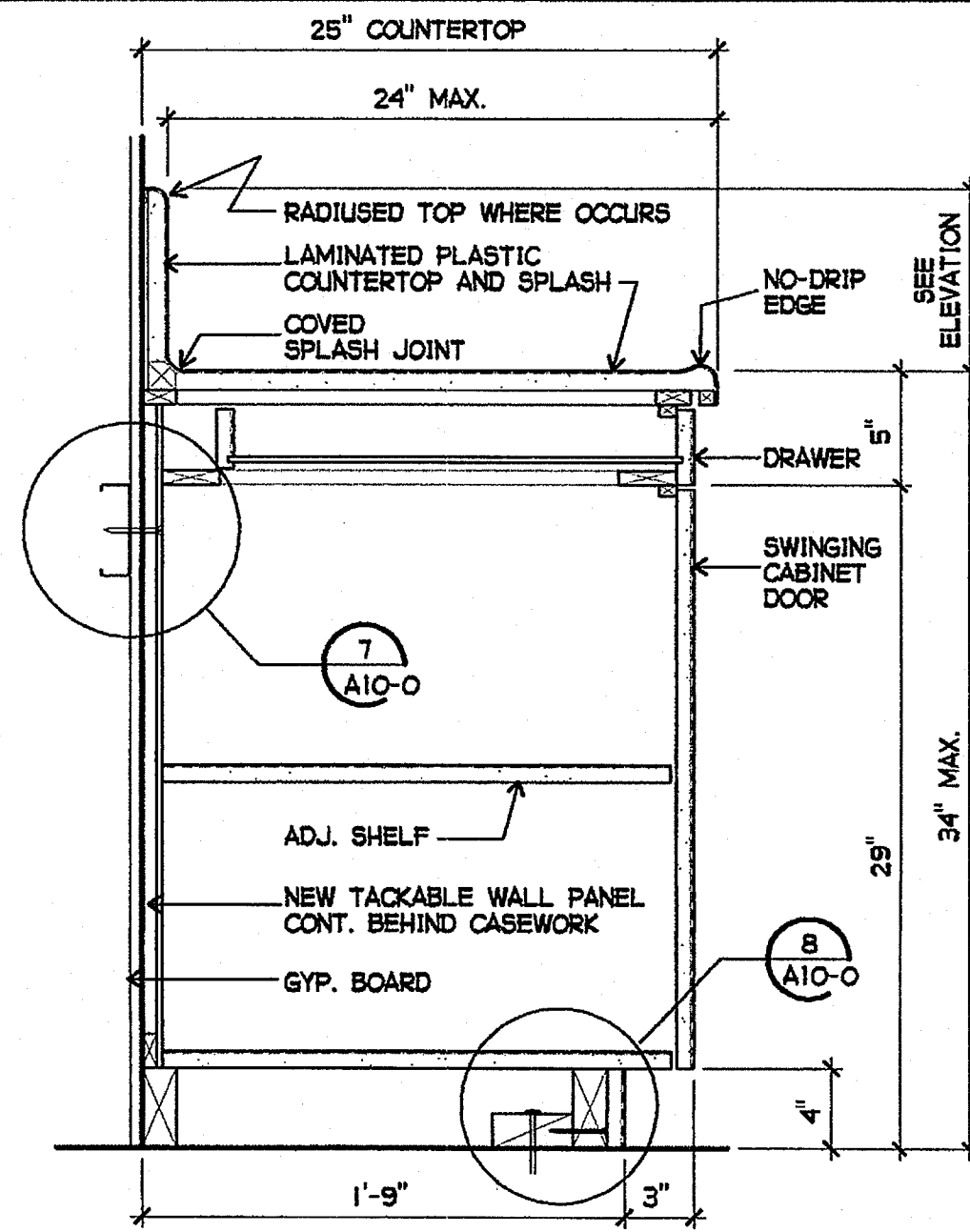
6 TEACHING UNIT PLAN SECTION
3/8" = 1'-0"



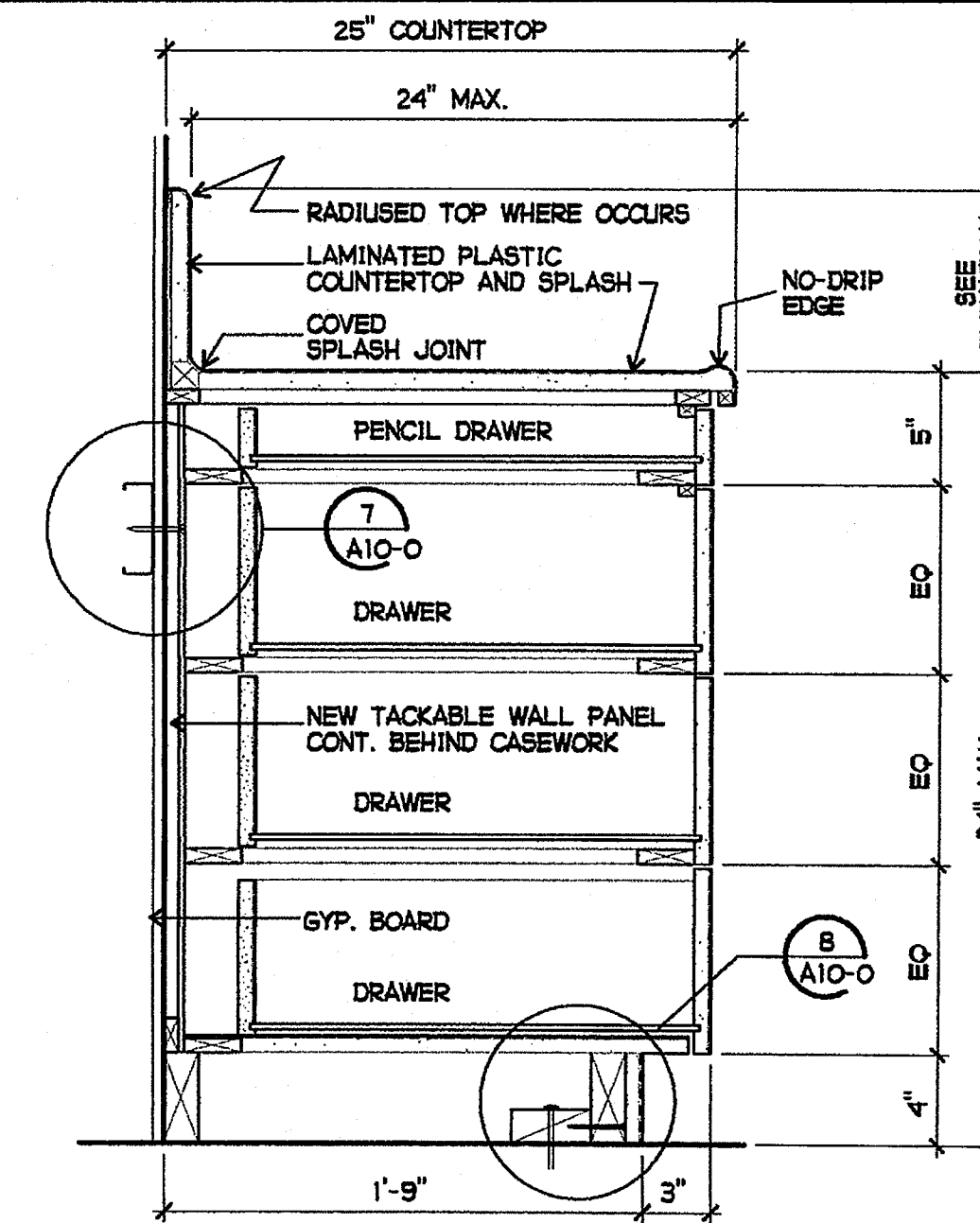
10 ACCESSIBLE BASE
1-1/2" = 1'-0"



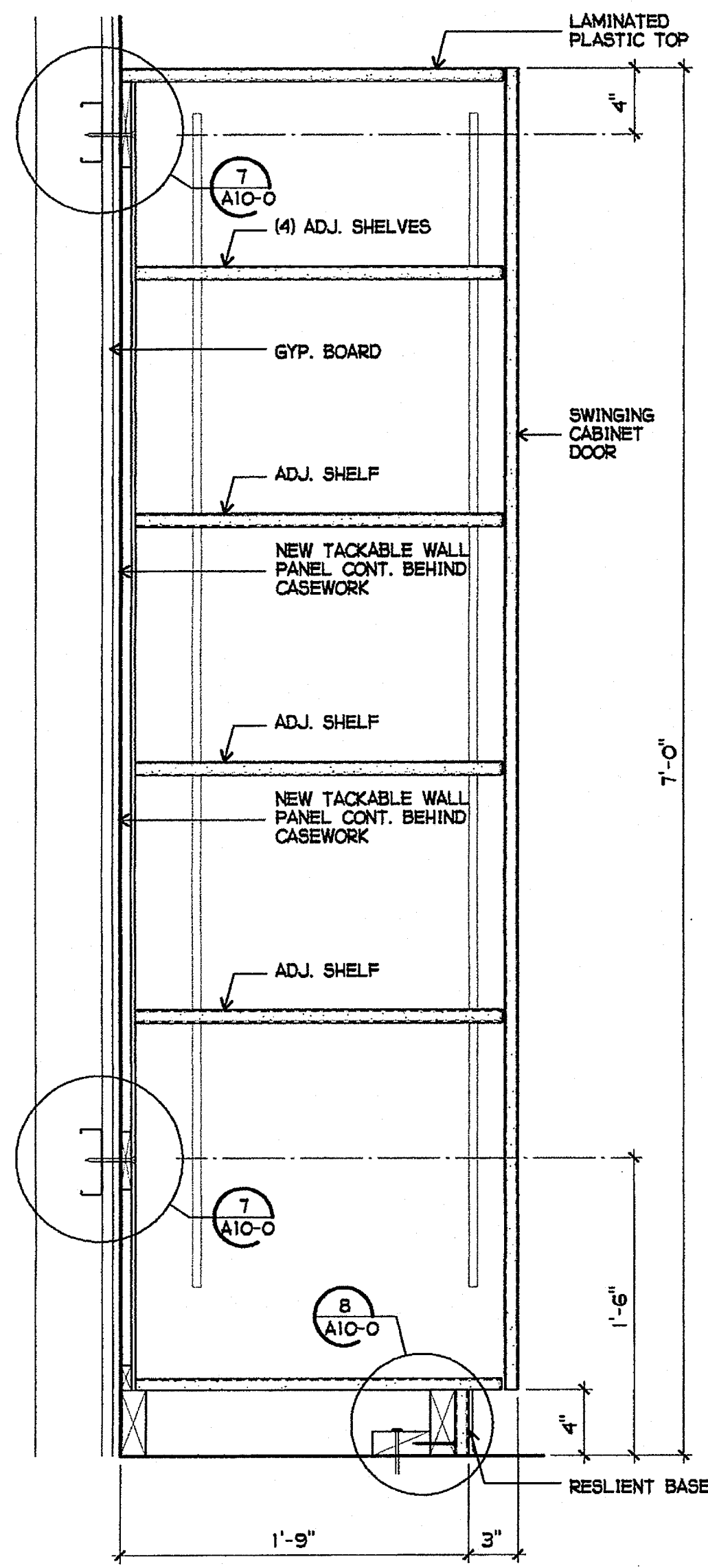
11 ACCESSIBLE SINK BASE
1-1/2" = 1'-0"



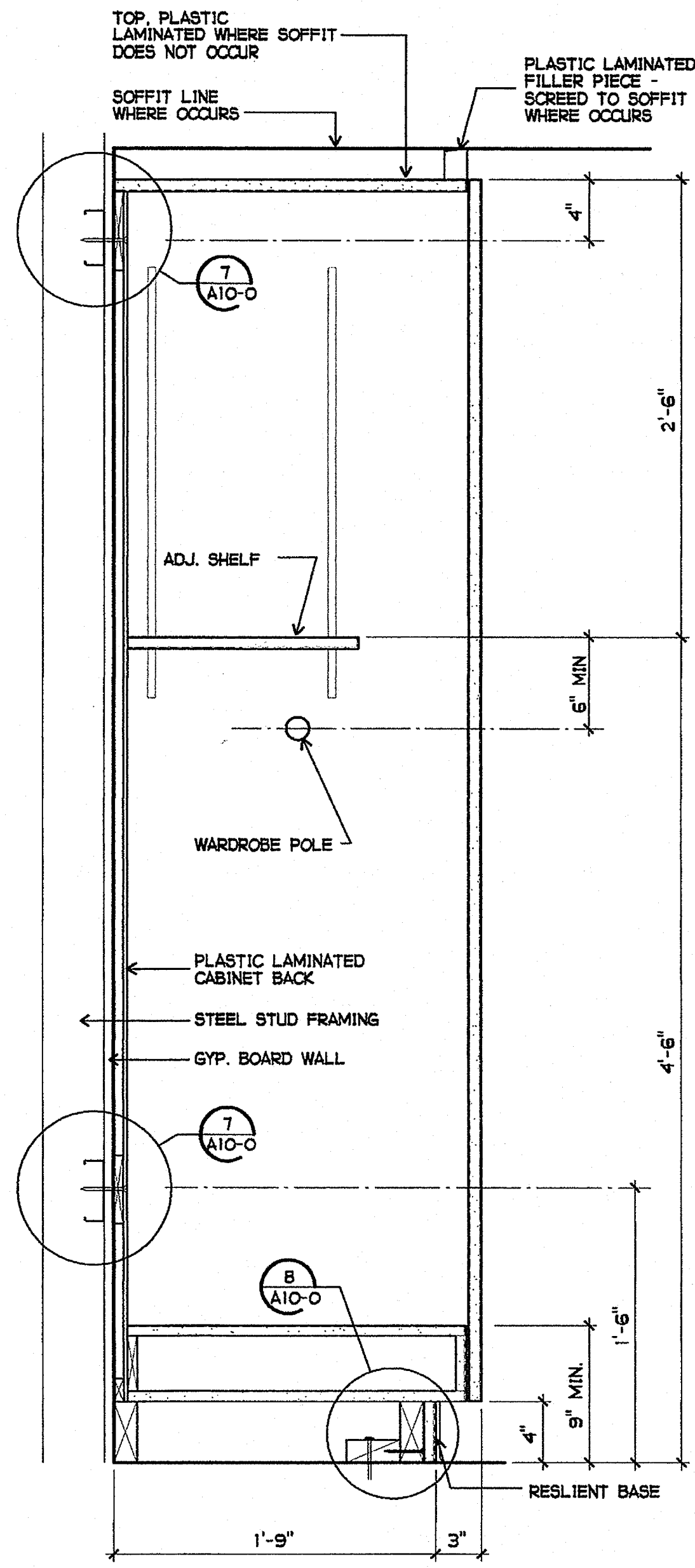
12 BASE CABINET
1-1/2" = 1'-0"



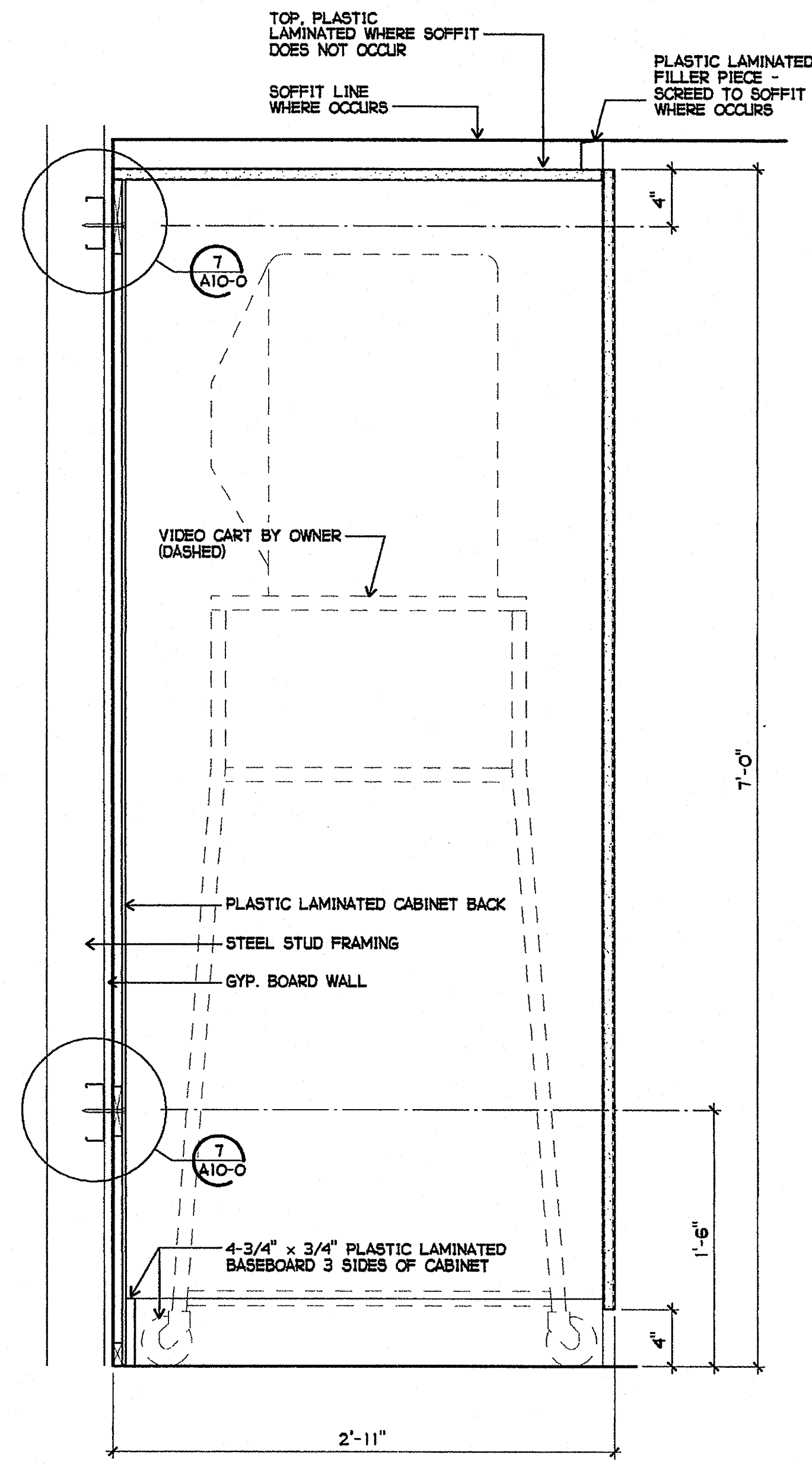
13 DRAWERS AT BASE CABINET
1-1/2" = 1'-0"



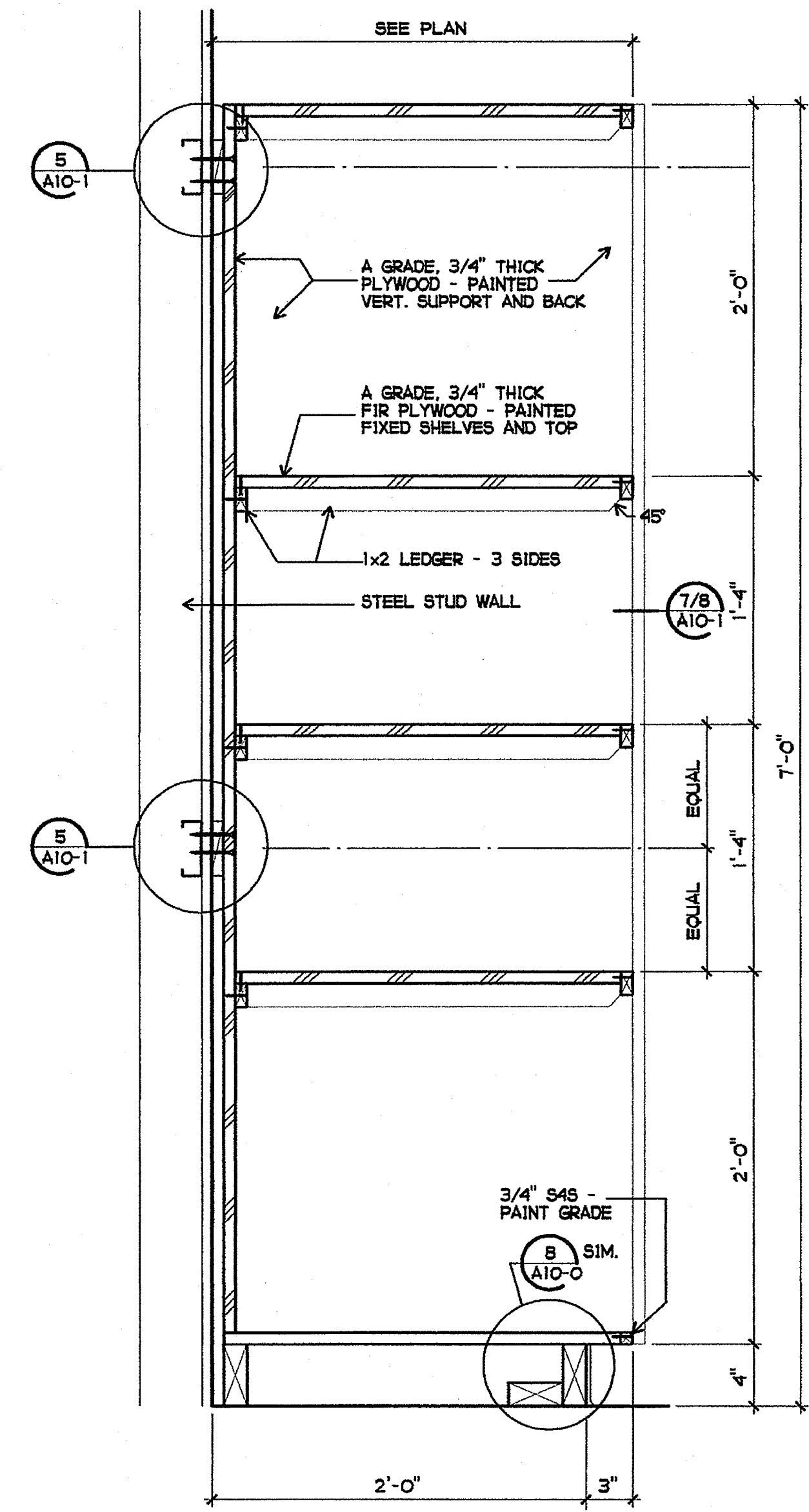
1 STORAGE CABINET
1-1/2" = 1'-0"



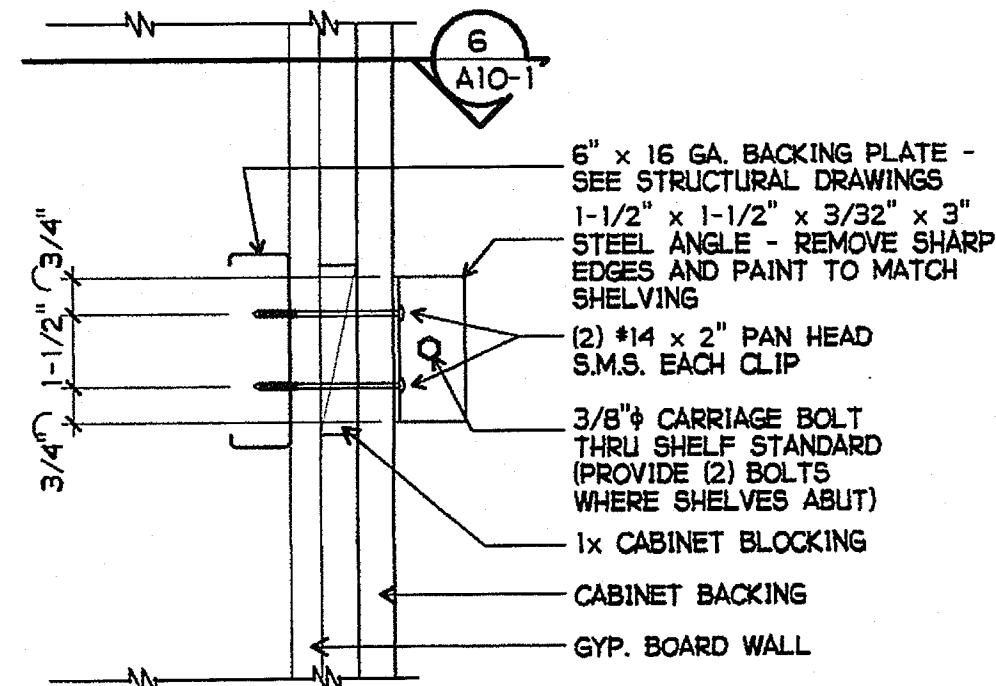
2 TEACHERS WARDROBE
1-1/2" = 1'-0"



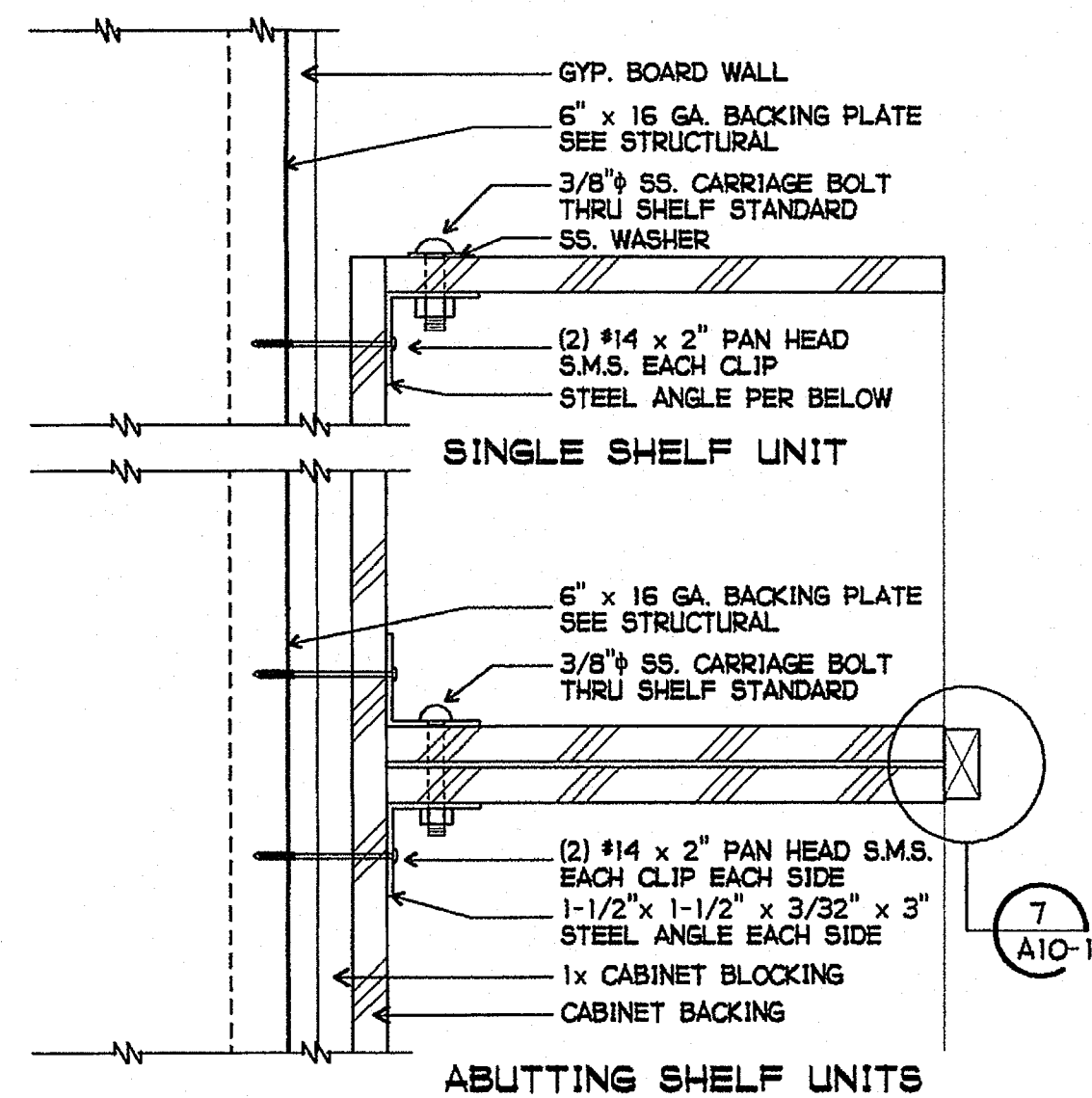
3 TV/VIDEO CART CABINET
1-1/2" = 1'-0"



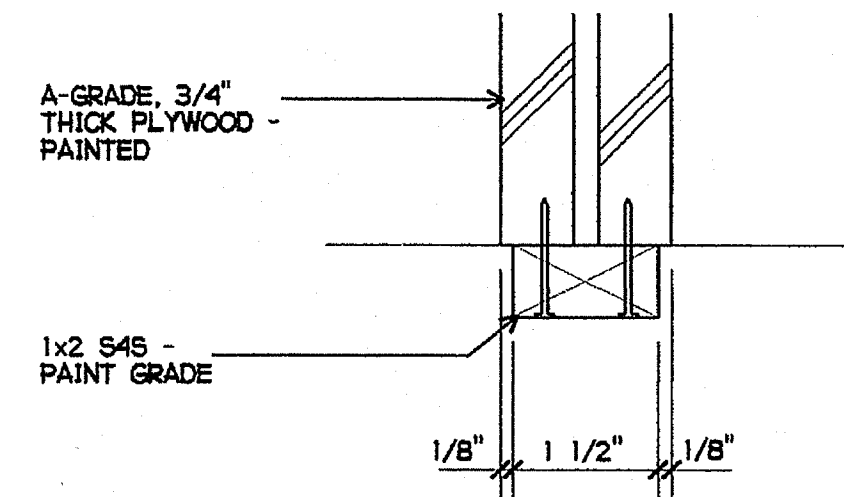
4 OPEN, FIXED STORAGE SHELVING
1-1/2" = 1'-0"



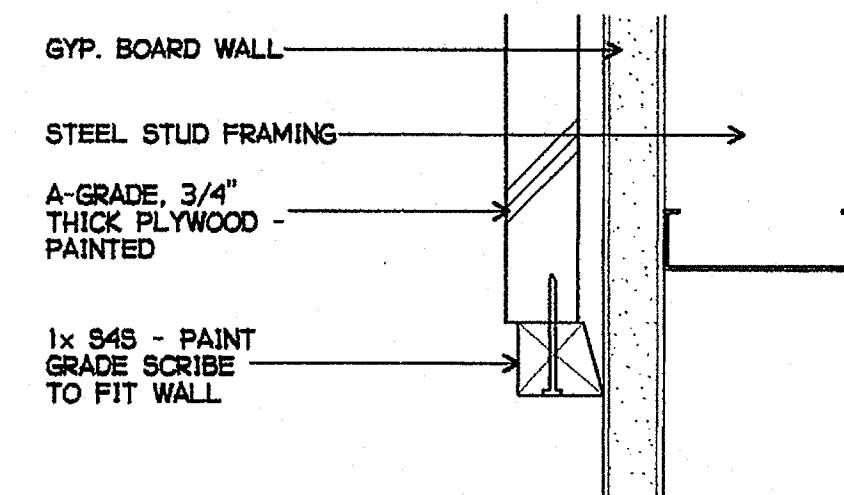
5 OPEN, FIXED STORAGE SHELVING
3" = 1'-0"



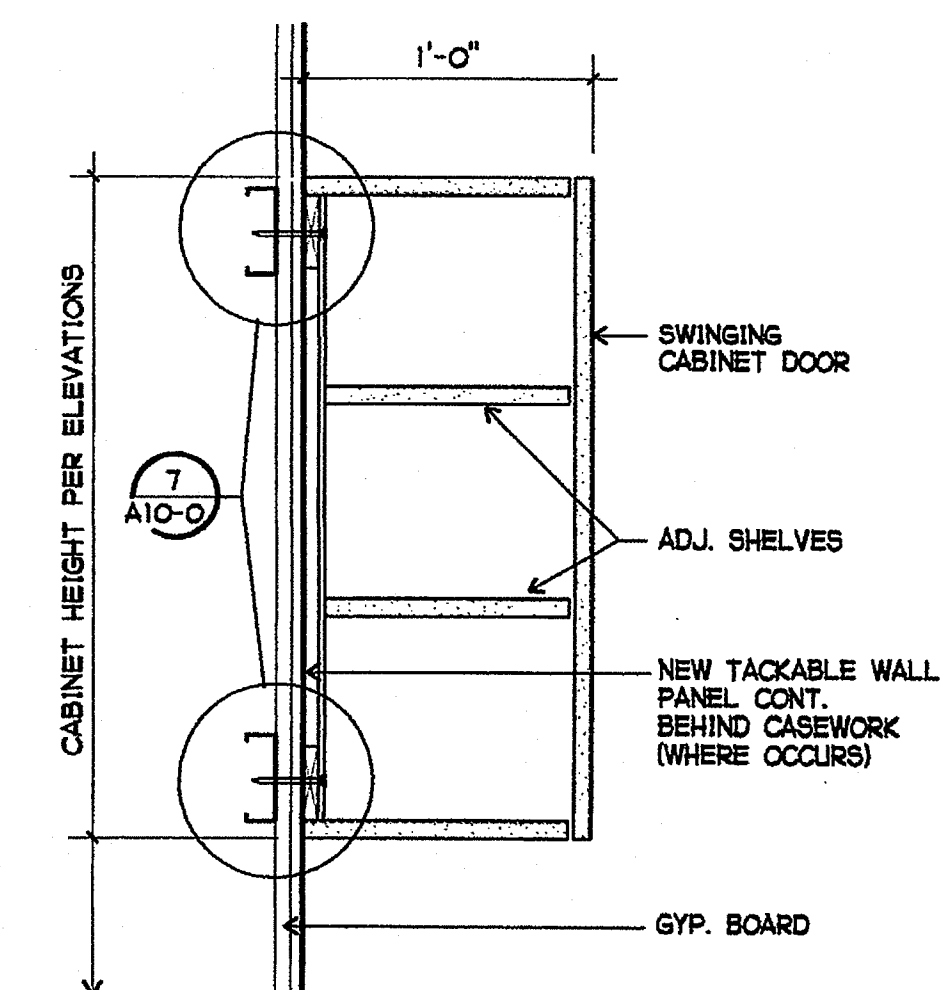
6 OPEN, FIXED STORAGE SHELVING
3" = 1'-0"



7 BETWEEN OPEN, FIXED STORAGE SHELVING - PLAN
6" = 1'-0"



8 OPEN, FIXED STORAGE SHELVING AT WALL - PLAN
6" = 1'-0"



9 WALL CABINET, 12" DEEP
1-1/2" = 1'-0"

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CLSD NO. 758-000
PROJECT NOS. 025
P. T. N. 73569-9
DATE
REVISIONS

JEFFERSON MS NEW CONSTRUCTION
823 ACACIA STREET
OCEANSIDE, CA 92054
OCEANSIDE UNIFIED S.D.

space art
function time

GROTH ARCHITECTS, INC.

DSA
IDENTIFICATION STAMP
DIV. OF THE STATE ARCHITECT
OFFICE OF REGULATION SERVICES
4-106494
AC PLS. SS.
DATE MAR 28 2005

REGISTERED ARCHITECT
JOHN SCOTT GROTH
C-26609
4/30/2007
RENEWAL
DATE OF EXPIRATION

SHEET TITLE
CASEWORK DETAILS

A10-1

GENERAL NOTES

THE FOLLOWING GENERAL NOTES ARE A SUMMARY OF THE SPECIFICATIONS FOR THE CONVENIENCE OF THE CONTRACTOR. REFER TO THE SPECIFICATIONS AND ALL OTHER CONTRACT DOCUMENTS FOR ALL REQUIREMENTS.

GENERAL

- 1. CONTRACTOR SHALL VERIFY ALL DIMENSIONS BEFORE STARTING WORK AND NOTIFY THE ARCHITECT IMMEDIATELY OF ANY DISCREPANCIES FOUND.
2. SPECIFIC CODES AND DETAILS SHALL TAKE PRECEDENCE OVER THESE GENERAL NOTES AND THE TYPICAL DETAILS ON SQL1.1 THROUGH SQL1.4 IN CASE OF CONFLICT. NOTIFY ENGINEER FOR CLARIFICATION.
3. WHERE NO CONSTRUCTION DETAILS ARE SHOWN OR NOTED FOR ANY PART OF THE WORK, THE DETAILS USED SHALL BE THE SAME AS FOR OTHER SIMILAR WORK, PROVIDED THAT PRIOR APPROVAL IS OBTAINED FROM THE ARCHITECT OR ENGINEER.
4. THE DESIGN IS BASED ON TITLE 24, CODE OF REGULATIONS (C.C.R.) 2001 EDITION.
5. NEITHER THE OWNER NOR THE ARCHITECT WILL ENFORCE SAFETY MEASURES OR REGULATIONS. THE CONTRACTOR SHALL DESIGN, CONSTRUCT AND MAINTAIN ALL SAFETY DEVICES, INCLUDING SHORING AND BRACING, AND SHALL BE SOLELY RESPONSIBLE FOR CONFORMING TO ALL LOCAL, STATE AND FEDERAL SAFETY AND HEALTH STANDARDS, LAWS AND REGULATIONS.

FOUNDATIONS:

A PRELIMINARY EXPLORATION OF THE SOILS UNDERLYING THIS SITE WAS MADE BY CONSTRUCTION TESTING & ENGINEERING INC. AND IS DESCRIBED IN A REPORT DATED JUNE 10, 2004.
THE FOLLOWING ARE RECOMMENDATIONS AND INTERPRETATIONS FROM THIS REPORT. FLORES LUND CONSULTANTS (FLC) CAN NOT BE HELD LIABLE FOR RECOMMENDATIONS PREPARED BY A DIFFERENT FIRM. (FLC'S EXPERIENCE IN THIS FIELD IS NIL).

- 1. SITE PREPARATION
BEFORE GRADING, THE SITE SHOULD BE CLEARED OF ANY EXISTING DEBRIS AND OTHER DELETERIOUS MATERIALS. IN AREAS TO RECEIVE STRUCTURES OR DISTRESS-SENSITIVE IMPROVEMENTS, EXPANSION, BURROWED, OR OTHERWISE LOOSE OR DISTURBED SOILS SHOULD BE REMOVED TO THE DEPTH OF COMPETENT MATERIALS. A DETERMINATION OF THE SUITABILITY OF THE EXPOSED SUBGRADES SHOULD BE MADE IN THE FIELD BY AN ENGINEER OR GEOLOGIST FROM THIS FIRM. ORGANIC AND OTHER DELETERIOUS MATERIALS NOT SUITABLE FOR STRUCTURAL BACKFILL SHOULD BE DISPOSED OF OFFSITE AT A LEGAL DISPOSAL SITE.
2. SITE EXCAVATION
BASED ON THE PRESENCE OF SHALLOW UNDOCUMENTED FILL OR TOPSOIL MATERIALS, AREAS BENEATH PROPOSED NEW STRUCTURES ARE TO BE EXCAVATED TO COMPETENT NATIVE MATERIALS AND TO A MINIMUM DEPTH OF 18 INCHES BELOW ALL PROPOSED FOUNDATIONS TO MINIMIZE EFFECTS OF DIFFERENTIAL SETTLEMENTS. THESE EXCAVATIONS CAN GENERALLY BE ACCOMPLISHED USING HEAVY-DUTY CONSTRUCTION EQUIPMENT. HOWEVER, LOCALIZED CEMENTED OR VERY HARD ZONES MAY BE ENCOUNTERED DURING THESE OPERATIONS. GRADING ACTIVITIES SHOULD BE CONTINUOUSLY MONITORED BY CTE. SUCH OBSERVATIONS ARE ESSENTIAL TO IDENTIFY FIELD CONDITIONS THAT DIFFER FROM THOSE IDENTIFIED DURING OUR SUBSURFACE INVESTIGATION AND ADJUST DESIGNS TO ACTUAL FIELD CONDITIONS ENCOUNTERED.
3. FILL PLACEMENT AND COMPACTION
AS STATED, AN ENGINEER OR GEOLOGIST FROM CTE SHOULD BE CALLED UPON TO VERIFY THAT THE PROPER SITE PREPARATION HAS OCCURRED BEFORE FILL PLACEMENT BEGINS. FOLLOWING THE REMOVAL OF LOOSE OR DISTURBED SOILS, AREAS TO RECEIVE FILLS OR CONCRETE OR SLABS ON GRADE SHOULD BE SCARIFIED, MOISTURE CONDITIONED TO ABOVE OPTIMUM MOISTURE CONTENT, AND PROPERLY COMPACTED. FILL AND BACKFILL SHOULD BE COMPACTED TO A MINIMUM RELATIVE COMPACTION OF 90 PERCENT AS EVALUATED BY ASTM D-1557 AT MOISTURE CONTENTS BETWEEN OPTIMUM AND TWO PERCENT ABOVE OPTIMUM. THE OPTIMUM LIFT THICKNESS FOR BACKFILL SOIL WILL BE DEPENDENT ON THE TYPE OF COMPACTION EQUIPMENT USED. GENERALLY BACKFILL SHOULD BE PLACED IN UNIFORM LIFTS NOT EXCEEDING EIGHT INCHES IN LOOSE THICKNESS. BACKFILL PLACEMENT AND COMPACTION SHOULD BE DONE IN OVERALL CONFORMANCE WITH GEOTECHNICAL RECOMMENDATIONS AND LOCAL ORDINANCES.
4. FILL MATERIALS
SOILS DERIVED FROM ON-SITE MATERIALS ARE CONSIDERED SUITABLE FOR REUSE ON THE SITE AREA AS FILL, PROVIDED THEY ARE SCREENED OF ORGANIC MATERIALS AND MATERIALS GREATER THAN THREE INCHES IN MAXIMUM DIMENSION. IMPORTED FILL BENEATH STRUCTURES, PAVEMENTS AND WALKS SHOULD HAVE AN EXPANSION INDEX LESS THAN OR EQUAL TO 30 (PER UBC 18-1-B) WITH LESS THAN 35 PERCENT PASSING THE NO. 200 SIEVE. IMPORTED FILL SOILS FOR USE IN STRUCTURAL OR SLOPE AREAS SHOULD BE EVALUATED BY THE SOILS ENGINEER TO DETERMINE STRENGTH CHARACTERISTICS BEFORE PLACEMENT ON THE SITE.
5. THE ALLOWABLE SOIL BEARING PRESSURE IS 2,000 PSF FOR CONTINUOUS AND ISOLATED SPREAD FOOTINGS. FOOTINGS SHALL EXTEND A MINIMUM DEPTH OF 18 INCHES BELOW LOWEST ADJACENT SUBGRADE.
6. FOOTING ELEVATIONS SHOWN ARE FOR BIDDING PURPOSES ONLY AND ARE ASSUMED TO BE IN SUITABLE BEARING MATERIALS. THE ACTUAL ADEQUACY OF THE BEARING MATERIAL SHALL BE DETERMINED BY A PROFESSIONAL, LICENSED GEOTECHNICAL ENGINEER REGISTERED IN THE STATE OF CALIFORNIA PRIOR TO PLACING OF REINFORCING OR POURING OF CONCRETE, AND FOOTING ELEVATIONS SHALL BE ADJUSTED, OR OTHER REMEDIAL ACTION TAKEN, AS DIRECTED BY THIS PROFESSIONAL AND APPROVED BY THE ARCHITECT, AND DSA.
7. ALL ANCHORS SHALL BE TIED IN PLACE PRIOR TO CALLING FOR FOUNDATION INSPECTION.

METAL DECKS

- 1. METAL DECK SHALL BE STEEL AND SHALL BE "VERCO" PER PLAN. STEEL DECK OF OTHER MANUFACTURERS WILL BE ACCEPTABLE, PROVIDED THE DECK HAS PHYSICAL DIMENSIONS AND PHYSICAL PROPERTIES EQUAL TO OR GREATER THAN THE VERCO STEEL DECK, INCLUDING I.C.B.O. APPROVED DIAPHRAGM SHEAR VALUES.
2. ALL METAL SHALL BE GALVANIZED.
3. THE METAL DECK IS USED AS A DIAPHRAGM.
4. METAL DECK SHALL BE PLACED IN THREE SPAN LENGTHS WHEREVER POSSIBLE.
5. METAL DECK SHALL BE ERECTED SUCH THAT RIBS ARE NORMAL TO THE SUPPORTING MEMBERS.
6. HOLES OR OTHER OPENINGS LARGER THAN 6" IN DIAMETER, IF NOT SHOWN ON STRUCTURAL DRAWINGS, SHALL BE APPROVED BY ENGINEER PRIOR TO CUTTING.

STEEL STUD SHEAR CONNECTORS

- 1. STEEL STUDS WELDED TO STEEL BEAMS OR GIRDERS SHALL BE MADE FROM COLD DRAWN BAR STOCK CONFORMING TO ASTM A-108, GRADE 1010 THROUGH 1020 AND SHALL CONFORM WITH ICBO REPORT ER-2614 AND SECTION 7.2 OF AWS D1.1, LATEST EDITION.
2. STEEL STUDS SHALL BE INSTALLED IN CONFORMANCE WITH THE REQUIREMENTS OF CHAPTER 7 OF AWS D1.1, LATEST EDITION.
3. STUDS WELDED THROUGH THE STEEL DECK SHALL COMPLY WITH SECTION 7.6 OF AWS D1.1 "STUD APPLICATION QUALIFICATION REQUIREMENTS".
4. THE PRODUCTION CONTROL TEST SHALL COMPLY WITH SECTION 7.7 OF AWS D1.1

COLD-FORMED METAL

- 1. PROVIDE METAL STUDS AND ACCESSORIES AS INDICATED ON THE DRAWINGS, AS SPECIFIED HEREIN, AND AS NEEDED FOR A COMPLETE AND PROPER INSTALLATION.
2. CONTRACTOR SHALL PROVIDE EFFECTIVE, FULL TIME QUALITY CONTROL OVER ALL FABRICATION AND ERECTION COMPLYING WITH THE PERTINENT CODES AND REGULATIONS OF GOVERNMENT AGENCIES (DSA) HAVING JURISDICTION.
3. ALL PRODUCTS TO BE MANUFACTURED BY THE CURRENT MEMBERS OF THE STEEL STUD MANUFACTURERS ASSOCIATION AND THE ICBO REPORT ER-4943P, INCLUDING THE SUPPLEMENTAL ICBO SUBMITTAL AND THE PRODUCT TECHNICAL INFORMATION.
4. ALL GALVANIZED STUDS AND JOISTS SHALL BE FORMED FROM STEEL THAT CORRESPONDS TO THE MINIMUM REQUIREMENTS OF 1996 A.I.S.I. STANDARDS.
5. ALL STRUCTURAL MEMBERS SHALL BE DESIGNED IN ACCORDANCE WITH THE AMERICAN IRON AND STEEL INSTITUTE (AISI) "SPECIFICATION FOR THE DESIGN OF COLD-FORMED STEEL STRUCTURAL MEMBERS" 1996 EDITION.
6. ALL LIGHT GAGE STEEL MEMBERS: STUDS AND TRACKS SHALL BE FORMED FROM STEEL HAVING A MINIMUM 33,000 PSI YIELD POINT (ASTM A653 SS GRADE 33 OR ASTM A1011 SS GRADE 33) FOR THICKNESSES OF 0.0179 INCH THROUGH 0.0451 INCH, AND A MINIMUM 50,000 PSI YIELD POINT (ASTM A653 SS GRADE 50, CLASS 1 OR 3, OR ASTM A1011 SS GRADE 50) FOR THICKNESSES OF 0.0538 INCH THROUGH 0.1180 INCH.
7. PROVIDE ALL ACCESSORIES INCLUDING, BUT NOT LIMITED TO, TRACKS, CLIPS, WEB STIFFENERS, ANCHORS, FASTENING DEVICES, RESILIENT CLIPS, AND OTHER ACCESSORIES REQUIRED FOR A COMPLETE AND PROPER INSTALLATION, AND AS RECOMMENDED BY THE MANUFACTURER AND APPROVED BY DSA FOR THE STEEL MEMBERS USED.
8. FASTENING OF COMPONENTS SHALL BE WITH SELF-DRILLING SCREWS OR WELDING. SCREWS OR WELDS SHALL BE OF SUFFICIENT SIZE TO INSURE THE STRENGTH OF THE CONNECTION. ALL WELDS OF GALVANIZED STEEL SHALL BE TOUCHED UP WITH A ZINC-RICH PAINT. ALL WELDS OF CARBON SHEET STEEL SHALL BE TOUCHED UP WITH PAINT. WIRE TYING OF COMPONENTS SHALL NOT BE PERMITTED.

SPECIAL INSPECTION

SPECIAL INSPECTION BY SPECIAL INSPECTORS SATISFACTORY TO THE BUILDING OFFICIAL IS REQUIRED FOR THE FOLLOWING TYPES OF WORK IN CONFORMANCE WITH THE "CALIFORNIA BUILDING CODE (1997 UNIFORM BUILDING CODE AND REVISIONS INCLUDED IN THE TITLE 24) CHAPTER 17A.

- 1. CONCRETE - SPECIAL INSPECTION IS REQUIRED FOR THE FOLLOWING:
A. CONCRETE PLACEMENT: DURING THE TAKING OF TEST SPECIMENS AND PLACING OF REINFORCED CONCRETE AND PNEUMATICALLY PLACED CONCRETE.
B. BOLTS INSTALLED IN CONCRETE: SPECIAL INSPECTION IS REQUIRED PRIOR AND DURING THE INSTALLATION OF THE BOLTS AND PLACING OF THE CONCRETE AROUND SUCH BOLTS.
C. REINFORCING STEEL: DURING THE PLACING OF REINFORCING STEEL FOR ALL CONCRETE.
2. WELDING - SPECIAL INSPECTION IS REQUIRED FOR THE FOLLOWING:
A. DURING ALL FIELD WELDING.
B. DURING ALL STRUCTURAL WELDING INSPECTION OF ALL SHOP AND FIELD WELDING OPERATIONS, INCLUDING THE INSTALLATION OF AUTOMATIC END-WELDED STUD SHEAR CONNECTORS SHALL BE MADE BY A QUALIFIED WELDING INSPECTOR APPROVED BY DSA. SUCH INSPECTOR SHALL BE A PERSON TRAINED AND THOROUGHLY EXPERIENCED IN INSPECTING WELDING OPERATIONS. THE INSPECTOR'S ABILITY TO DISTINGUISH BETWEEN SOUND AND UNSOUND WELDING SHALL BE RELIABLY ESTABLISHED. THE MINIMUM REQUIREMENTS FOR A QUALIFIED WELDING INSPECTOR SHALL BE AS THOSE FOR AN AWS CERTIFIED WELDING INSPECTOR (CWI), AS DEFINED IN THE PROVISIONS OF THE 1992 EDITION OF AWS QCI, STANDARD AND GUIDE FOR QUALIFICATION AND CERTIFICATION OF WELDING INSPECTORS PUBLISHED BY THE AMERICAN WELDING SOCIETY. ALL WELDING INSPECTORS SHALL BE AS APPROVED BY DSA.
C. DURING ALL STRUCTURAL WELDING OF REINFORCING STEEL.
D. INSTALLATION AND INSPECTION OF HIGH STRENGTH BOLTS SHALL BE IN ACCORDANCE WITH THE SPECIFICATION FOR STRUCTURAL JOINTS USING ASTM A-325 BOLTS OF THE RESEARCH COUNCIL OF THE ENGINEERING FOUNDATION, NOVEMBER 13, 1985, 2727 (I), IR 27-7 & UBC STANDARD SECTION 27.705 (c).
E. DURING THE SHEAR STUD APPLICATION QUALIFICATION REQUIREMENTS PER SECTION 7.6 OF AWS D1.1 AND FOR THE PRODUCTION CONTROL PER SECTION 7.7 OF AWS D1.1 LATEST EDITION.
3. GRADING EXCAVATIONS AND FILLING PER SEC. 1701A.4.13

STRUCTURAL OBSERVATION (AS REQUIRED BY SECTION 1702A OF THE C.B.C.)

- 1. THE DESIGN PROFESSIONAL RESPONSIBLE FOR THE STRUCTURAL DESIGN SHALL SUBMIT A STATEMENT IN WRITING TO DSA, STATING THAT DURING THE CONSTRUCTION OF THIS STRUCTURE, SITE VISITS HAVE BEEN PERFORMED TO OBSERVE GENERAL COMPLIANCE WITH THE APPROVED STRUCTURAL PLANS, SPECIFICATIONS AND CHANGE ORDERS. FURTHER, THE STATEMENT SHALL, IN DETAIL, NOTE HOW ANY DEFICIENCIES HAVE BEEN CORRECTED.
2. AT A MINIMUM, THIS STRUCTURE REQUIRES SITE OBSERVATIONS BEFORE THE FOLLOWING HAS BEEN STARTED:
A. POURING THE INITIAL FOOTINGS.
B. DURING THE START OF ERECTION OF THE ROUGH STRUCTURE.
C. AFTER COMPLETION OF ALL THE ROUGH FRAMING.
D. POURING THE INITIAL CONCRETE TOPPING.

- 10. MINIMUM PROTECTIVE CONCRETE COVERAGE OF REINFORCING:
A. ON EARTH SIDE WHEN PLACED AGAINST EARTH..... 3 IN.
B. ON EARTH SIDE WHEN FORMED..... 2 IN.
C. EXTERIOR WALL STEEL ABOVE GRADE..... 1 1/2 IN.
D. INTERIOR WALL STEEL AND SUPPORTED SLABS..... 1 IN.
E. TIED COLUMNS (TO TIES) ABOVE GRADE..... 1 1/2 IN.
F. BEAMS (TO STIRRUPS) ABOVE GRADE..... 1 1/2 IN.
G. WELDED WIRE FABRIC..... CENTER LINE OF SLAB
11. NO PIPES OR DUCTS SHALL BE PLACED IN CONCRETE COLUMNS, WALLS OR SLABS UNLESS SPECIFICALLY DETAILED OR UNLESS SLEEVES ARE PROVIDED IN ACCORDANCE WITH THE TYPICAL DETAIL 13 ON SHEET SQL1.1.
12. WALLS AND COLUMNS SHALL BE DOWELED FROM SUPPORTS WITH BARS OF THE SAME SIZE AND SPACING.
13. SPLICE CONTINUOUS REINFORCING IN ACCORDANCE WITH THE TYPICAL DETAIL 5 ON SHEET SQL1.1 STAGGER SPLICE ALL REINFORCING.
14. PROVIDE MINIMUM EMBEDMENT OF REINFORCING IN CONFORMANCE WITH THE TYPICAL DETAIL 1 ON SHEET SQL1.1
15. REFER TO THE ARCHITECTURAL, MECHANICAL AND ELECTRICAL DRAWINGS FOR LOCATIONS OF PIPES, DUCTS, VENTS AND SIMILAR OPENINGS.
16. REINFORCING, ANCHOR BOLTS AND ALL OTHER EMBEDDED ITEMS SHALL BE SECURELY HELD IN POSITION AND SHALL BE INSPECTED PRIOR TO PLACING CONCRETE.
17. CHAMFER: 3/4" ON ALL EXPOSED CORNERS.
18. ALL ANCHOR BOLTS SHALL BE TIED IN PLACE PRIOR TO PLACING CONCRETE.
19. NO FIELD BENDING OF REINFORCEMENT WILL BE PERMITTED UNLESS APPROVED BY DSA AND THE ENGINEER.

REINFORCING STEEL

- 1. DETAILS OF REINFORCEMENT SHALL BE IN ACCORDANCE WITH CHAPTER 7 OF "AMERICAN CONCRETE INSTITUTE" 318 UNLESS OTHERWISE NOTED. REINFORCING STEEL DETAILING, BENDING AND PLACING SHALL BE IN ACCORDANCE WITH THE CONCRETE REINFORCING STEEL INSTITUTE "MANUAL OF STANDARD PRACTICE", LATEST EDITION.
2. WELDING OF REINFORCING STEEL, IF PERMITTED BY THE ARCHITECT, SHALL BE IN ACCORDANCE WITH THE "STRUCTURAL WELDING CODE - REINFORCING STEEL" OF THE AMERICAN WELDING SOCIETY, AWS D1-4, AND SHALL BE PERFORMED BY WELDERS QUALIFIED UNDER THE PROCEDURES CONTAINED THEREIN.
3. ALL REINFORCING STEEL SHOP DRAWINGS SHALL BE REVIEWED BY THE STRUCTURAL ENGINEER AND THE ARCHITECT PRIOR TO FABRICATION.
4. ALL REINFORCING STEEL SHALL CONFORM TO ASTM A-615, GRADE 60 FOR NO.4 AND LARGER, OTHERWISE GRADE 40. WELDED REINFORCING WHERE PERMITTED SHALL CONFORM TO ASTM 706, OR A VERIFIED AND APPROVED EQUIVALENT.
5. WALLS AND COLUMNS SHALL BE DOWELED FROM SUPPORTS WITH BARS OF THE SAME SIZE AND SPACING.
6. SPACER TIES: PROVIDE A MINIMUM OF #3 TIES AT 24 INCHES IN ALL BEAMS AND FOOTINGS.
7. SPLICE MINIMUM REINFORCING IN ACCORDANCE WITH THE TYPICAL DETAIL 5 ON SHEET SQL1.1
8. PROVIDE MINIMUM EMBEDMENT OF REINFORCING IN CONFORMANCE WITH THE TYPICAL DETAIL 1 ON SHEET SQL1.1
9. BAR SUPPORTS SHALL BE PROVIDED IN ACCORDANCE WITH THE PROVISIONS OF "BAR SUPPORT SPECIFICATION" BY THE CONCRETE REINFORCING STEEL INSTITUTE (CRSI).
10. ALL REINFORCING STEEL AND OTHER EMBEDDED ITEMS SHALL BE SECURELY HELD IN POSITION AND SHALL BE INSPECTED PRIOR TO PLACING CONCRETE AND OR GROUT.
11. NO FIELD BENDING OF REINFORCEMENT WILL BE PERMITTED UNLESS APPROVED BY DSA AND THE ENGINEER.

STRUCTURAL STEEL

- 1. STRUCTURAL WIDE FLANGE STEEL INCLUDES "WT" SHAPES SHALL COMPLY WITH ASTM A 992 GRADE 50, EXCEPT FOR ANGLES, PLATES AND MISC. STEEL WITH ASTM A-36.
2. MATERIALS AND WORKMANSHIP SHALL COMPLY WITH A.I.S.C. "SPECIFICATION FOR THE DESIGN, FABRICATION AND ERECTION OF STRUCTURAL STEEL FOR BUILDINGS".
3. ALL WELDING SHALL BE BY THE ARC WELDING PROCESS USING E-70 ELECTRODES AND CERTIFIED WELDERS. THE USE OF E70T-4 IS PROHIBITED.
4. CONNECTED MEMBERS SHALL BEAR ONLY UPON THE UNTHREADED PORTION OF BOLTS.
5. ROUND HSS (HOLLOW STRUCTURAL SECTION) SHALL COMPLY WITH ASTM A-53, GRADE B (FY=35 KSI).
6. RECTANGULAR HSS (HOLLOW STRUCTURAL SECTION) SHALL COMPLY WITH ASTM A 500, GRADE B (FY=46 KSI).
7. HIGH STRENGTH BOLTS SHALL COMPLY WITH ASTM A-325, UNLESS OTHERWISE NOTED.
8. MACHINE BOLTS SHALL COMPLY WITH ASTM A-307, UNLESS OTHERWISE NOTED.
9. ALL FIELD AND SHOP WELDING AND HIGH STRENGTH BOLTING SHALL BE CONTINUOUSLY INSPECTED BY A D.S.A. APPROVED SPECIAL INSPECTOR.
10. STEEL SHALL BE IDENTIFIED BY MILL CERTIFICATES.
11. BOLT HOLES- STANDARD BOLT HOLES. FOR BEAM AND GIRDERS- 1/16" LARGER THAN BOLT SIZE. FOR BASE PLATES- 1/8" OVERSIZE BOLT HOLES.
12. BASE PLATES: BEDDED ON 1 1/2" MIN. NON-SHRINK GROUT, U.O.N.
13. FILLER METAL USED IN ARC WELDING SHALL BE IN ACCORDANCE WITH TABLE 4.1.1 OF "STRUCTURAL WELDING CODE - STEEL" (AWS D1.1-00). THE MIN. TENSILE STRENGTH FOR FILLER METAL SHALL BE 70 KSI
14. ALL FULL PENETRATION BUTT WELDS ARE REQUIRED TO HAVE ULTRASONIC TESTING (UT) PERFORMED BY A CERTIFIED TESTING INSPECTION LABORATORY.
15. STRUCTURAL STEEL SHOP DRAWINGS: TO BE REVIEWED BY THE ARCHITECT AND STRUCTURAL ENGINEER BEFORE FABRICATION.
16. EXPOSED STRUCTURAL STEEL AND MISCELLANEOUS STEEL: HOT DIP GALVANIZED.
17. ALL WELDS USED IN PRIMARY MEMBERS AND CONNECTIONS IN THE LATERAL FORCE-RESISTING SYSTEM SHALL BE MADE WITH A FILLER METAL THAT HAS A MINIMUM CHARPY V-NOTCH TOUGHNESS OF 20 ft.-lbs., AT MINUS 20 DEGREES F, AS DETERMINED BY AWS CLASSIFICATION OR MANUFACTURE CERTIFICATION. IN ADDITION, ALL WELDS DENOTED BY "C.V.N." MUST MEET THIS REQUIREMENT.

DESIGN CRITERIA

- 1. "CALIFORNIA CODE OF REGULATIONS" (C.C.R.), TITLE 24, 2001 EDITION AND STANDARDS REFERENCED THEREIN.
2. "RECOMMENDED LATERAL FORCE REQUIREMENTS AND COMMENTARY" BY THE STRUCTURAL ENGINEERS ASSOCIATION OF CALIFORNIA, 1999 EDITION.
3. DESIGN LOADS:
A. LIVE LOADS
ROOF.....16 PSF (REDUCIBLE)
FLOOR.....50 PSF (REDUCIBLE)
STAIRS & CORRIDORS.....100 PSF (REDUCIBLE)
B. WIND (PER UBC)
LESS THAN 40 FEET ABOVE GROUND..... 17.4 PSF (HRZ)
9.4 PSF (UPWARD)
BASIC WIND SPEED..... 70 MPH
EXPOSURE CATEGORY = C
C. SEISMIC -
SEISMIC ZONE 4, Z=0.4 I = 1.15
SEISMIC SOURCE TYPE "B" R = 5.6
No = 1.0 Vmax = 2.5 Ca / R
Soil PROFILE TYPE: Sd = 0.196 W (ULTIMATE STRENGTH)
Ca = 0.64(Na) = 0.64 = 0.140 W (WORKING STRESS)
Ca = 0.44(Na) = 0.44

SLAB-ON-GRADE

- 1. THE PURPOSE OF THESE NOTES IS TO ACHIEVE THE BEST POSSIBLE FLOOR FINISH UTILIZING THE EXPERIENCE OF THE CONTRACTOR SINCE THE CONTRACTOR'S MEANS AND METHODS SIGNIFICANTLY AFFECT THE QUALITY AND THEREFORE THE SUCCESS OR FAILURE OF THE DESIGN.
2. THE CONTRACTOR SHALL SUBMIT A POUR SEQUENCE SCHEDULE FOR REVIEW BY THE ARCHITECT AND STRUCTURAL ENGINEER PRIOR TO CASTING ANY SLAB-ON-GRADES. THE SUBMITTAL MUST CONTAIN THE FOLLOWING: AMOUNT OF CEMENT, STRENGTH OF CONCRETE, AGGREGATE SIZE, SLUMP AMOUNT AND THE CONTRACTOR'S ENDORSEMENT THAT HE CAN PRODUCE A SUCCESSFUL SLAB-ON-GRADE.
3. IF A PUMP MIX IS PROPOSED, IT SHOULD BE PROPORTIONED TO MINIMIZE SHRINKAGE IN ADDITION TO CONFORMING TO ALL OTHER REQUIREMENTS.
4. AS A GUIDELINE TO THE CONTRACTOR, THE SLAB-ON-GRADE SHALL BE CAST IN SQUARE OR RECTANGULAR SECTIONS APPROXIMATELY 400 SQUARE FEET MAXIMUM IN AREA WITH MAXIMUM DISTANCE OF 20 FEET BETWEEN CONSTRUCTION OR WEAKENED JOINTS.
5. AS A FURTHER GUIDELINE TO THE CONTRACTOR, THE DRAWINGS MAY CONTAIN SUGGESTED LOCATIONS FOR CONSTRUCTION JOINTS (C.J.) AND WEAKENED JOINTS (W.J.).
6. REFER TO DETAIL 7 ON SHEET SQL1.1 FOR WEAKENED JOINT (W.J.) AND CONSTRUCTION JOINT (C.J.) DETAIL.

REINFORCED CONCRETE

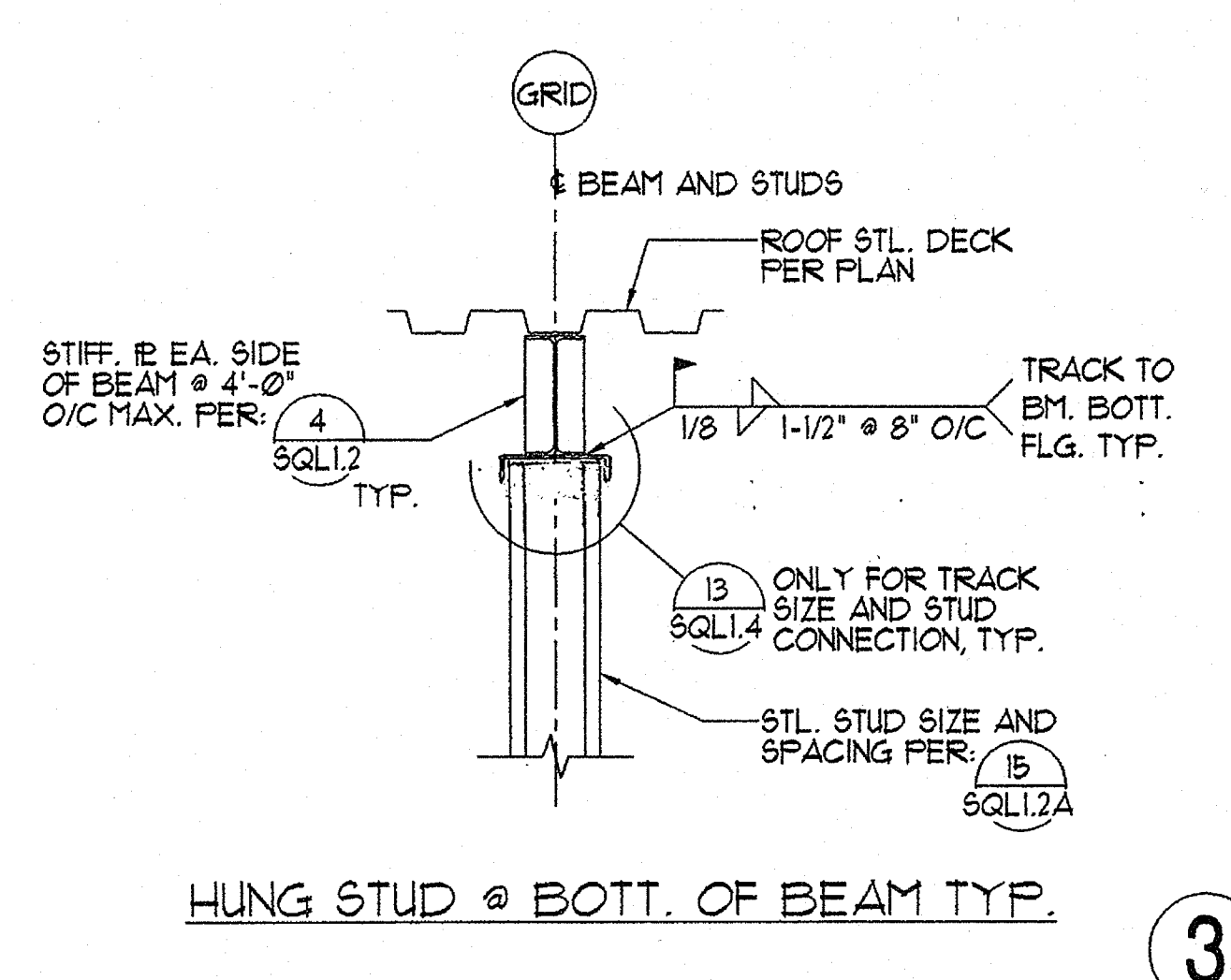
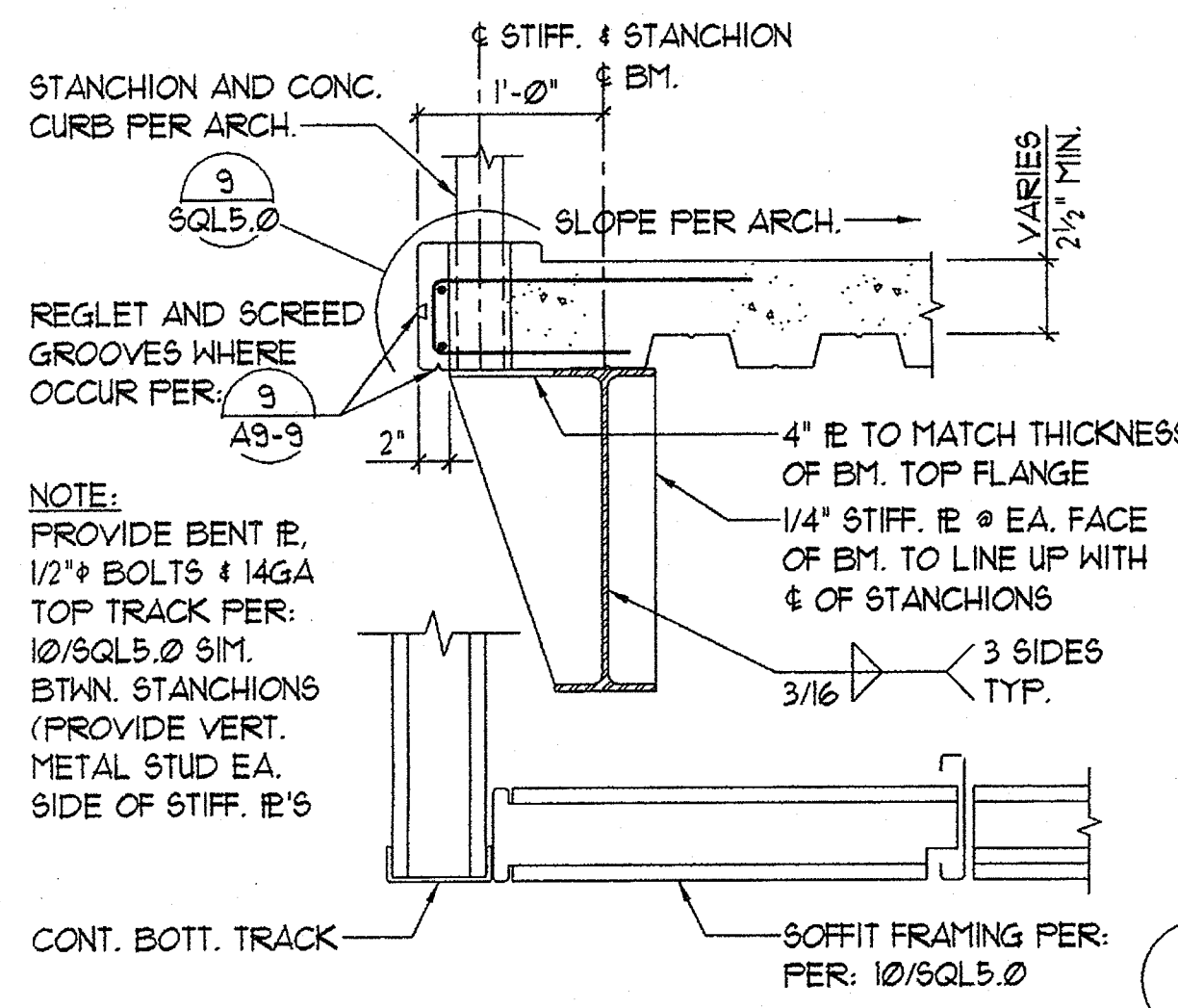
- 1. ALL CONCRETE WORK SHALL CONFORM TO THE "REQUIREMENTS FOR REINFORCED CONCRETE" (ACI 318) AND THE "SPECIFICATIONS FOR STRUCTURAL CONCRETE FOR BUILDINGS" (ACI 301), LATEST APPROVED EDITIONS, WITH MODIFICATIONS AS NOTED IN THE DRAWINGS, SPECIFICATIONS, AND TITLE 24.
2. ALL REINFORCING DETAILING SHALL CONFORM TO THE CONCRETE REINFORCING STEEL INSTITUTE "MANUAL OF STANDARD PRACTICE" LATEST EDITION.
3. CONTINUOUS INSPECTION BY AN D.S.A. APPROVED INSPECTOR IS REQUIRED FOR ALL STRUCTURAL REINFORCED CONCRETE WORK.
4. WELDING OF REINFORCING STEEL IF PERMITTED BY THE D.S.A. AND THE STRUCTURAL ENGINEER, SHALL BE IN ACCORDANCE WITH THE "STRUCTURAL WELDING CODE - REINFORCING STEEL" OF THE AMERICAN WELDING SOCIETY, AWS D1-4, AND SHALL BE PERFORMED BY WELDERS QUALIFIED UNDER THE PROCEDURES CONTAINED THEREIN.
5. ALL REINFORCING STEEL SHOP DRAWINGS SHALL BE REVIEWED BY THE ARCHITECT AND STRUCTURAL ENGINEER PRIOR TO FABRICATION.
6. CONCRETE STRENGTHS: THE CONCRETE STRENGTHS SHOWN IN THE FOLLOWING TABLE ARE MINIMUM COMPRESSIVE STRENGTH AT 28 DAYS, THE AGGREGATES SHOWN ARE THE MAXIMUM SIZE (INCHES) AND THE SLUMP SHOWN IS THE MAXIMUM (INCHES). THE COMPRESSIVE STRENGTH OF CONCRETE SHALL BE PROPORTIONAL AS REQUIRED IN C.C.R. TITLE 24, SECTION 1905A.2 FOR EITHER METHOD A, B, OR C.

Table with 3 columns: ITEM OF CONSTRUCTION (145 P.C.F. AVE.), STRENGTH, AGGREGATE, SLUMP. Includes rows for Foundation System, Slab on Grade, Misc. Patching, Concrete Topping, and Batch Plant Inspection.

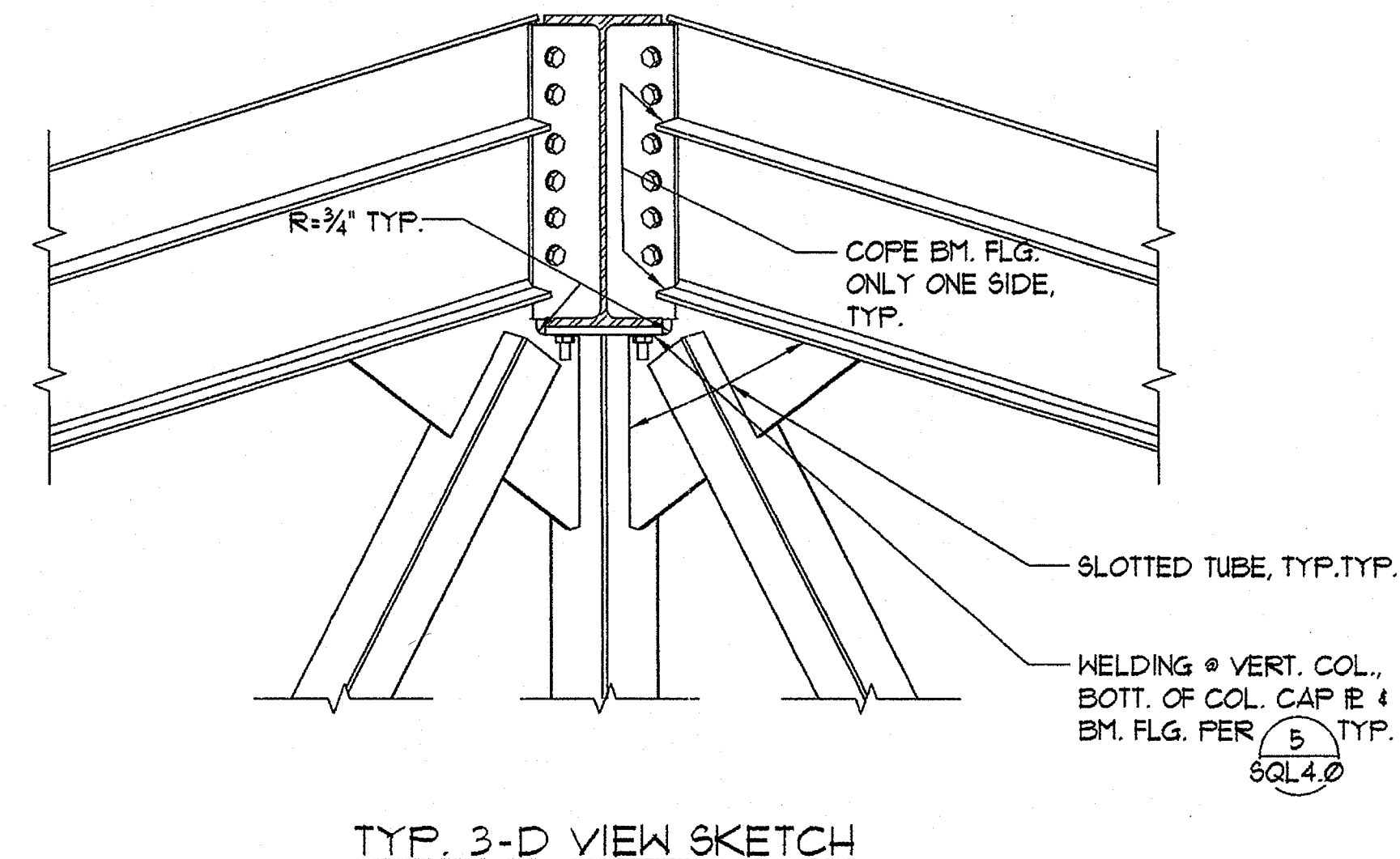
WHEN BATCH PLANT INSPECTION IS WAIVED, THE FOLLOWING REQUIREMENTS SHALL APPLY AND SHALL BE DESCRIBED IN THE CONTRACT SPECIFICATIONS: APPROVED INSPECTOR OF THE TESTING LABORATORY SHALL CHECK THE FIRST BATCHING AT THE START OF WORK AND FURNISH MIX PROPORTIONS TO THE LICENSED WEIGHMASTER. LICENSED WEIGHMASTER TO POSITIVELY IDENTIFY MATERIALS AS TO QUANTITY AND CERTIFY TO EACH LOAD BY A TICKET. TICKETS SHALL BE TRANSMITTED TO THE PROJECT INSPECTOR BY A TRUCK DRIVER WITH LOAD IDENTIFIED THEREON. INSPECTOR WILL NOT ACCEPT THE LOAD WITHOUT A LOAD TICKET IDENTIFYING THE MIX AND WILL KEEP A DAILY RECORD OF PLACEMENTS, IDENTIFYING EACH TRUCK, ITS LOAD AND TIME OF RECEIPT, AND APPROXIMATE LOCATION OF DEPOSIT IN THE STRUCTURE WILL TRANSMIT A COPY OF THE DAILY RECORD TO THE ENFORCEMENT AGENCY. AT THE END OF THE PROJECT, THE WEIGHMASTER SHALL FURNISH AN AFFIDAVIT TO THE ENFORCEMENT AGENCY ON FORM SSS 411-8 CERTIFYING THAT ALL CONCRETE FURNISHED CONFORMS IN EVERY PARTICULAR TO PROPORTIONS ESTABLISHED BY MIX DESIGNS. THE EVALUATION AND ACCEPTANCE OF CONCRETE SHALL CONFORM TO THE REQUIREMENTS OF C.B.C. TITLE 24 SECTION 1905A.6. ALL REINFORCING STEEL SHALL CONFORM TO ASTM A615, GRADE 60 FOR NO. 4 AND LARGER, OTHERWISE GRADE 40.

FOR GENERAL NOTES CONT. SEE SQL1.0A

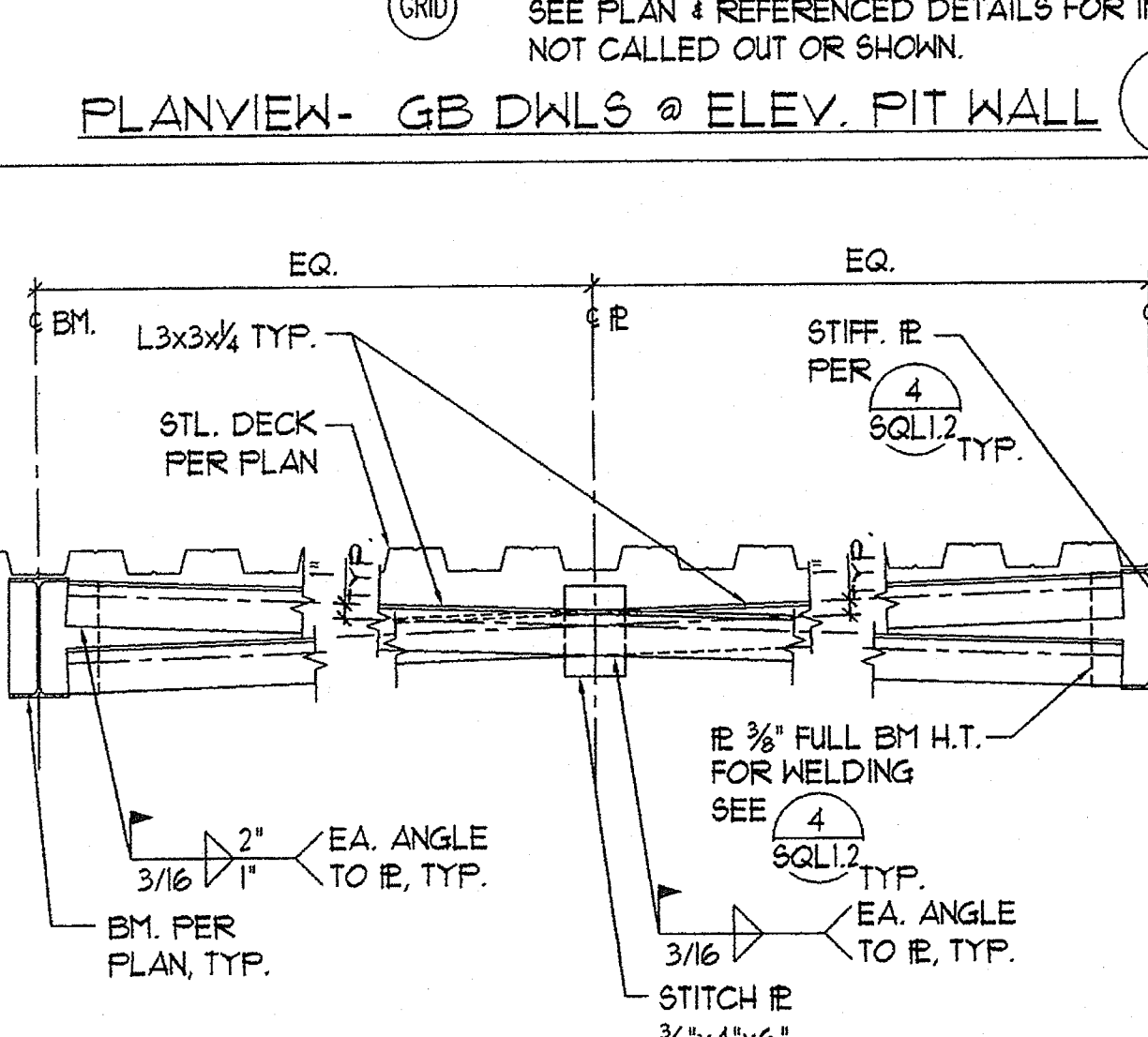
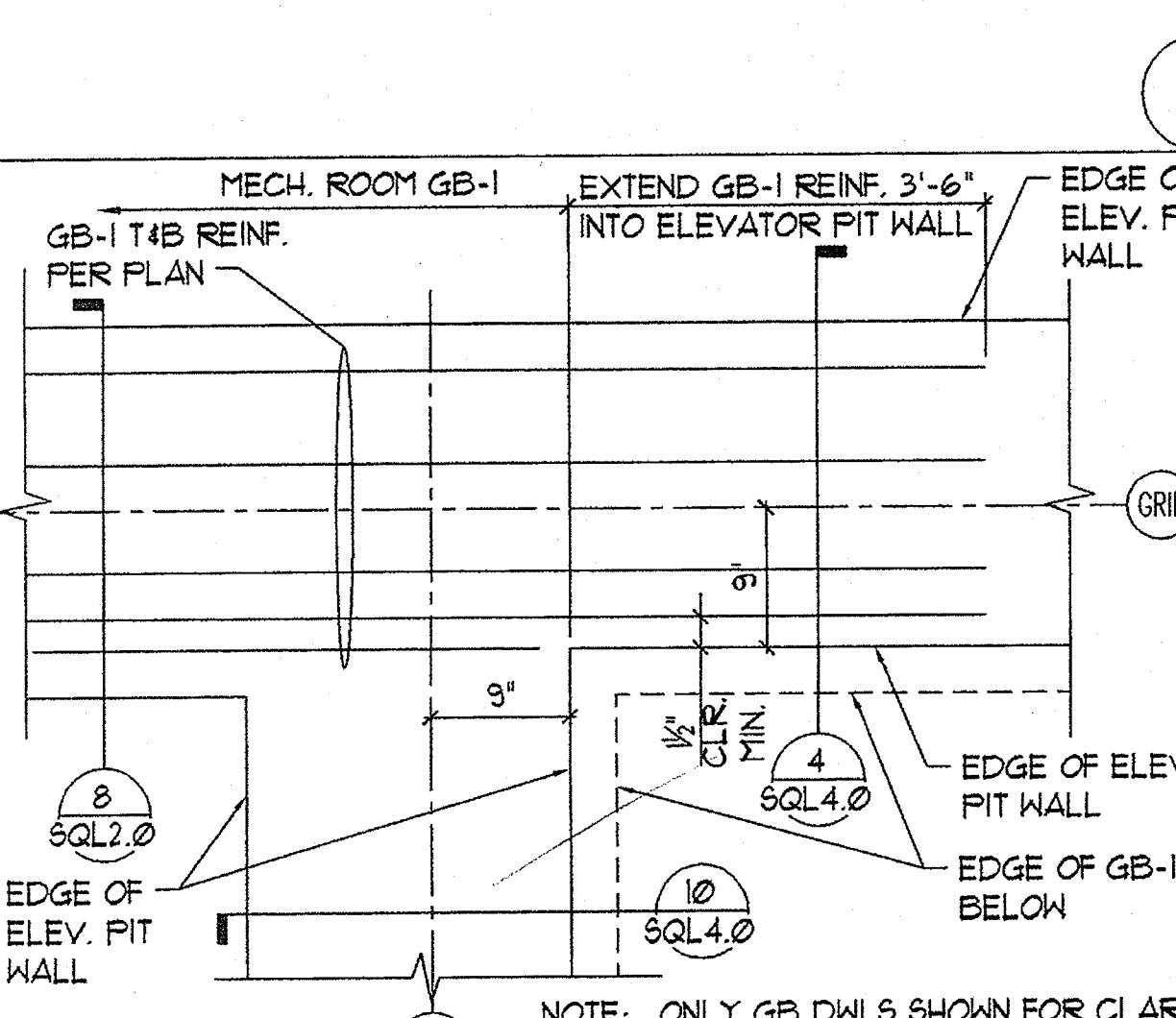
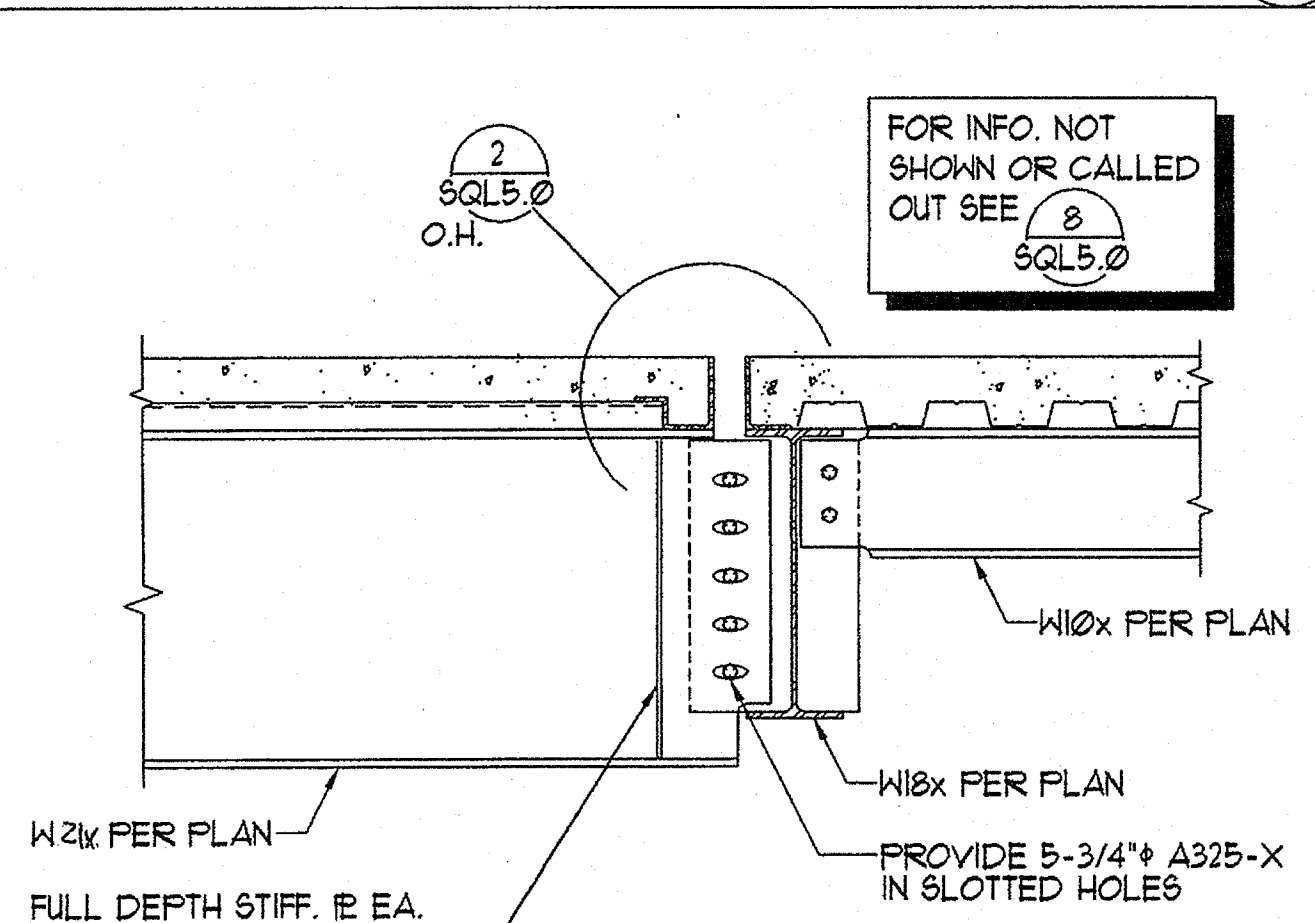
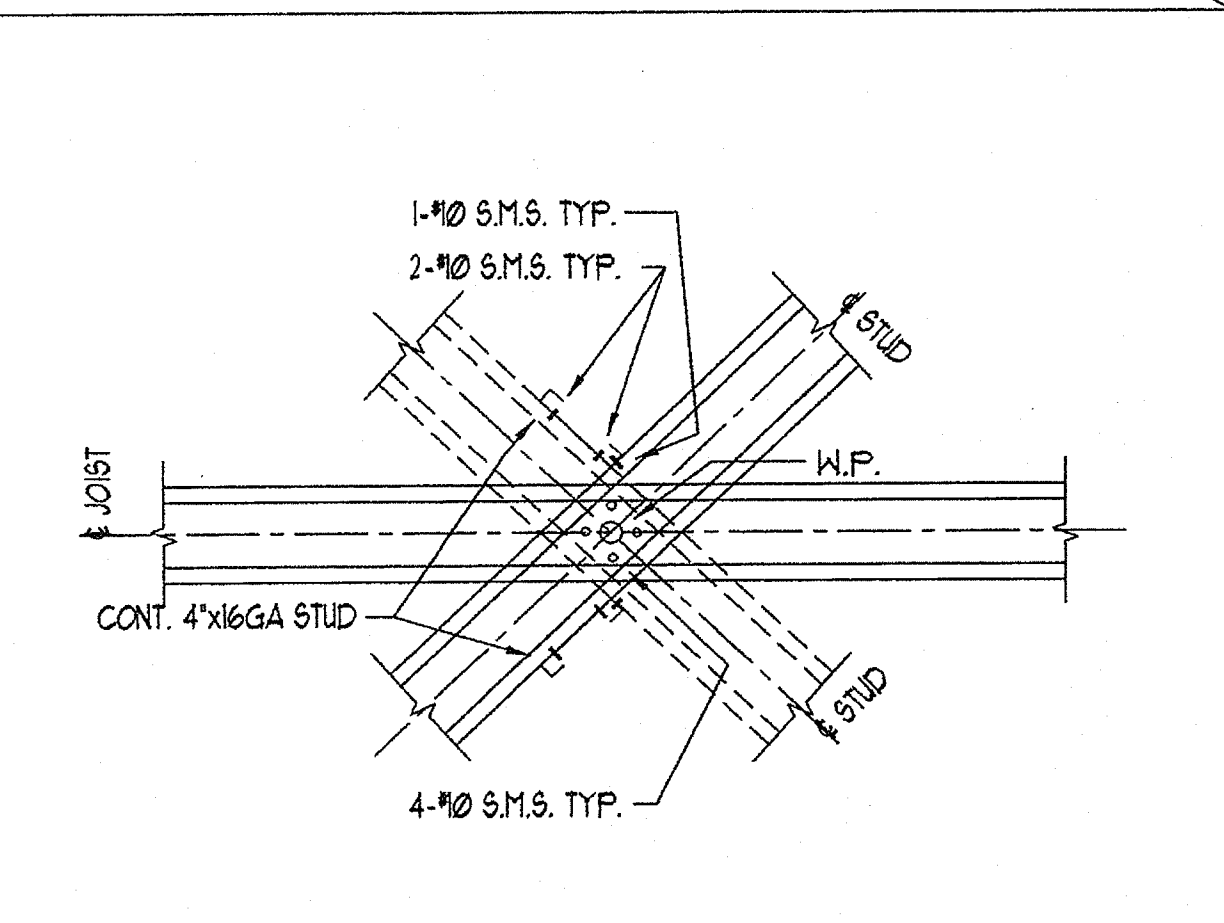
Vertical sidebar containing project information: GROTTH ARCHITECTS, INC., 3555 MISSION AVE., OCEANSIDE, CALIFORNIA 92054. Phone: 760-754-8191, Fax: 760-754-8291. Includes logos for DSA, FLC, and Groth Architects, and a 'GENERAL NOTES' title.



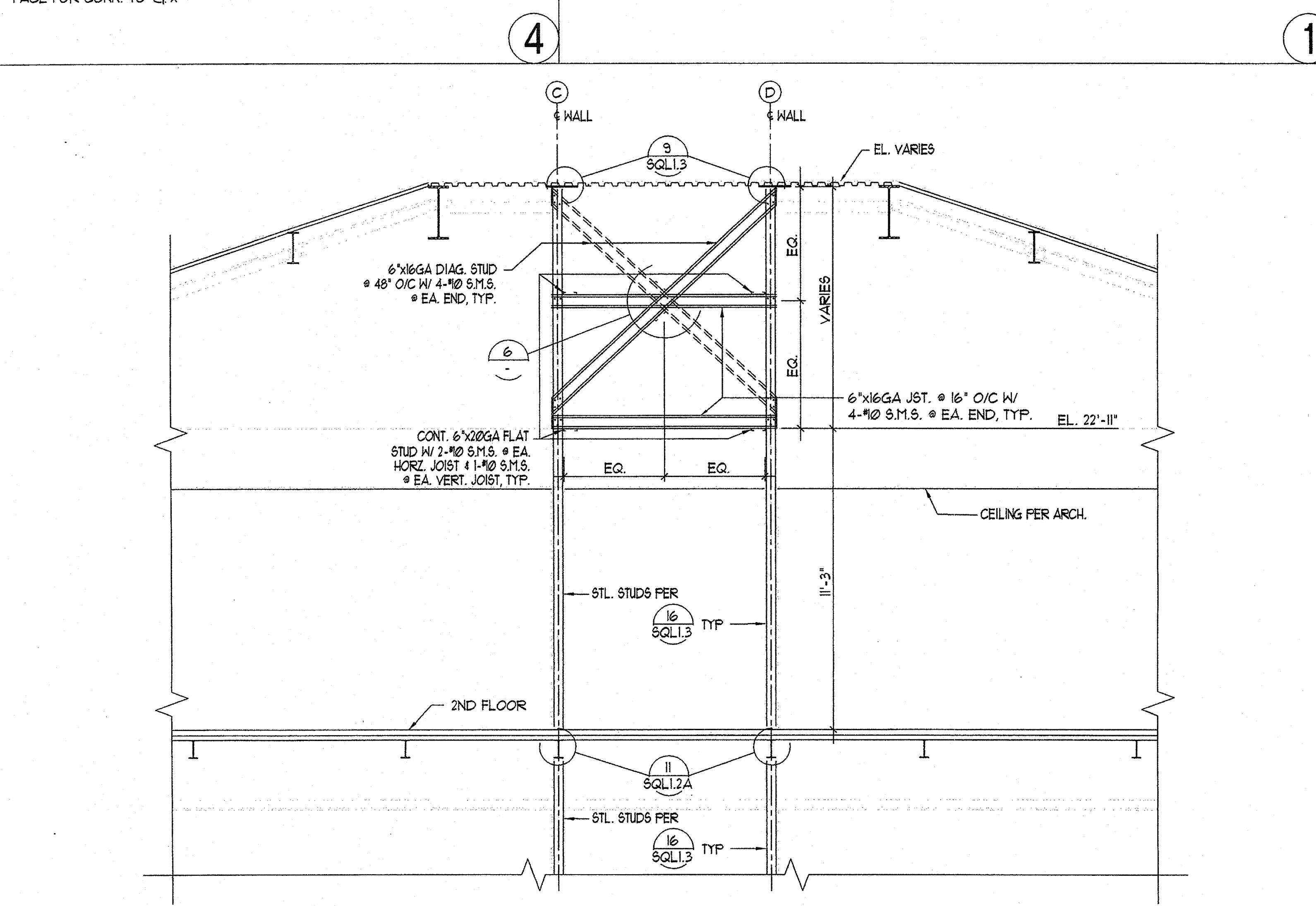
- POWDER DRIVEN SHOT PINS (LOW VELOCITY)**
1. QUALIFICATION FOR USE OF ALL POWER ACTUATED TOOLS MUST MEET ANSI A10.3 STANDARD AS REQUIRED BY THE MANUFACTURER AND ALL OSHA REQUIREMENTS.
 2. TESTING: THE OPERATOR, TOOL, AND FASTENER SHALL BE PRE-QUALIFIED BY THE PROJECT INSPECTOR. HE SHALL OBSERVE THE TESTING OF THE FIRST 10 FASTENER INSTALLATIONS. A TEST "PULL-OUT" LOAD OF NOT LESS THAN TWICE THE DESIGN LOAD SHALL BE APPLIED TO THE PIN IN SUCH A MANNER AS NOT TO RESIST THE SPALLING TENDENCY OF THE CONCRETE SURROUNDING THE PIN. THEREAFTER, RANDOM TESTS UNDER THE PROJECT INSPECTOR'S SUPERVISION SHALL BE MADE OF APPROXIMATELY 1 IN 10 PINS. IF ANY PIN FAILS TESTING, TEST ALL PINS OF THE SAME CATEGORY NOT PREVIOUSLY TESTED UNTIL TWENTY (20) CONSECUTIVE PASS, THEN RESUME THE INITIAL TESTING FREQUENCY.



TYP. 3-D VIEW SKETCH



TYP. BEAM BOTT. FLANGE BRACING



PARTIAL BUILDING CROSS SECTION

- GENERAL NOTES (CONT.)**
- EXPANSION BOLTS OR EPOXY-TYPE ANCHORS IN CONCRETE (HARD ROCK OR LIGHTWEIGHT) PER 2001 CBC, TITLE 24, CHAPTER 19A, SEC. 1923A.3.5 AND IR 19-1**
1. ALL EXPANSION ANCHORS SHOULD MEET THE MINIMUM DEPTH OF EMBEDMENT CRITERIA ESTABLISHED BY THE ICGO REPORT.
 2. INSTALLED ANCHORS ARE TO BE IN ACCORDANCE WITH THE REQUIREMENTS GIVEN IN THE ICGO RESEARCH COMMITTEE RECOMMENDATIONS FOR THE SPECIFIC ANCHOR, OR AS REQUIRED BY THE MANUFACTURER FOR THE ANCHORS SUBMITTED WITHOUT ICGO RESEARCH COMMITTEE RECOMMENDATIONS.
 3. WHEN EXPANSION ANCHORS ARE INSTALLED INTO CONCRETE SUPPORTED BY A METAL DECK, THE ANCHORS SHALL BE CENTERED ON THE "LOW" FLUTE OF THE DECKING WHERE PRACTICABLE. THE DECK SHOULD HAVE A MINIMUM THICKNESS OF 20 GAGE, WITH FLUTES AS WIDE AS POSSIBLE.
 4. WHEN INSTALLED FROM THE BOTTOM, EMBEDMENT SHALL BE 1 1/2" ABOVE THE TOP OF THE DECKING FLUTE (EXCEPT 1/4" DIA. AND 5/16" DIA. ANCHORS FOR CEILINGS). THE EFFECTIVE DEPTH OF EMBEDMENT SHOULD BE CONSIDERED TO BE 1/3 OF THE METAL DECK HEIGHT PLUS THE 1 1/2" NOTED ABOVE. THERE SHALL BE A ONE INCH (0'-1") MINIMUM CLEAR DISTANCE FROM THE TOP OF CONCRETE TO END OF BOLT.
 5. WHEN INSTALLED FROM THE TOP OF CONCRETE, THE DEPTH OF EMBEDMENT SHALL BE THE DEPTH OF CONCRETE ABOVE THE TOP OF THE FLUTE PLUS ONE THIRD (1/3) THE FLUTE HEIGHT. A MINIMUM ONE INCH (0'-1") CLEAR DISTANCE FROM THE STEEL DECK TO THE BOTTOM OF THE ANCHOR MUST BE MAINTAINED.
 6. HOLES SHALL BE CLEAN AND FREE FROM DUST IMMEDIATELY PRIOR TO INSTALLATION OF THE ANCHOR.
 7. TEST LOADS, JOB SITE TESTING FOR VERIFYING SATISFACTORY INSTALLATION AND WORKMANSHIP IS REQUIRED.
 8. THE TEST LOAD MAY BE APPLIED BY ANY METHOD THAT WILL EFFECTIVELY MEASURE THE TENSION IN THE ANCHOR (REF. ASTM E488-90), SUCH AS DIRECT PULL WITH A HYDRAULIC JACK, CALIBRATED SPRING LOADING DEVICES, OR A CALIBRATED TORQUE WRENCH.
 9. ANCHORS IN WHICH TORQUE IS USED TO EXPAND THE ANCHOR WITHOUT APPLYING TENSION TO THE BOLT (TORQUE CONTROLLED ANCHOR) REQUIRES SUFFICIENT DATA FROM EITHER THE MANUFACTURER, OR FROM INDEPENDENT TESTING, TO ESTABLISH APPROPRIATE TORQUE TEST VALUES. THE TEST VALUES MUST BE SHOWN ON THE CONTRACT DOCUMENTS.
 10. THE FOLLOWING CRITERIA APPLY FOR THE ACCEPTANCE OF INSTALLED ANCHORS:
 - A. HYDRAULIC RAM METHOD: THE ANCHOR MUST NOT EXHIBIT OBSERVABLE MOVEMENT AT THE APPLICABLE TEST LOAD. FOR WEDGE AND SLEEVE TYPE ANCHORS, A PRACTICAL WAY TO DETERMINE OBSERVABLE MOVEMENT IS THAT THE WASHER UNDER THE NUT BECOMES LOOSE. DROP-IN ANCHORS ARE ONLY TO BE TESTED WITH THIS METHOD.
 - B. TORQUE WRENCH METHOD: THE APPLICABLE TEST TORQUE FOR WEDGE OR SLEEVE-TYPE ANCHORS, MUST BE REACHED WITHIN THE FOLLOWING LIMITS, ONE-HALF (1/2) TURN OF THE NUT; ONE-QUARTER (1/4) TURN OF THE NUT FOR THE 3/8" DIA. SLEEVE ANCHOR ONLY.
 - C. IF ANY ANCHOR FAILS TESTING, ALL ANCHORS OF THE SAME CATEGORY, NOT PREVIOUSLY TESTED, SHALL BE TESTED UNTIL 20 CONSECUTIVE ANCHORS PASS THE TEST REQUIREMENTS. THE INITIAL TESTING FREQUENCY SHALL THEN BE RESUMED.

TEST VALUES HARDROCK OR LIGHTWEIGHT CONCRETE						
ANCHOR	WEDGE	SLEEVE		SHELL		
DIA. (in)	LOAD (lbs)	TORQUE (ft-lbs)	LOAD (lbs)	TORQUE (ft-lbs)	LOAD (lbs)	TORQUE (ft-lbs)
1/4	800	10	400	4	1000	-
5/16	-	-	400	5	1400	-
3/8	1100	25	700	10	1800	-
1/2	2000	50	900	20	2700	-
5/8	2300	80	1100	45	3700	-
3/4	3700	150	1400	90	5400	-
1	5800	250	-	-	-	-

- NOTES**
1. ANCHOR DIA. REFERS TO THE THREAD SIZE FOR THE WEDGE AND SHELL CATEGORIES AND TO THE ANCHOR OUTSIDE DIA. FOR THE SLEEVE CATEGORY.
 2. APPLY PROOF TEST LOADS TO WEDGE & SLEEVE ANCHORS WITHOUT REMOVING THE NUT IF POSSIBLE. IF NOT, REMOVE NUT AND INSTALL A THREADED COUPLER TO THE SAME TIGHTNESS OF THE ORIGINAL NUT USING A TORQUE WRENCH & APPLY LOAD.
 3. FOR SLEEVE/SHELL INTERNALLY THREADED CATEGORIES, VERIFY THAT THE ANCHOR IS NOT PREVENTED FROM WITHDRAWING BY A BASEPLATE OR OTHER FIXTURES. IF RESTRAINT IS FOUND, LOOSEN AND SHIM OR REMOVE FIXTURE(S) PRIOR TO TESTING.
 4. REACTION LOADS FROM TEST FIXTURES MAY BE APPLIED CLOSE TO THE ANCHOR BEING TESTED, PROVIDED THE ANCHOR IS NOT RESTRAINED FROM WITHDRAWING BY THE FIXTURE(S).
 5. SHELL TYPE ANCHORS SHOULD BE TESTED AS FOLLOWS: VISUALLY INSPECT 25% FOR FULL EXPANSION AS EVIDENCED BY THE LOCATION OF THE EXPANSION PLUS IN THE ANCHOR BODY. PLUG LOCATION OF A FULLY EXPANDED ANCHOR SHOULD BE AS RECOMMENDED BY THE MANUFACTURER, OR, IN THE ABSENCE OF SUCH RECOMMENDATION, AS DETERMINED ON THE JOB SITE FOLLOWING THE MANUFACTURER'S INSTALLATION INSTRUCTIONS, AND: PROOF LOAD 5% AS INDICATED IN THE TABLE ABOVE, BUT NOT LESS THAN THREE ANCHORS PER DAY FOR EACH DIFFERENT PERSON OR CREW INSTALLING ANCHORS, OR; TEST 50% OF THE INSTALLED ANCHORS PER 1923A.3.5.
 6. TEST EQUIPMENT IS TO BE CALIBRATED BY AN APPROVED TESTING LABORATORY IN ACCORDANCE WITH STANDARD RECOGNIZED PROCEDURES.
 7. TORQUE TEST VALUES FOR SHELL TYPE ANCHORS ARE OMITTED DUE TO LACK OF DATA. TORQUE TESTING CAN OCCUR ON AN INDIVIDUAL BASIS WHEN TEST PROCEDURES ARE SUBMITTED AND APPROVED BY THE ENFORCEMENT AGENCY. TABULATED VALUES MAY BE FORTHCOMING ONCE THE ENFORCEMENT AGENCY HAS MORE DATA TO EVALUATE THE FEASIBILITY OF STANDARD TORQUE VALUES.
 8. TESTING SHOULD OCCUR 24 HOURS MINIMUM AFTER INSTALLATION OF THE SUBJECT ANCHORS.

FLC FLORES LUND CONSULTANTS
PROFESSIONAL ENGINEERS
7220 TRADE STREET, SUITE 200, SAN DIEGO, CALIFORNIA 92121
(619) 566-0626 FAX (619) 566-0627

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GROTH ARCHITECTS, INC.
823 ACACIA STREET
OCEANSIDE, CA 92054

JEFFERSON MS NEW CONSTRUCTION

PROJECT NO. 758-000
O25
P. T. N. 73569-9
DATE
REVISIONS

3355 MISSION AVE. SUITE 234
OCEANSIDE, CALIFORNIA 92054

GROTH ARCHITECTS, INC. OCEANSIDE UNIFIED S.D.

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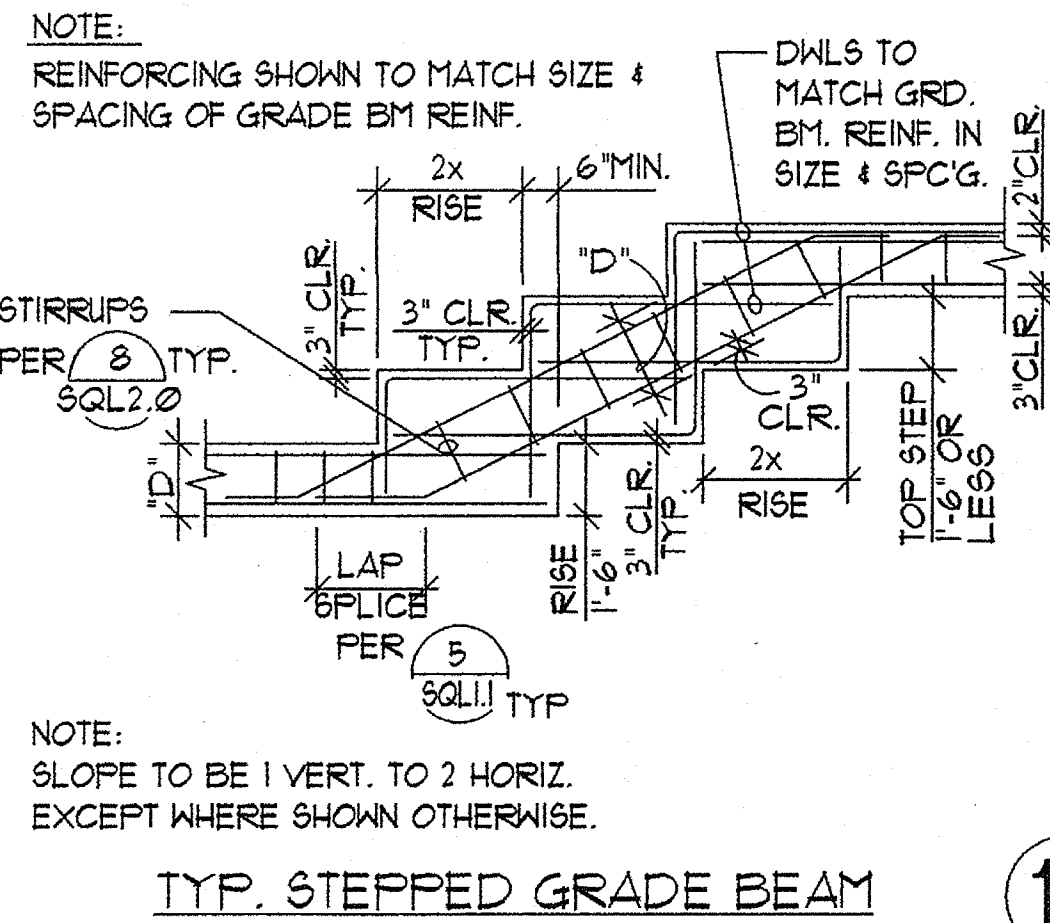
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JOAN SCOTT BROT
C-26609
4/30/2007
STATE OF CALIFORNIA

SHEET TITLE

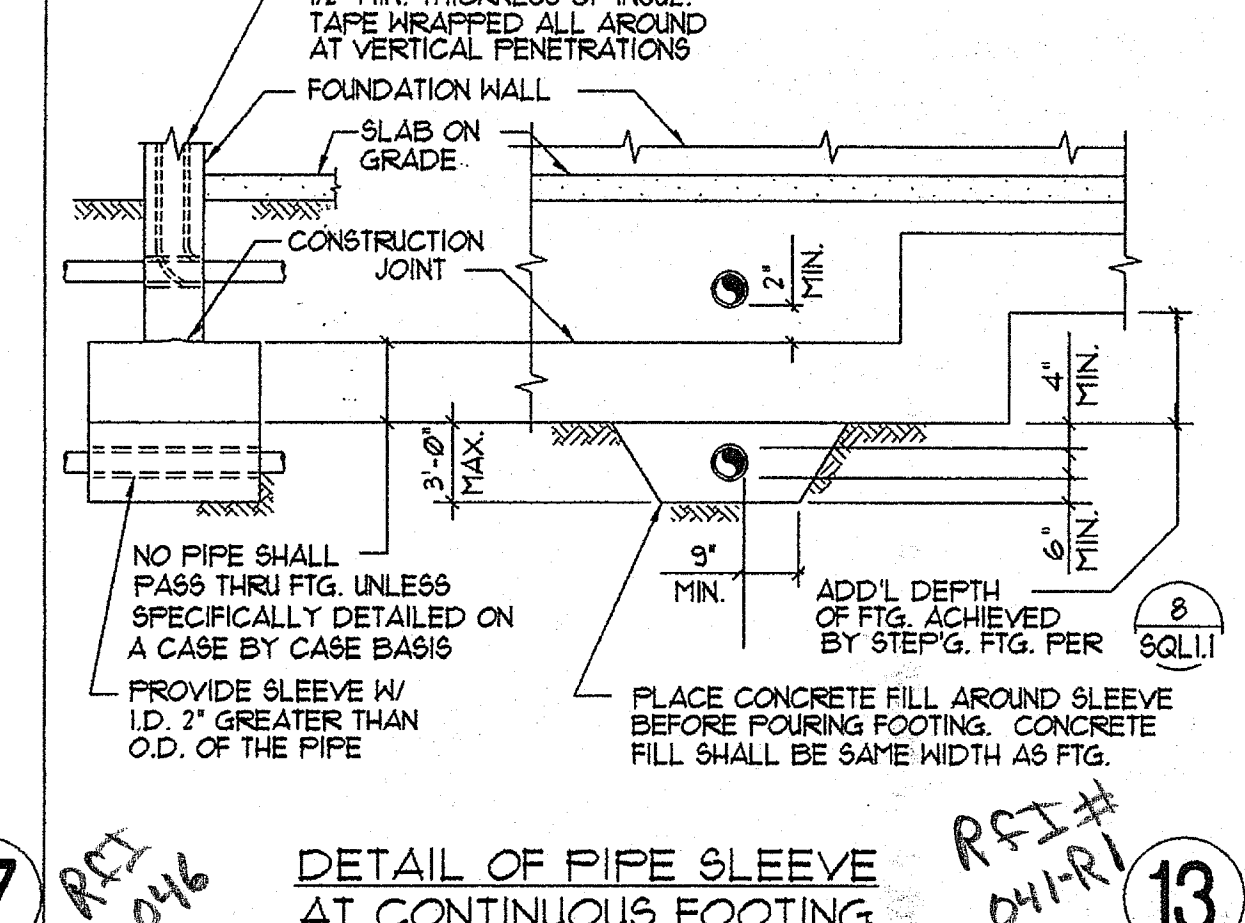
GENERAL NOTES

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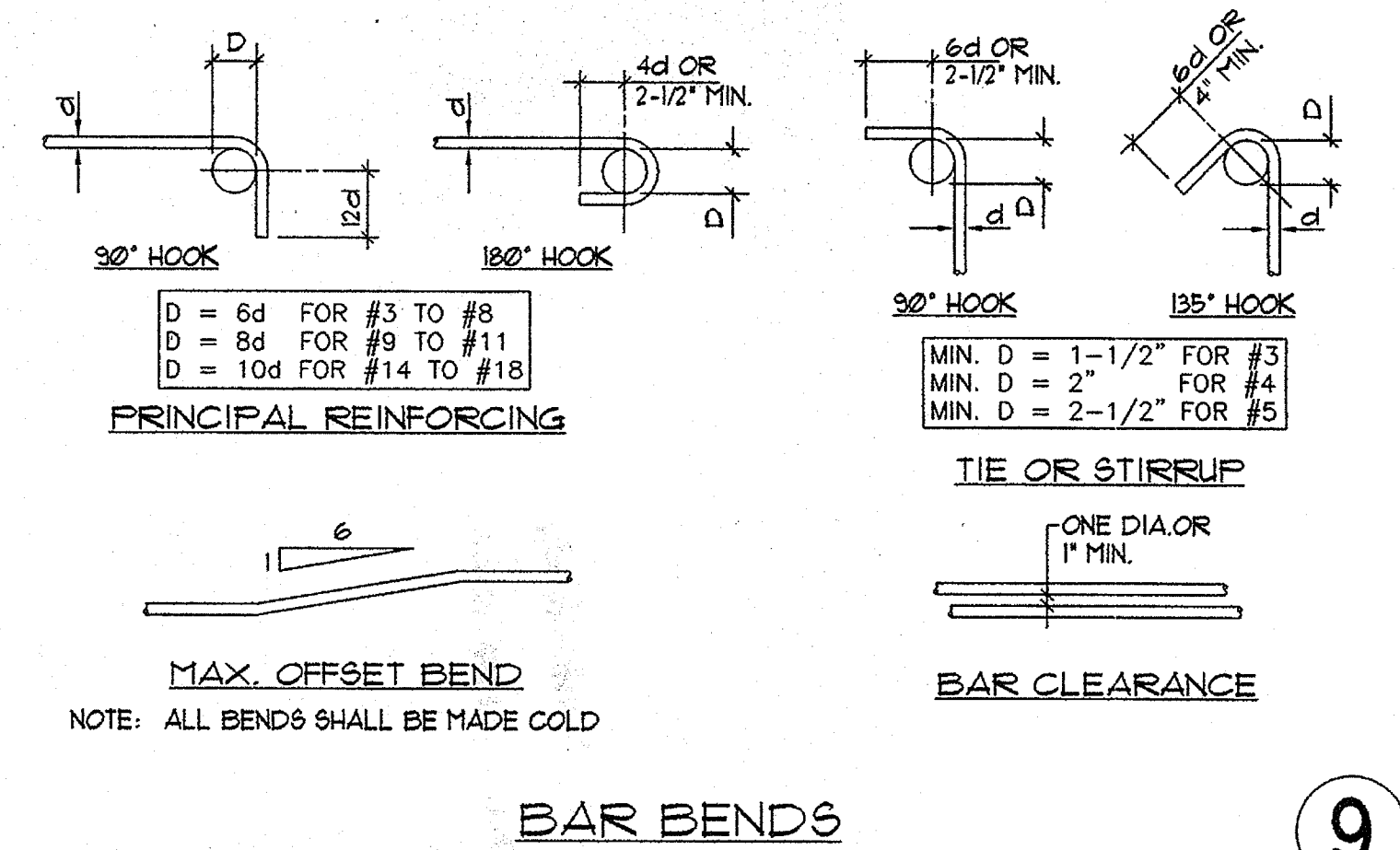
PHONE 760-754-8191
FAX 760-754-8391



TYP. STEPPED GRADE BEAM



DETAIL OF PIPE SLEEVE AT CONTINUOUS FOOTING



BAR BENDS

TENSION LAP SPLICES FOR GRADE 60 BARS $f_c = 3000$

BAR SIZE	BOTTOM BARS	TOP BARS
#3	16	17
#4	17	23
#5	26	34
#6	35	46
#7	57	74
#8	72	94
#9	87	113
#10	106	138
#11	125	163

(1) LAP SPLICES SHALL BE CLASS B SPLICES.
 (2) TOP BARS ARE HORIZONTAL BARS WITH MORE THAN 12" DEPTH OF CONCRETE CAST BELOW THE REINFORCEMENT.
 (3) BARS SHALL BE SPACED A MINIMUM OF 3 BAR DIAMETERS ON CENTER IN ORDER TO USE THE TABULATED SPLICE LENGTHS.

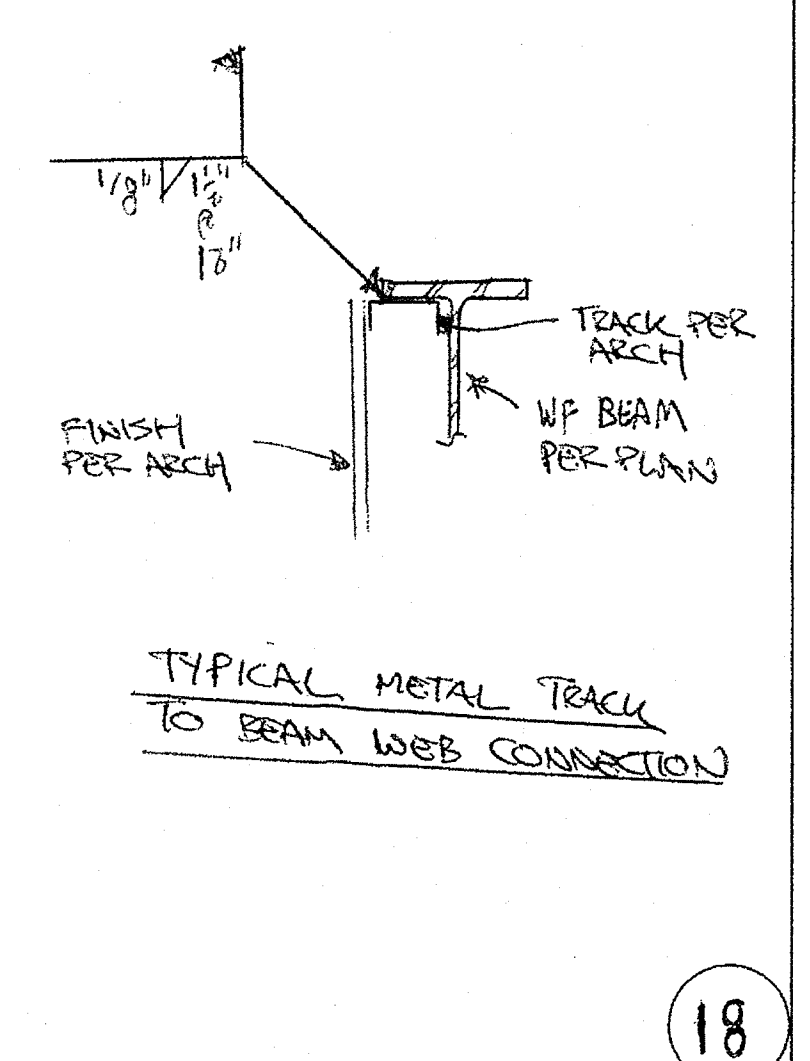
REINFORCING BAR SPLICES

MINIMUM STRAIGHT TENSION DEVELOPMENT FOR GRADE 60 BARS $f_c = 3000$

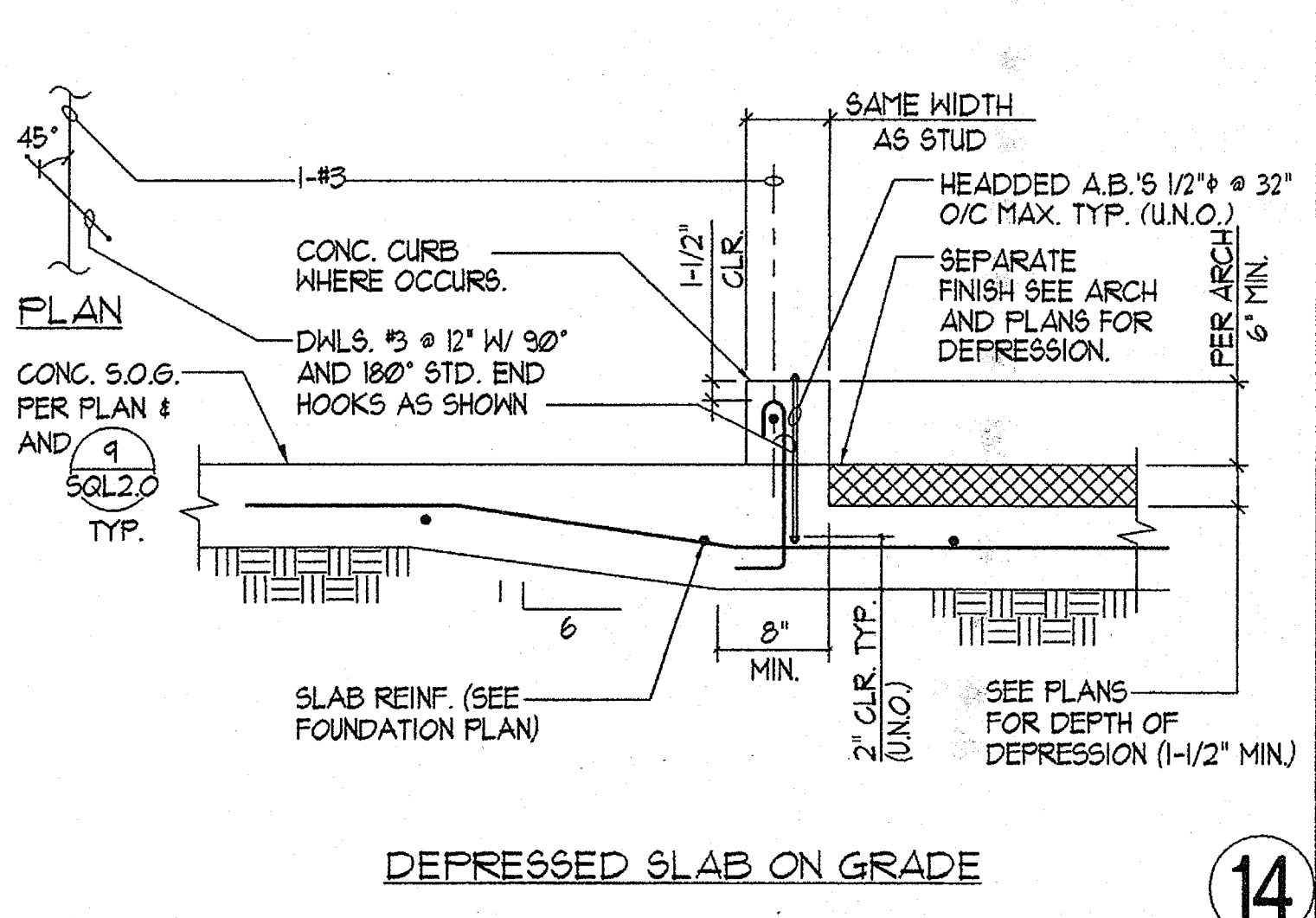
BAR SIZE	BOTTOM BARS	TOP BARS
#3	12	13
#4	13	17
#5	20	26
#6	27	35
#7	44	57
#8	55	72
#9	67	87
#10	81	106
#11	96	125

(1) BARS SHALL BE SPACED A MINIMUM OF 4 BAR DIAMETERS ON CENTER TO USE THE TABULATED DEVELOPMENT LENGTHS.
 (2) FOR DEFINITION OF LAP BARS SEE 5

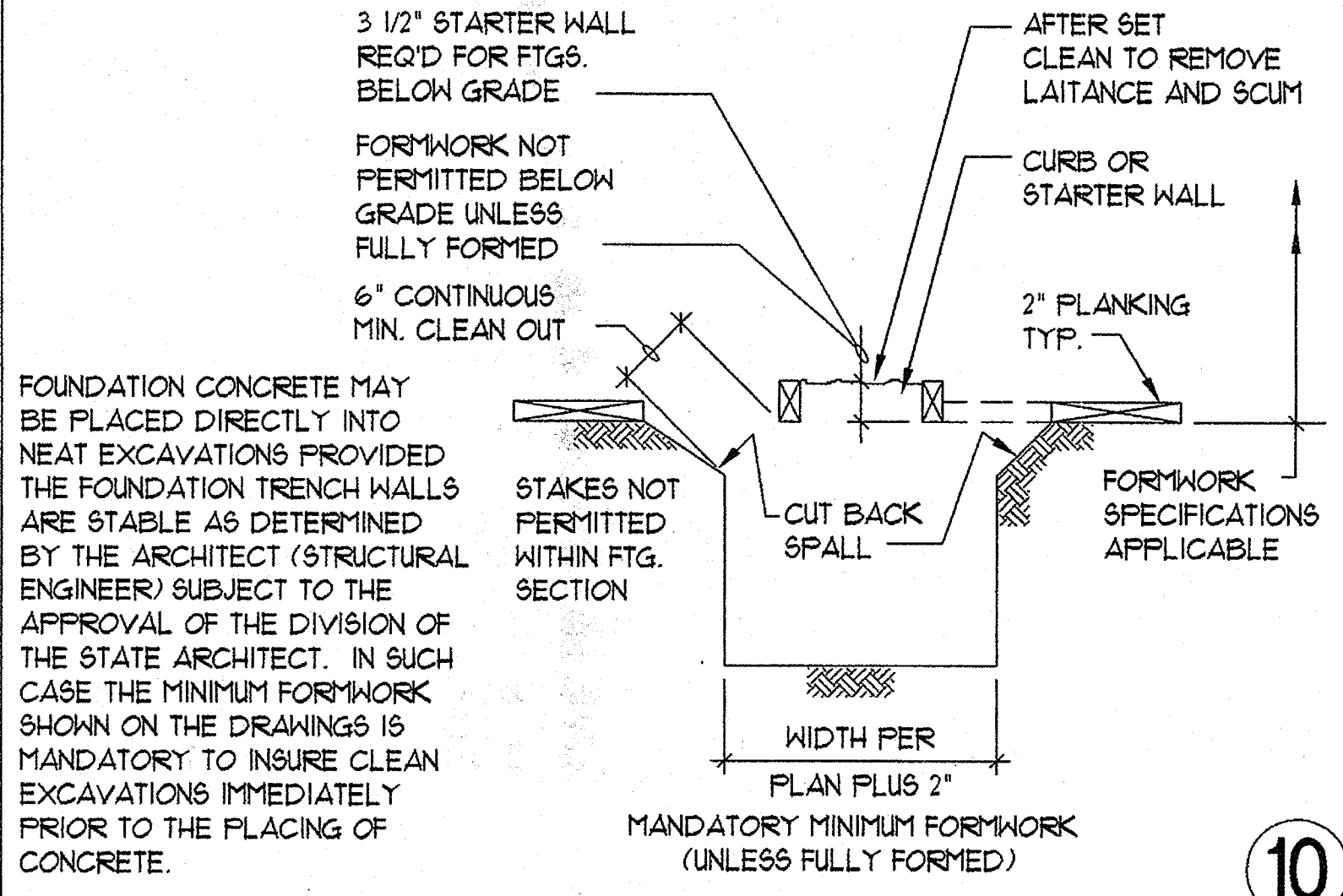
REINFORCING BAR EMBEDMENT



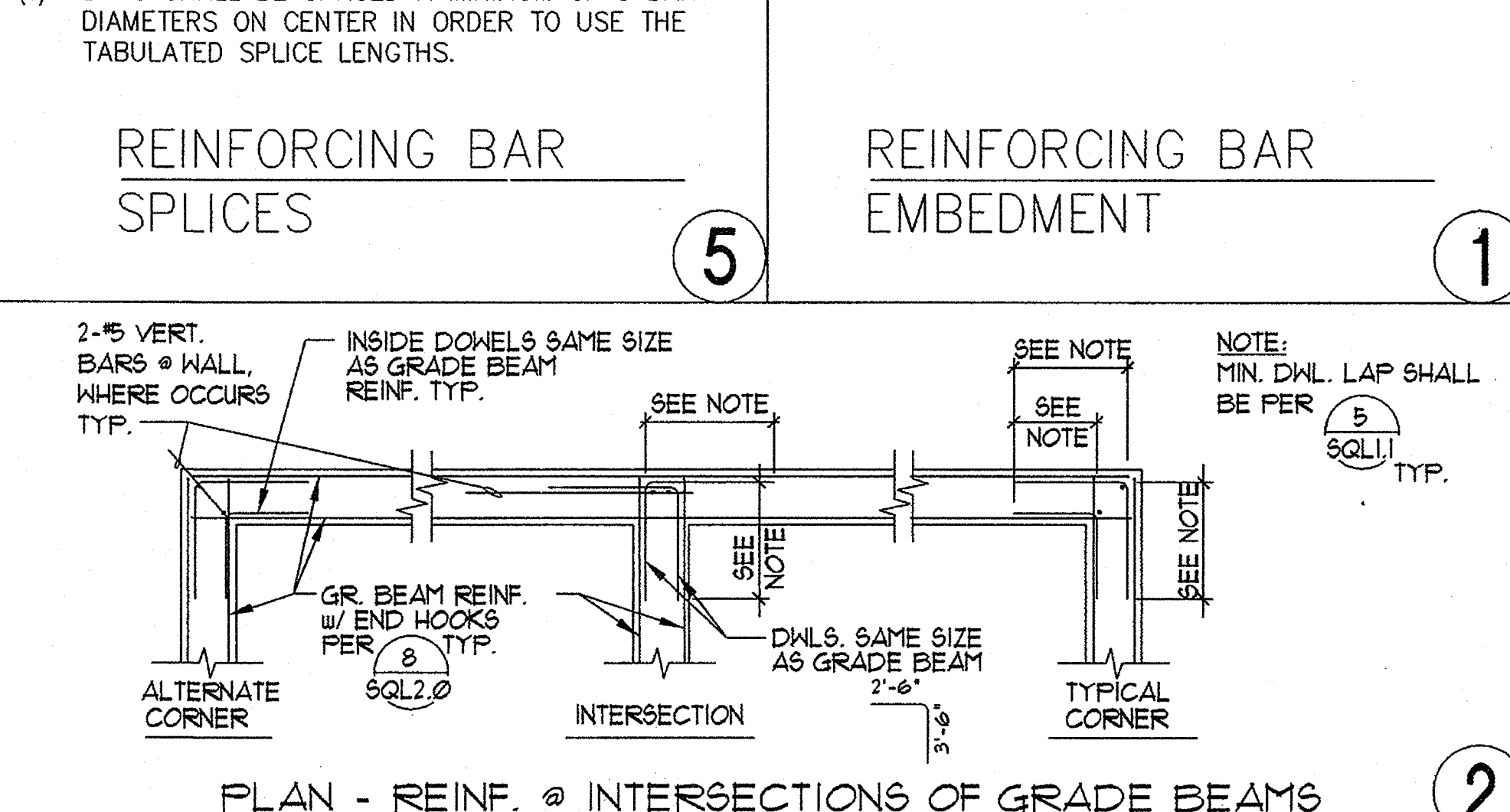
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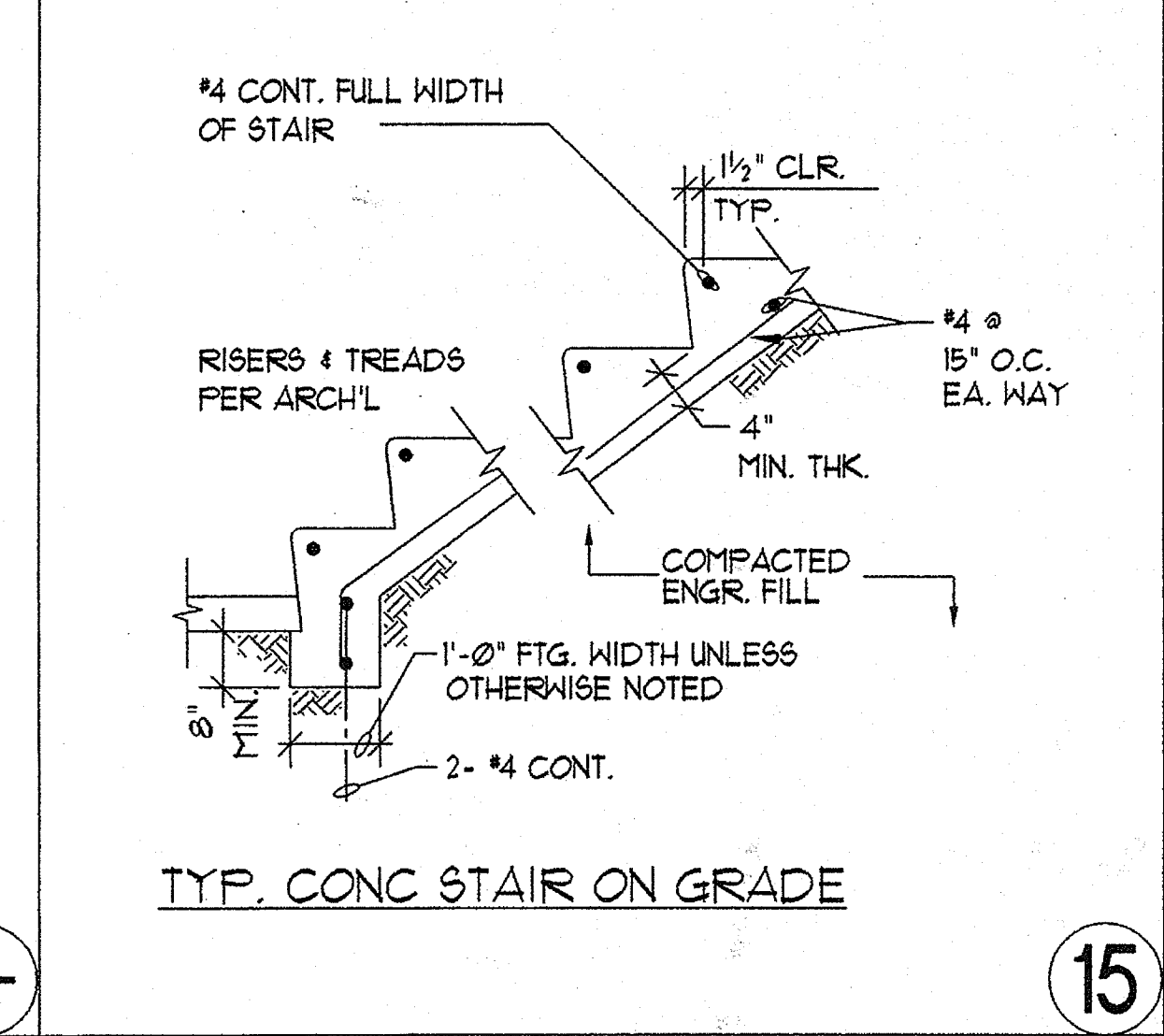
DEPRESSED SLAB ON GRADE



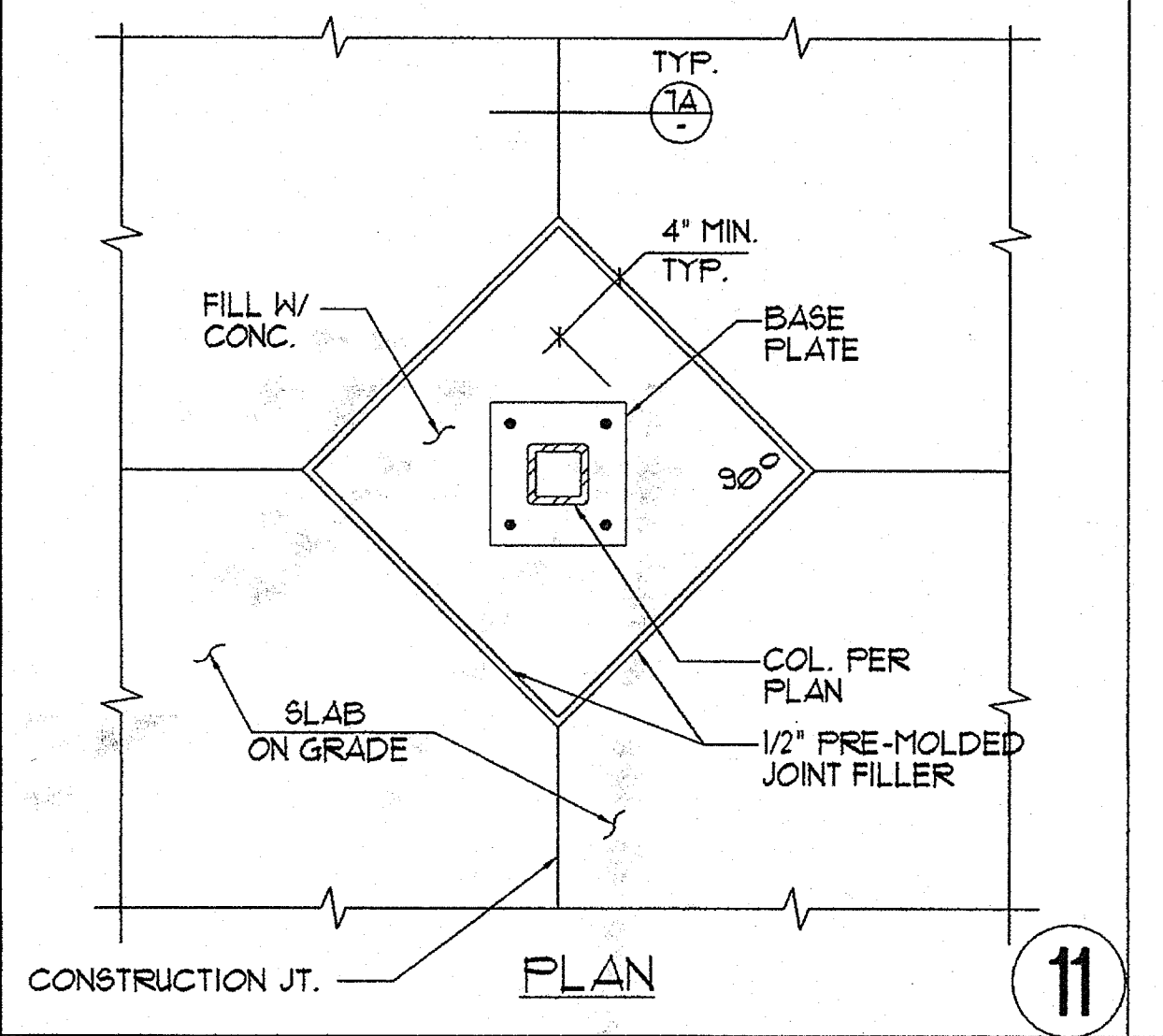
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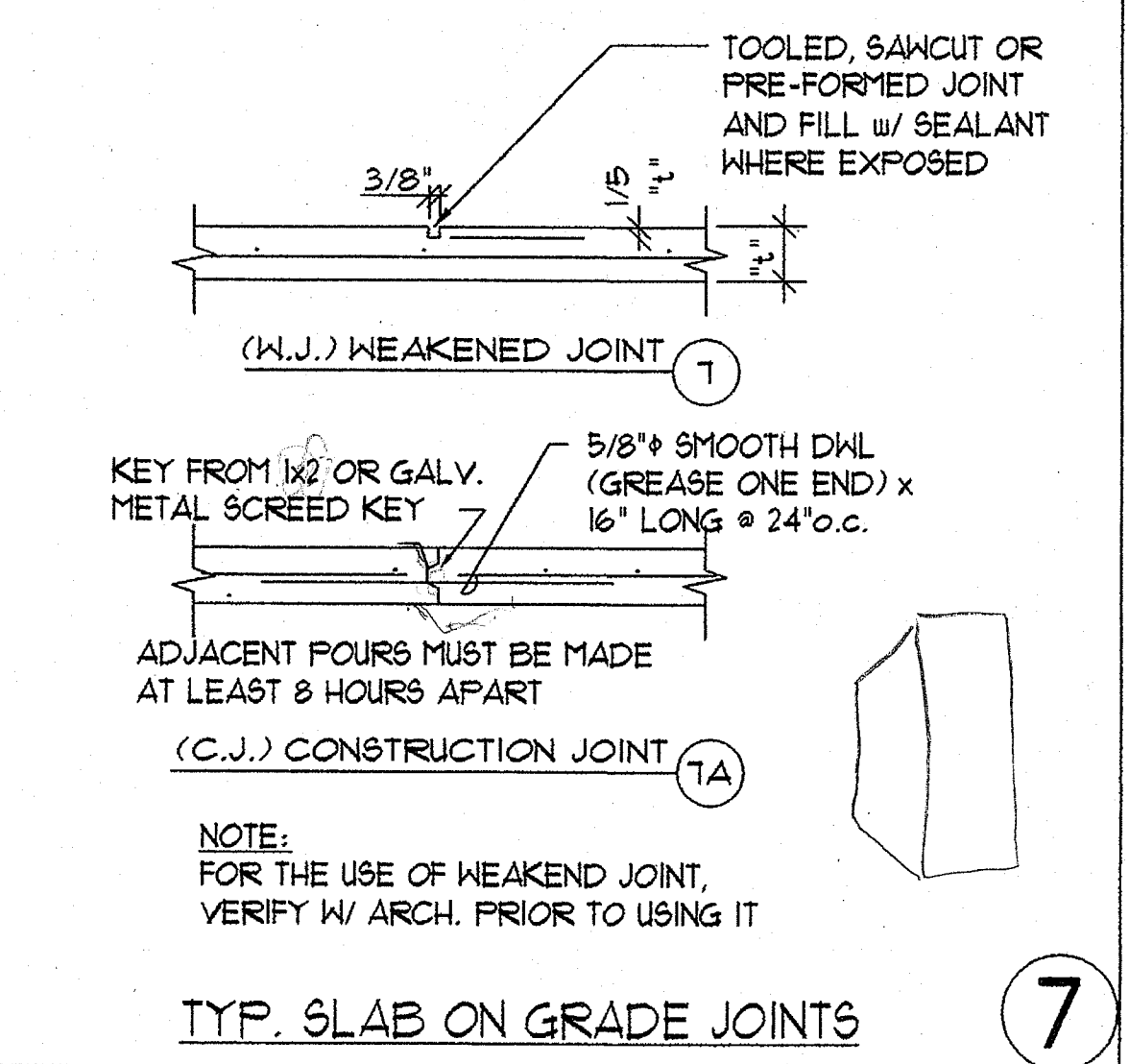
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15



11



3

SCHEDULE

BOLT SIZE	EMBED.	REMARKS
1/2"	4"	
5/8"	4"	
3/4"	5"	
1/2"	6"	
1"	7"	
1-1/8"	8"	
1-1/4"	9"	

BOLT EMBEDMENT AT CONCRETE WALL

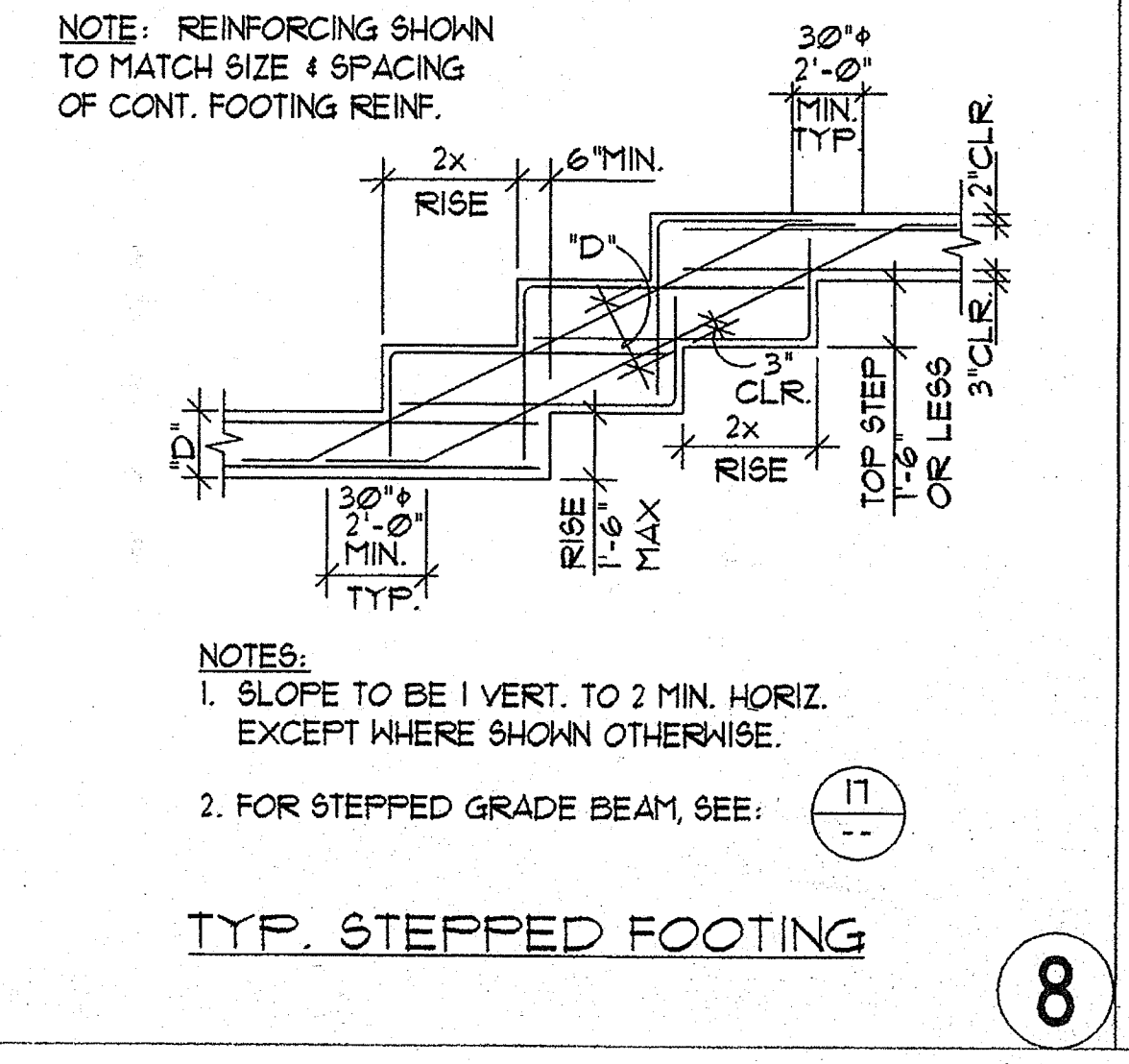
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SCHEDULE

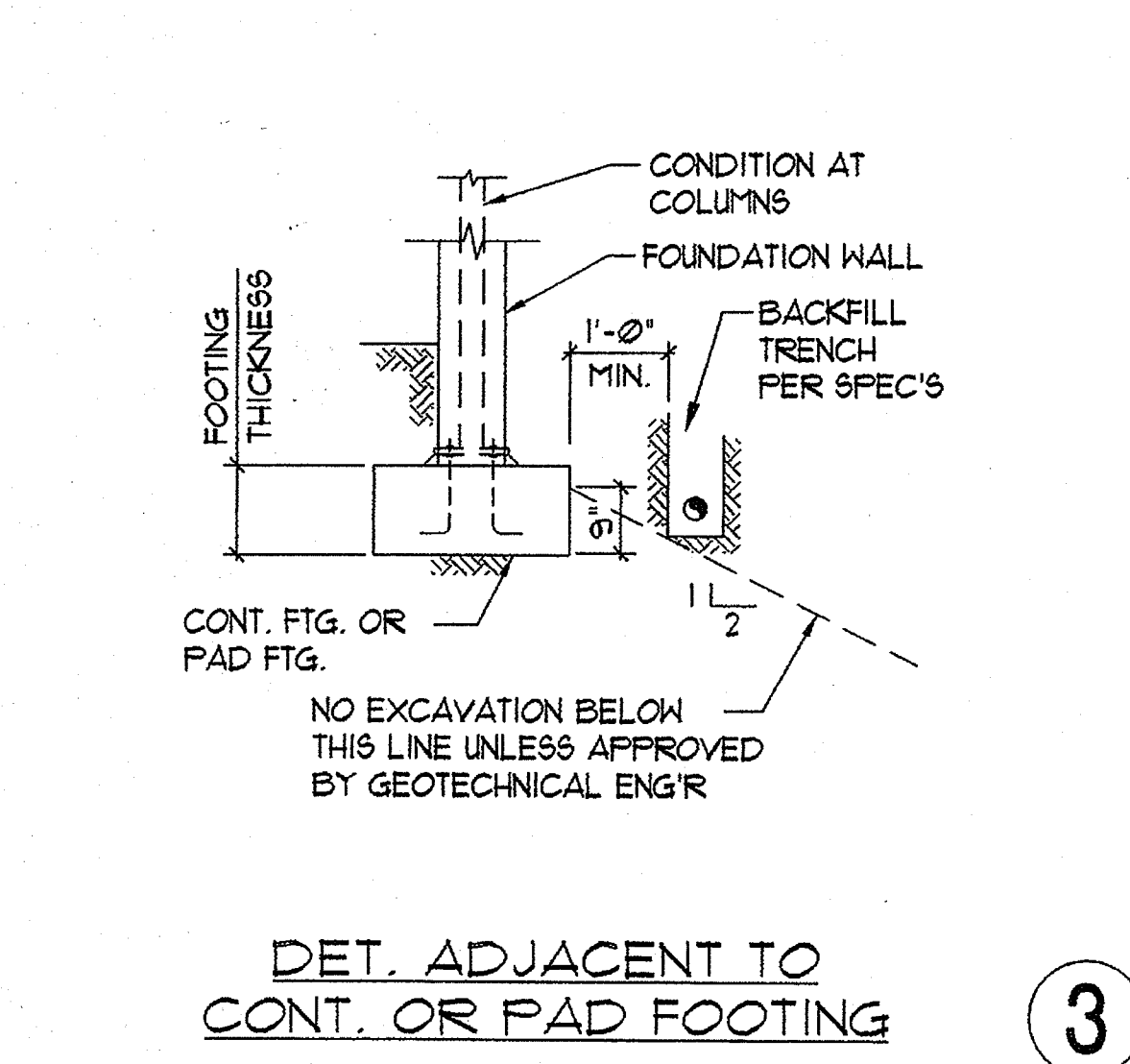
SIZE	EMBED.	REMARKS
1/2" DIA.	1"	
5/8" DIA.	9"	
3/4" DIA.	12"	
1" DIA.	15"	

FOUNDATION HEADED ANCHOR BOLTS

12



8



4

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JEFFERSON MS NEW CONSTRUCTION
 823 ACACIA STREET
 OCEANSIDE, CA 92054
 OCEANSIDE UNIFIED S.D.

GROTH ARCHITECTS, INC. 3355 MISSION AVE. SUITE 234 OCEANSIDE, CALIFORNIA 92054
 PHONE 760-754-8191 FAX 760-754-8291

PROJECT NOS. 025
 P. T. N. 73569-9
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REVISIONS

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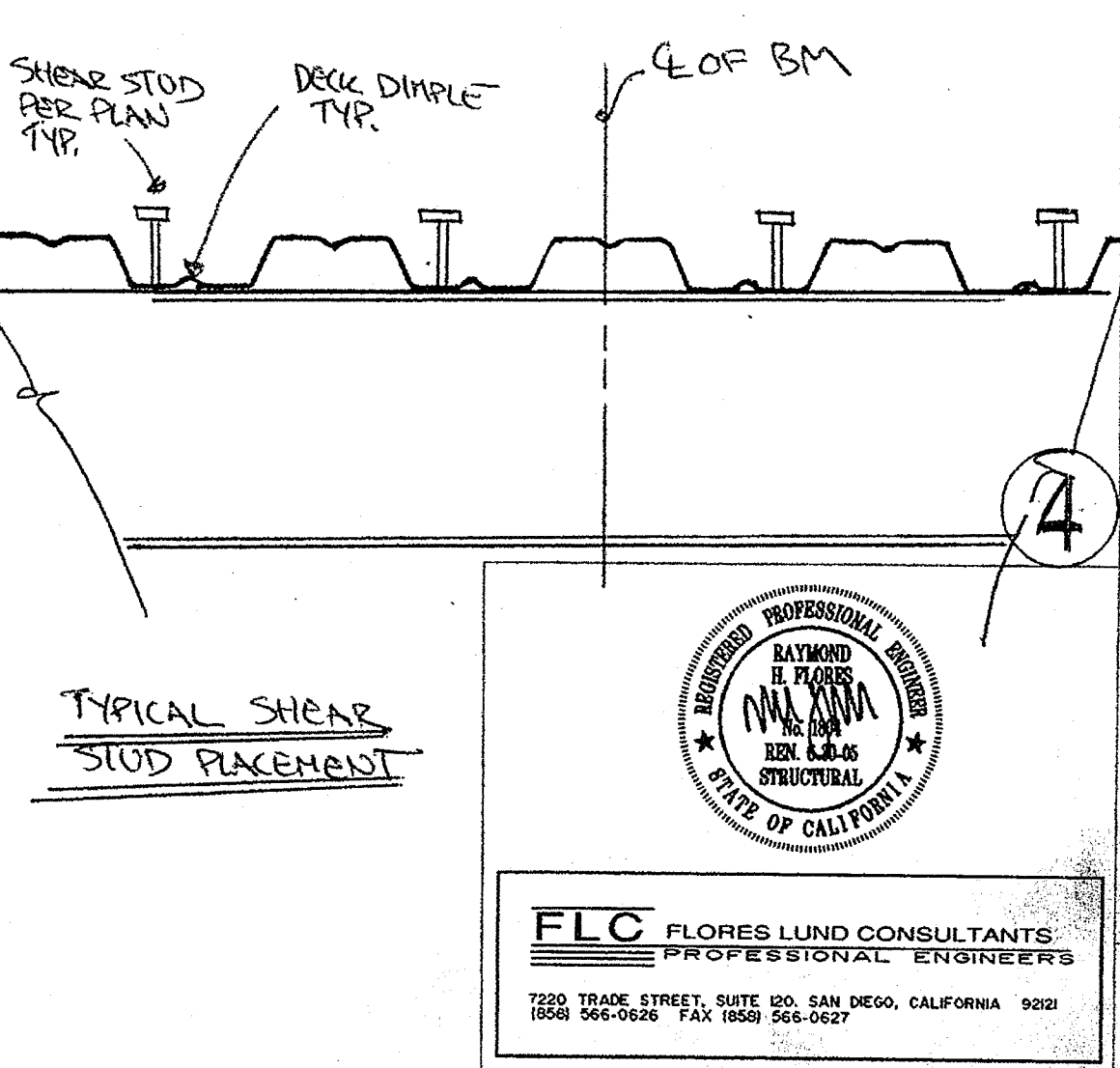
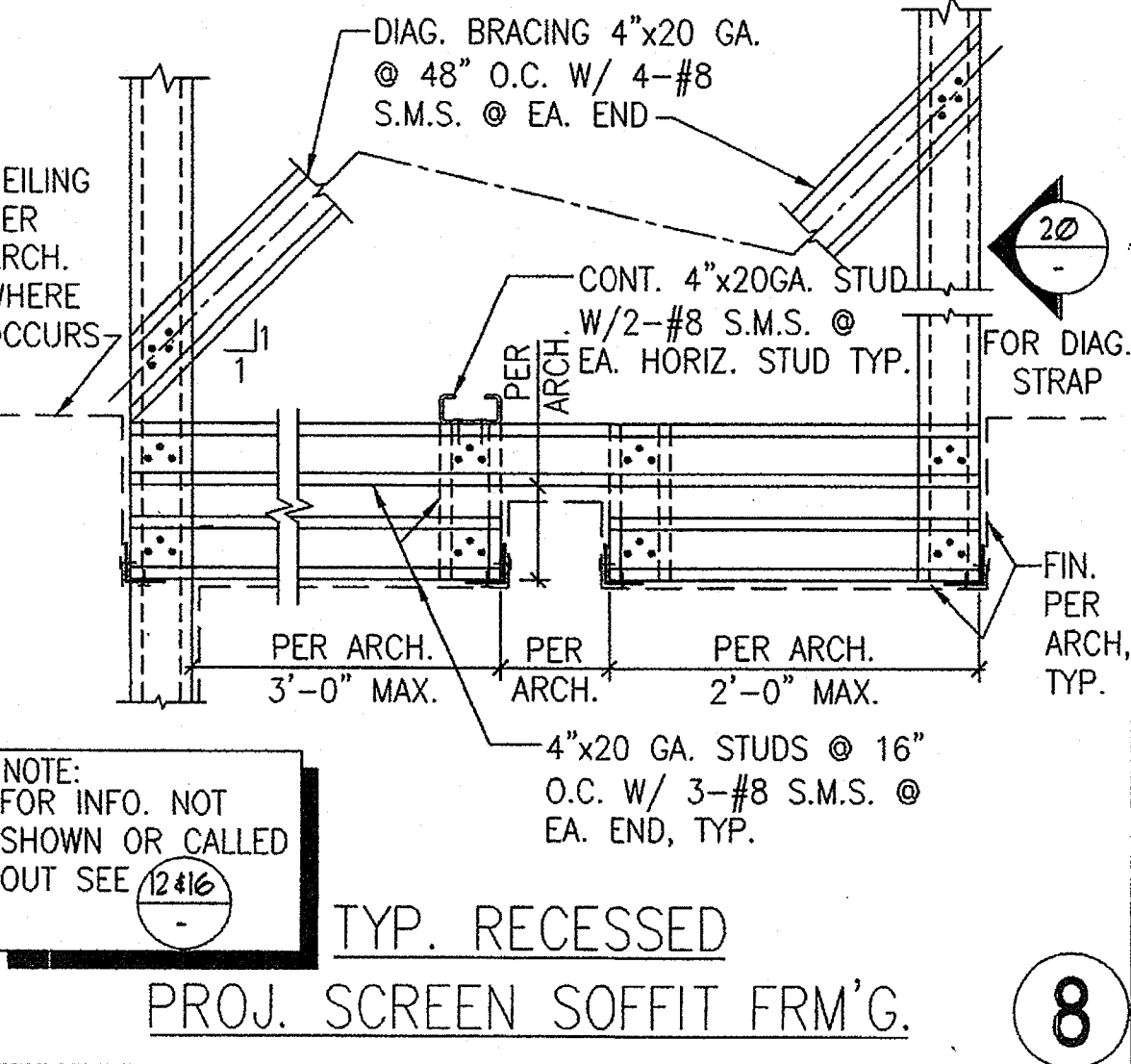
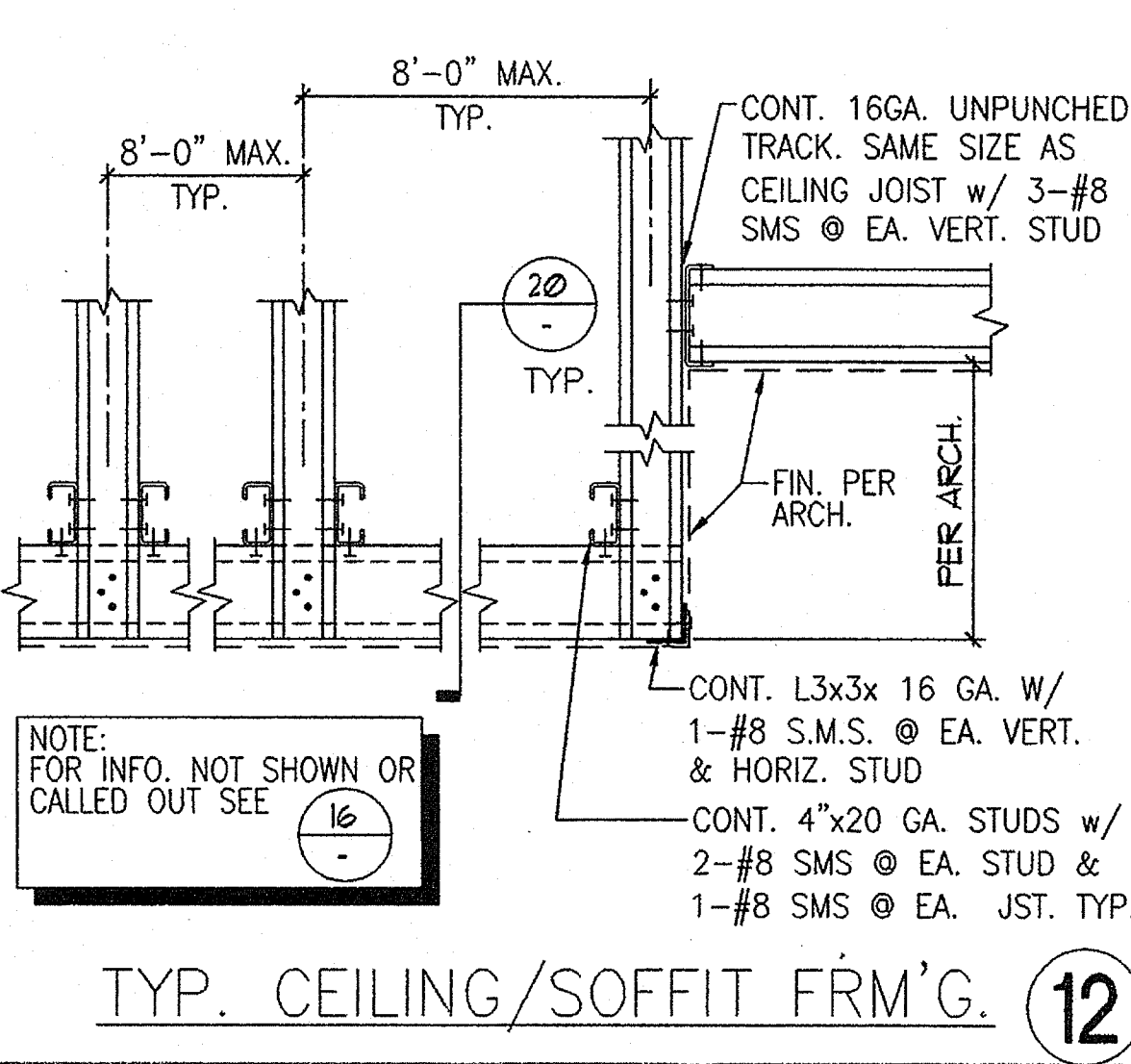
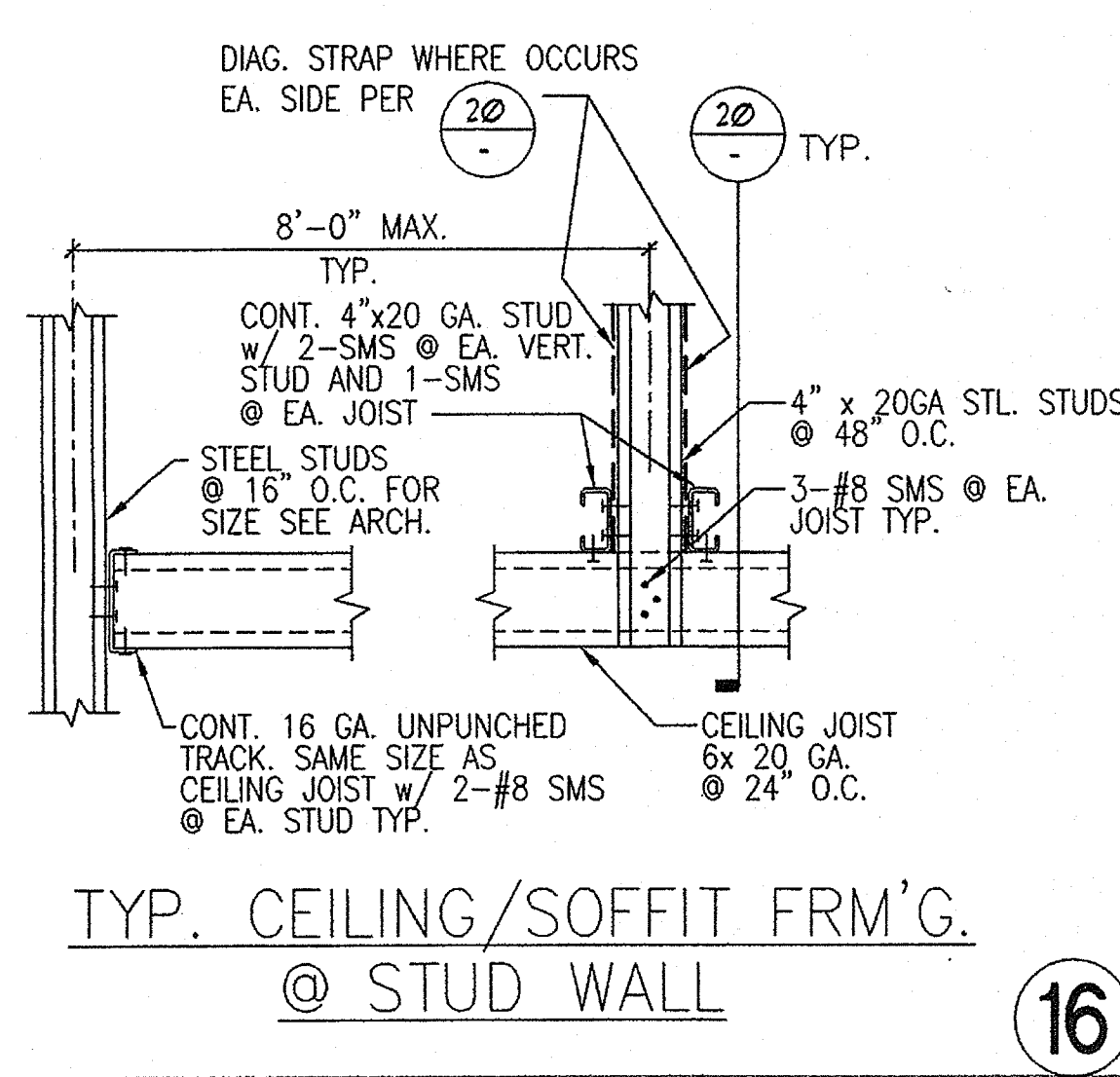
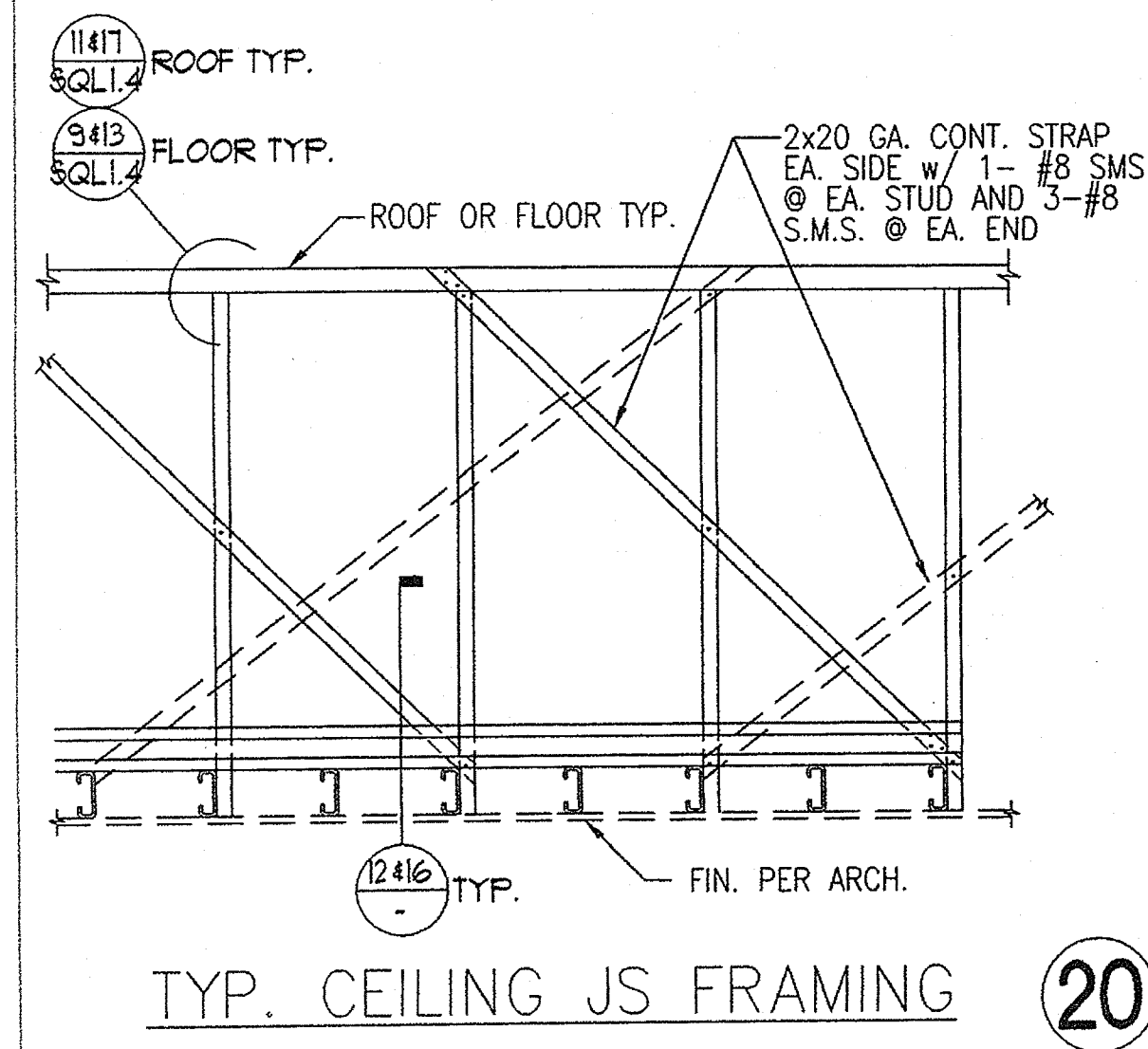
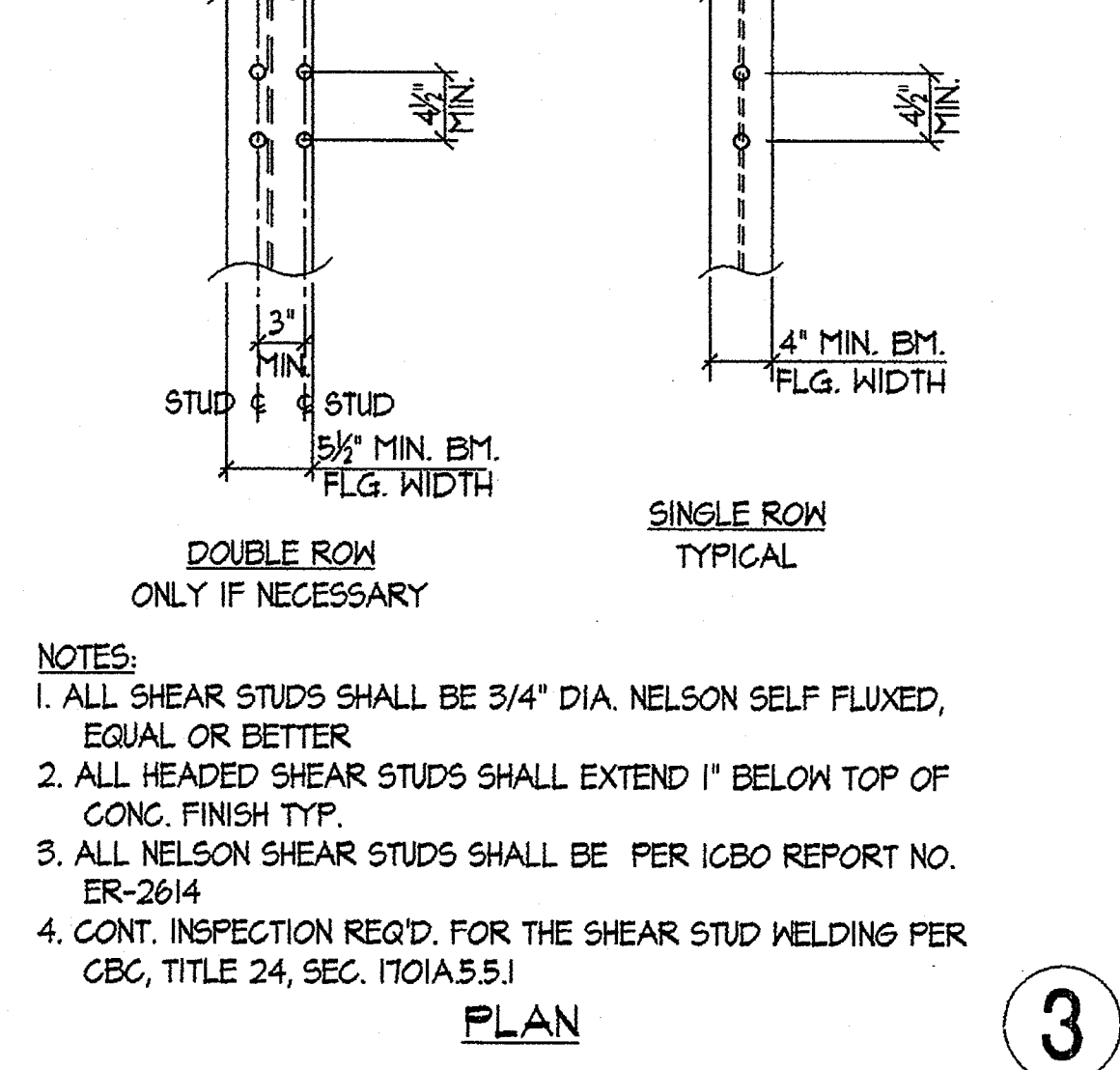
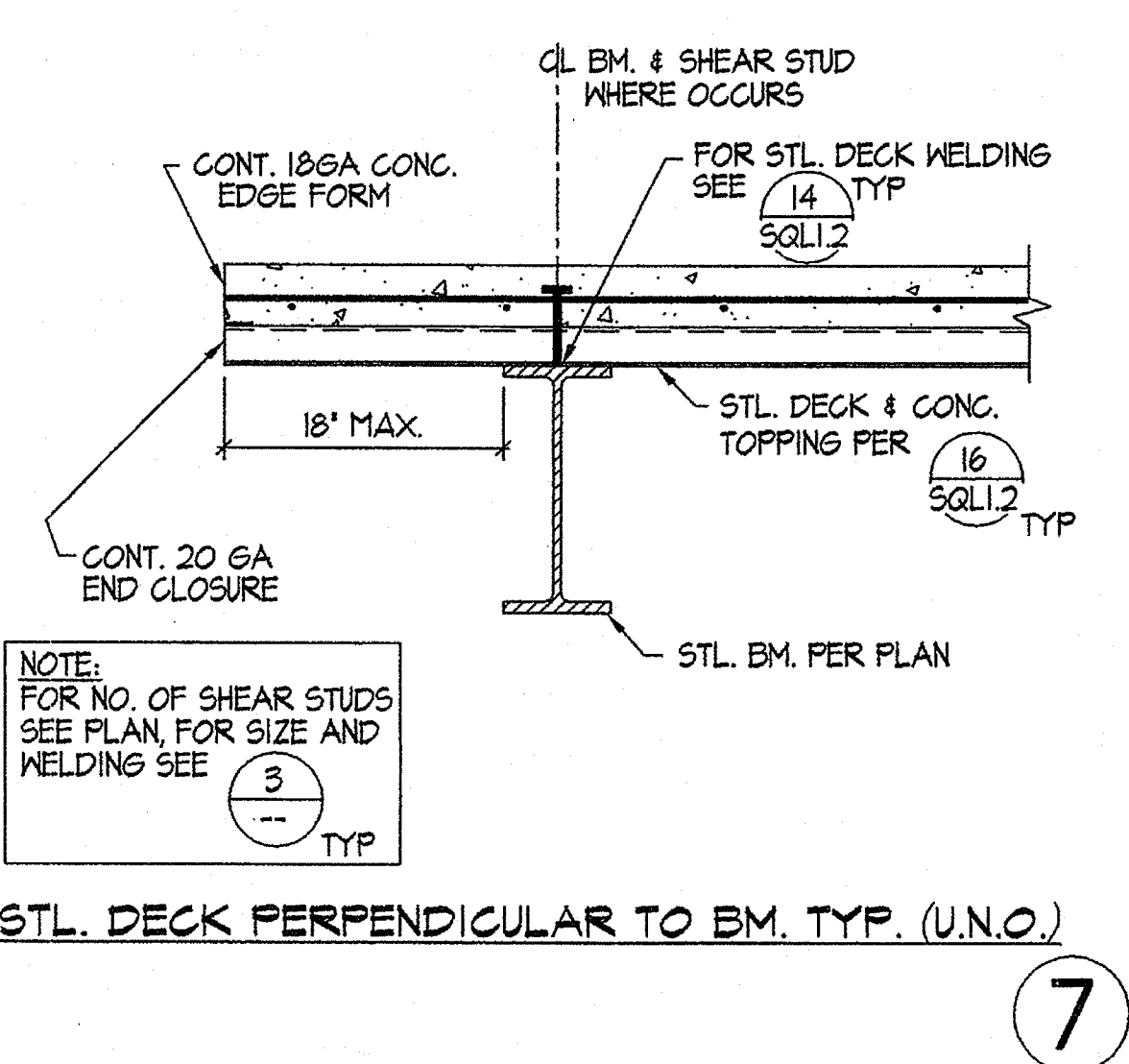
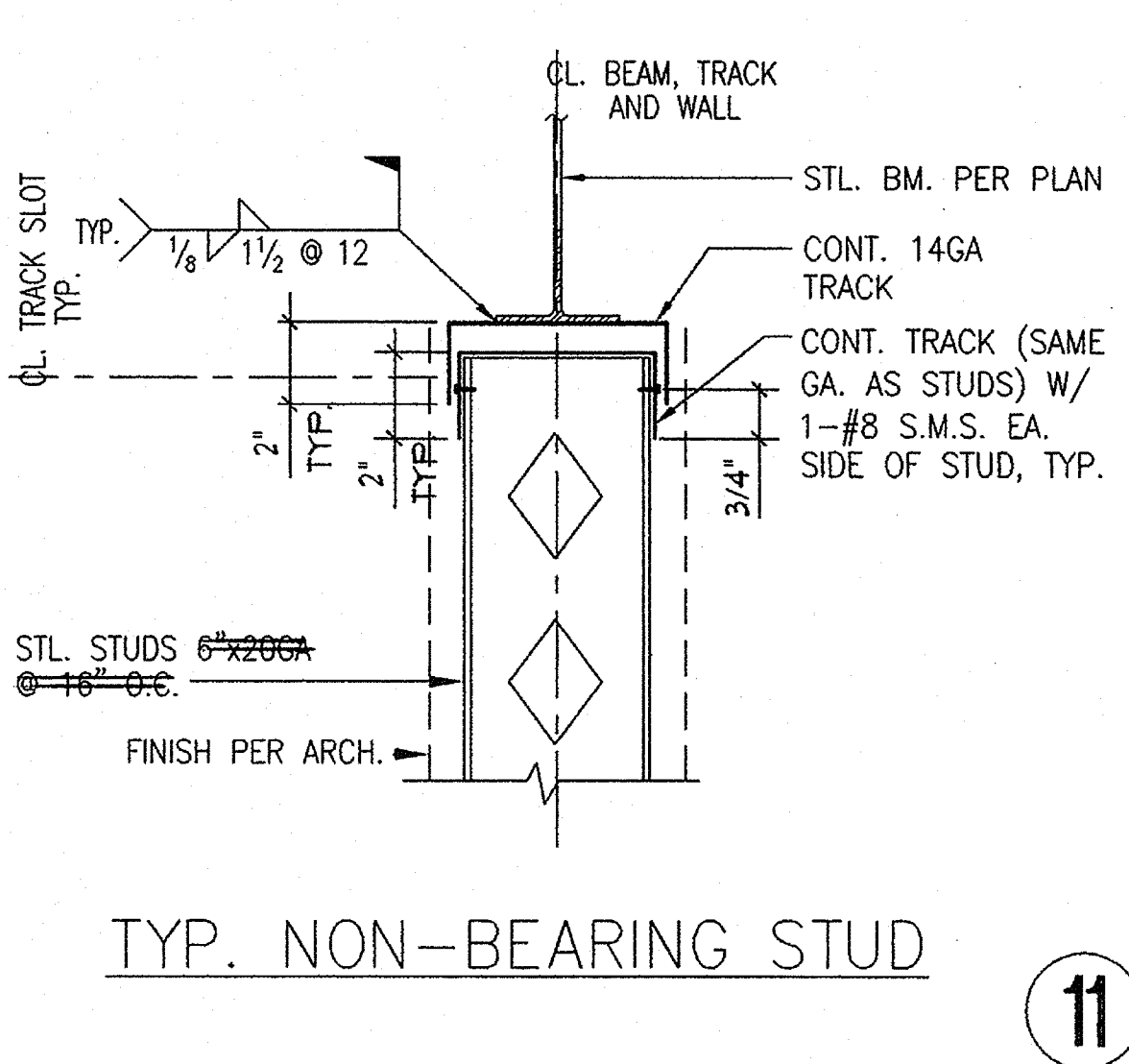
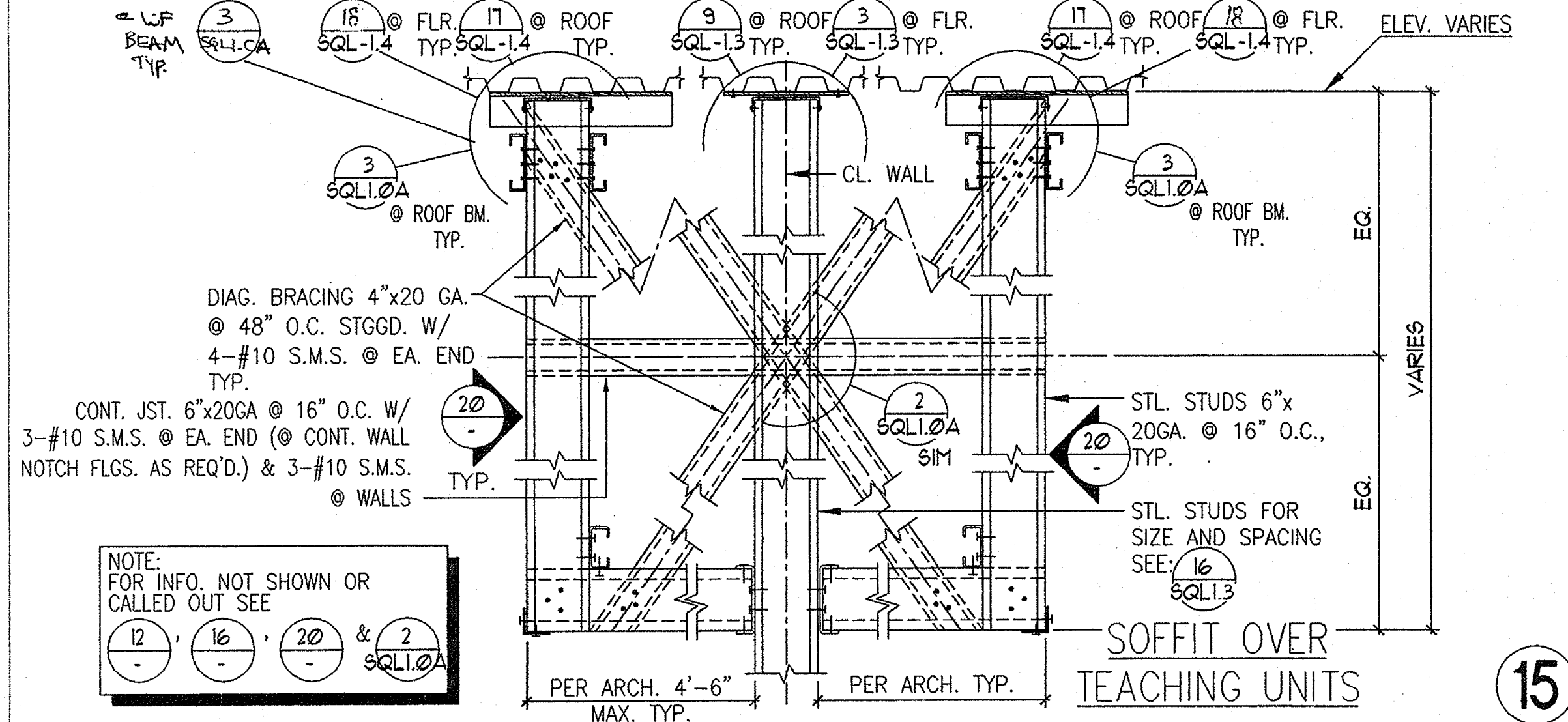
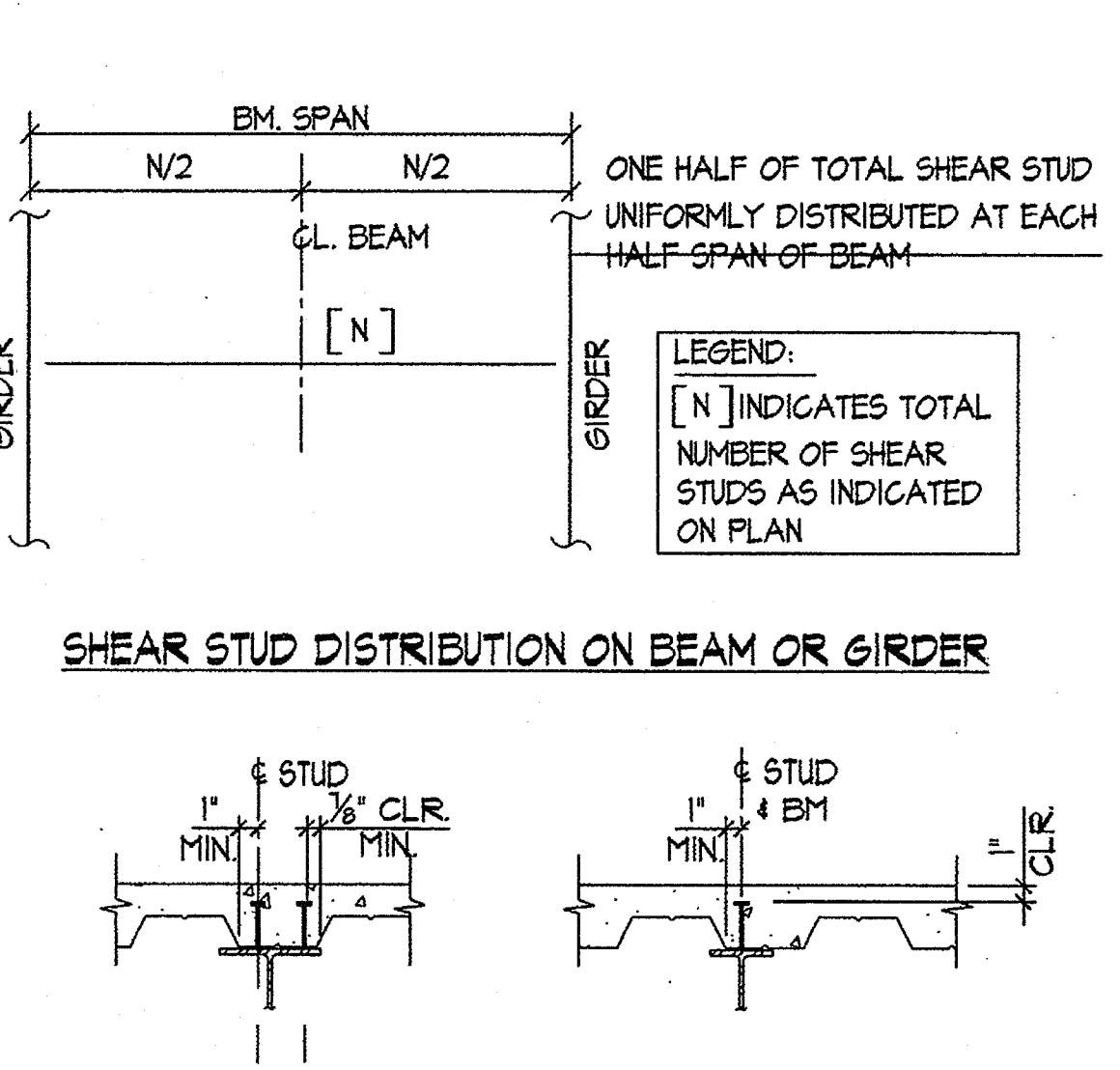
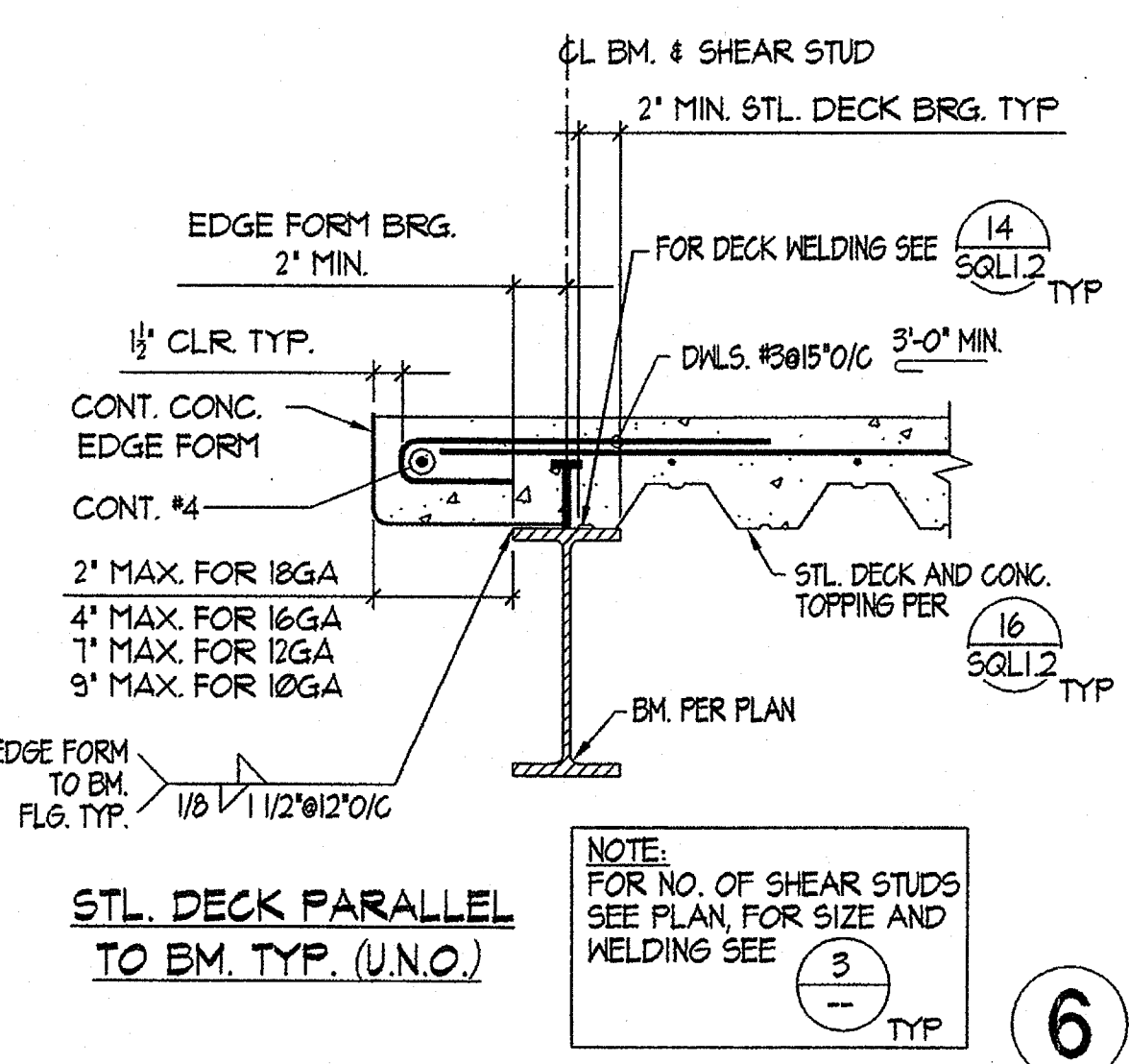
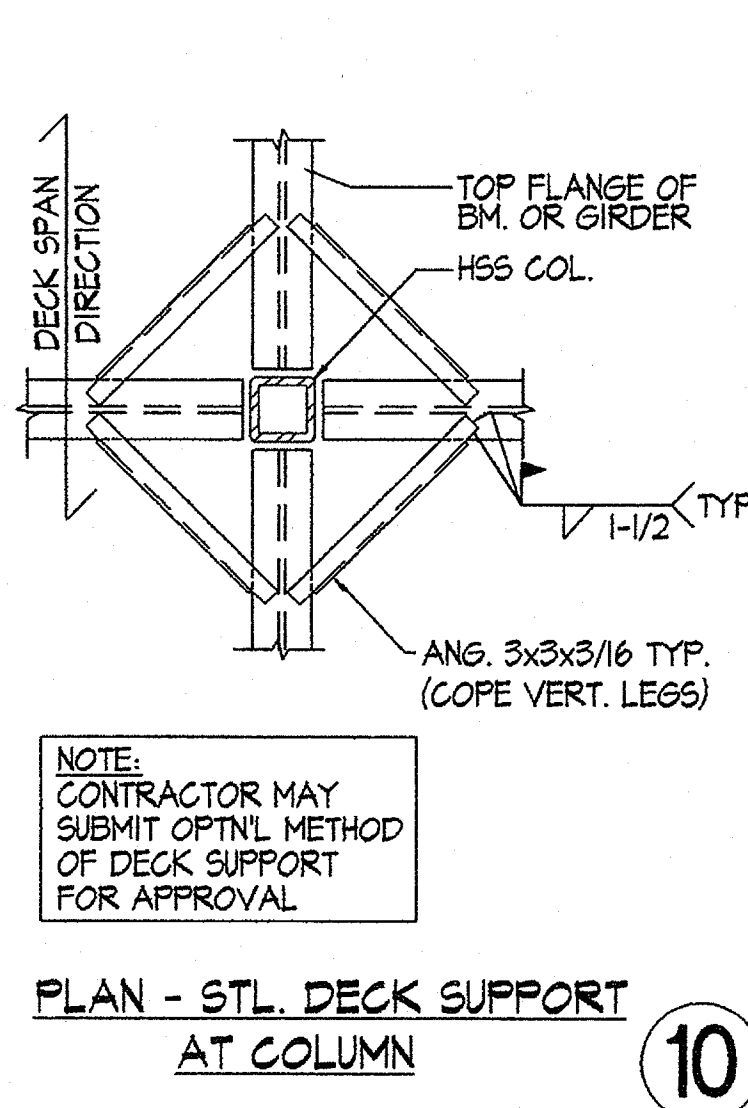
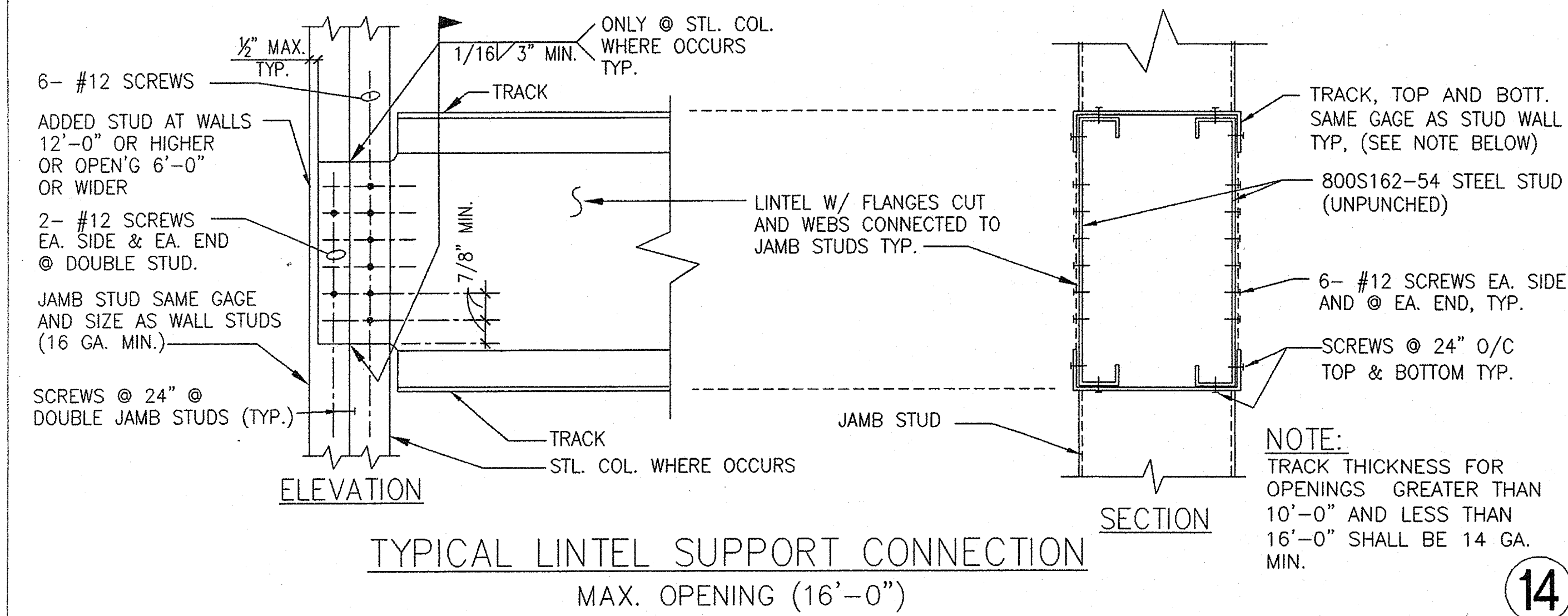
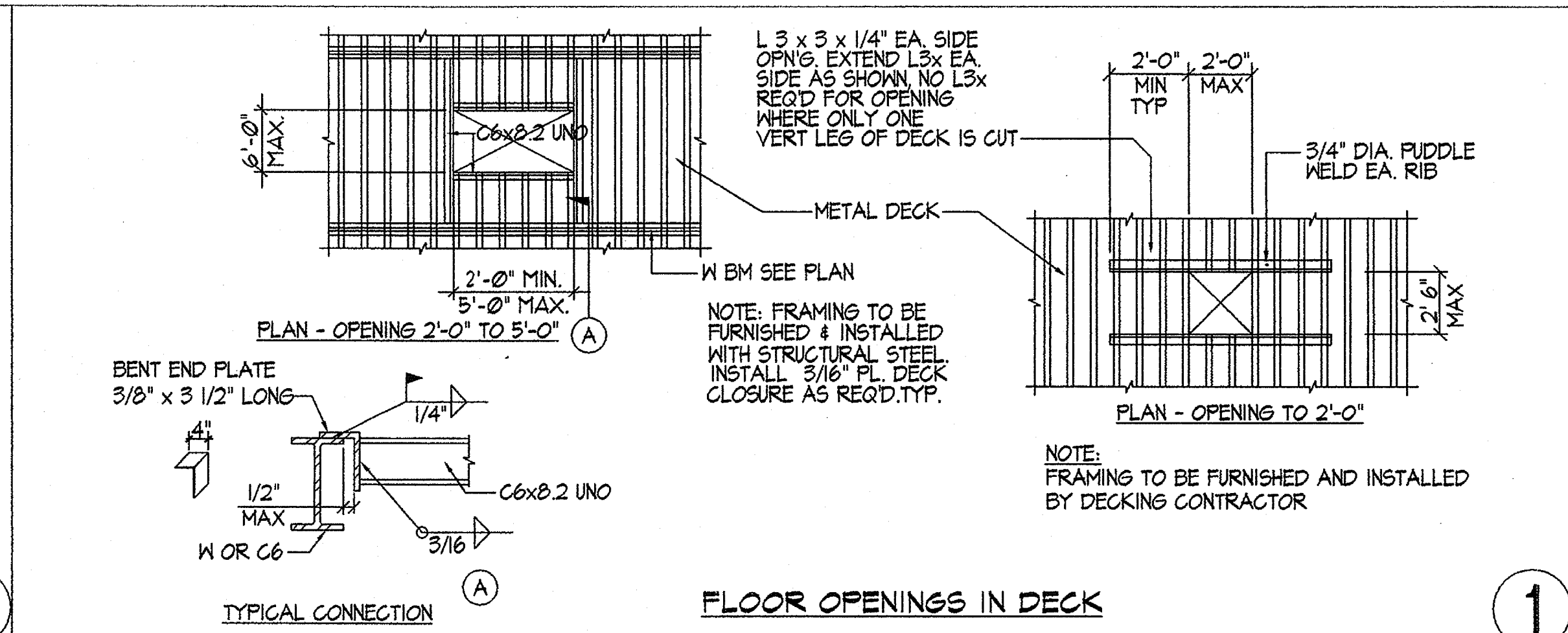
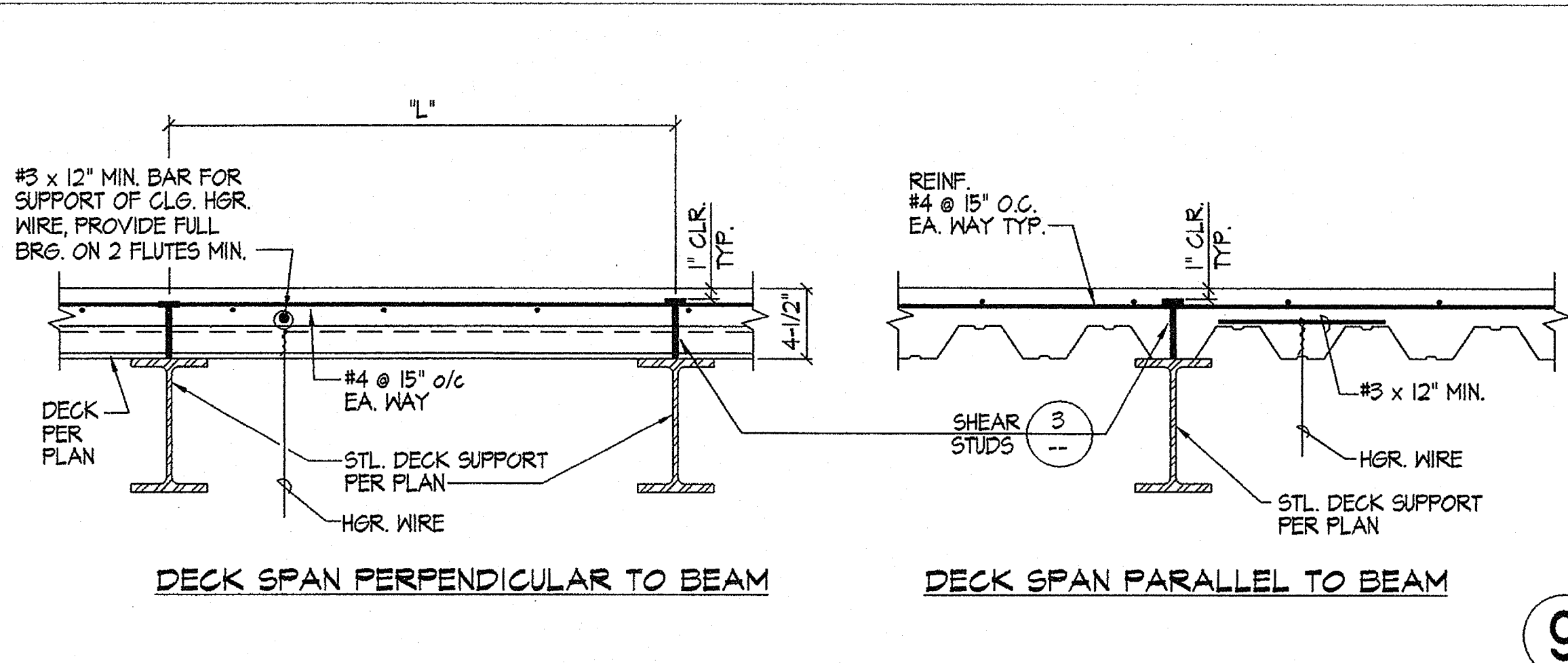
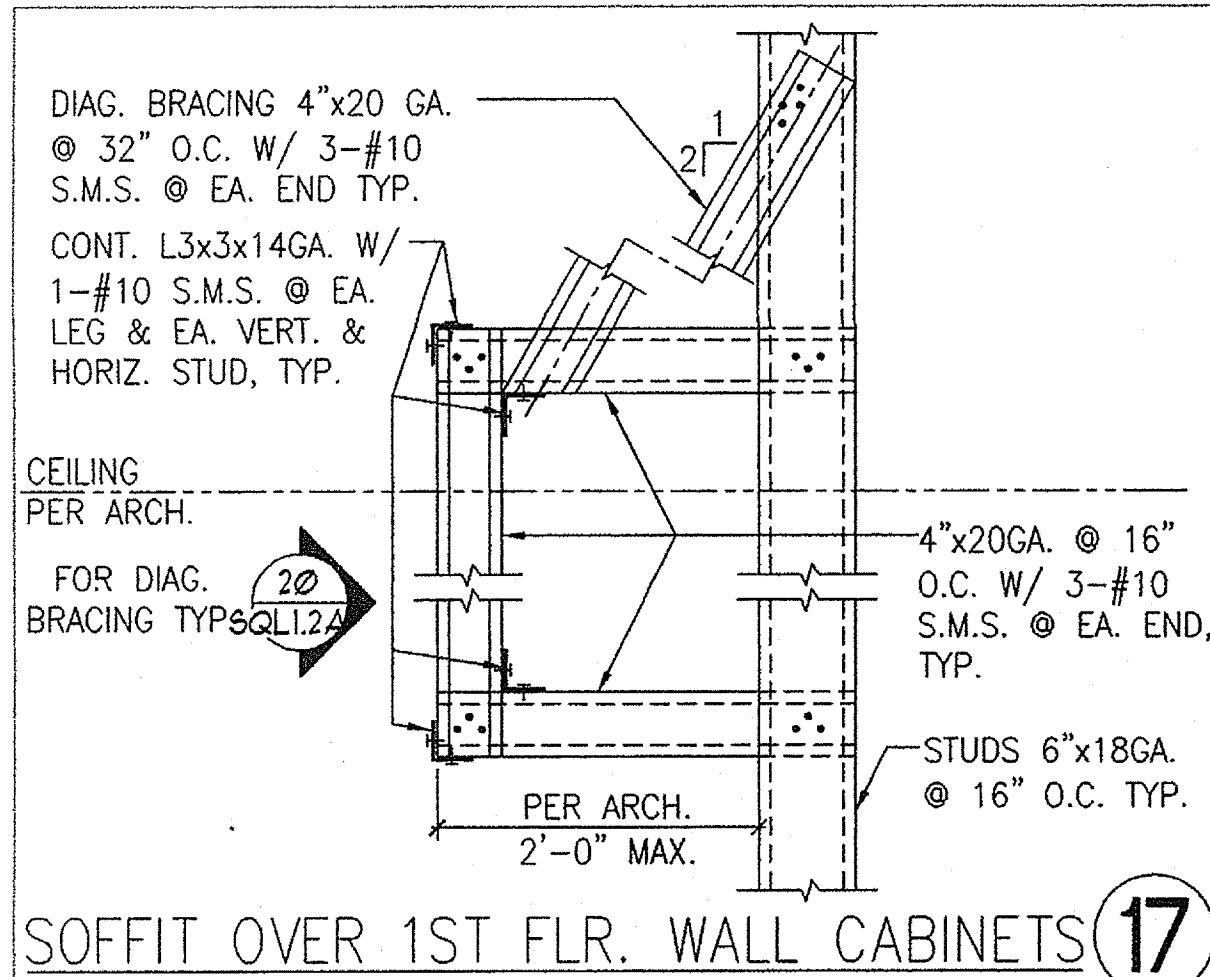
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 JOHN SCOTT GROTH
 C-26609
 4/30/2007
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SHEET TITLE

TYPICAL DETAILS

SQL1.1

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 PROFESSIONAL ENGINEERS
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PHONE 760-754-8191
FAX 760-754-8291

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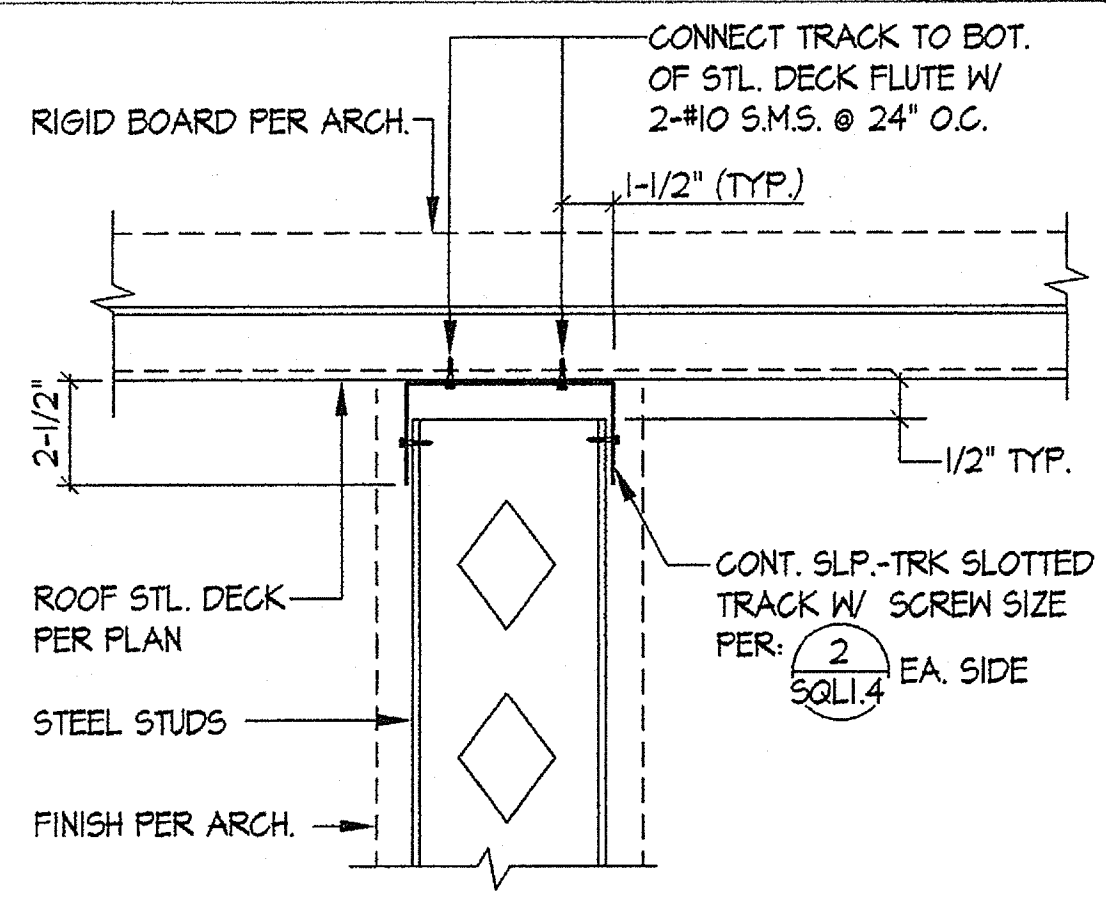
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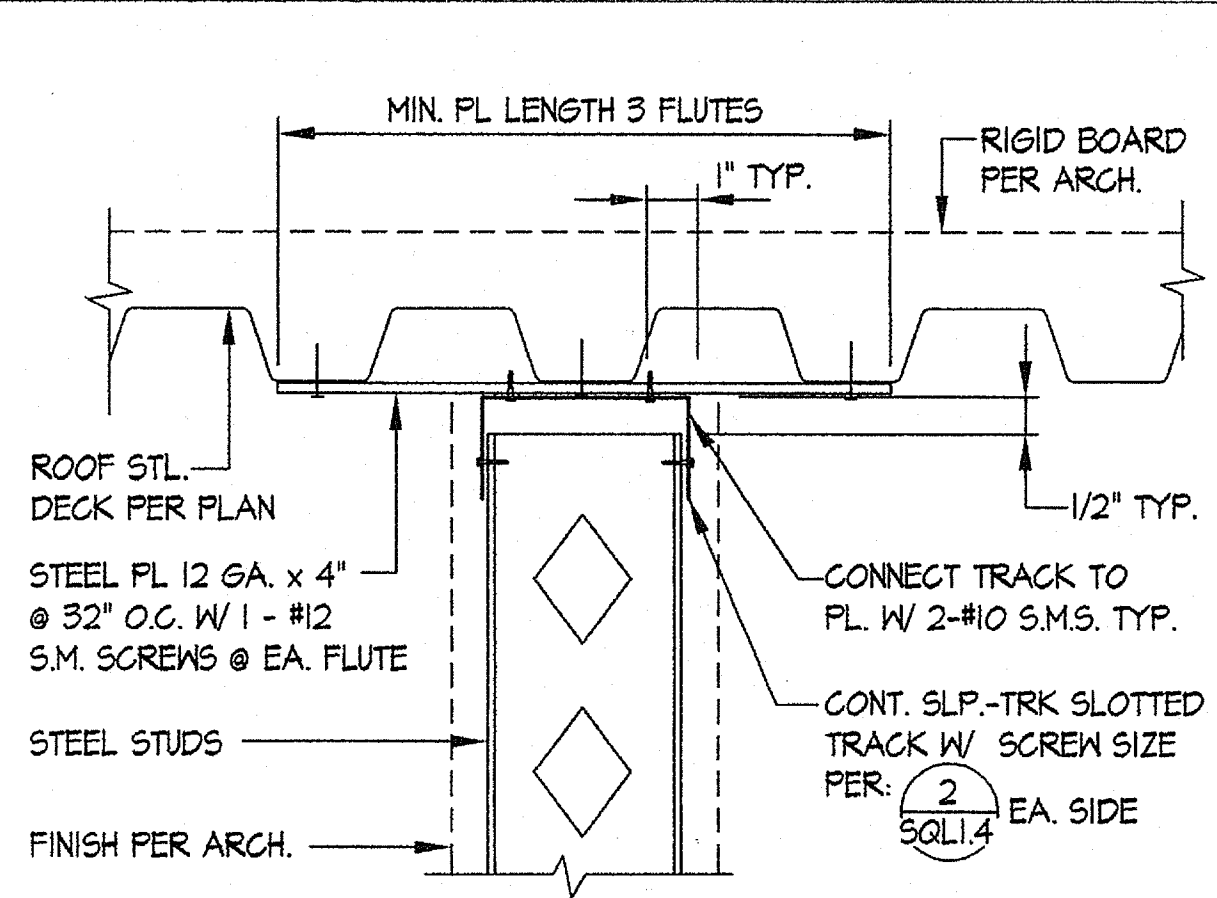
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FLC FLORES LUND CONSULTANTS
PROFESSIONAL ENGINEERS
7220 TRANK STREET, SUITE 100, SAN DIEGO, CALIFORNIA 92121
1856 565-0626 FAX 1856 565-0627

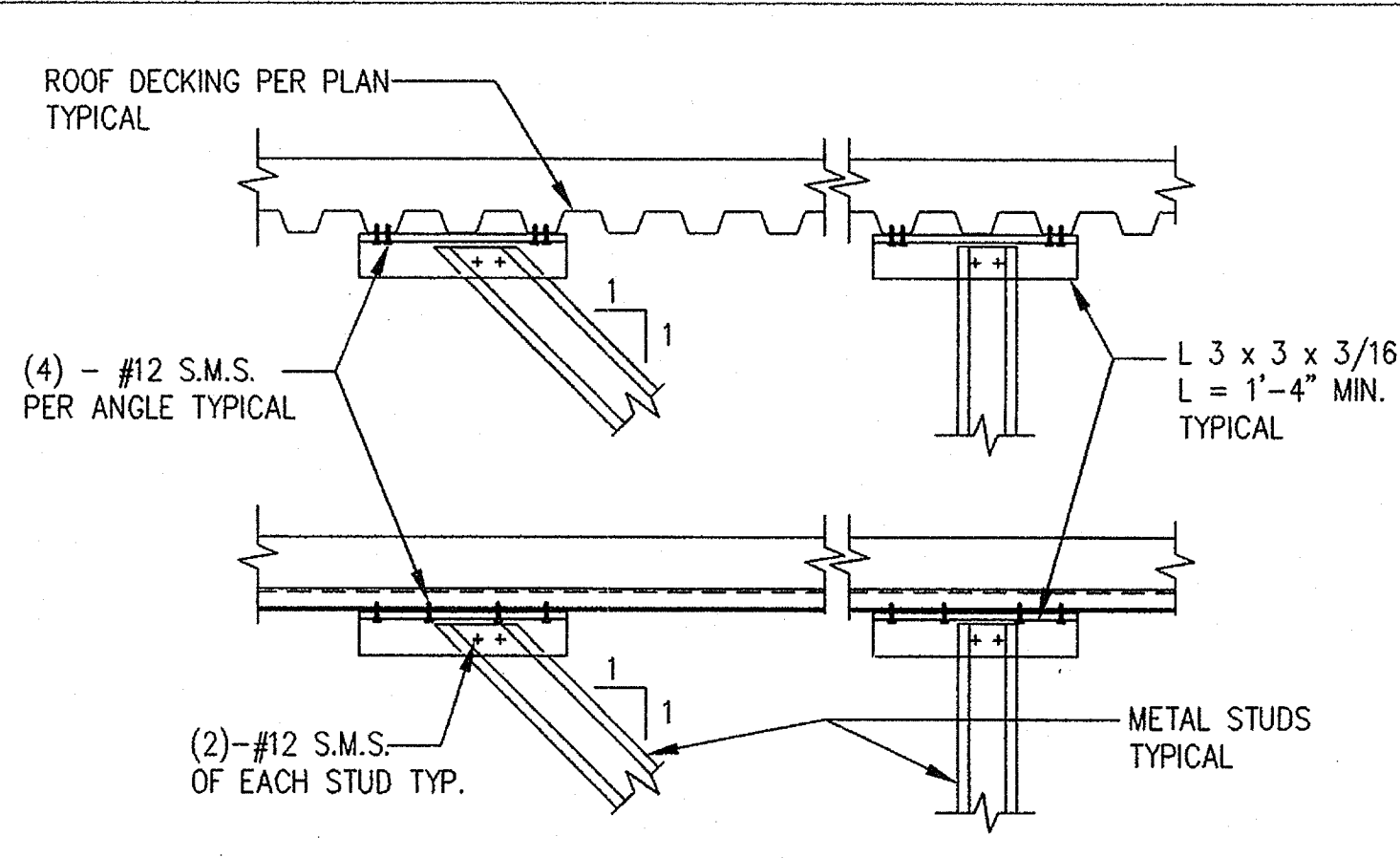
TYPICAL DETAILS
SQL1.2A



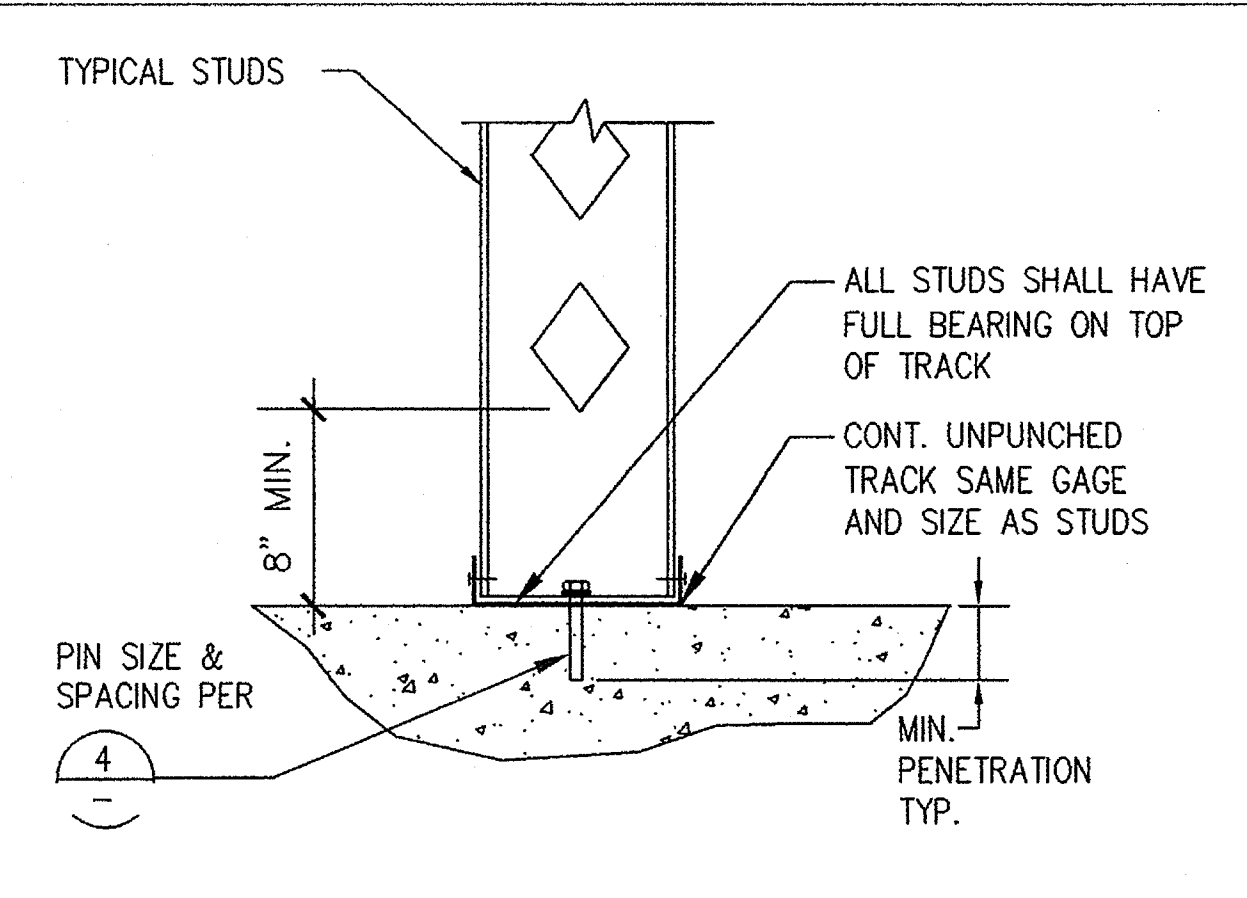
TYP. NON-BRG. STUD PERP. TO ROOF DECK (13)



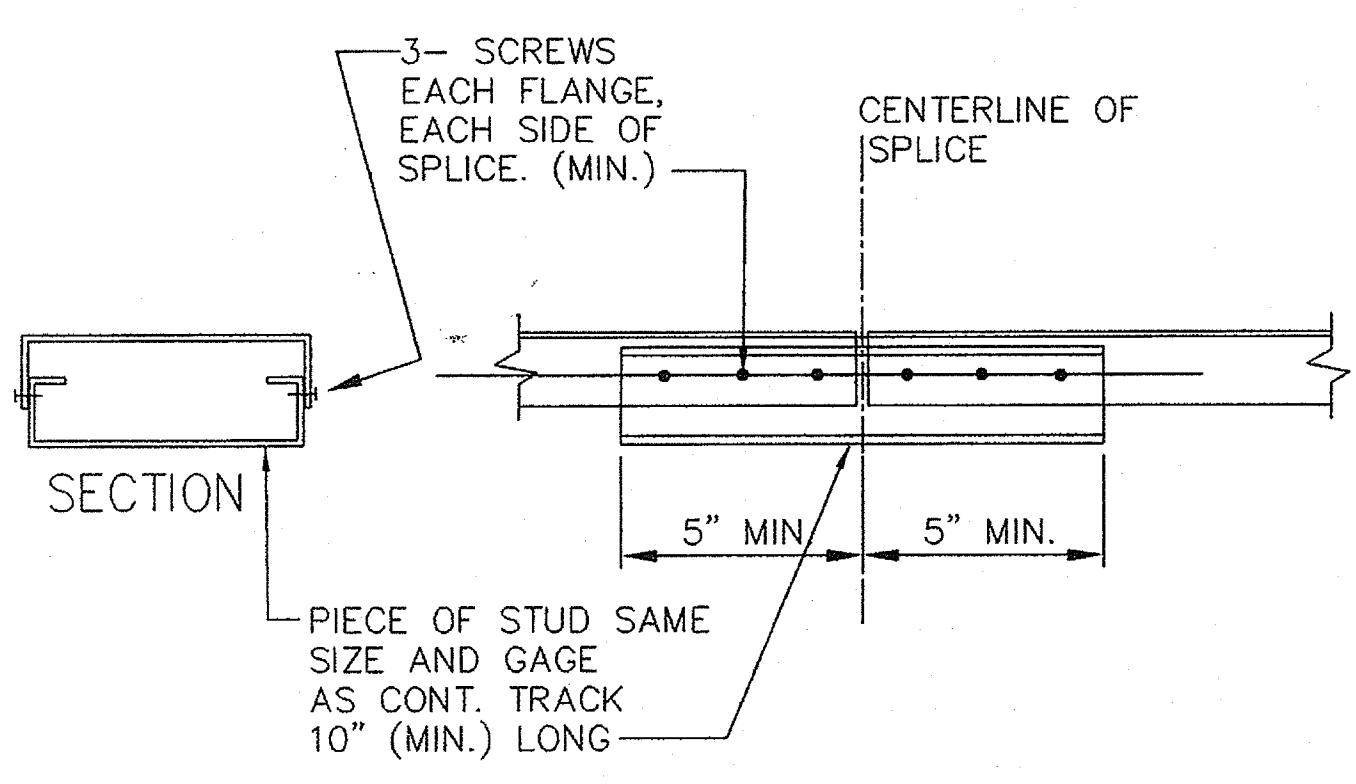
TYP. NON-BRG. STUD PARALLEL TO ROOF DECK (9)



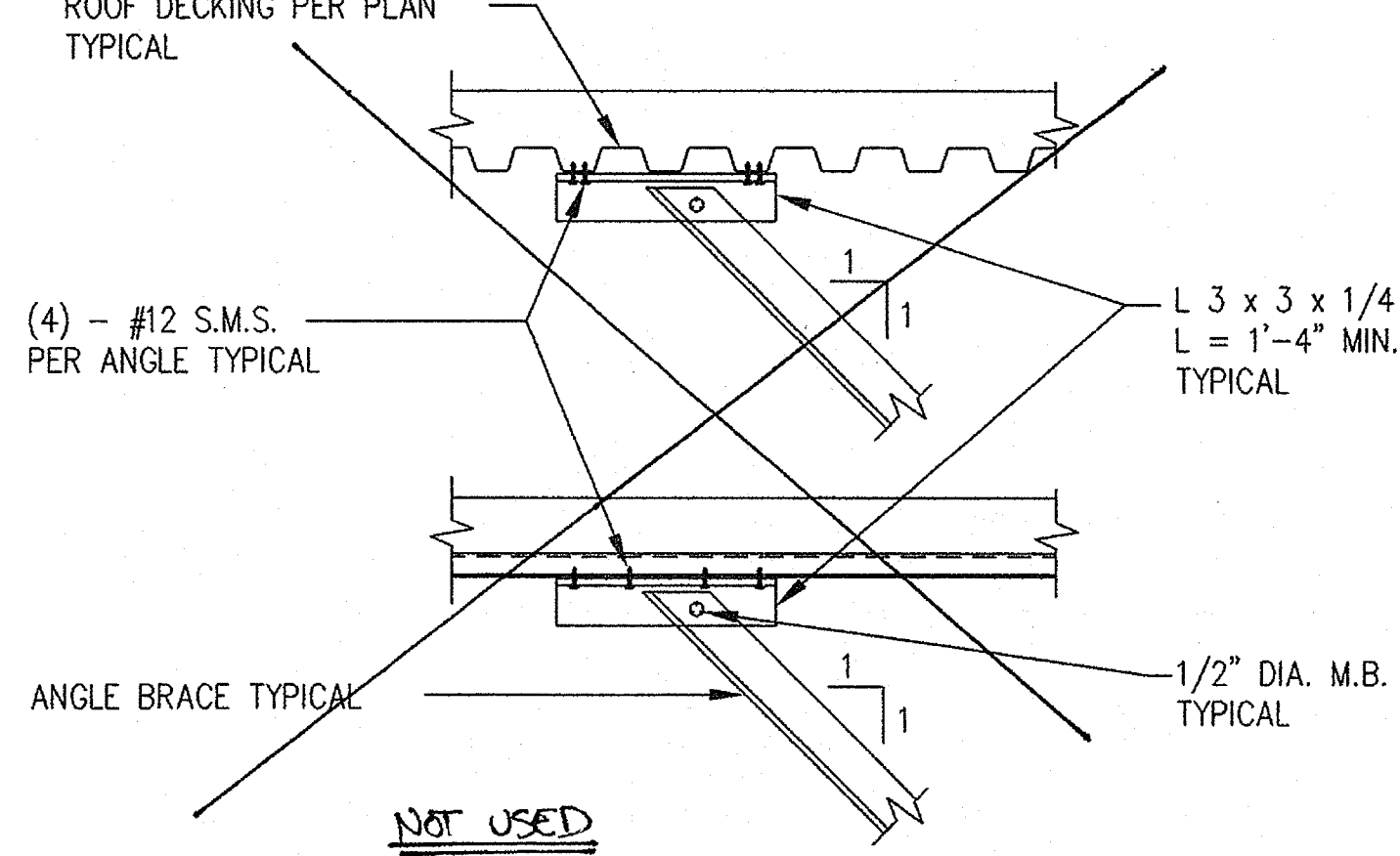
TYP. STUD BRACING AT ROOF (5)



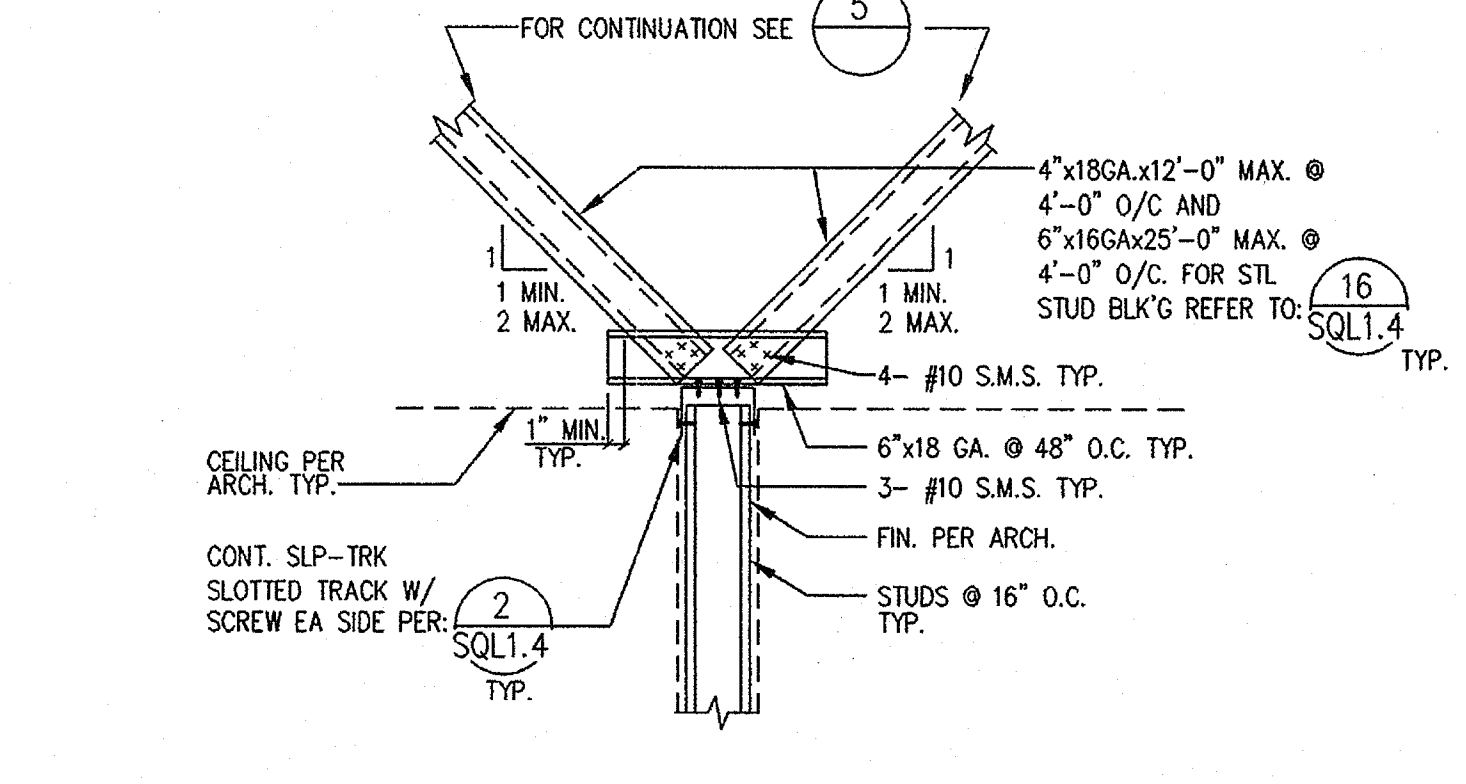
BOTTOM OF WALL AT CONC. (1)



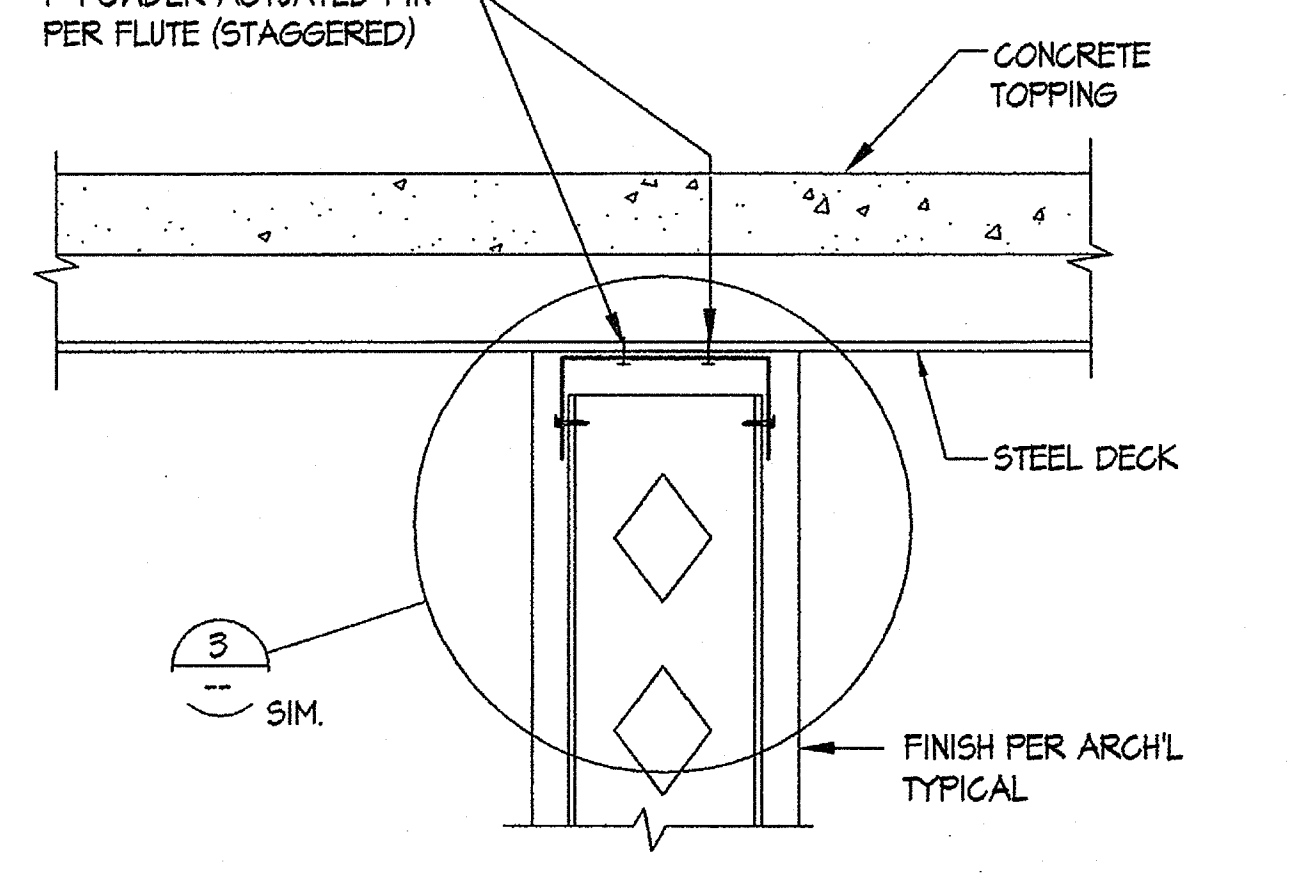
TYP. TOP STEEL TRACK SPLICE (14)



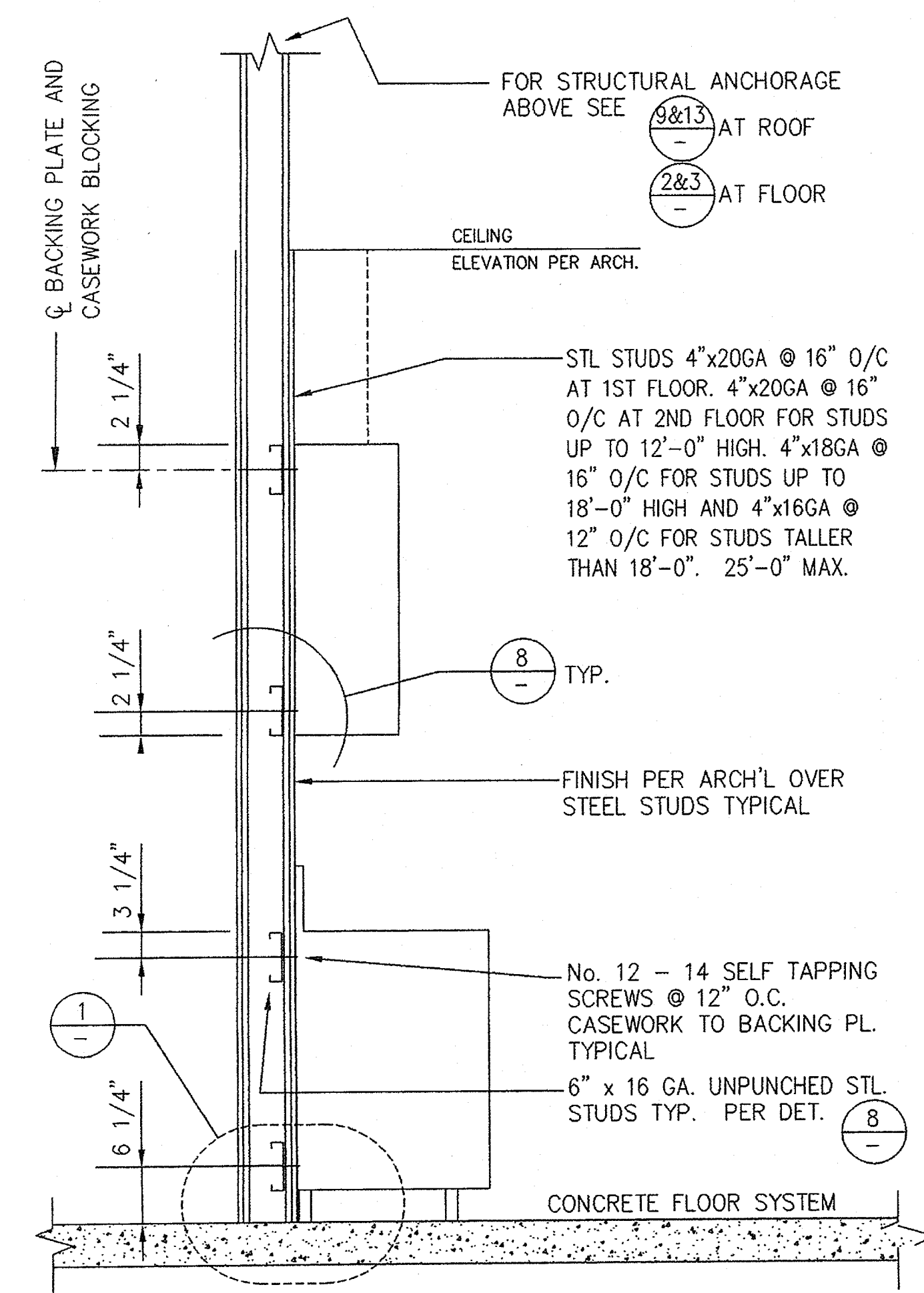
TYP. ANGLE BRACING AT ROOF (10)



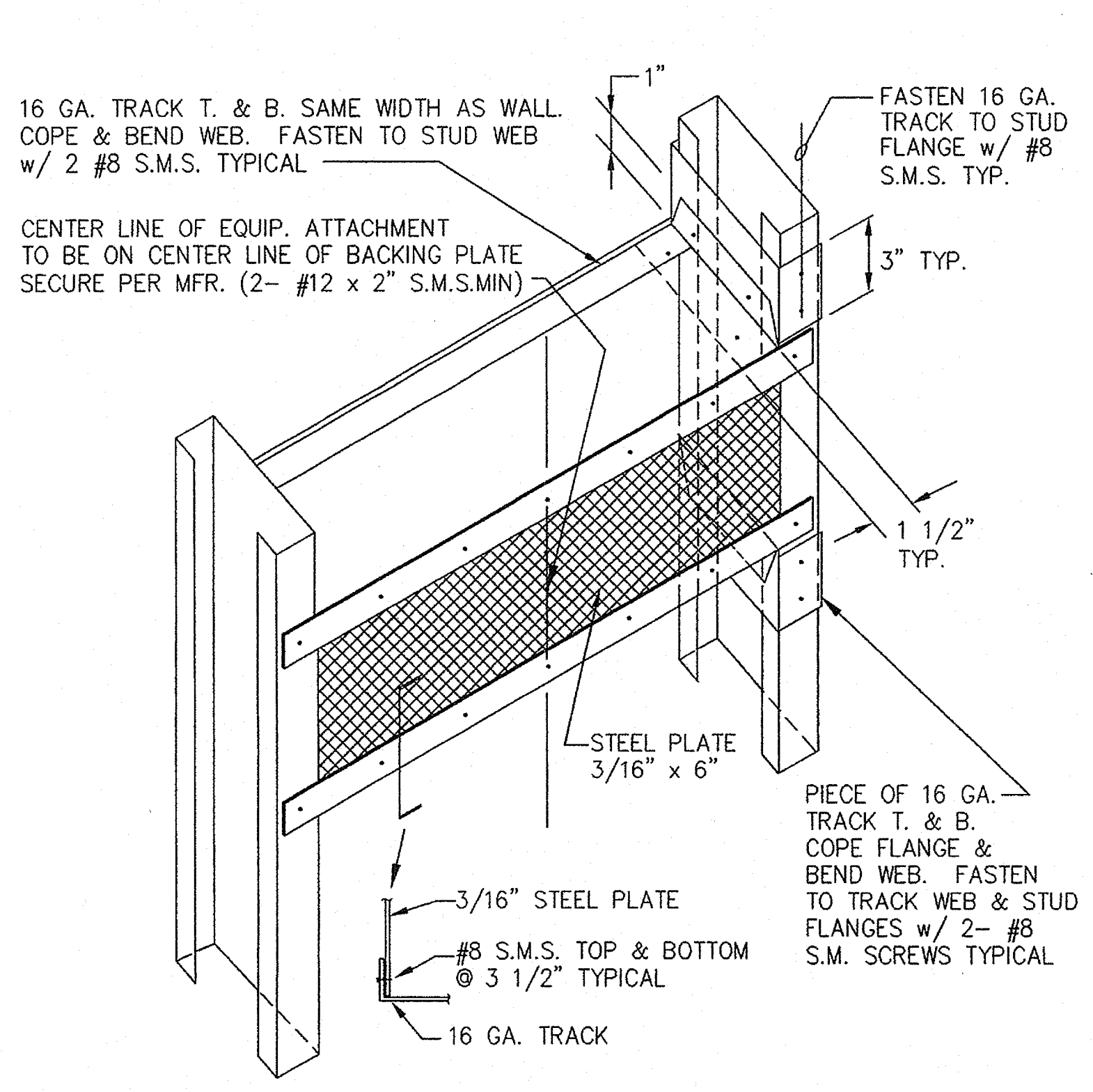
TYP. PARTITION CONN. @ CEILING LEVEL FOR EXACT LOCATION, SEE ARCH. (6)



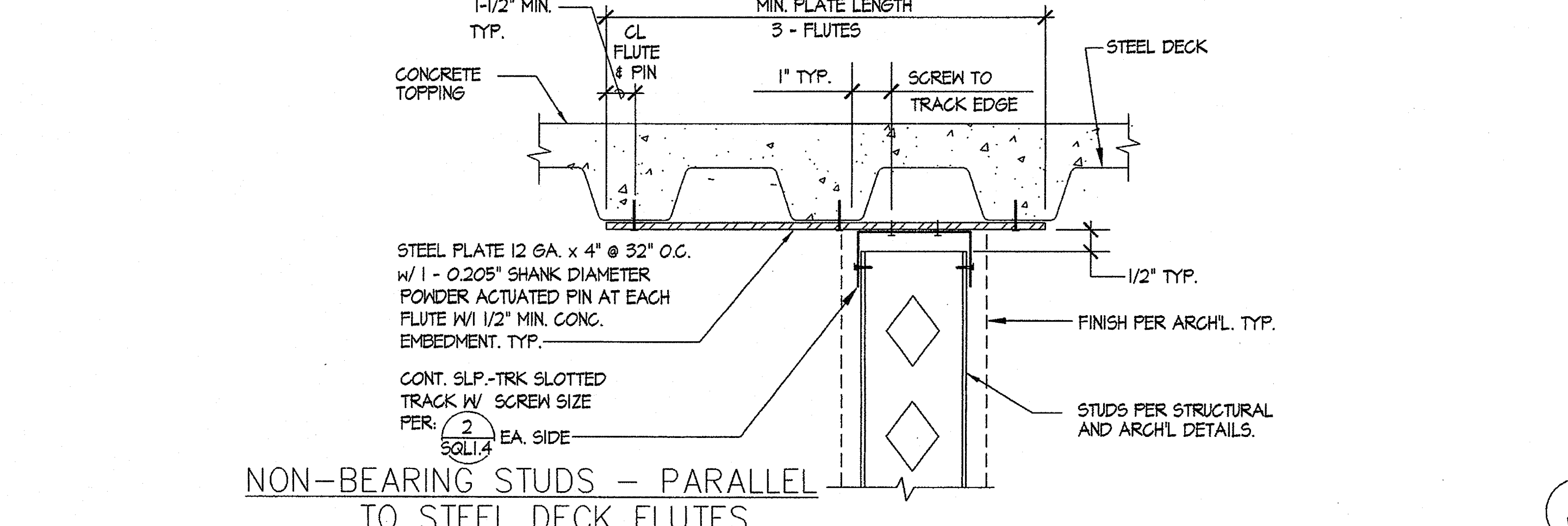
NON-BRG. STUDS - PERPENDICULAR TO STEEL DECK FLUTES (2)



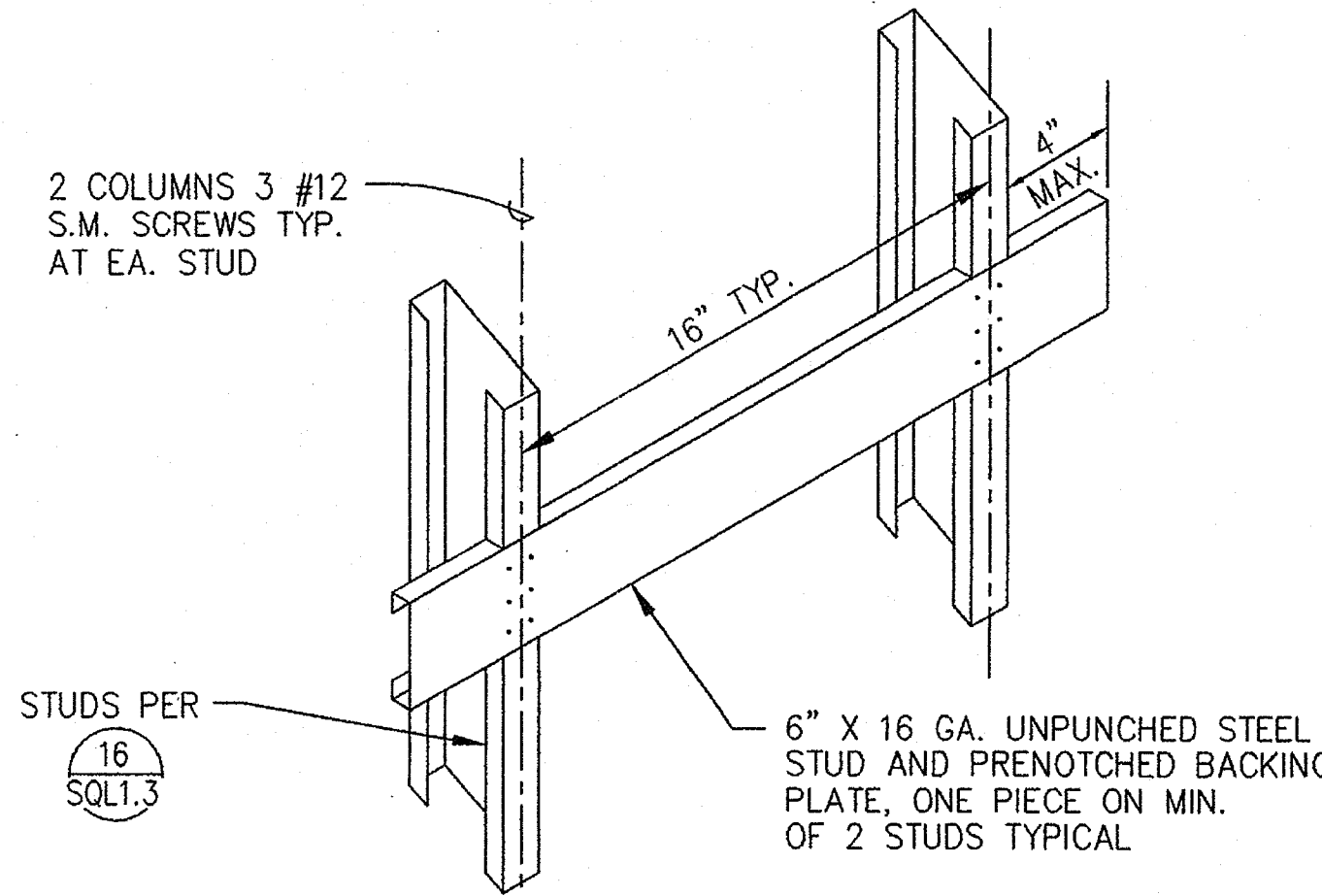
TYP. CASEWORK SUPPORT (16)



MECHANICAL, ELECTRICAL EQUIPMENT BACKING PLATE (12)



NON-BEARING STUDS - PARALLEL TO STEEL DECK FLUTES (3)



BACKING PLATE (8)

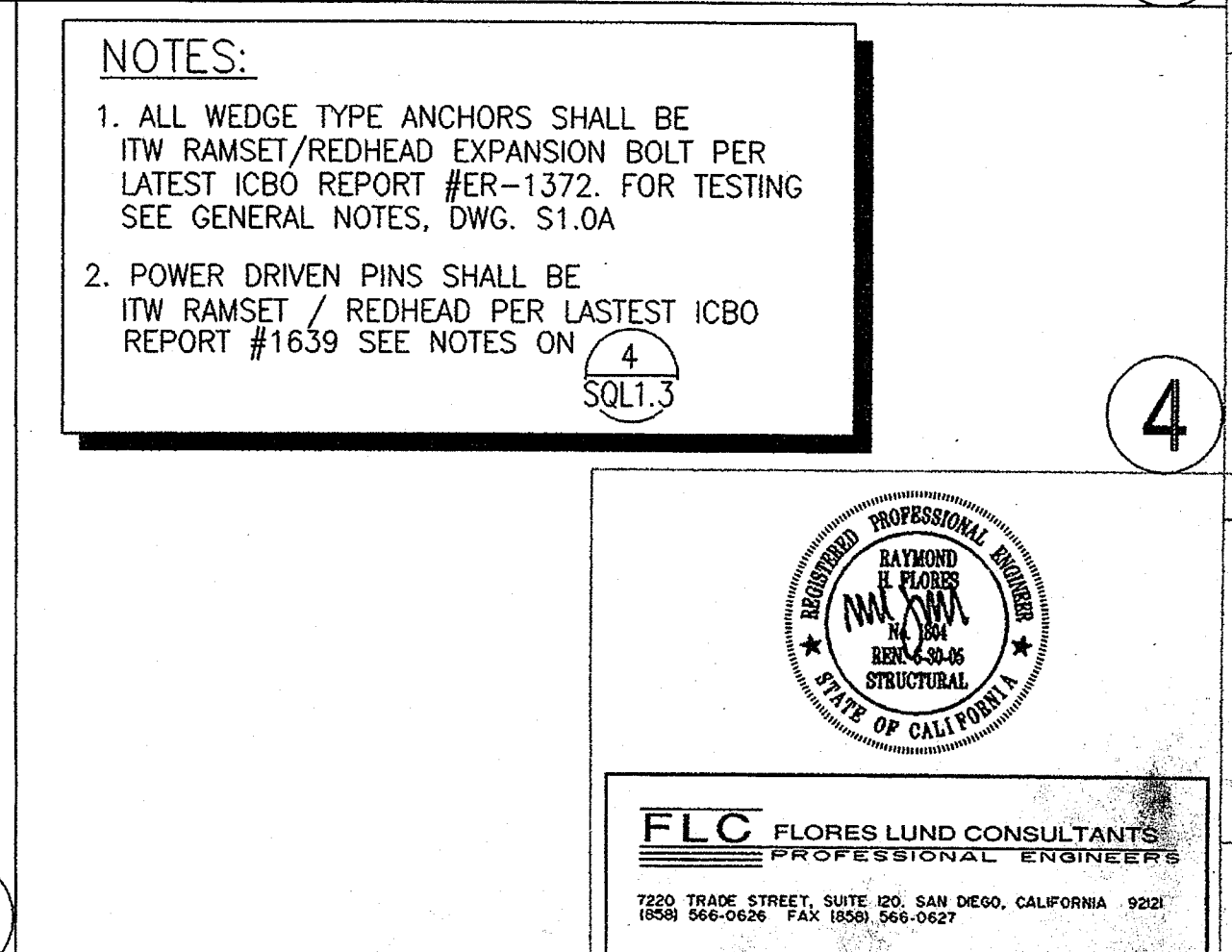
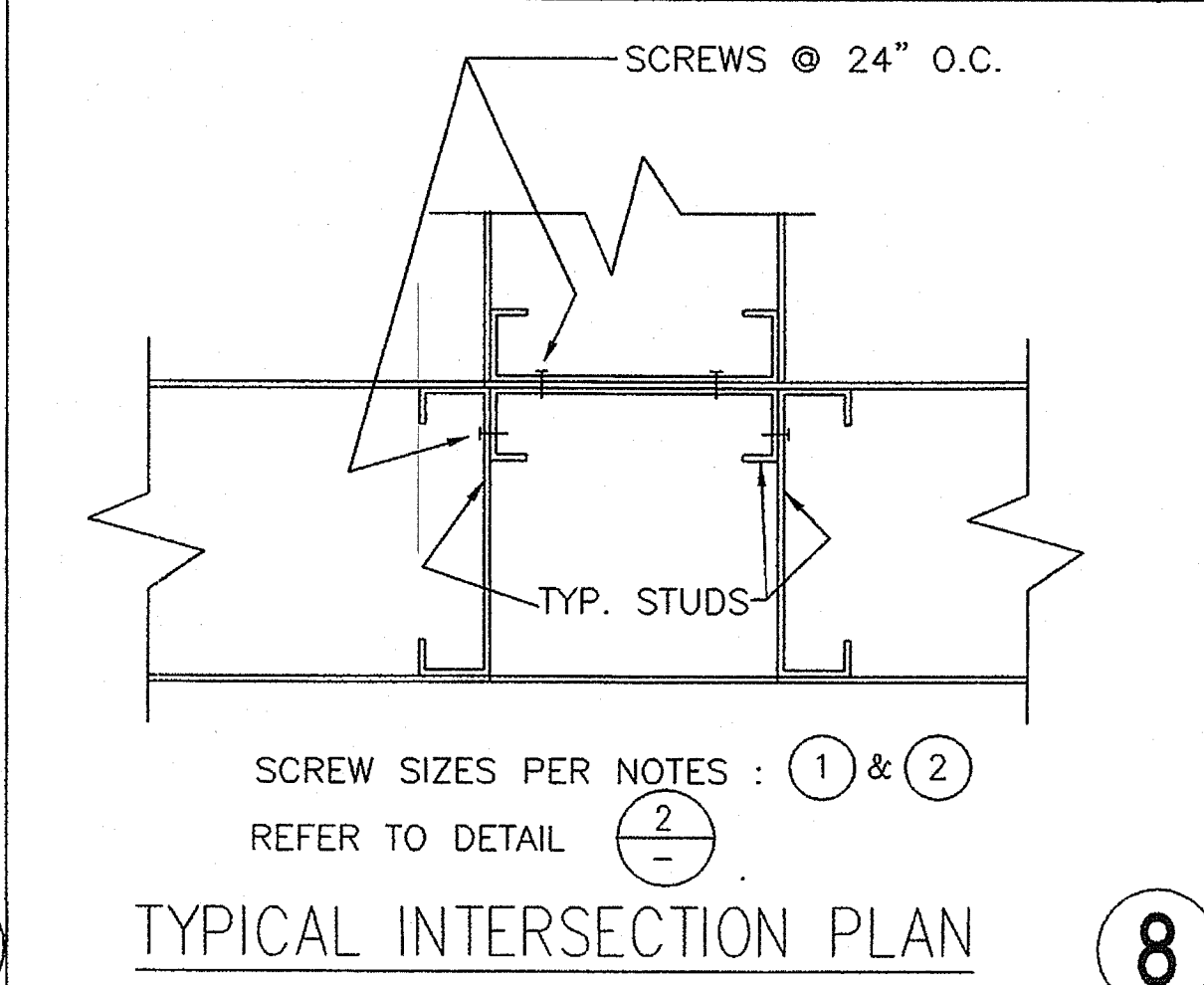
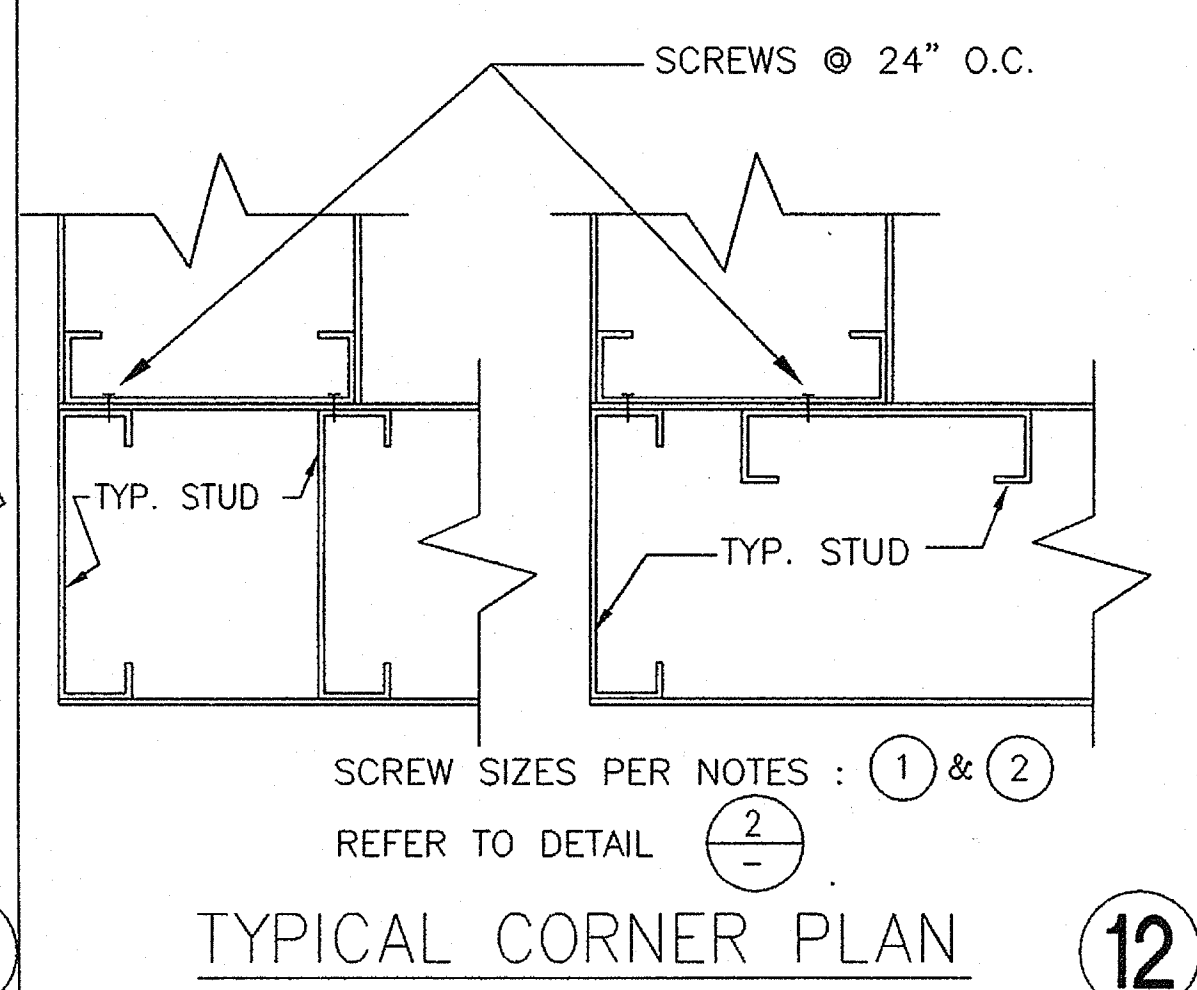
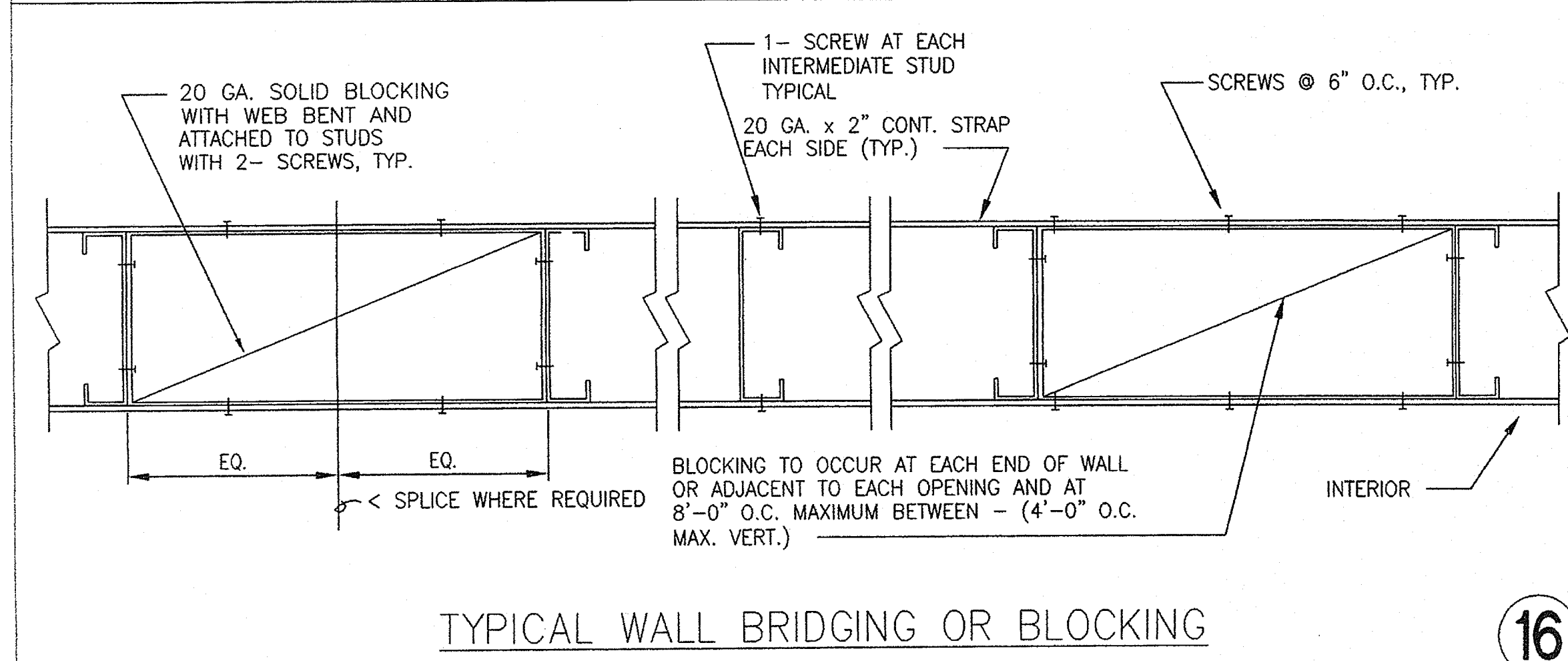
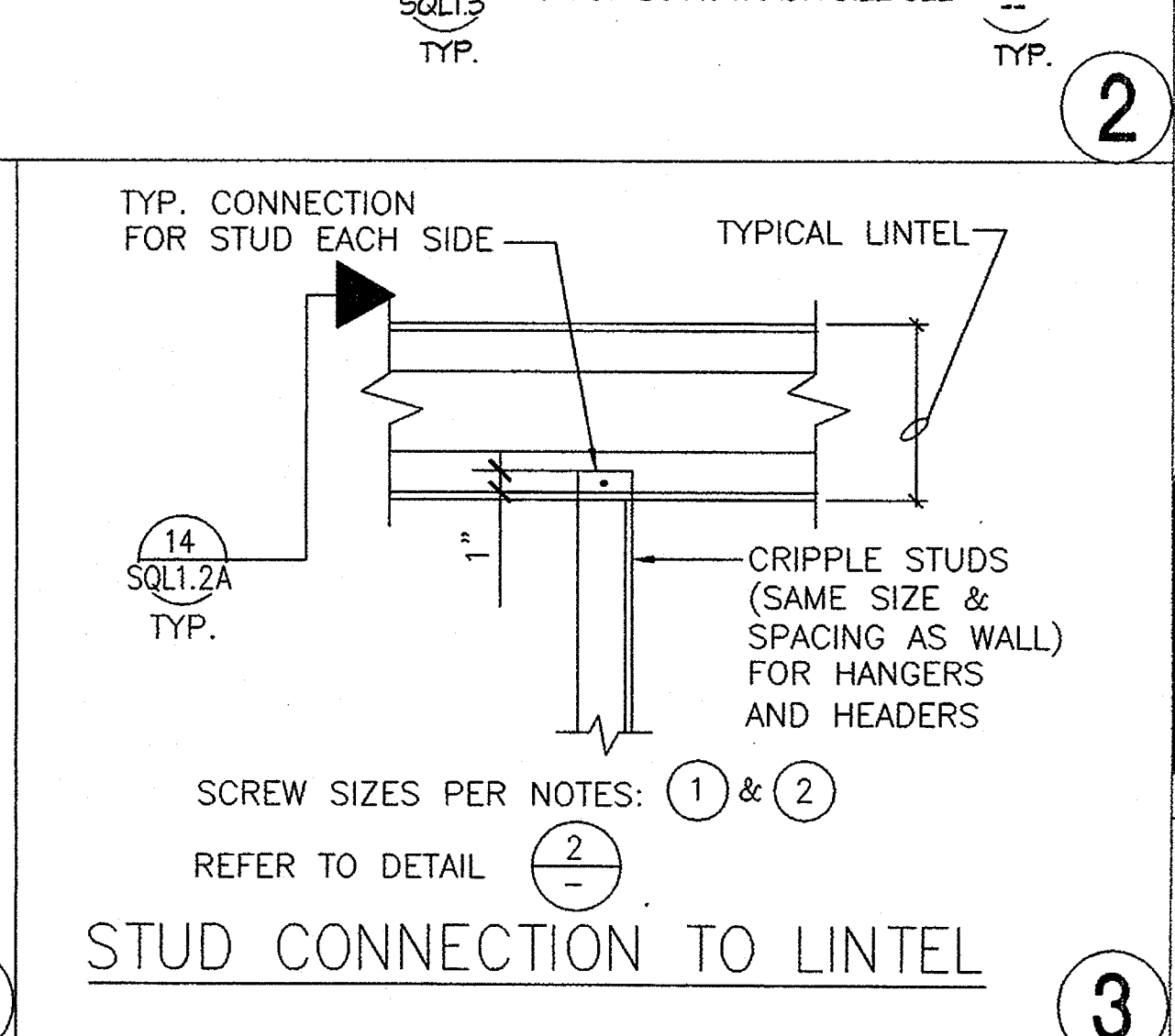
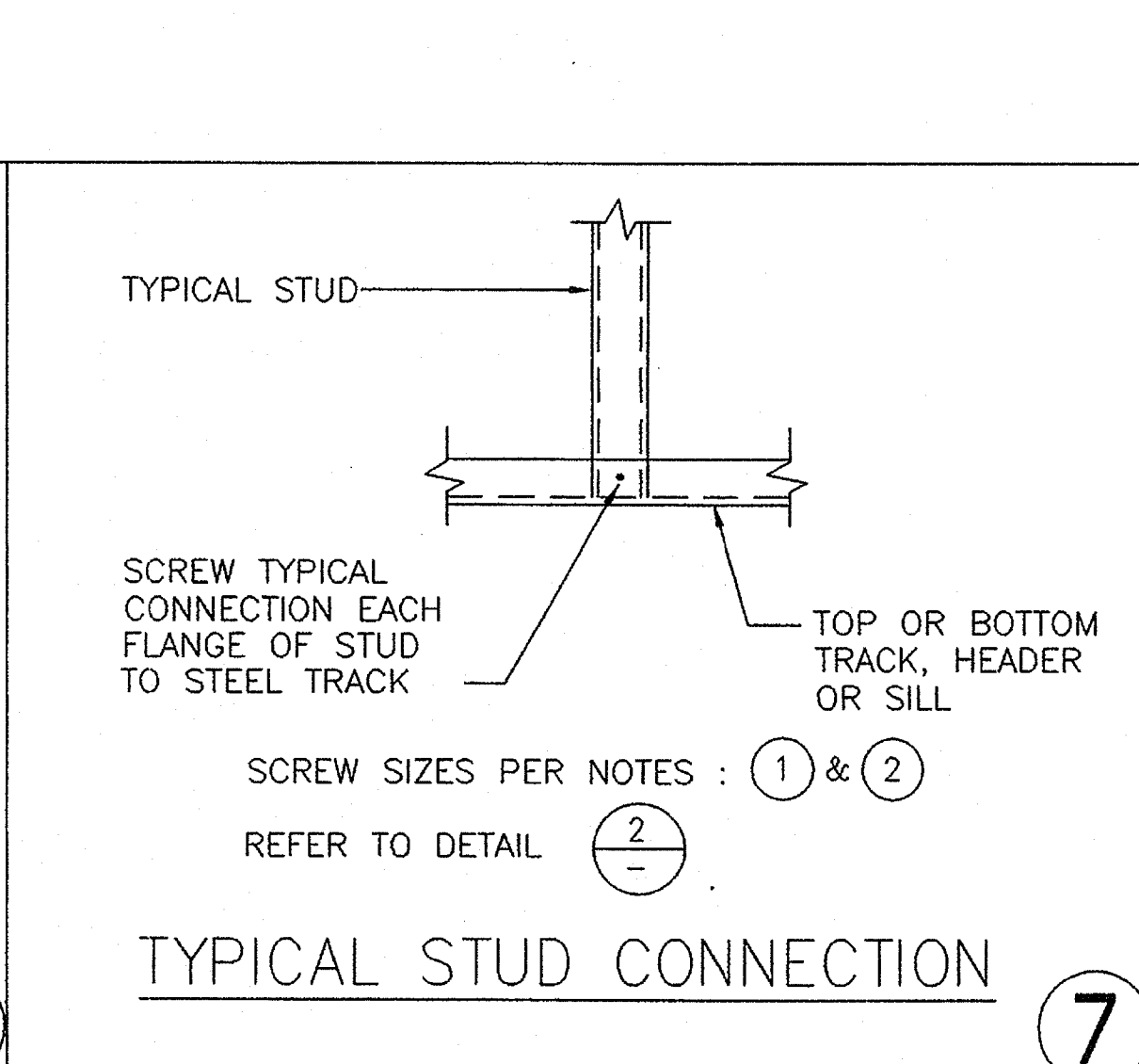
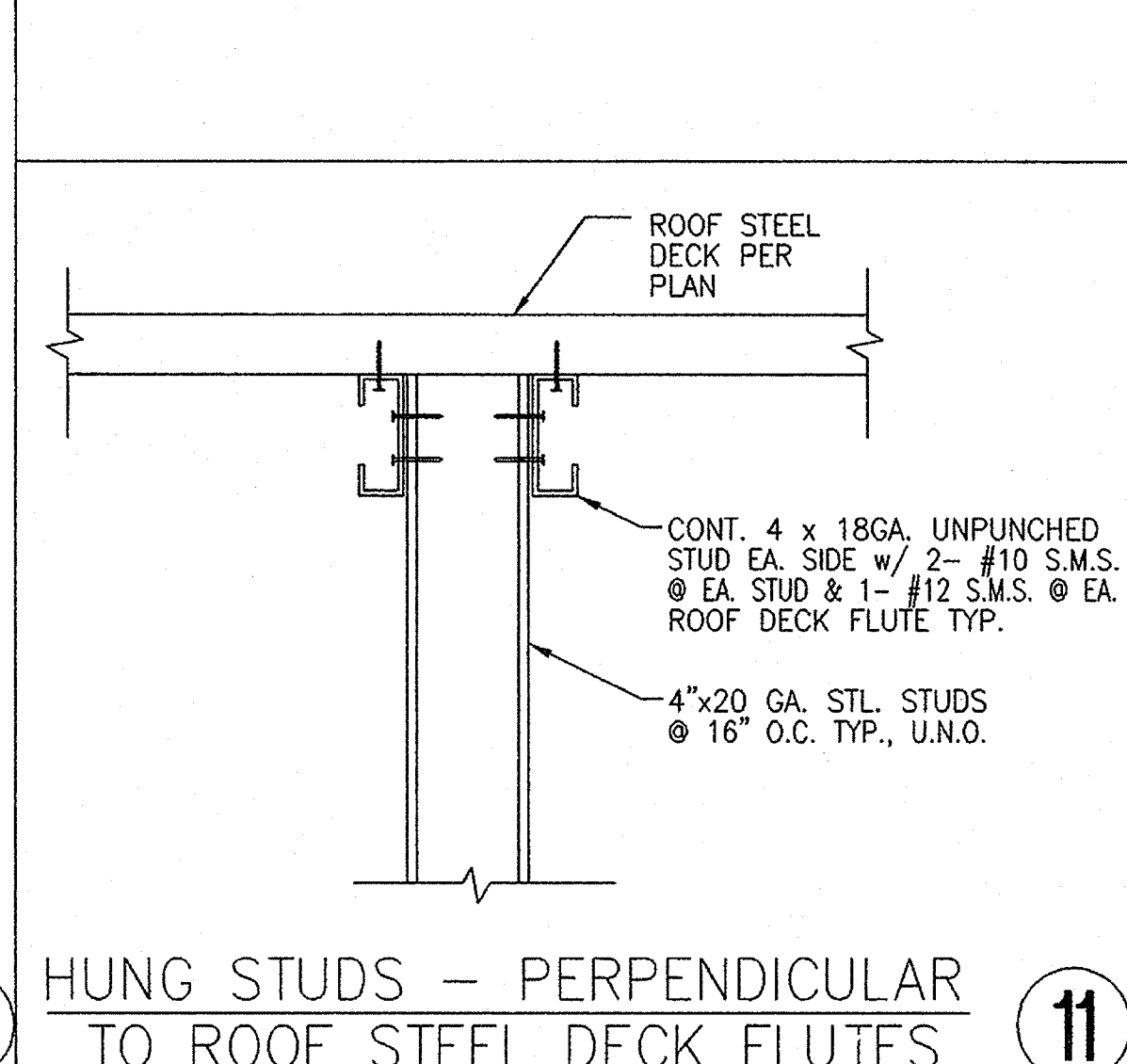
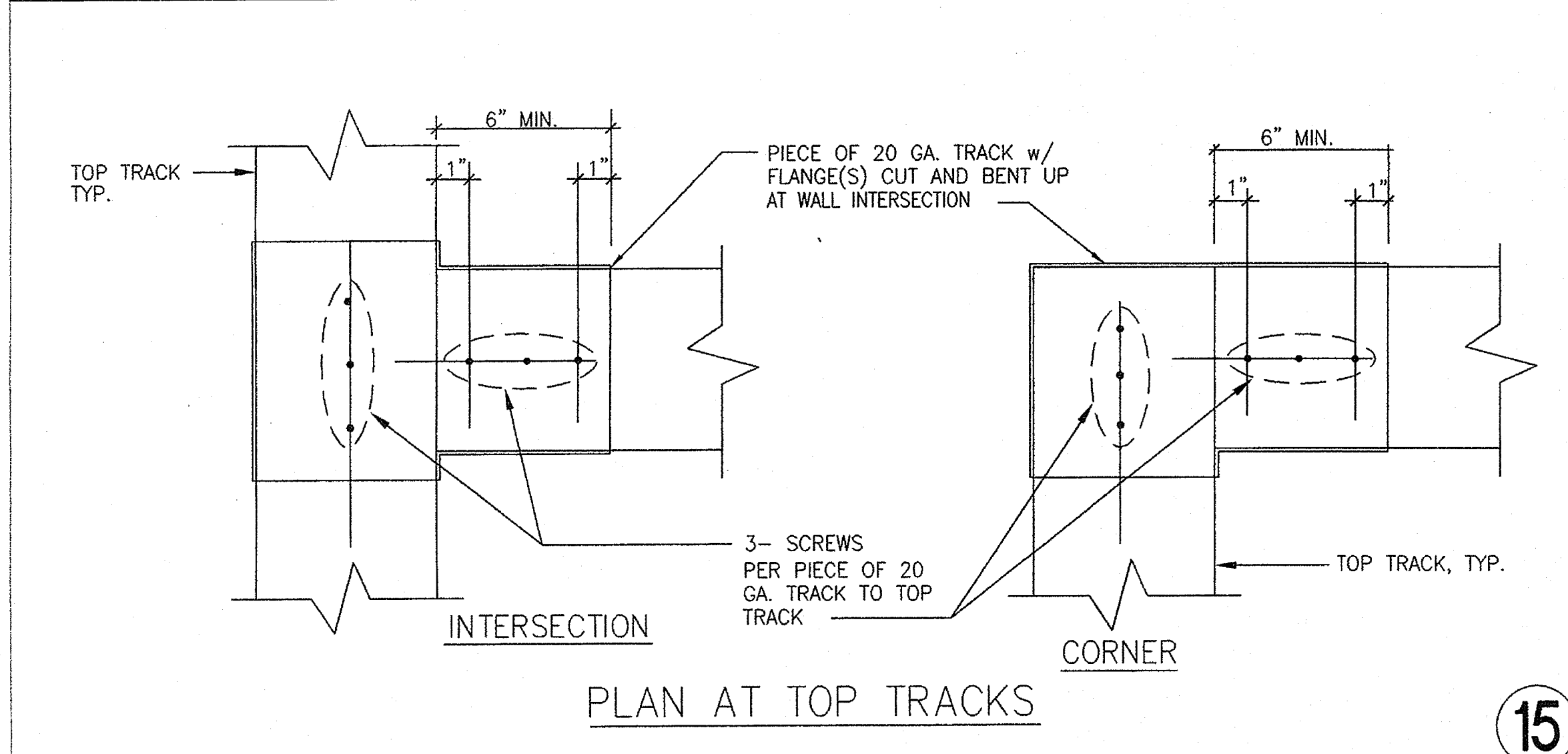
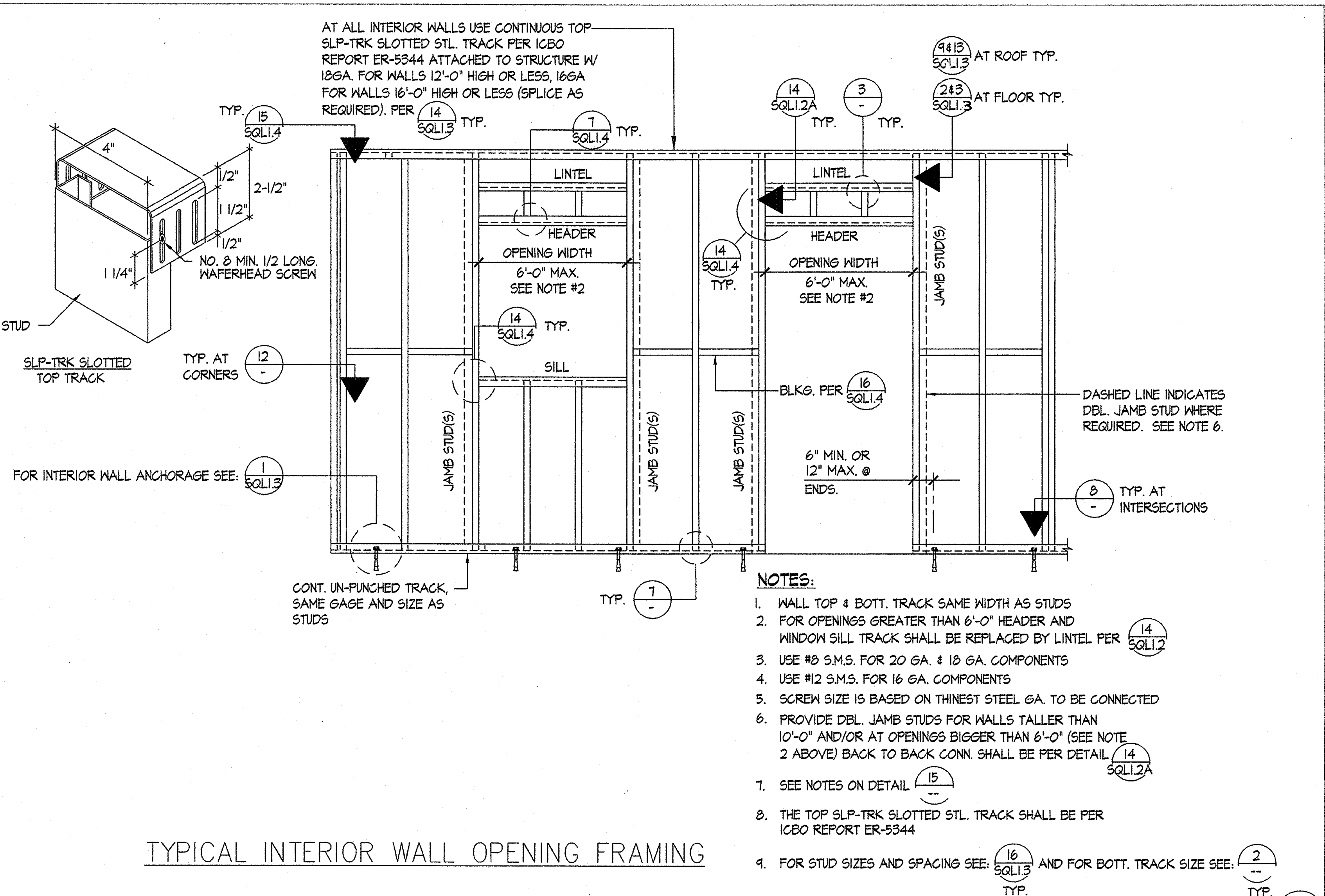
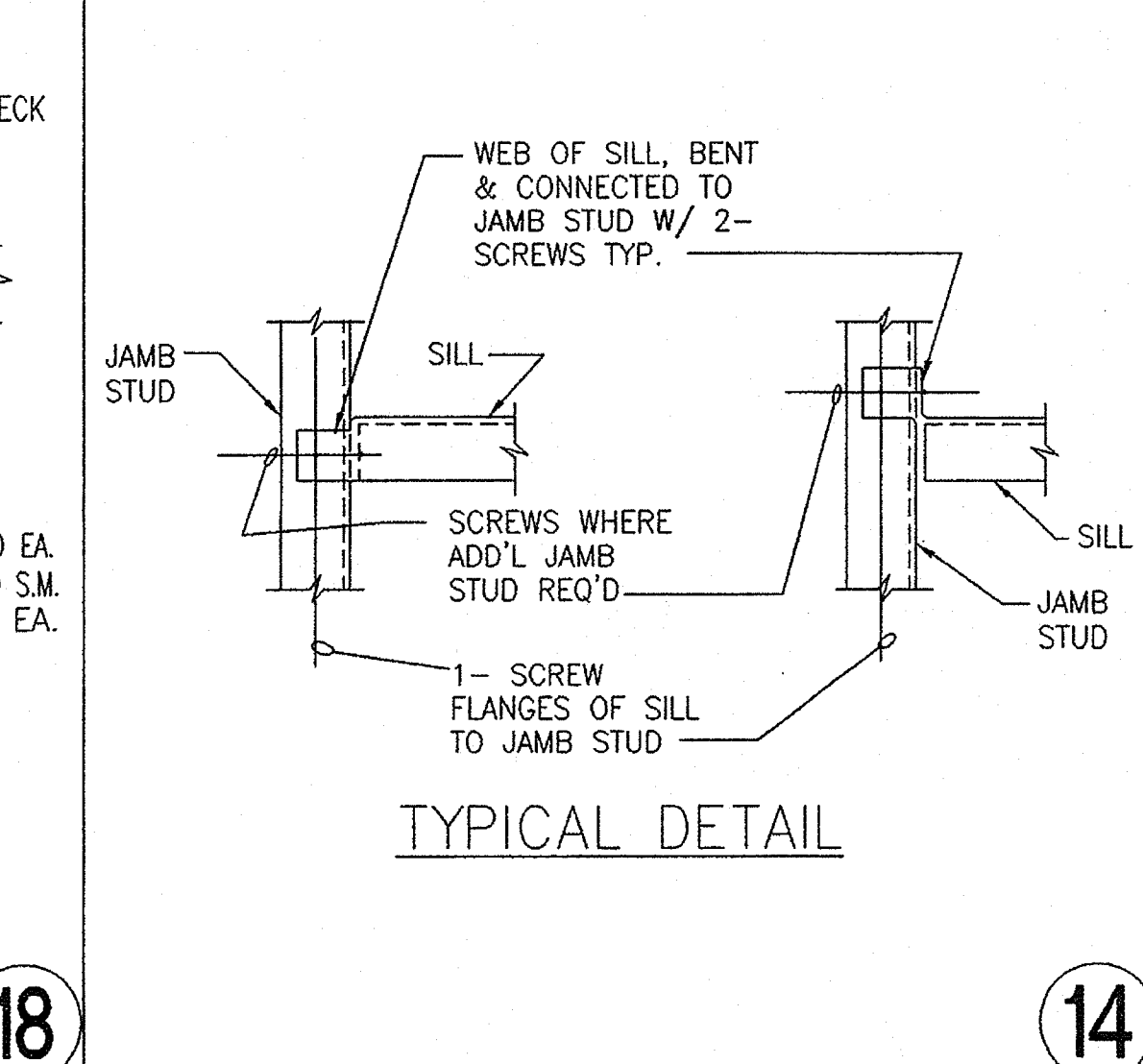
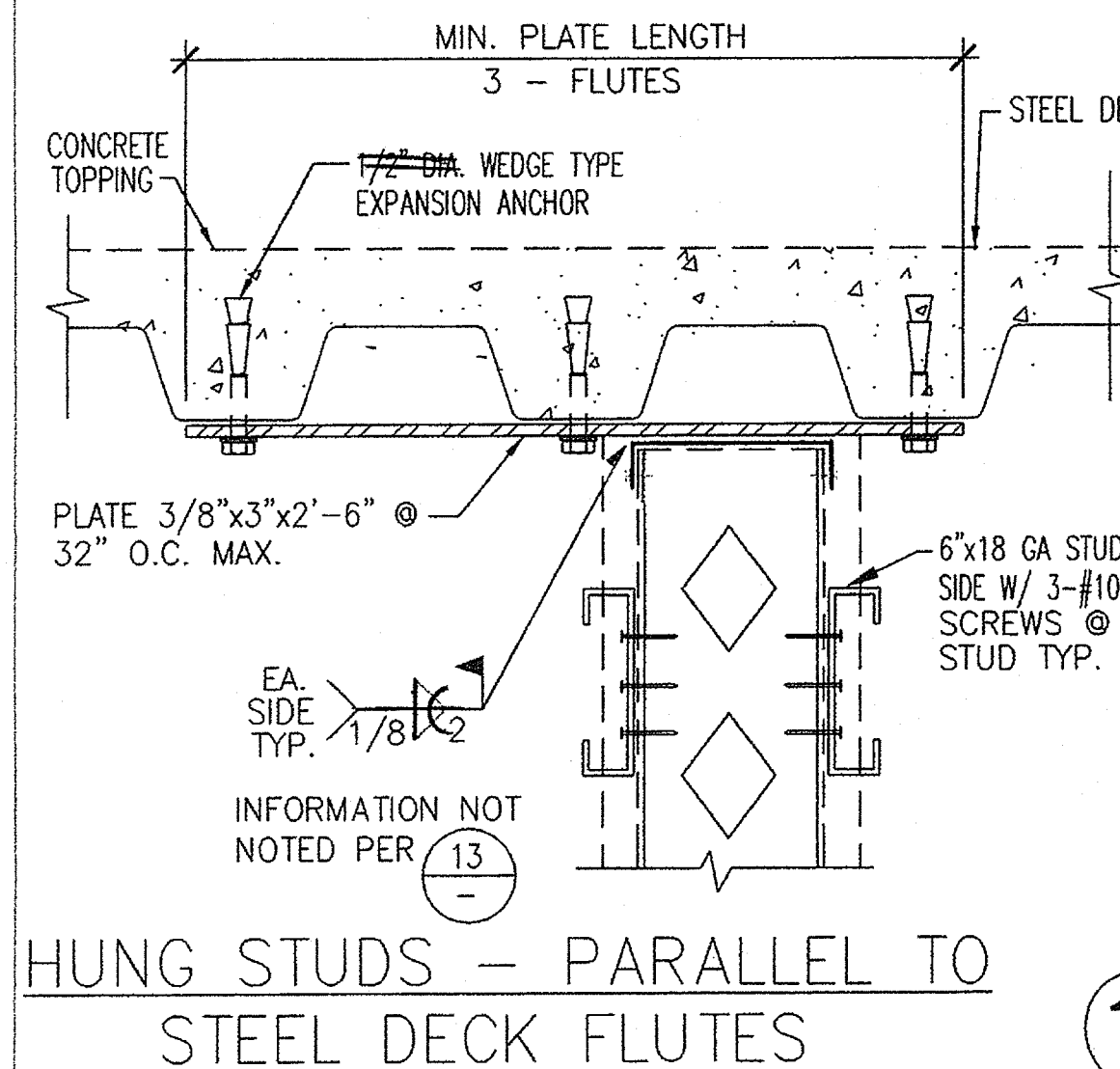
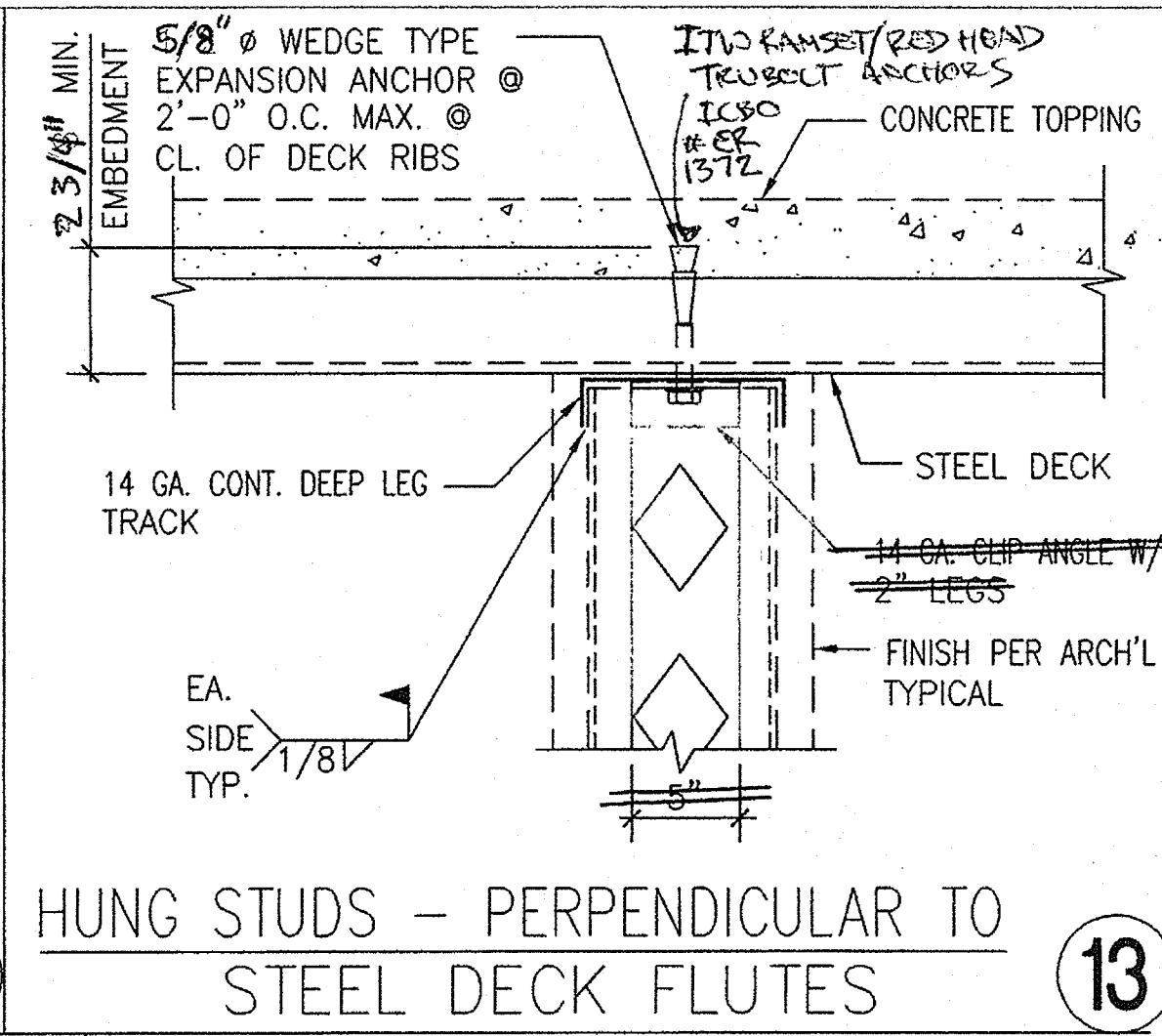
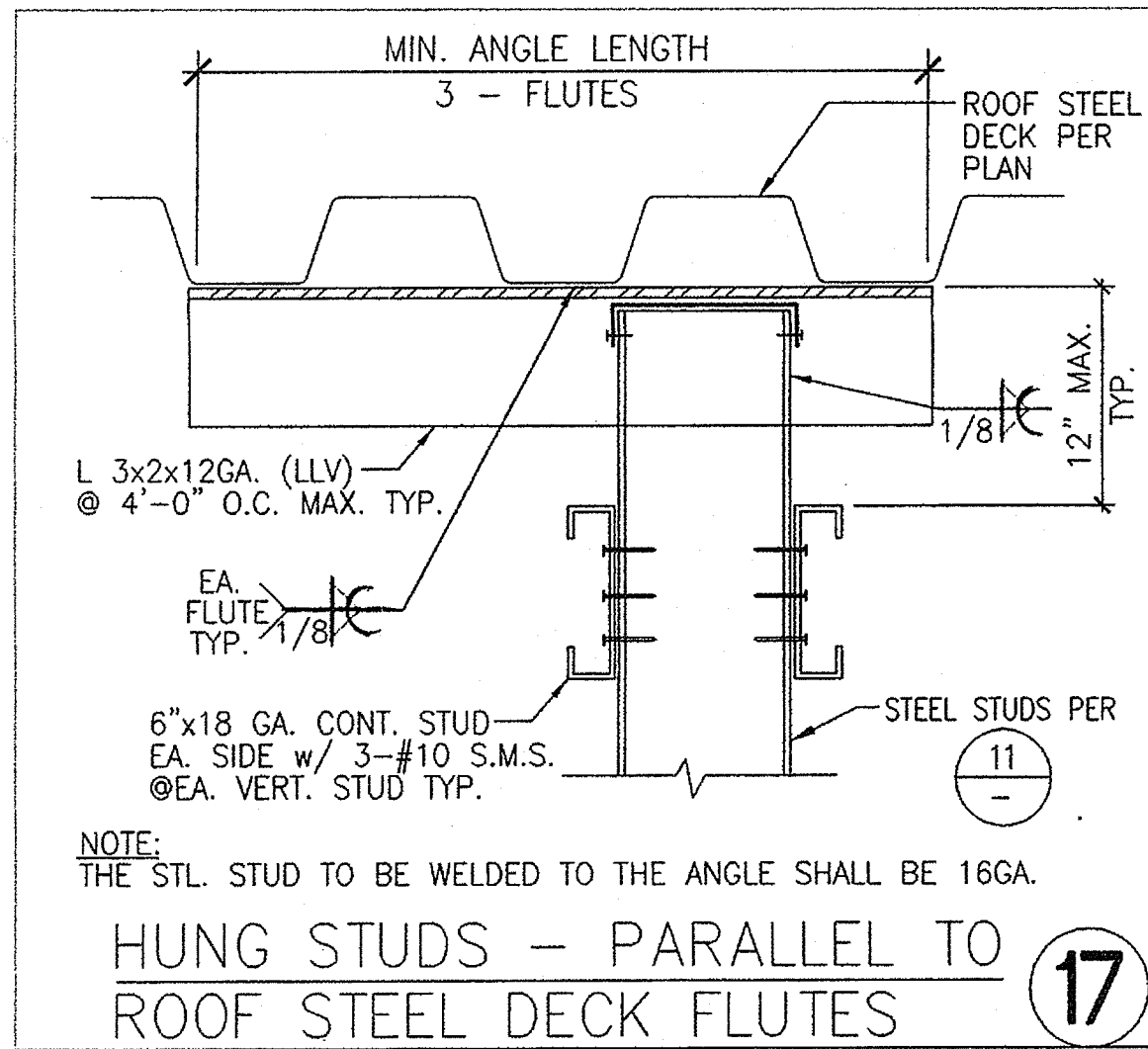
POWDER DRIVEN FASTENERS IN CONC.

MINIMUM SHANK DIAMETER	MINIMUM PENETRATION	TRACK DEPTH	TRACK GAGE	MAXIMUM SPACING	SPAC. DIST.
0.170"	1 1/4"	4" & 6"	20	32"	3"
0.170"	1 1/2"	4" & 6"	18	32"	3"
0.170"	1 1/2"	4" & 6"	16	24"	3"

- NOTES:
- FASTENERS SHALL BE PER ITW RMSET/ RED HEAD ACTUATED FASTENERS. LATEST I.C.B.O. REPORT No. 1639, (TYP.)
 - FOR TOOL QUALIFICATION AND FASTENER TESTING SEE "POWDER DRIVEN SHOT PINS" NOTES ON DWG. SQL1.0A

FASTENER SCHEDULE (4)

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 GROTH ARCHITECTS, INC.
 823 ACACIA STREET
 OCEANSIDE, CA 92054
 OCEANSIDE UNIFIED S.D.
 GROTH ARCHITECTS, INC. 3355 MISSION AVE. SUITE 234 OCEANSIDE, CALIFORNIA 92054
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 OUSD NO. 758-000
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 PROFESSIONAL ENGINEERS
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3355 MISSION AVE.
OCEANSIDE, CALIFORNIA 92054
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FAX 760-754-8291

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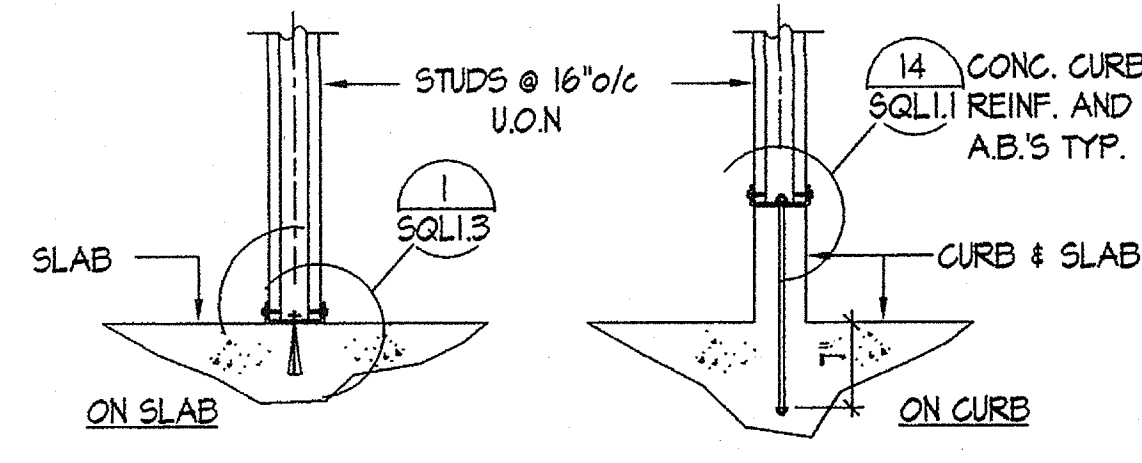
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C-26609
4/30/2007
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STATE OF CALIFORNIA

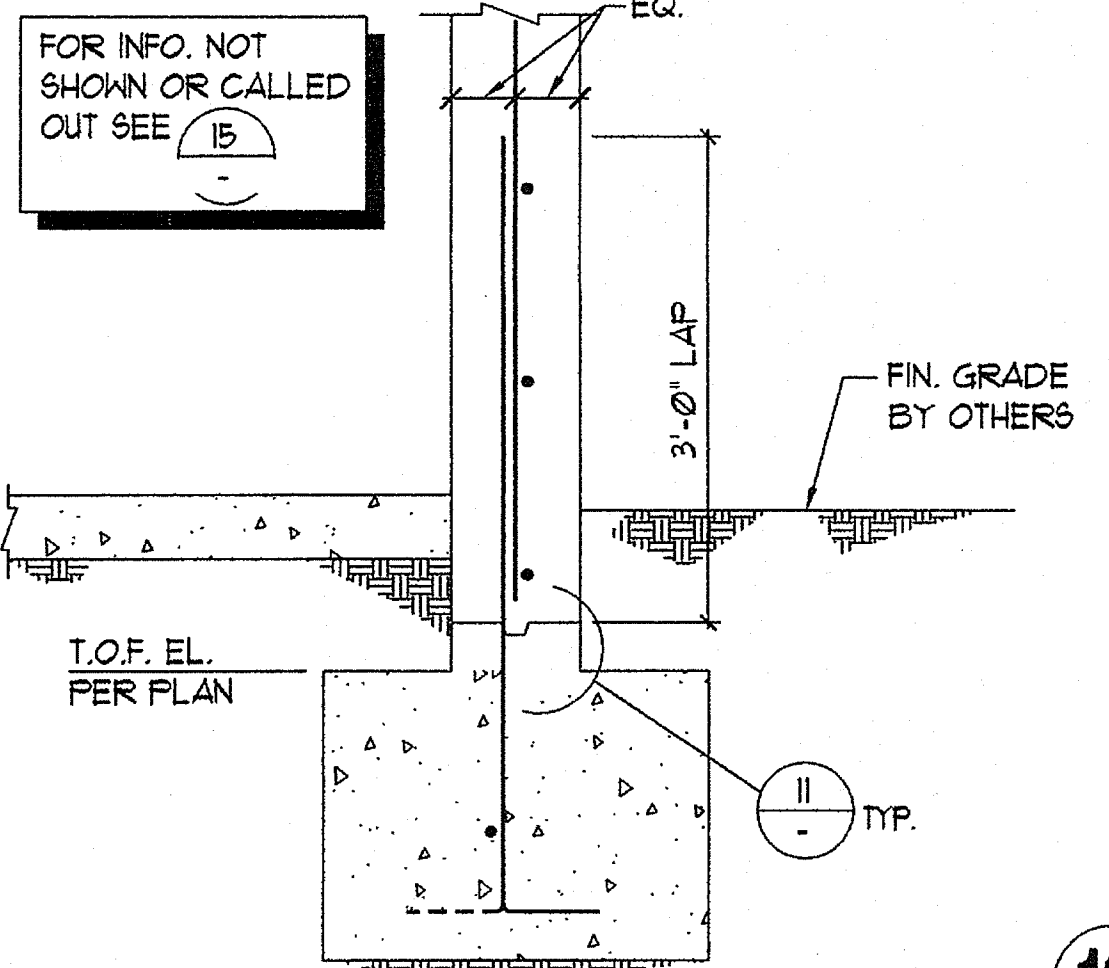
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PROFESSIONAL ENGINEERS
1230 TRADE STREET, SUITE 500, SAN DIEGO, CALIFORNIA 92101
TEL: 619-444-1000 FAX: 619-444-0427

TYPICAL DETAILS

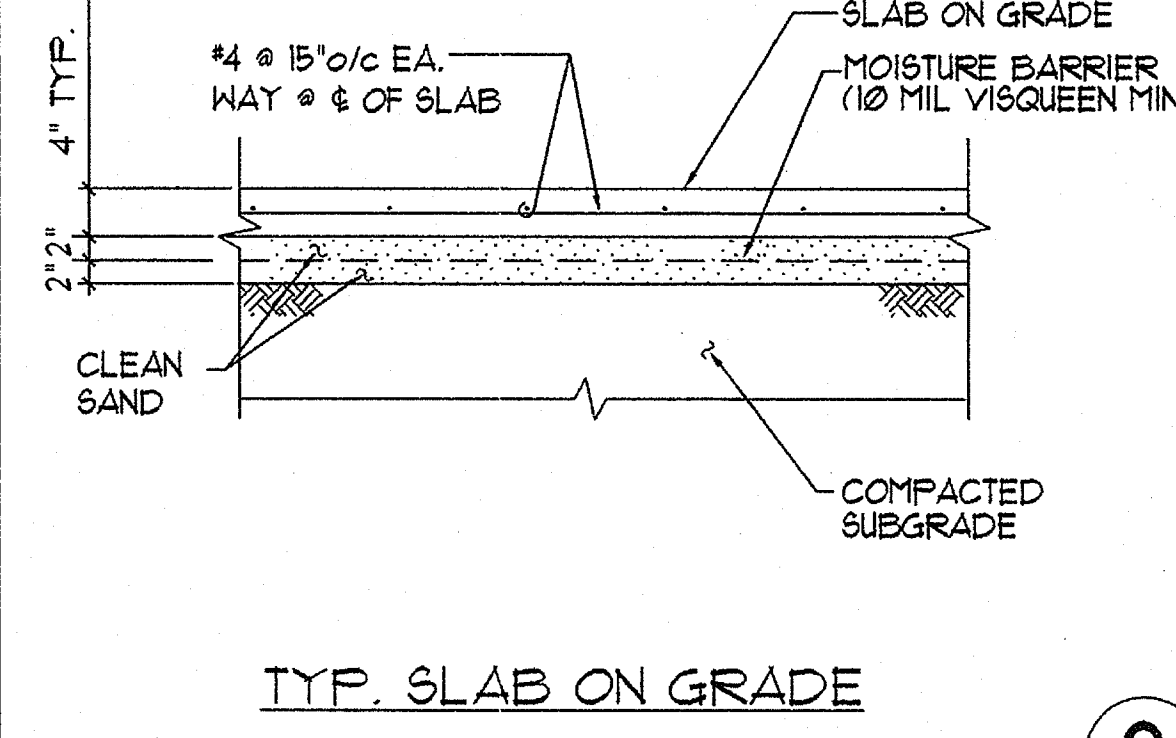
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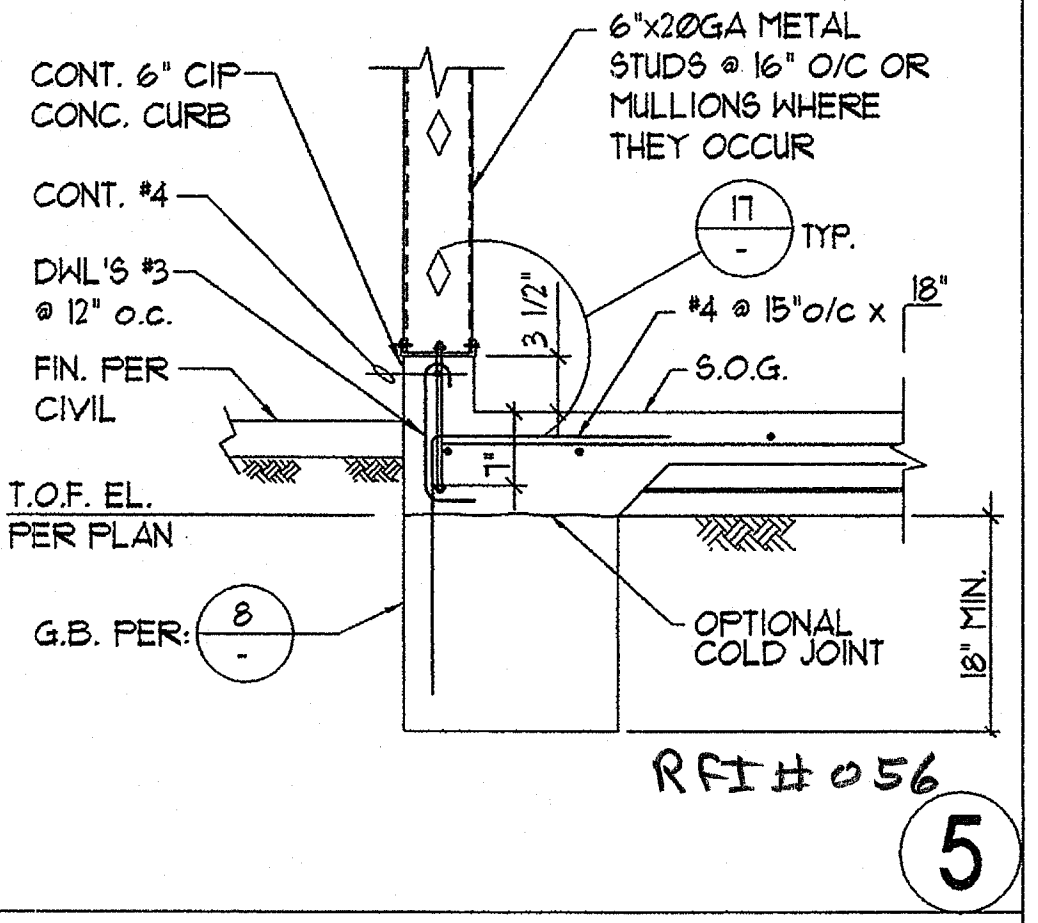
NON-BEARING PARTITION 17



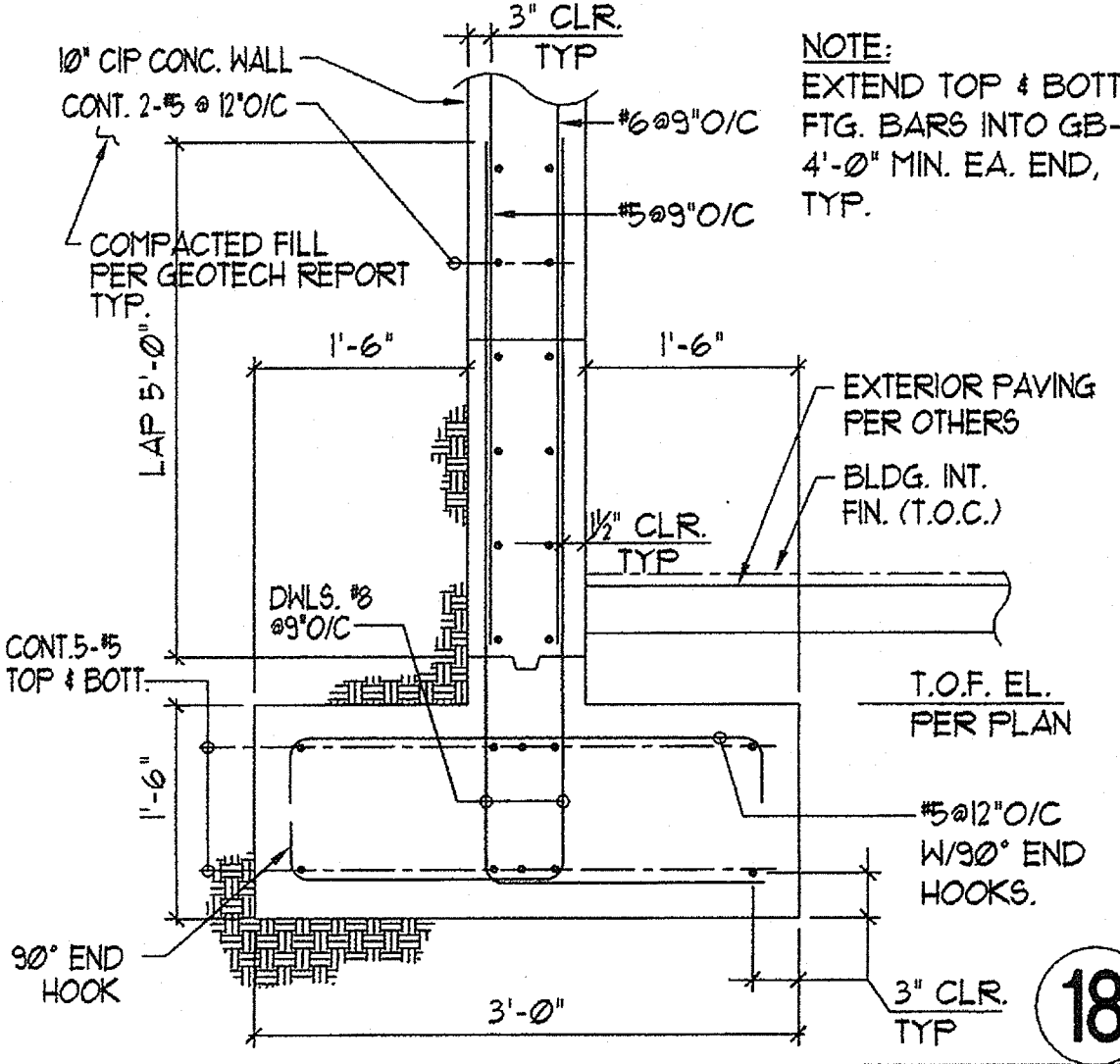
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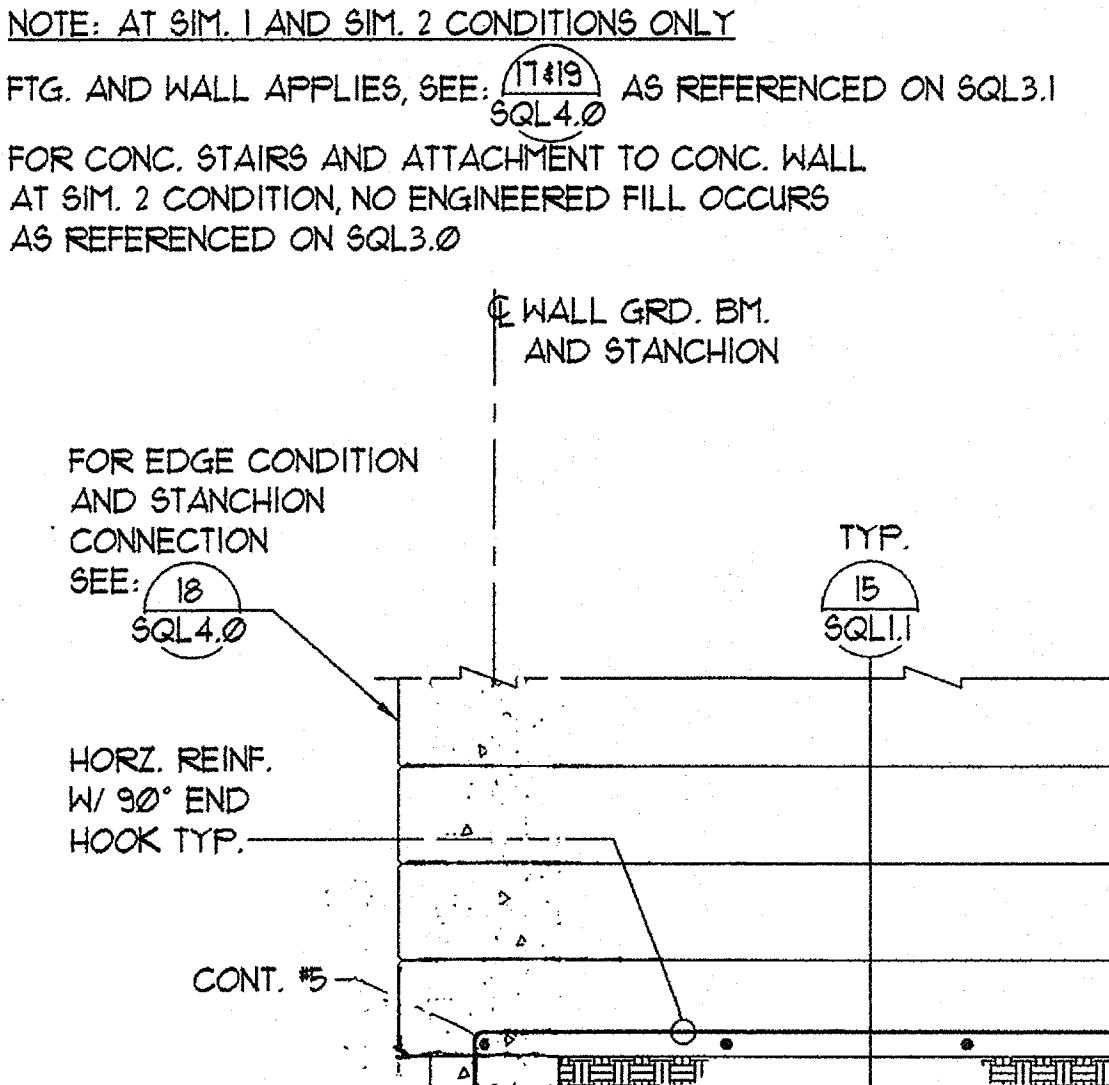
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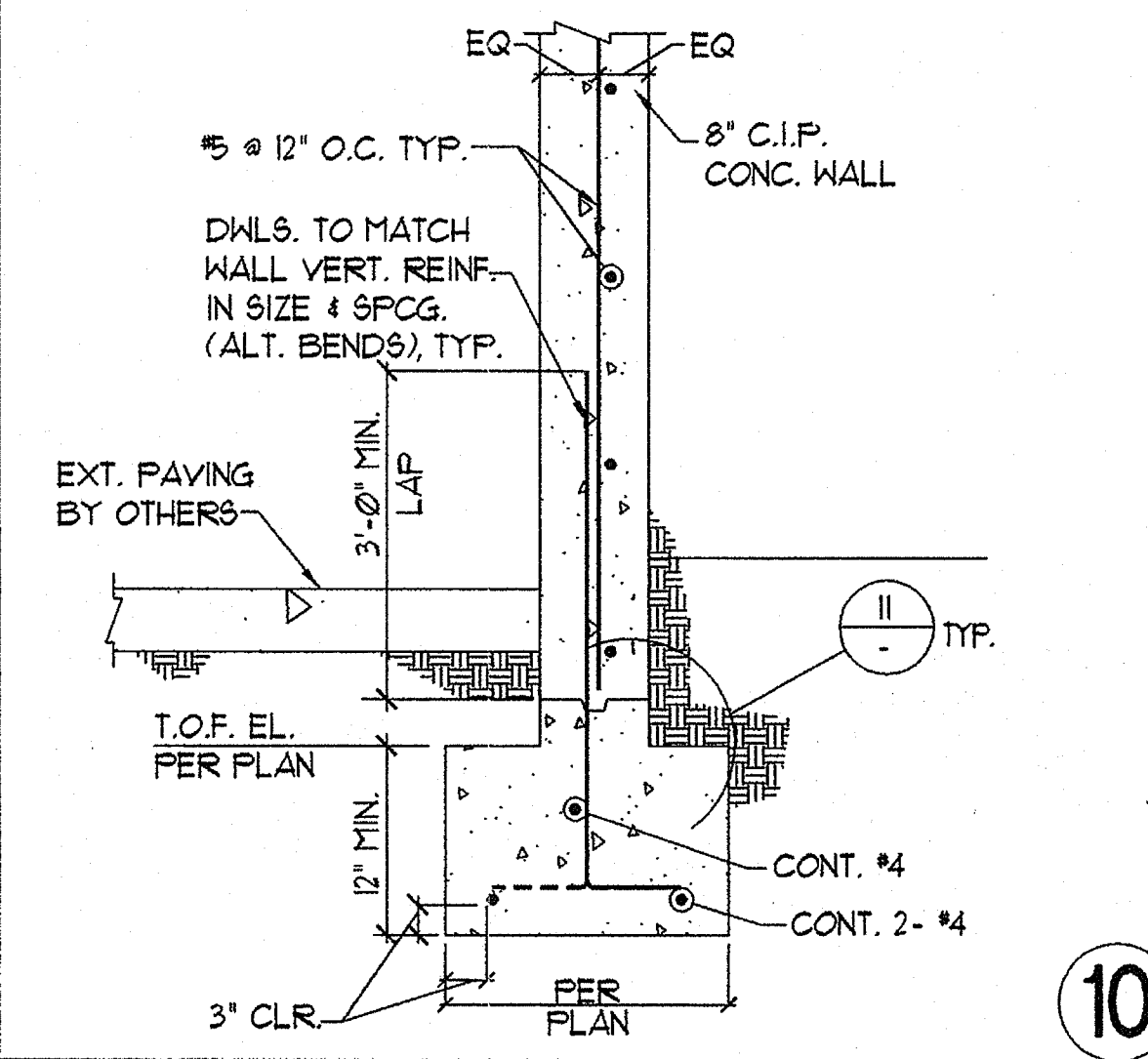
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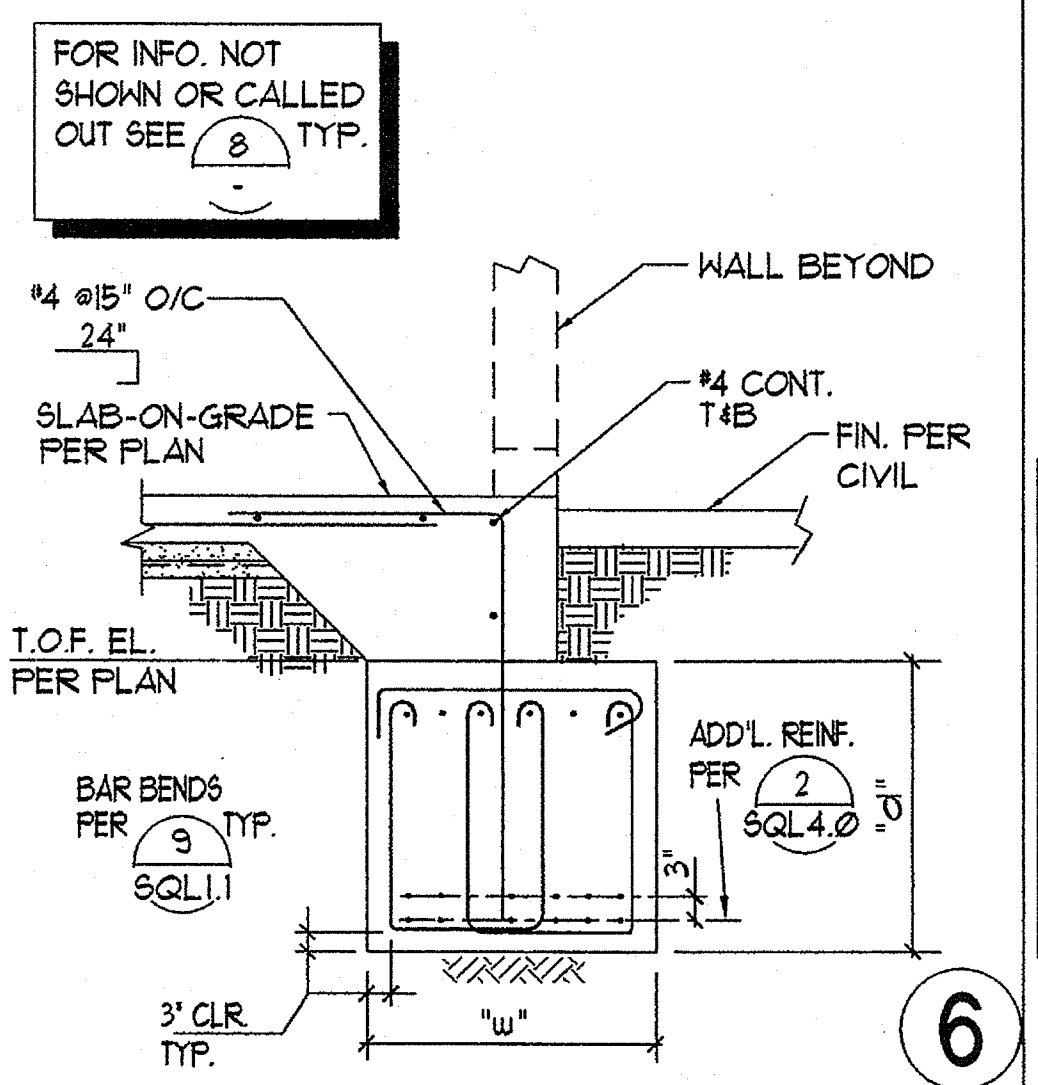
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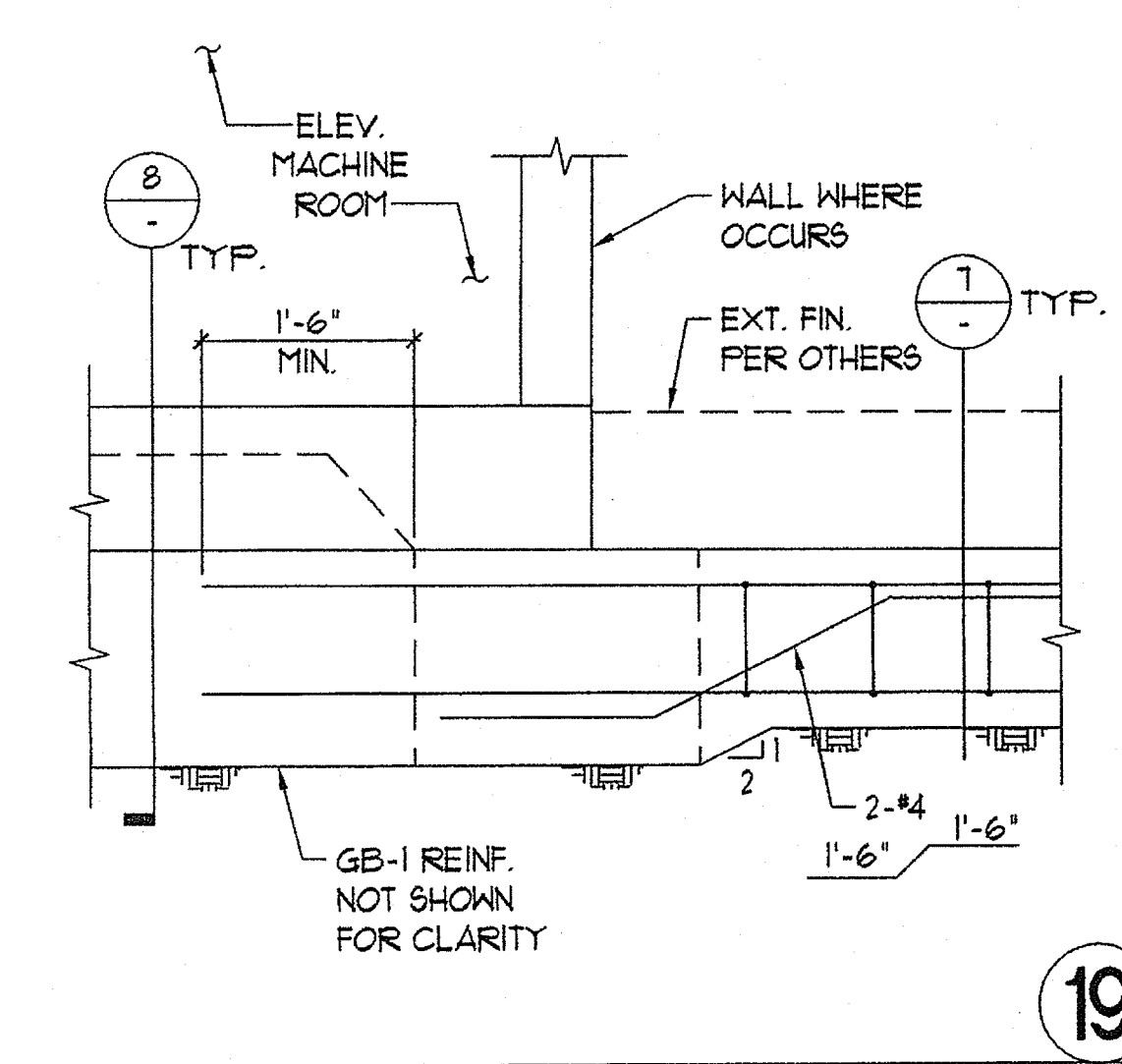
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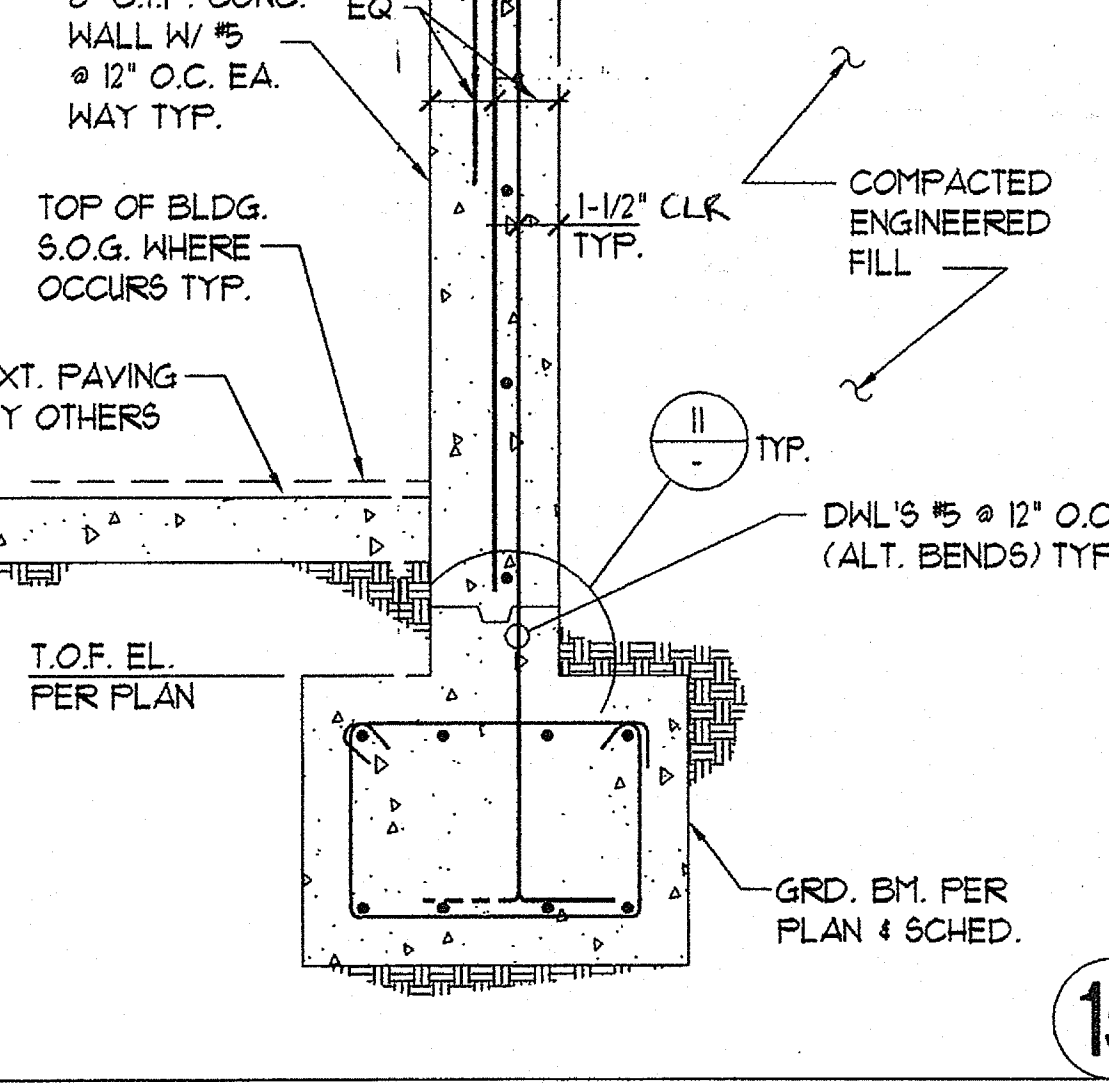
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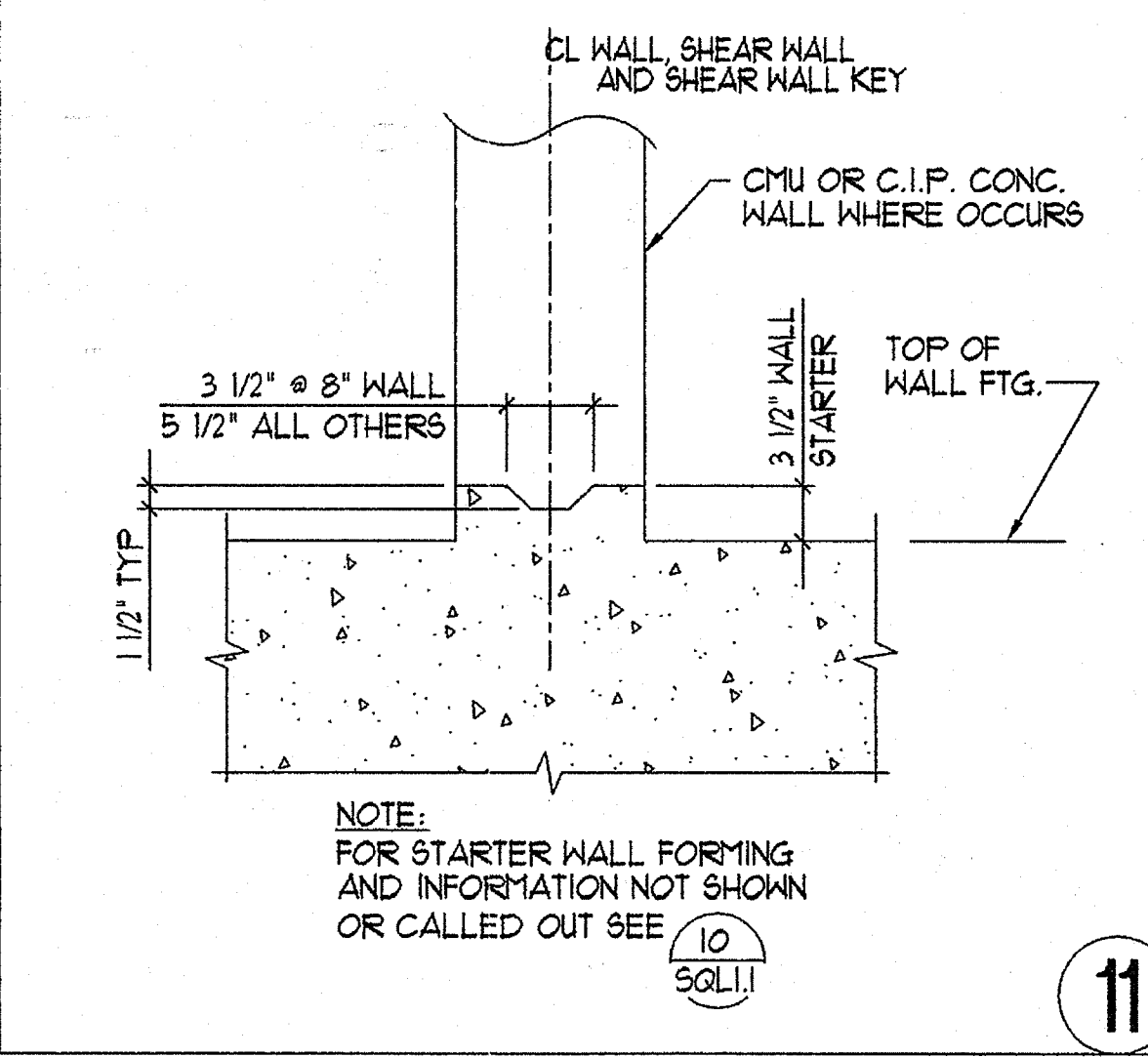
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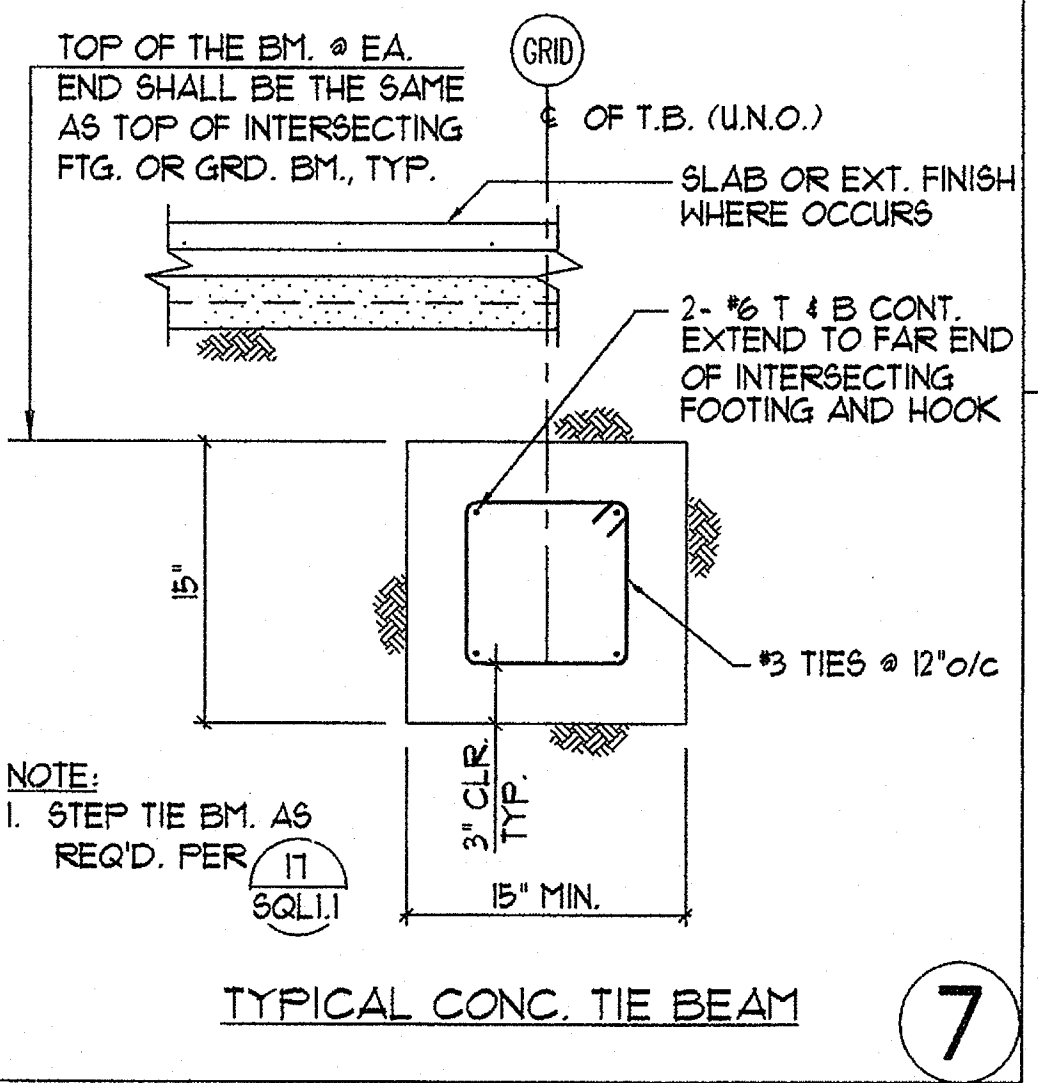
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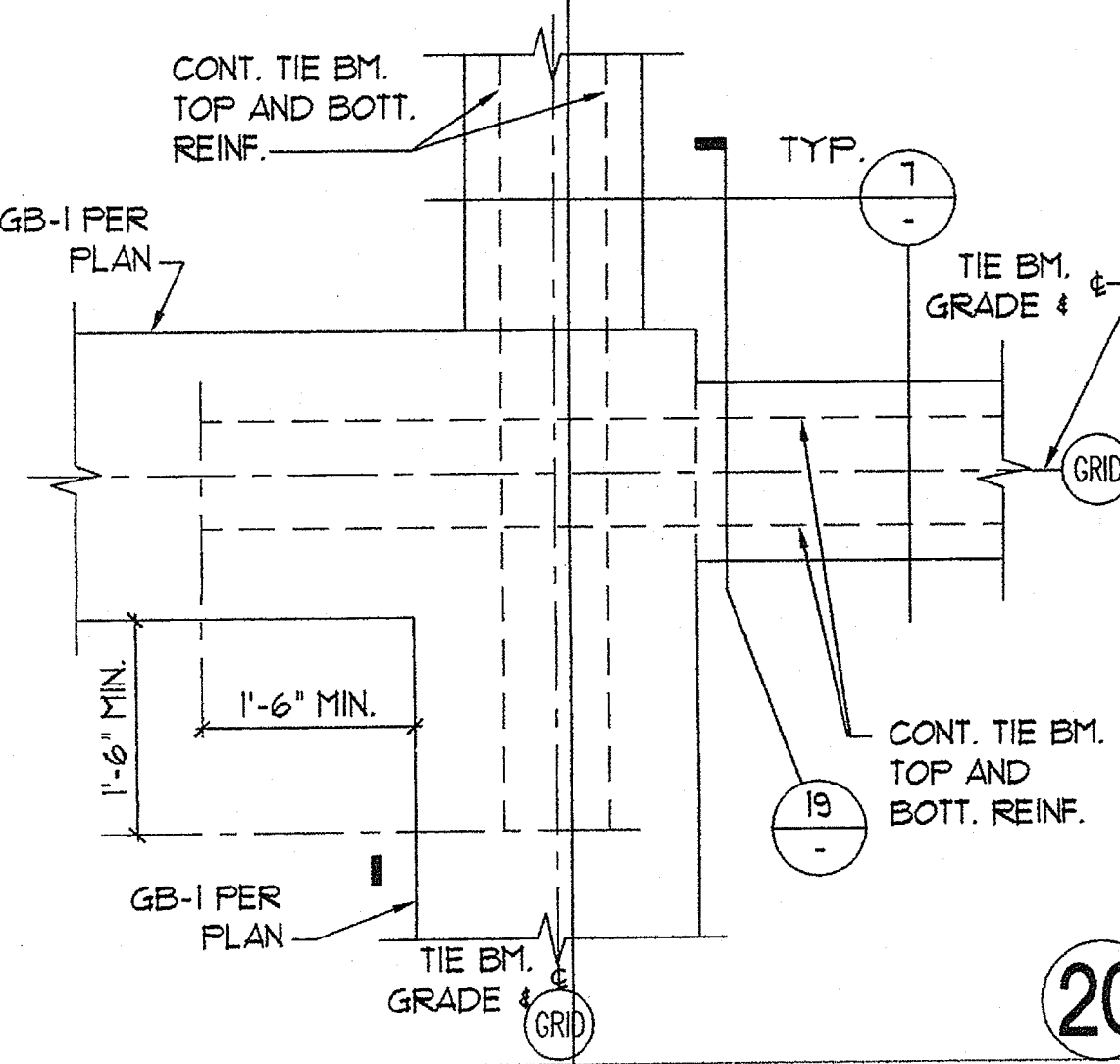
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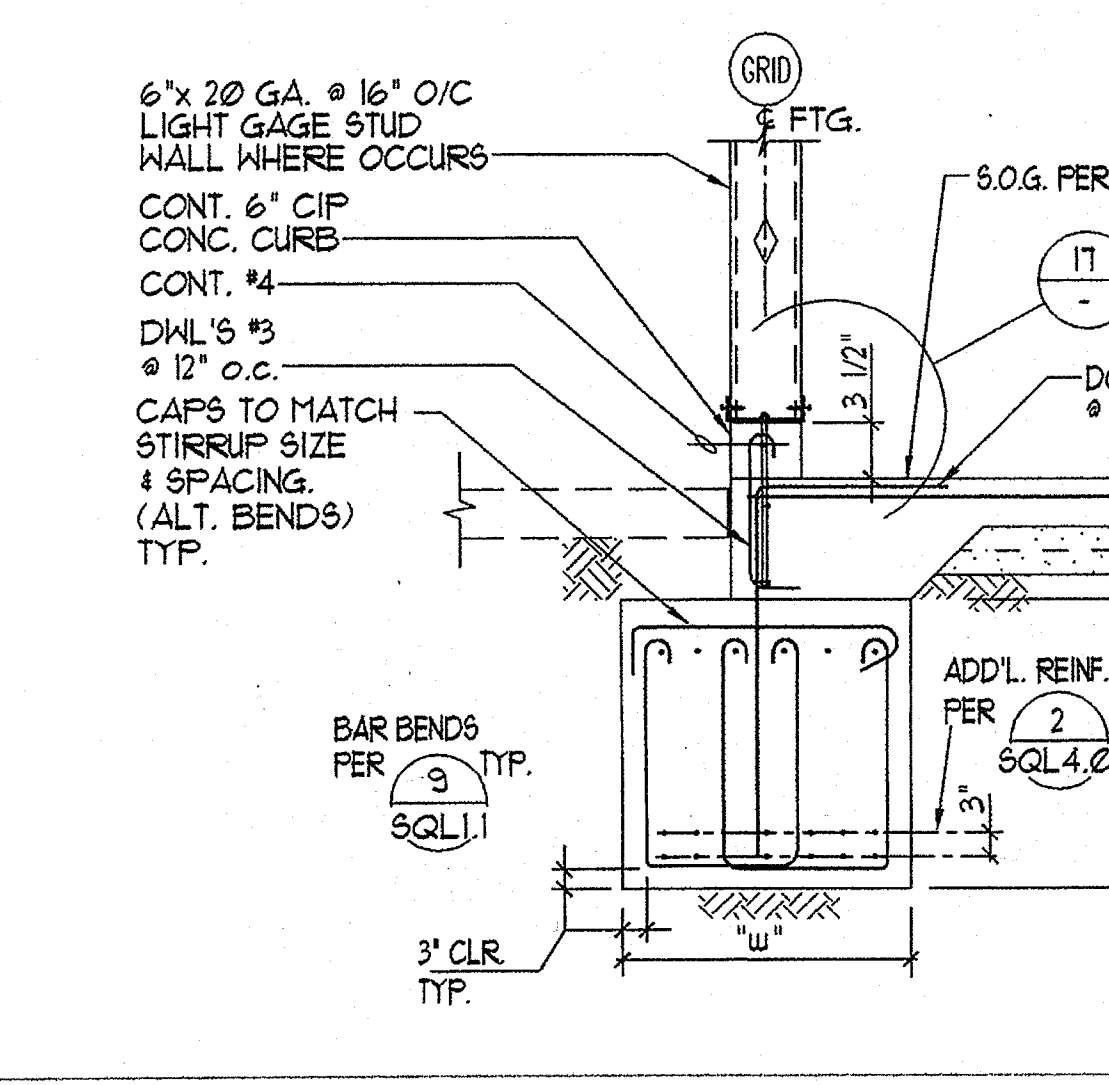
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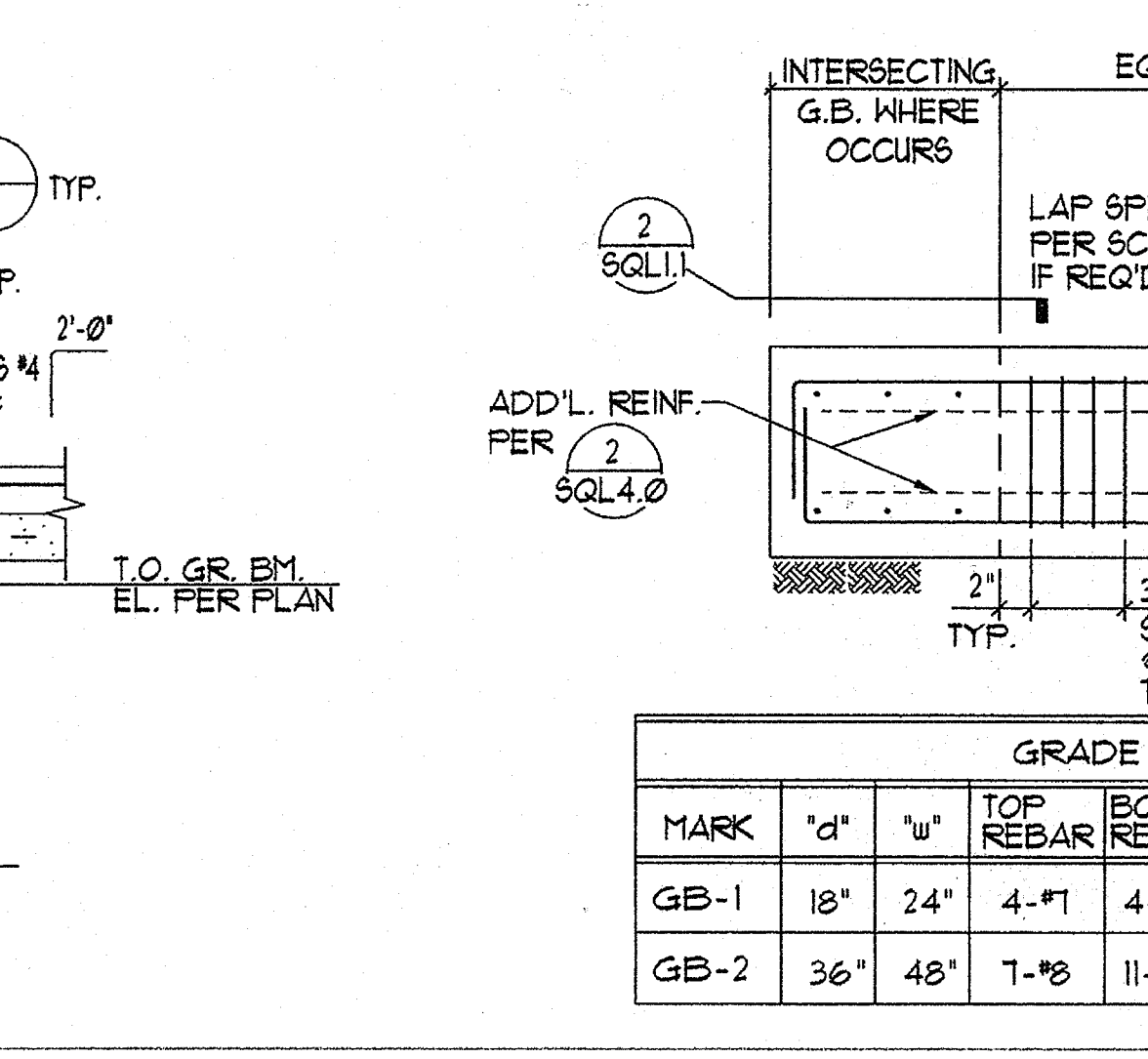
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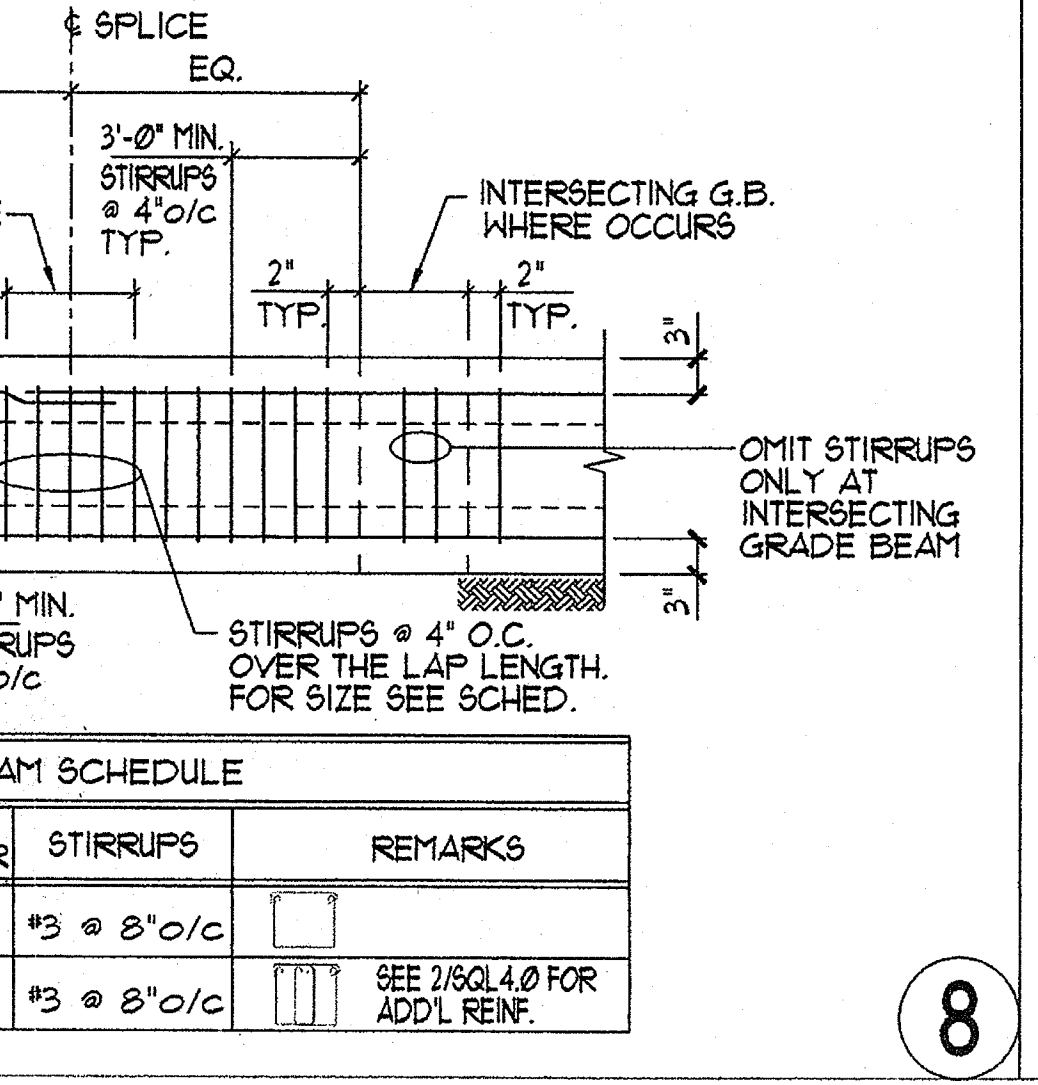
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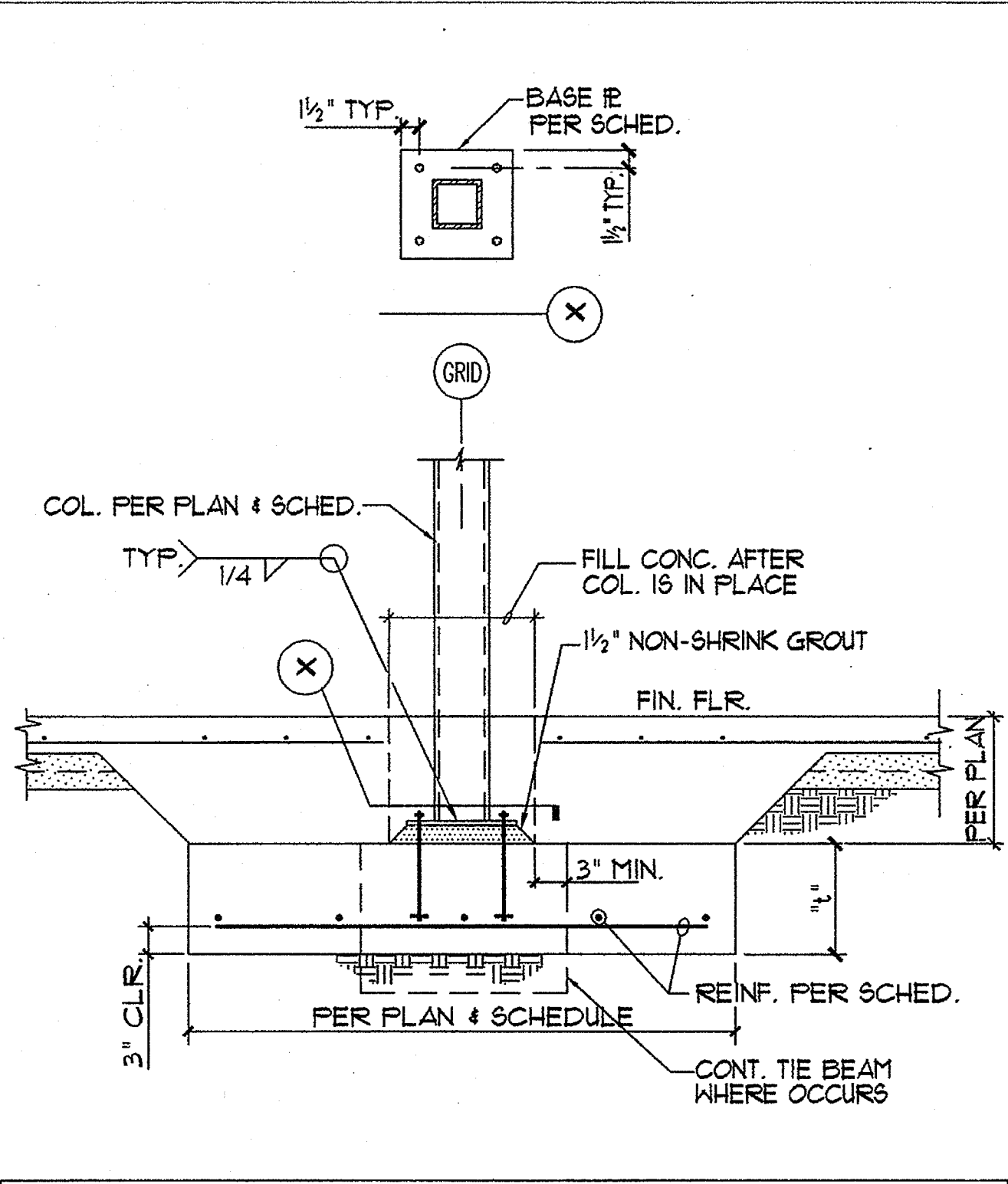
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5



4



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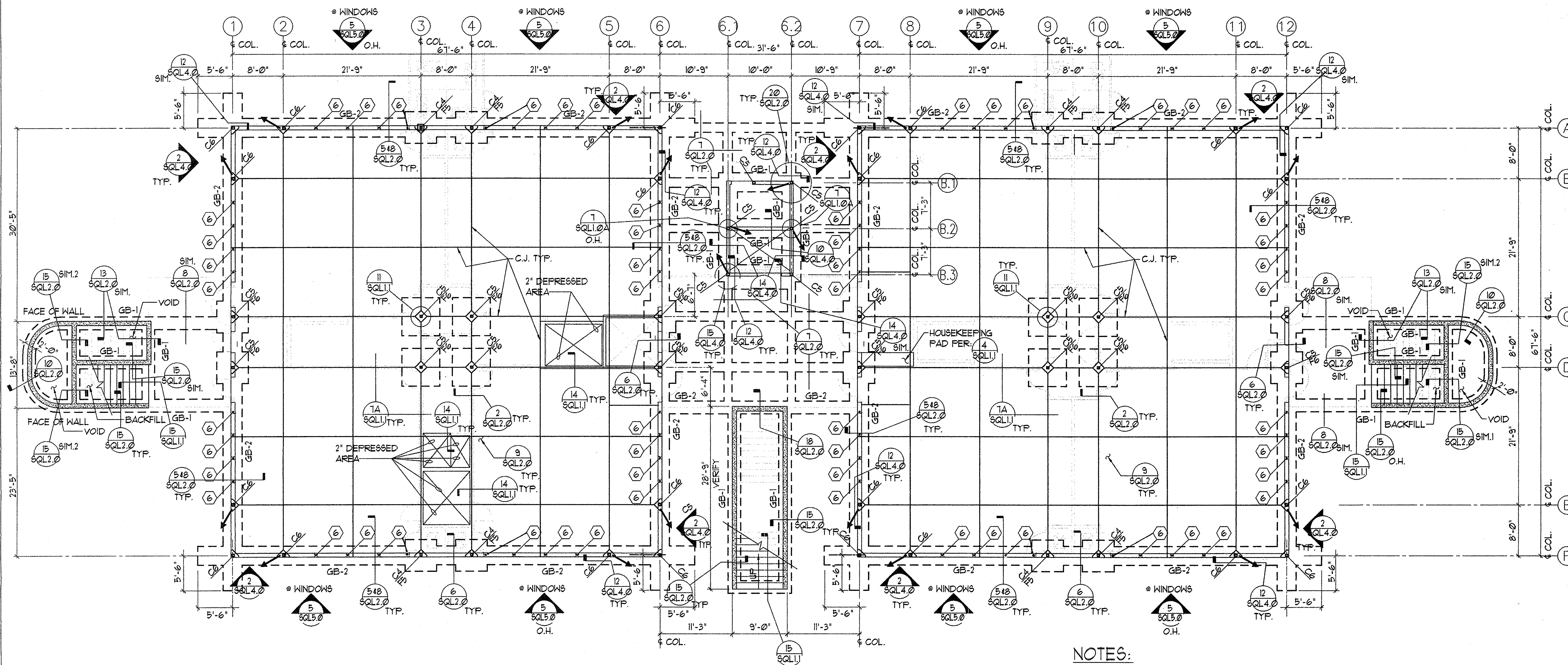
COLUMN SCHEDULE

MARK	SIZE	TYP. BASE RE AND A.B.'S U.N.O.	REMARKS
C4	HSS 4x4x1/4	IE 3/4"x11"x11" w/ 4-3/4"x3" EMBED. A.B.'S	
C5	HSS 5x5x1/4	IE 7/8"x12"x12" w/ 4-3/4"x12" EMBED. A.B.'S	
C6	HSS 6x6x3/8	SEE DET'S 4 & 12 ON SQL4.0	
C6*	HSS 6x6x3/8	SEE DETAIL SQL4.0	

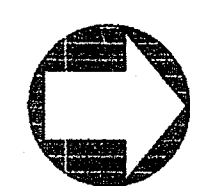
FOOTING SCHEDULE

MARK	SIZE	"	REINFORCING STEEL	REMARKS
F4	4'-0" x 4'-0"	12"	5-#5 EA. WAY	
F5	5'-0" x 5'-0"	12"	6-#6	
F6	6'-0" x 6'-0"	15"	7-#6	

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823 ACACIA STREET
OCEANSIDE CA 92054
OCEANSIDE UNIFIED S.D.
GROTH ARCHITECTS, INC. 3355 MISSION AVE. SUITE 234 OCEANSIDE CALIFORNIA 92054
PHONE 760-754-8191
FAX 760-754-8291
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JOHN SCOTT BOHL
C-26604
4/30/2007
RENEWAL
STATE OF CALIFORNIA
SHEET TITLE
FOUNDATION DETAILS
SQL2.0
FLC FLORES LUND CONSULTANTS
PROFESSIONAL ENGINEERS
2220 TRADE STREET, SUITE 200, SAN DIEGO, CALIFORNIA 92108
(619) 566-6650 FAX (619) 566-9227

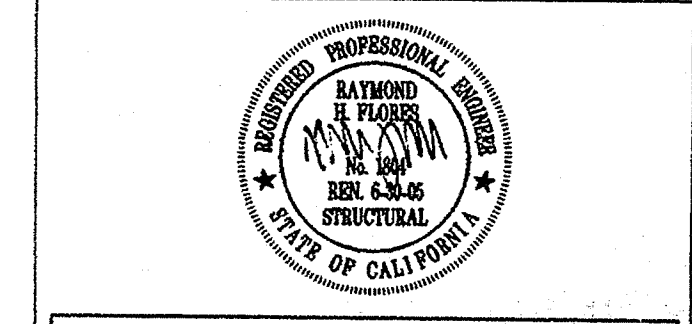
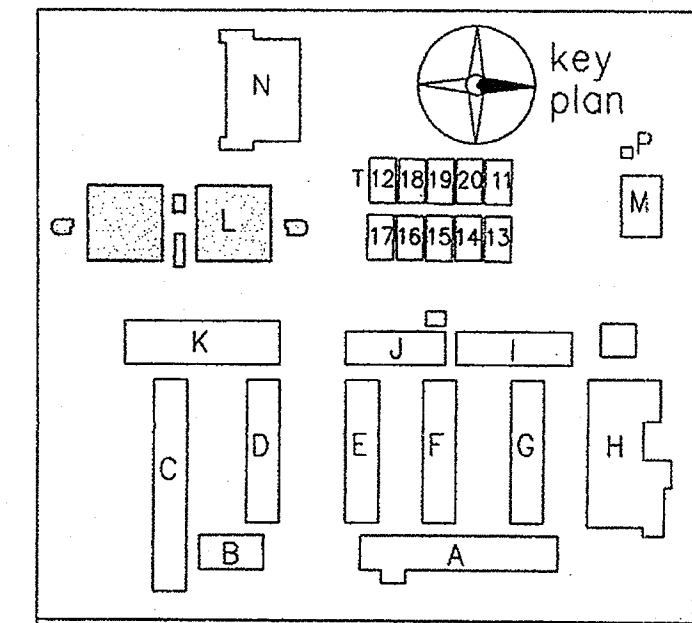


FOUNDATION PLAN
1/8" = 1'-0"



NOTES:

- ① TOP OF CONCRETE SLAB-ON-GRADE SHALL BE PER ARCH. DWG'S - DATUM ELEVATION = 119.5' REF ± 0'-0". TOP OF FTG. EL. & GRADE BM. SHALL BE 111.1'
- ② FOR TYP. CONCRETE SLAB-ON-GRADE SIZE AND REINFORCEMENT SEE $\frac{9}{SQL2.0}$ TYP.
- ③ $\frac{C4}{FB}$ DENOTES COLUMN AND FOOTING SIZE PER $\frac{2}{SQL2.0}$ TYP.
- ④ GB-1 DENOTES GRADE BEAM PER $\frac{8}{SQL2.0}$ TYP.
- ⑤ \nearrow DENOTES BRACED FRAME FROM THIS LEVEL TO THE NEXT PER $\frac{2}{SQL4.0}$ TYP.
- ⑥ \circ DENOTES BACK-TO-BACK 6" x 2" FLANGE x 16GA STEEL STUDS @ 4" OF WINDOW MULLION. CONNECT WITH 2-#10 S.M.S. @ 6" O/C TYP. STAGGERED.
- ⑦ FOR WINDOW FRAMING AT GRIDLINE:
① ⑥ ⑦ ⑧, AND AS SHOWN ON PLAN
* GRIDS A-F SEE ELEVATION ON $\frac{5}{SQL5.0}$



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OCEANSIDE UNIFIED S.D.

GROTH ARCHITECTS, INC.
3355 MISSION AVE. SUITE 234
OCEANSIDE, CALIFORNIA 92054
PHONE 760-754-8191
FAX 760-754-8291

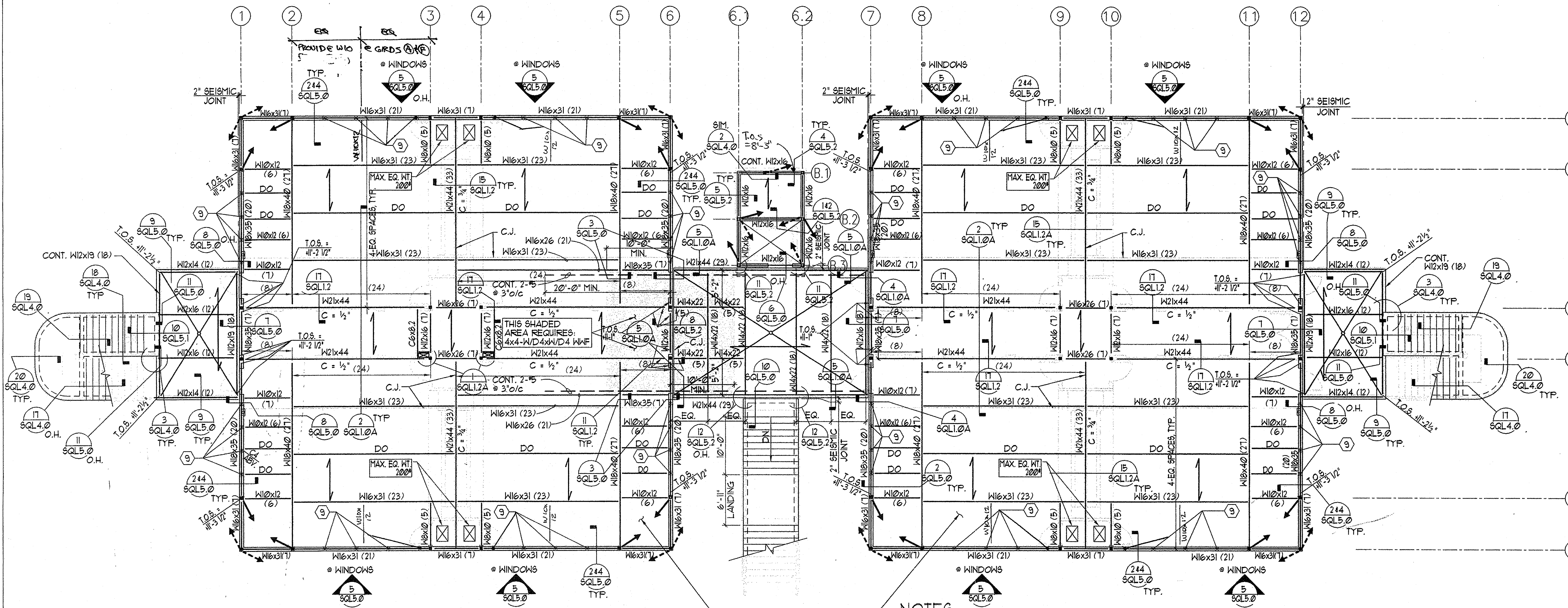
space art
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OFFICE OF REGULATION SERVICES
4-106494
AC FLS SS
DATE MAR 28 2005

LICENSED ARCHITECT
JOHN SCOTT BOCH
C-26609
4/30/2007
RENEWED
STATE OF CALIFORNIA

FOUNDATION PLAN

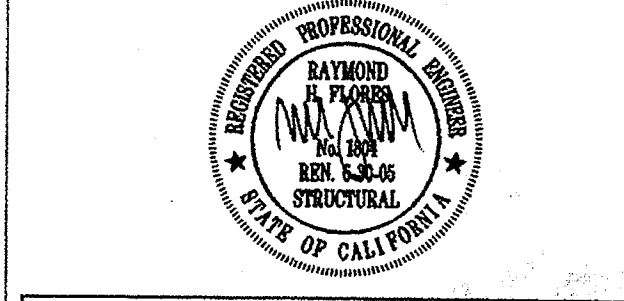
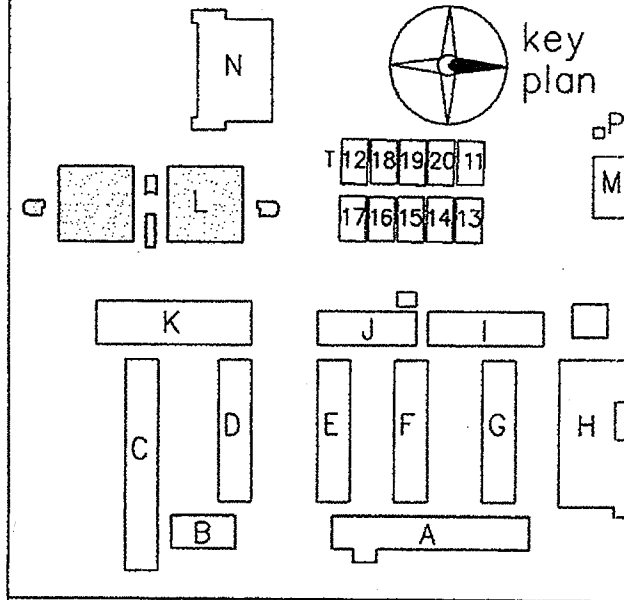
SQL3.0



2 1/2" REG. WT. CONC.
W#4 @ 15" o.c. EA. WAY
& SYNTHETIC FIBER
MESH OVER 2"x2" GA
STL. DECK FER #16
SQL1.2

NOTES:

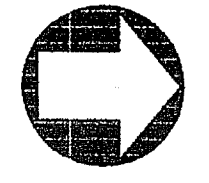
- 1 TOP OF CONC. FIN. EL. = 4'-11"-8". FOR EXACT ELEVATION OF TOP OF CONC. FIN. BTWN. GRID LINE (C) & (D) SEE ARCH'L DWG'S.
- 2 T.O.S. EL. = 4'-11"-3 1/2" TYP. (U.N.O.)
- 3 DENOTES DIRECTION OF STL. DECK FLUTES.
- 4 FOR STL. DECK PROPERTIES & WELDING SEE 14.416, SQL1.2
- 5 DENOTES DIRECTION OF DIAGONAL BRACED FRAME BELOW.
- 6 DENOTES DIRECTION OF DIAGONAL BRACED FRAME ABOVE.
- 7 FOR TYP. BM. TO BM. & BM. TO COL. CONNECTIONS SEE 2, SQL1.2
- 8 FOR TYP. CONN. ALONG GRID LINES 1, 6, 7, 12, (A), & (F) SEE 11, SQL1.2
- 9 DENOTES BACK-TO-BACK 6" x 2" FLANGE x 16GA STEEL STUDS ABOVE AND BELOW & OF WINDOW MULLION CONNECT WITH 2-#10 S.M.S. @ 6" O/C TYP. STAGGERED.
- 10 FOR WINDOW FRAMING AT GRIDLINE: 1, 6, 7, 12, AND AS SHOWN ON PLAN @ GRIDS (A) & (F) SEE ELEVATION ON 5, SQL5.0
- 11 (20) DENOTES NUMBER OF SHEAR STUDS PER 349, SQL1.2A
- 12 DENOTES DRAIN PER ARCH'L DWGS. CONC. TOPPING @ DRAIN LOCATION SHALL BE 2 1/2" MIN.
- 13 MAXIMUM TOPPING THICKNESS ABOVE STL. DECK = 5" BETWEEN GRIDS (4) AND (9) = 3-1/2" ALL OTHER AREAS
- 14 C=1/8" DENOTES CAMBER UP IN STEEL BEAM
- 15 FOR DEEP BEAM TO SHALLOW GIRDER CONN. SEE: 3, SQL5.2
- 16 CONTRACTOR NOTE: THE MAJORITY OF THE FLOOR BEAMS ARE SLOPED. MATCH TOP OF STEEL OF INTERSECTING BEAMS.
- 17 PROVIDE SHORING FOR DECK WHERE DECK SPANS ARE 7'-4" OR GREATER W/ A CONC. TOPPING 3" THICKER THEN 3/4" ABOVE THE TOP FLUTE'S

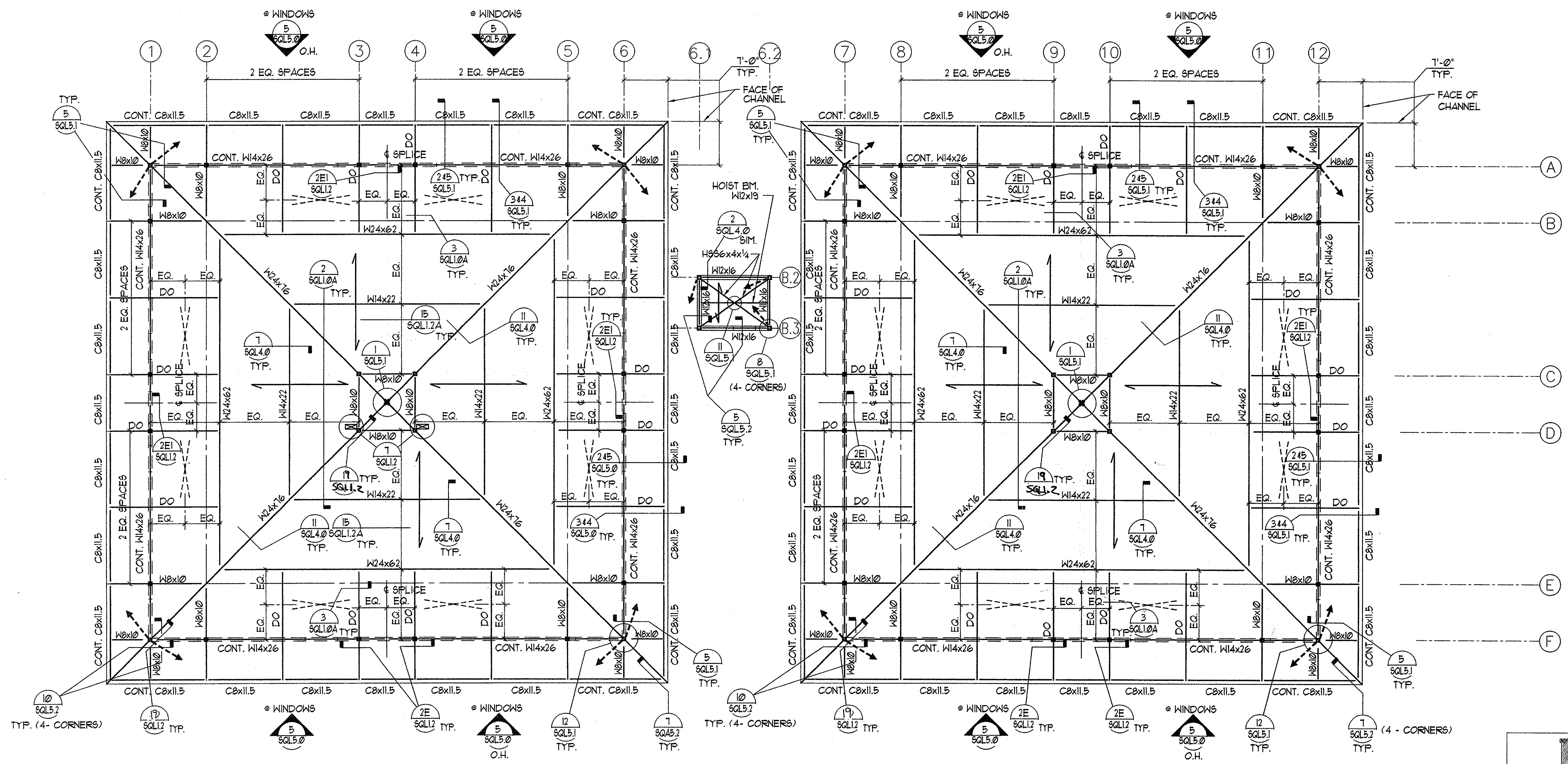


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SECOND FLOOR FRAMING PLAN

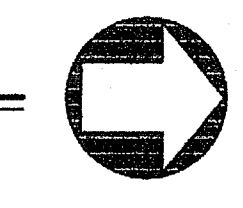
1/8" = 1'-0"





ROOF FRAMING PLAN

1/8" = 1'-0"

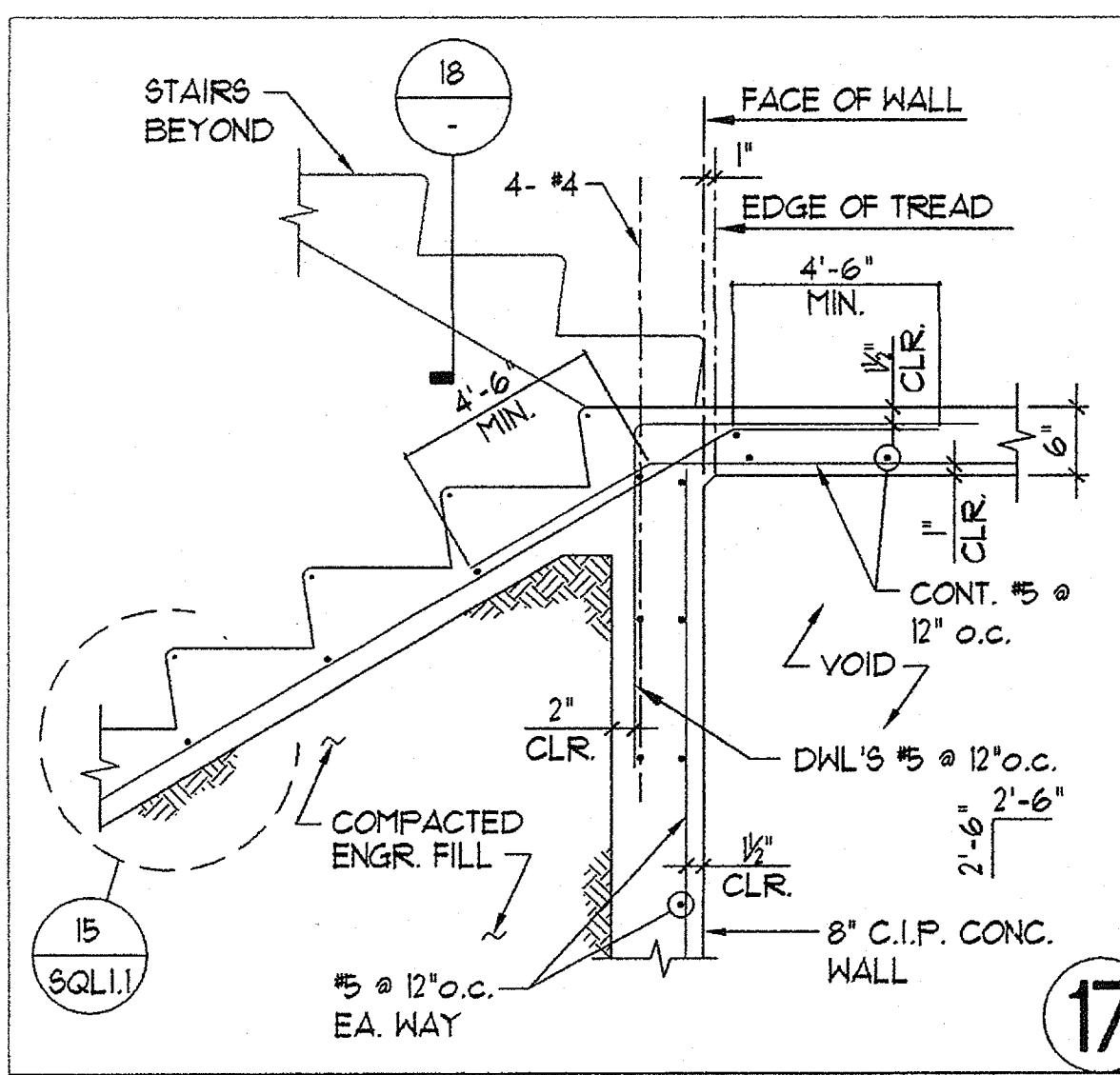


- NOTES:
- ← DENOTES DIRECTION OF STL. DECK FLUTES. FOR SIZE & WELDING SEE (3420) SQL1.2
 - DENOTES DIAGONAL BRACED FRAME BELOW PER PLAN AND ELEVATIONS FOR T.O.S. ELEVATION SEE ARCH'L DWGS.
 - FOR TYPICAL BM. TO BM. AND/OR BM. TO COL. CONN. SEE (2) SQL1.2
 - ROOF OPENINGS IF ANY SHALL BE COORDINATED WITH ARCH'L & MECH'L DWGS FOR SIZE & LOCATION. OPENING FRAME PER (3) SQL1.2 (1) TYP.
 - FOR WINDOW FRAMING AT GRIDLINE: (1) (6) (7) (12) AND AS SHOWN ON PLAN * GRIDS (A) & (F) SEE ELEVATION ON (5) SQL5.0
 - TOP OF STL. EL. (T.O.S.) ALONG GRID LINES (1) (6) (7) (12) (A) AND (F) = 22'-11"
 - TOP OF STL. EL. (T.O.S.) ALONG GRID LINES (6) (7) (12) AND (3) = 25'-2-1/2"
 - DENOTES BM. BOTT. FLG. ANGLE BRACING PER (8) SQL1.0A

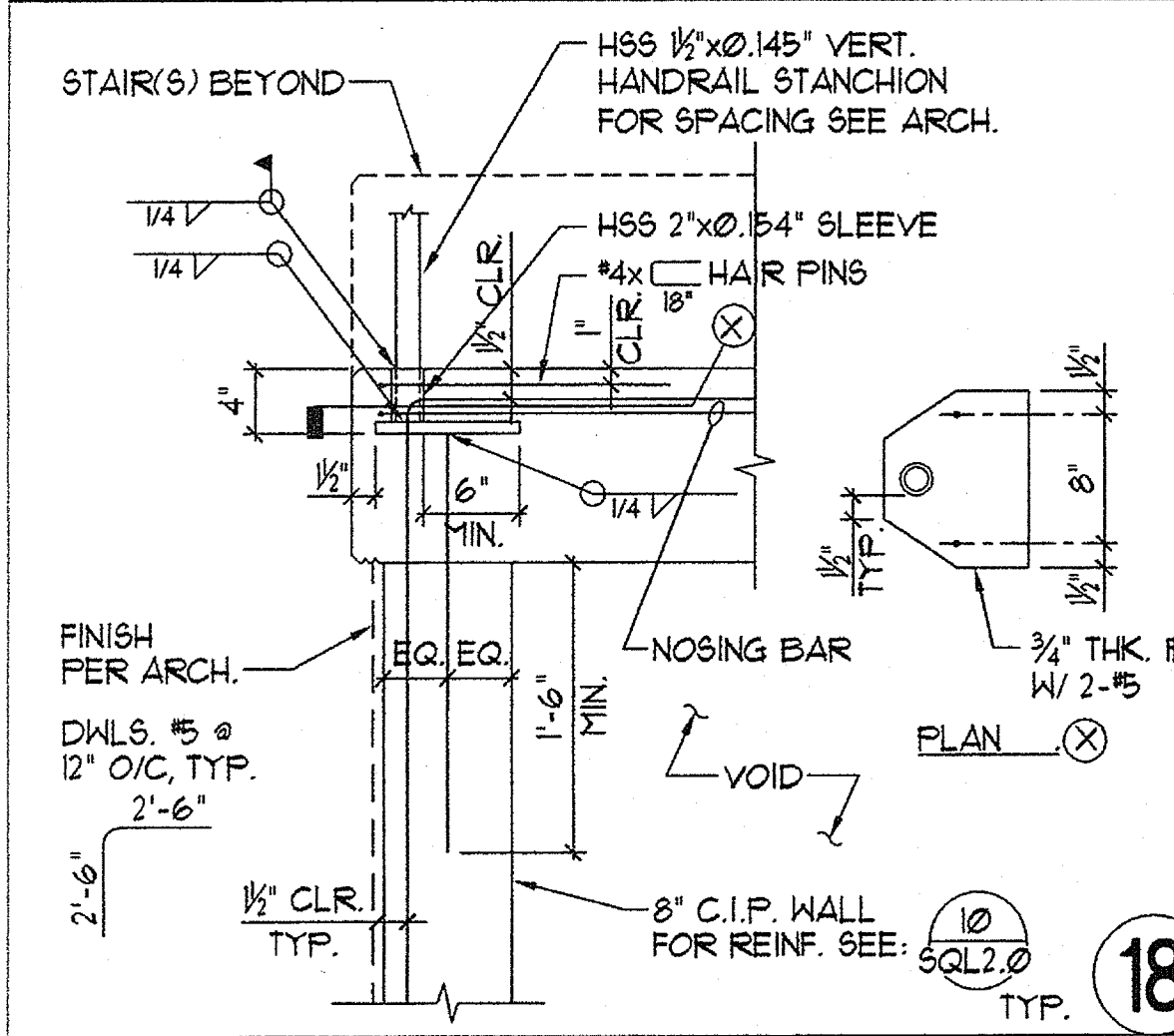
key plan

PROFESSIONAL ENGINEER
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 STATE OF CALIFORNIA

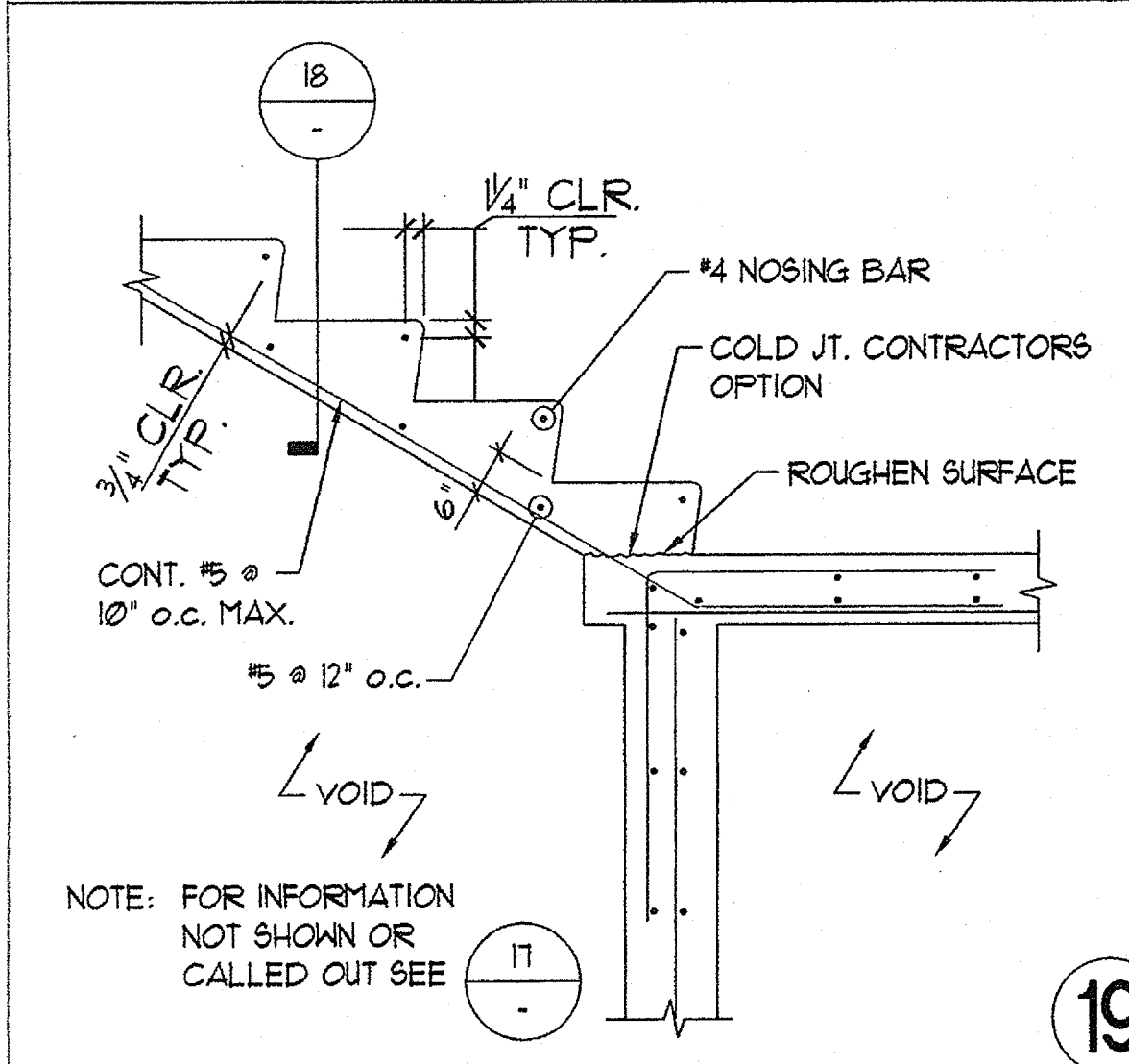
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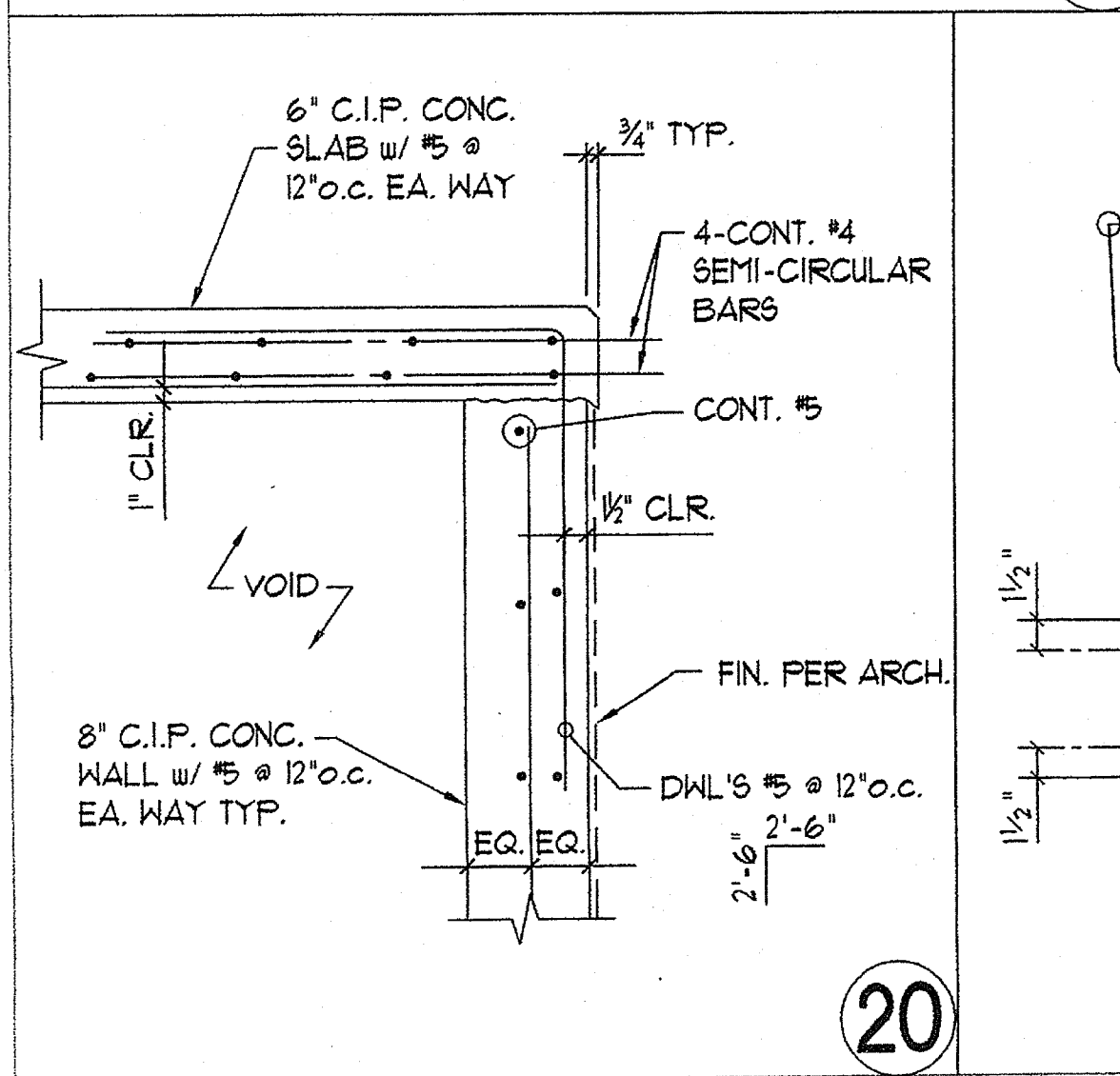
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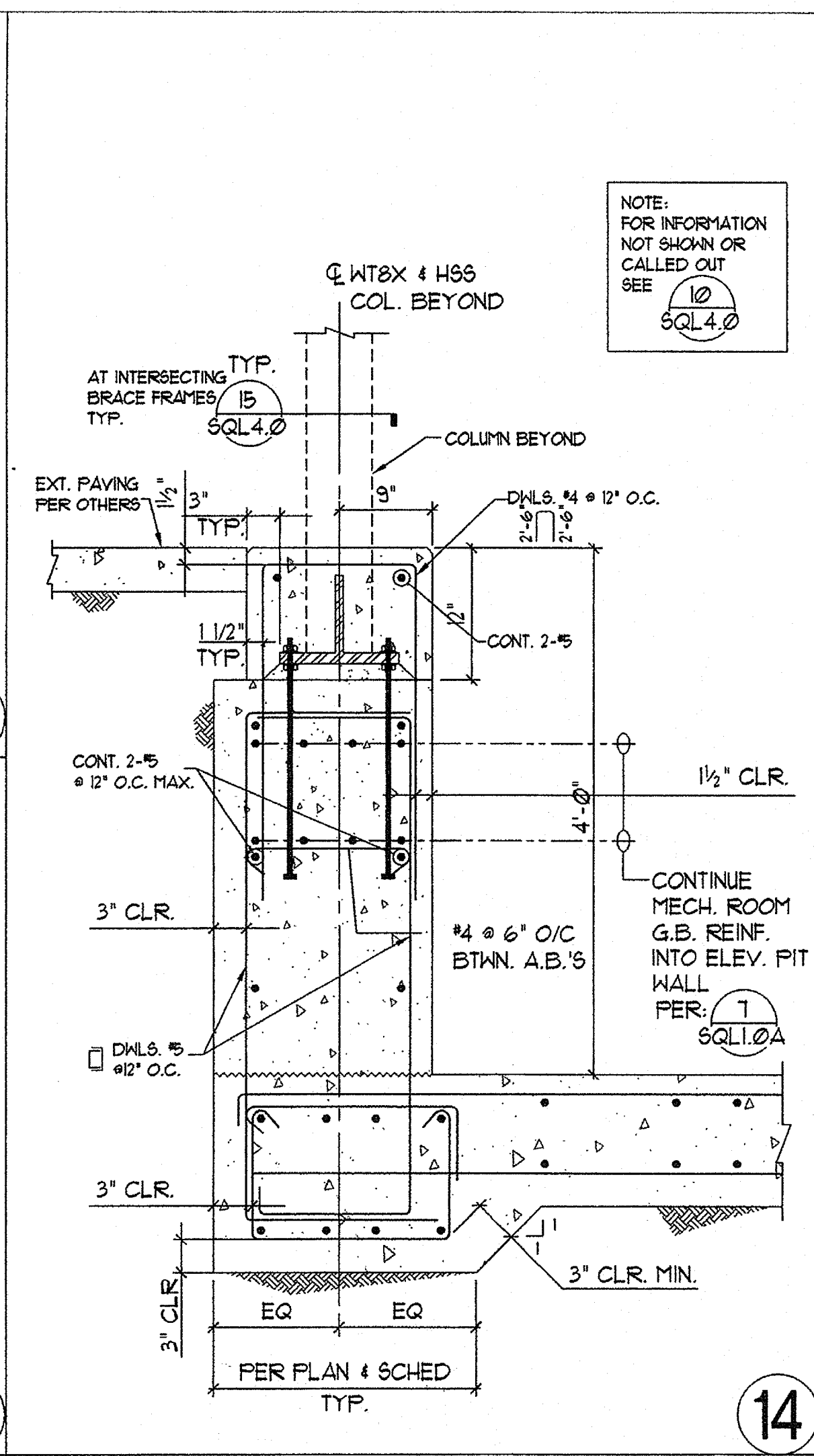
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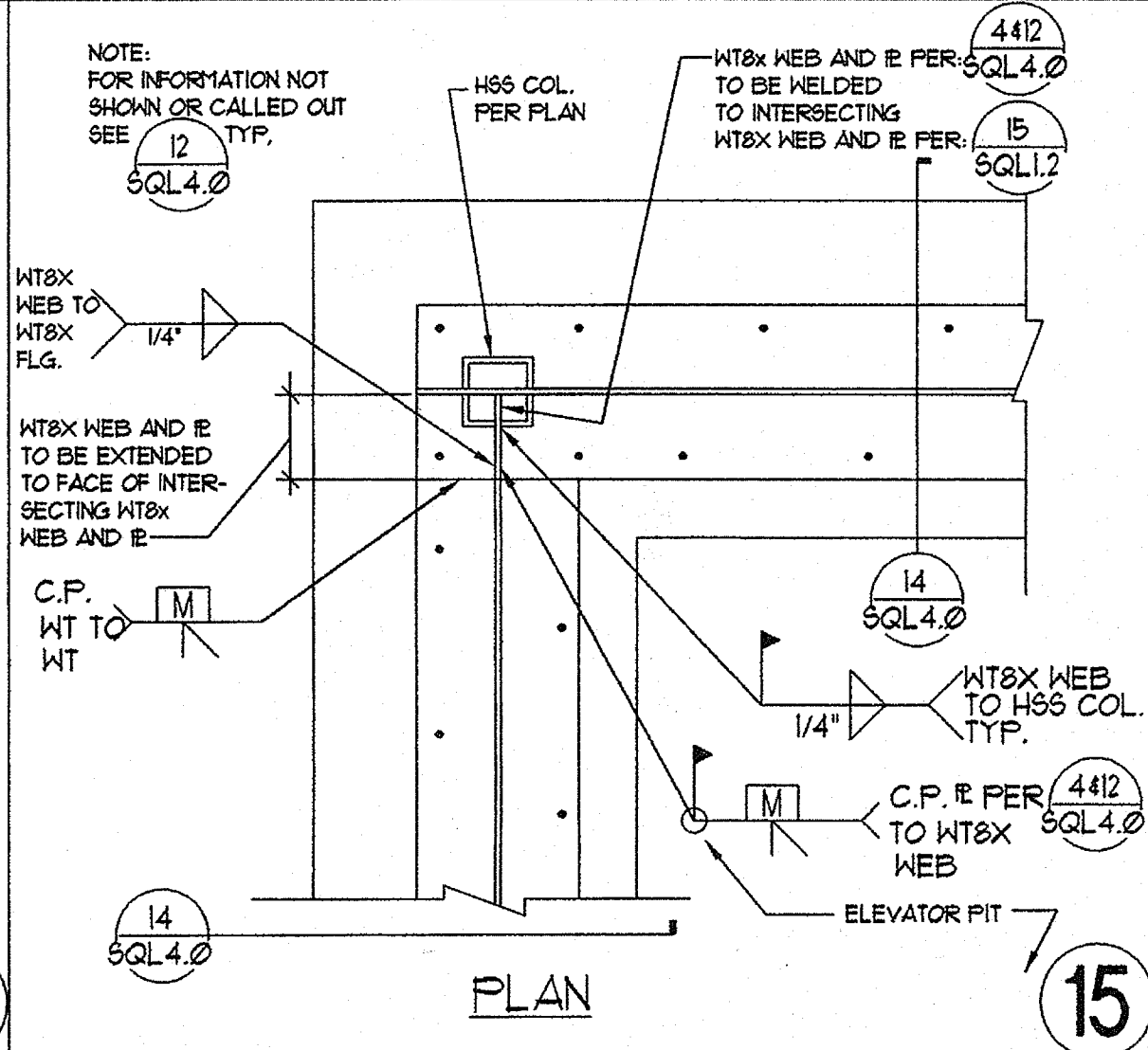
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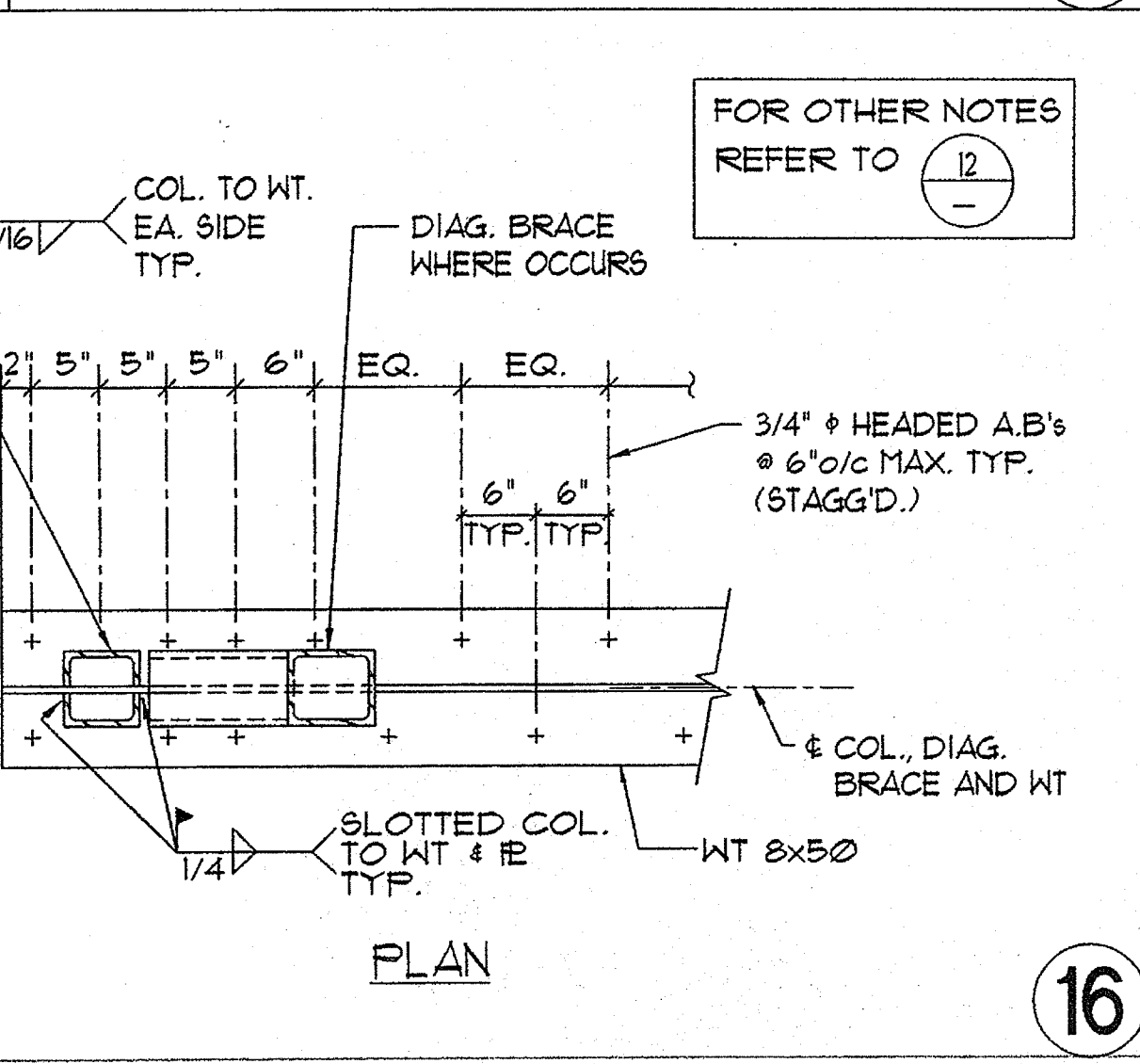
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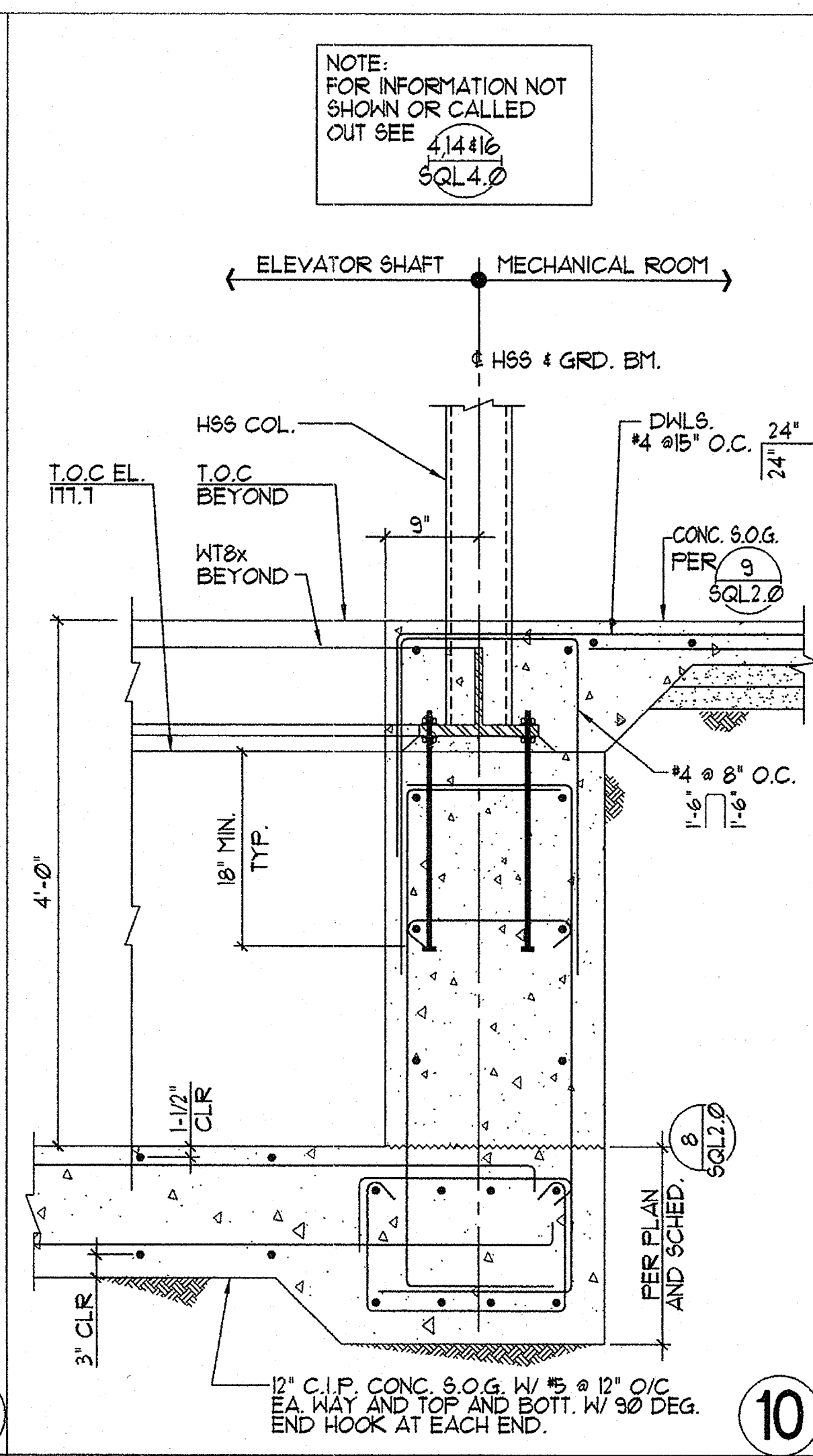
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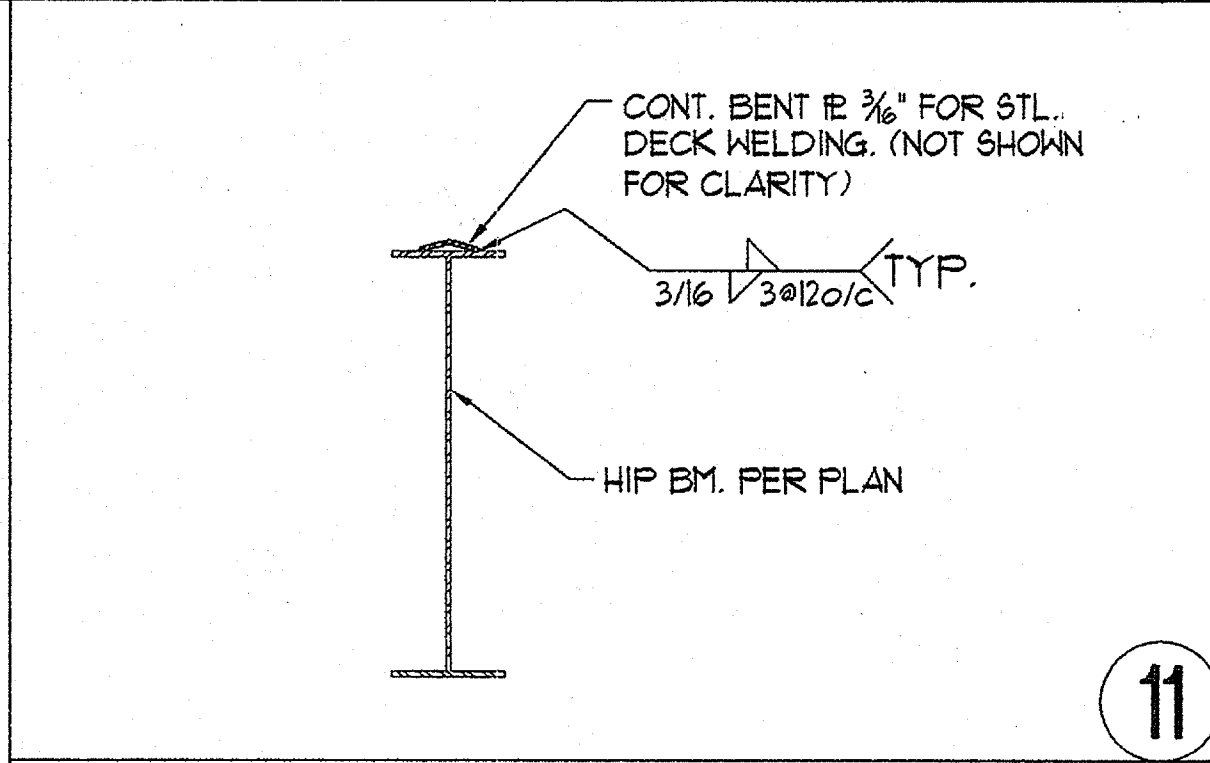
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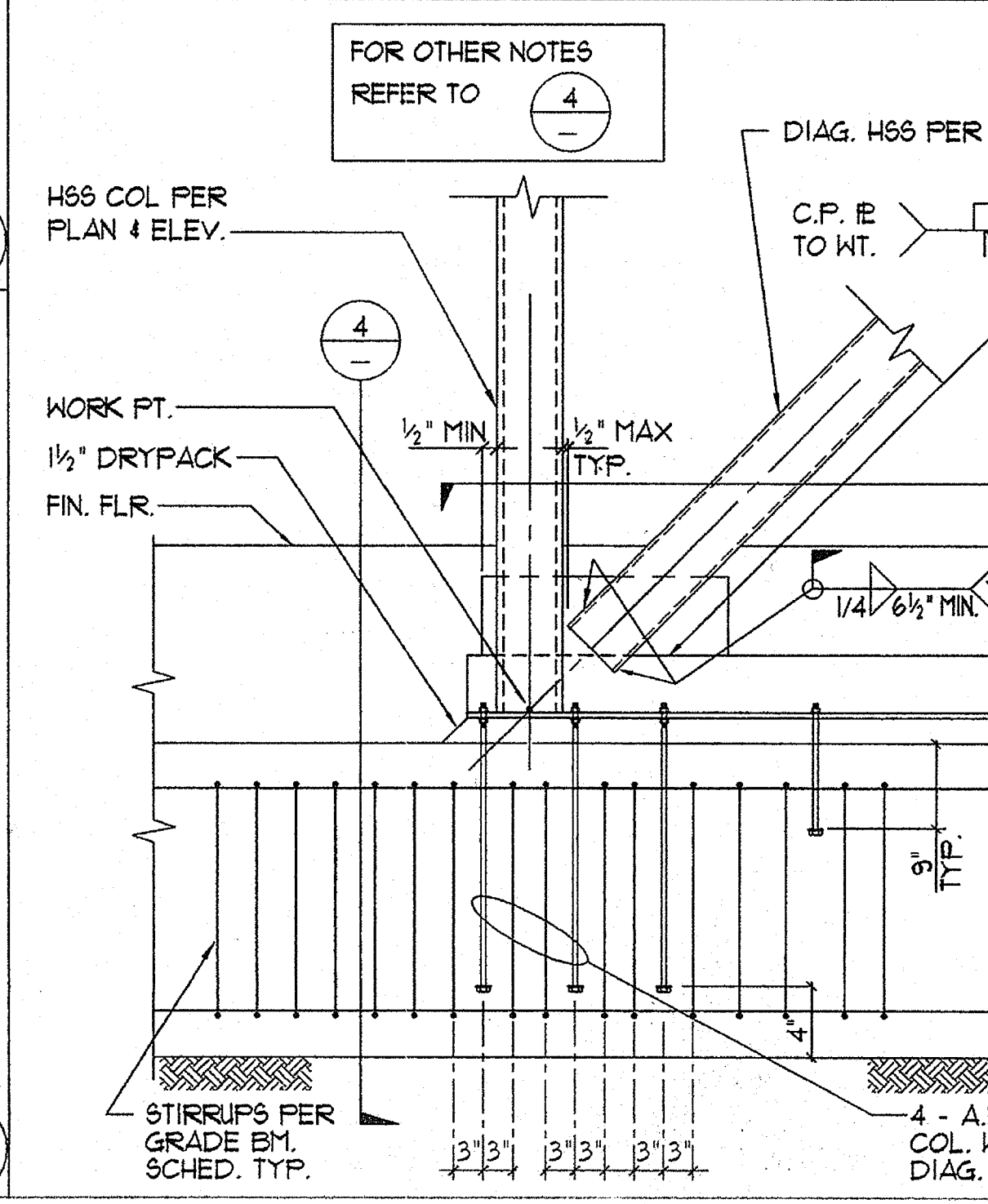
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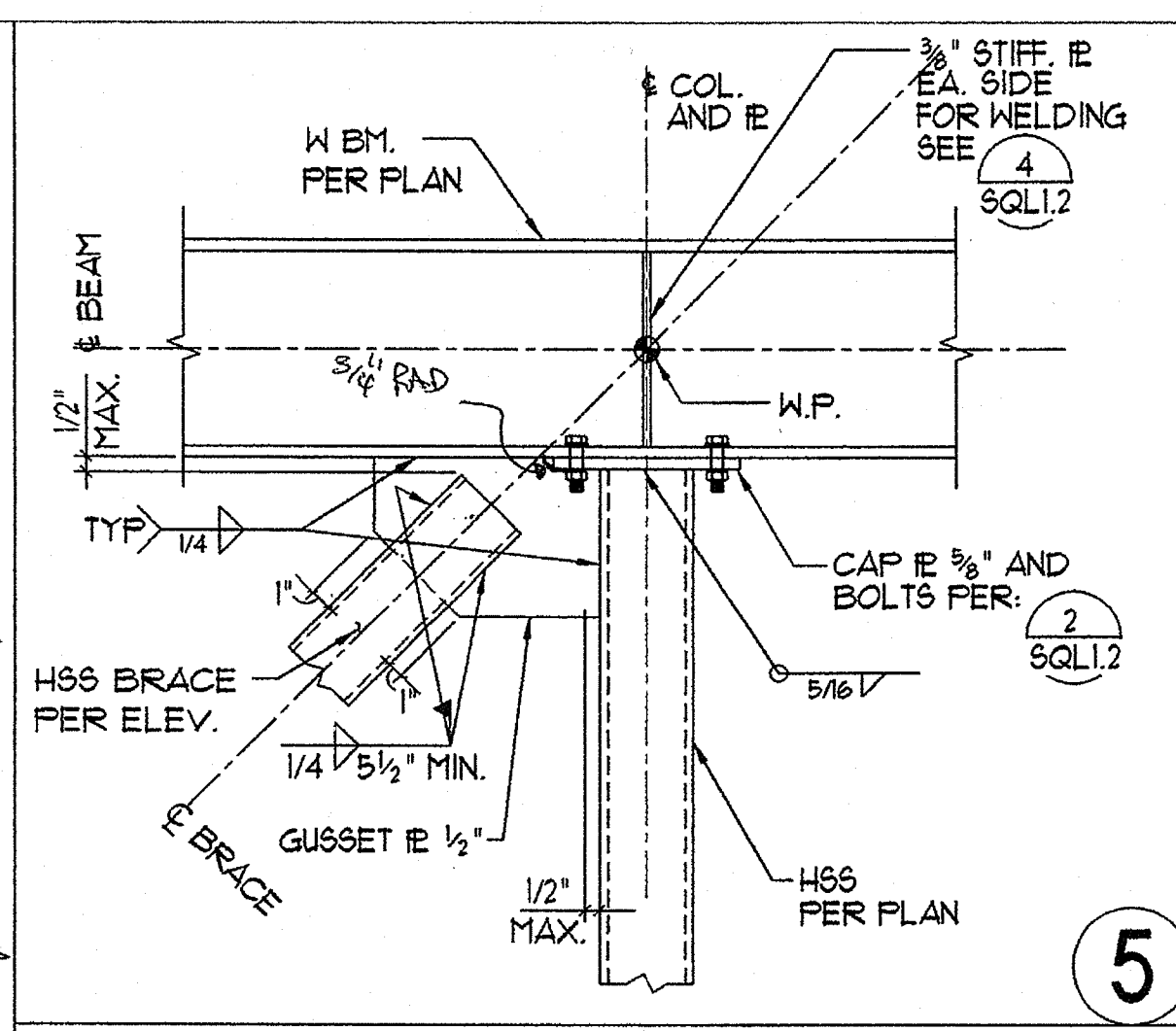
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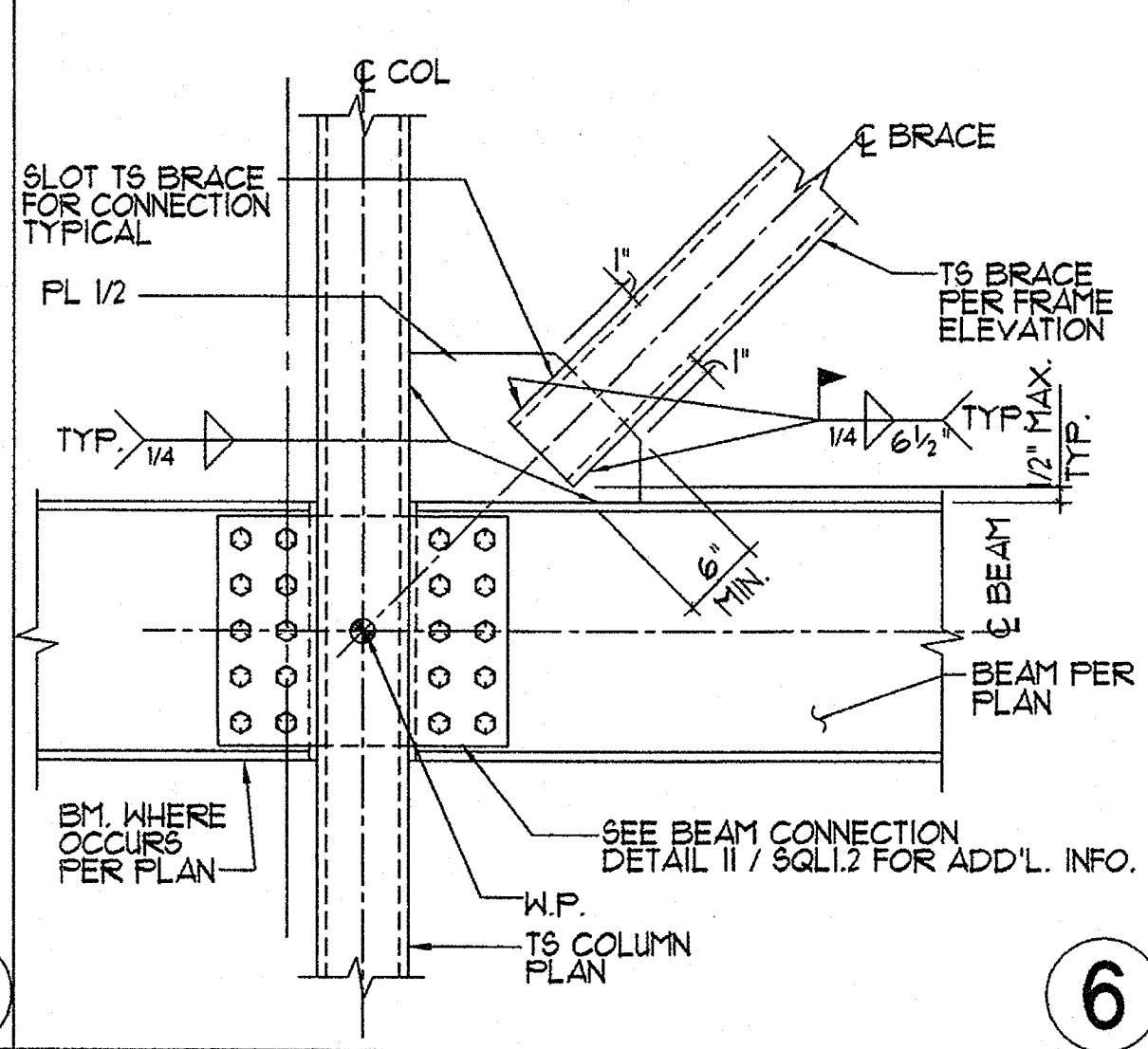
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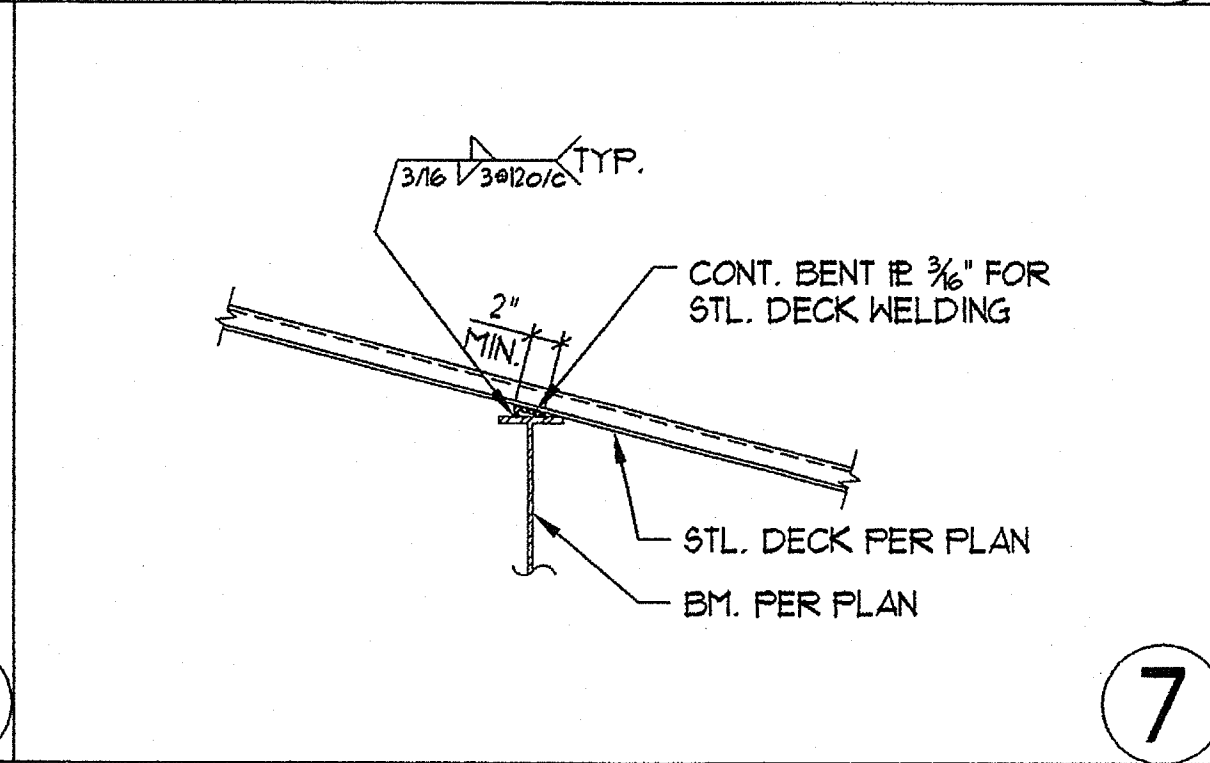
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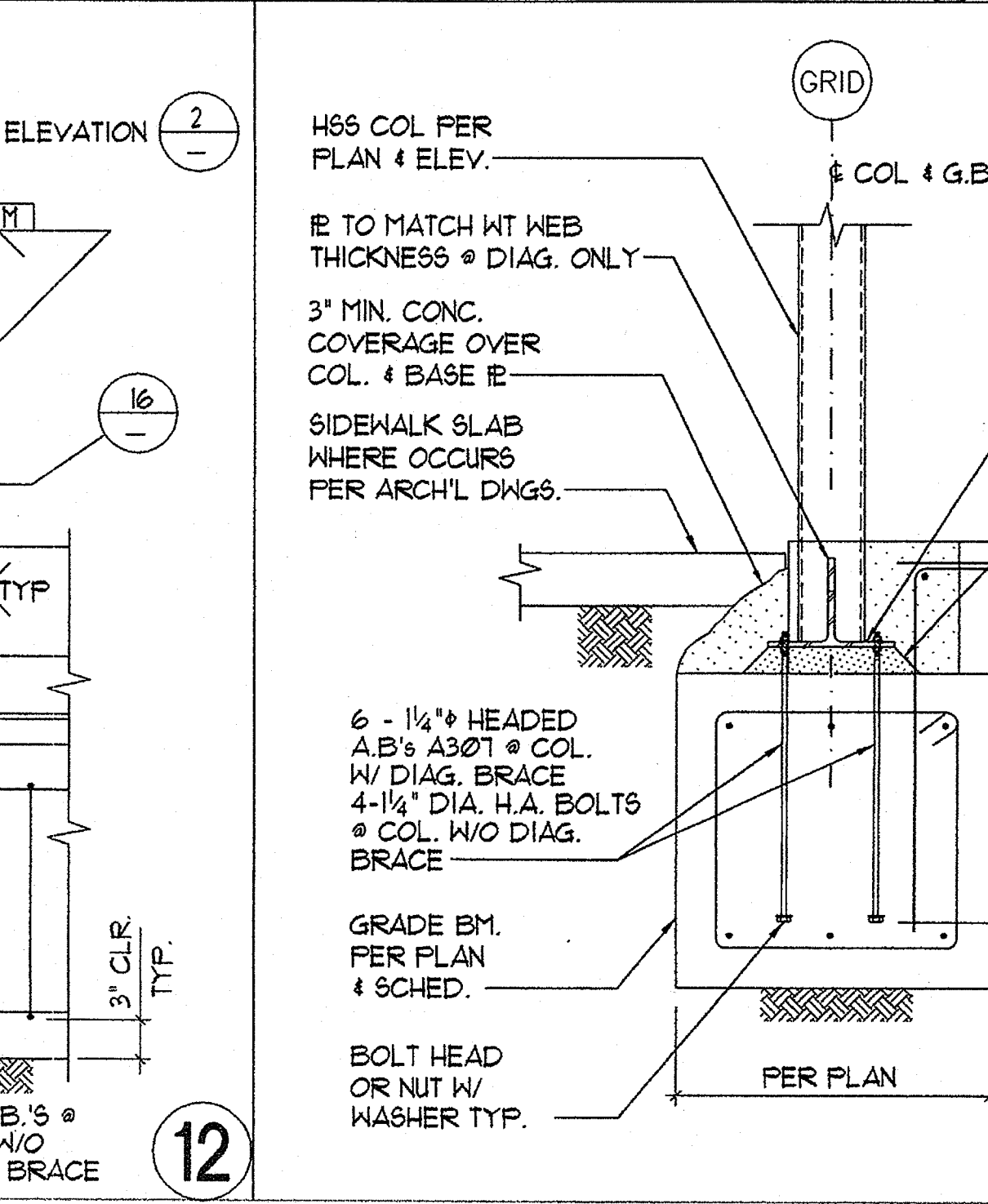
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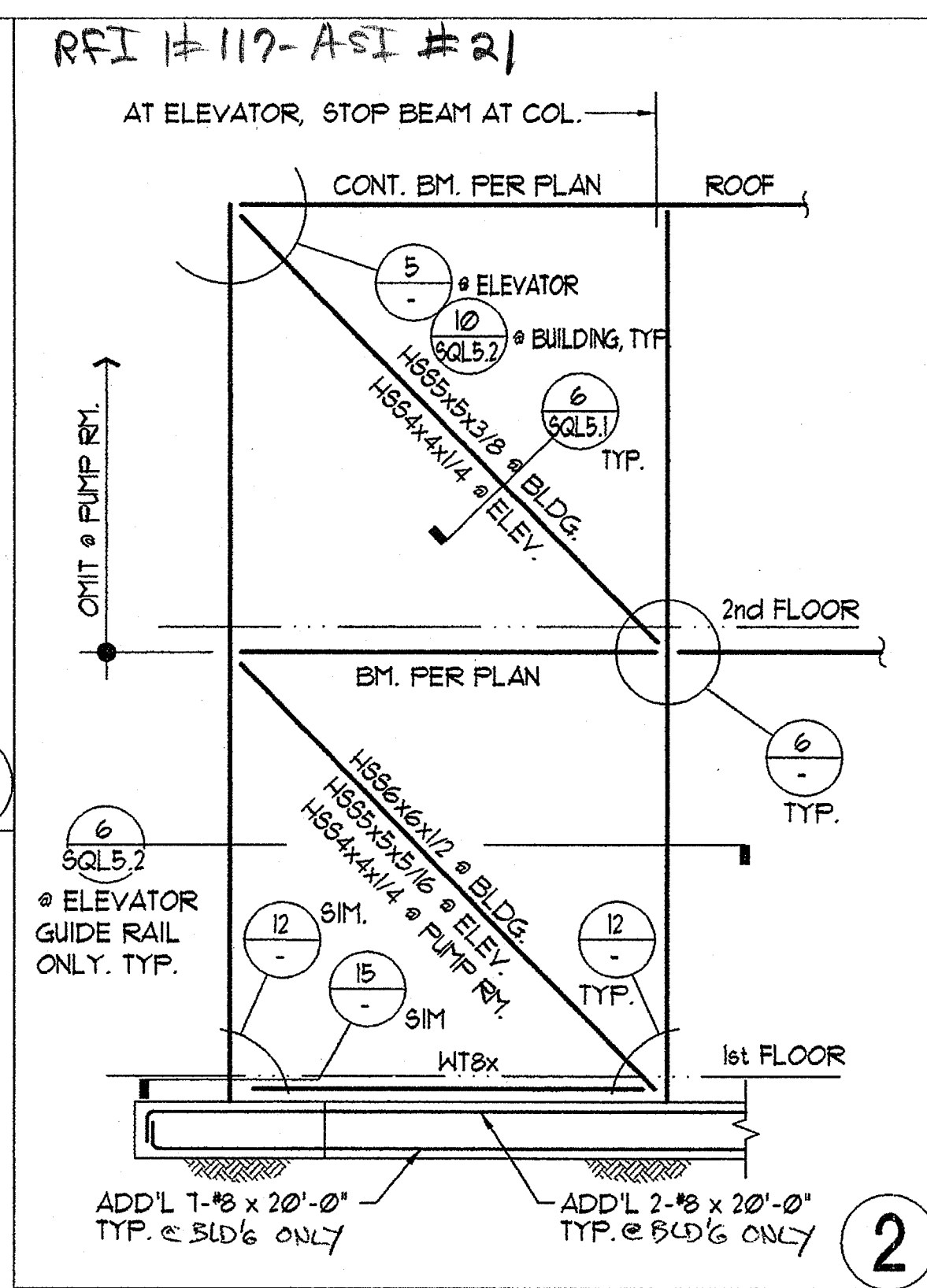
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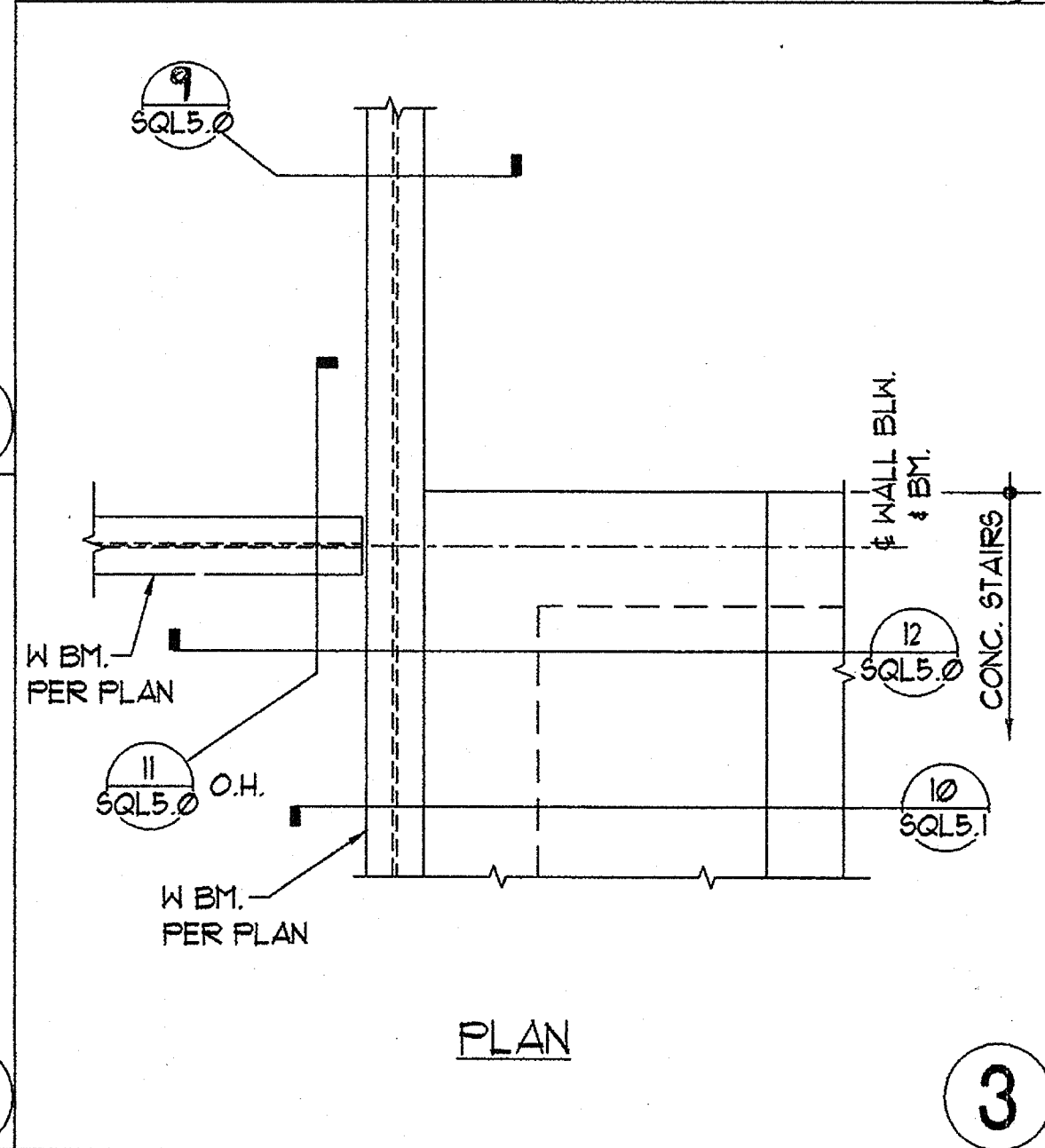
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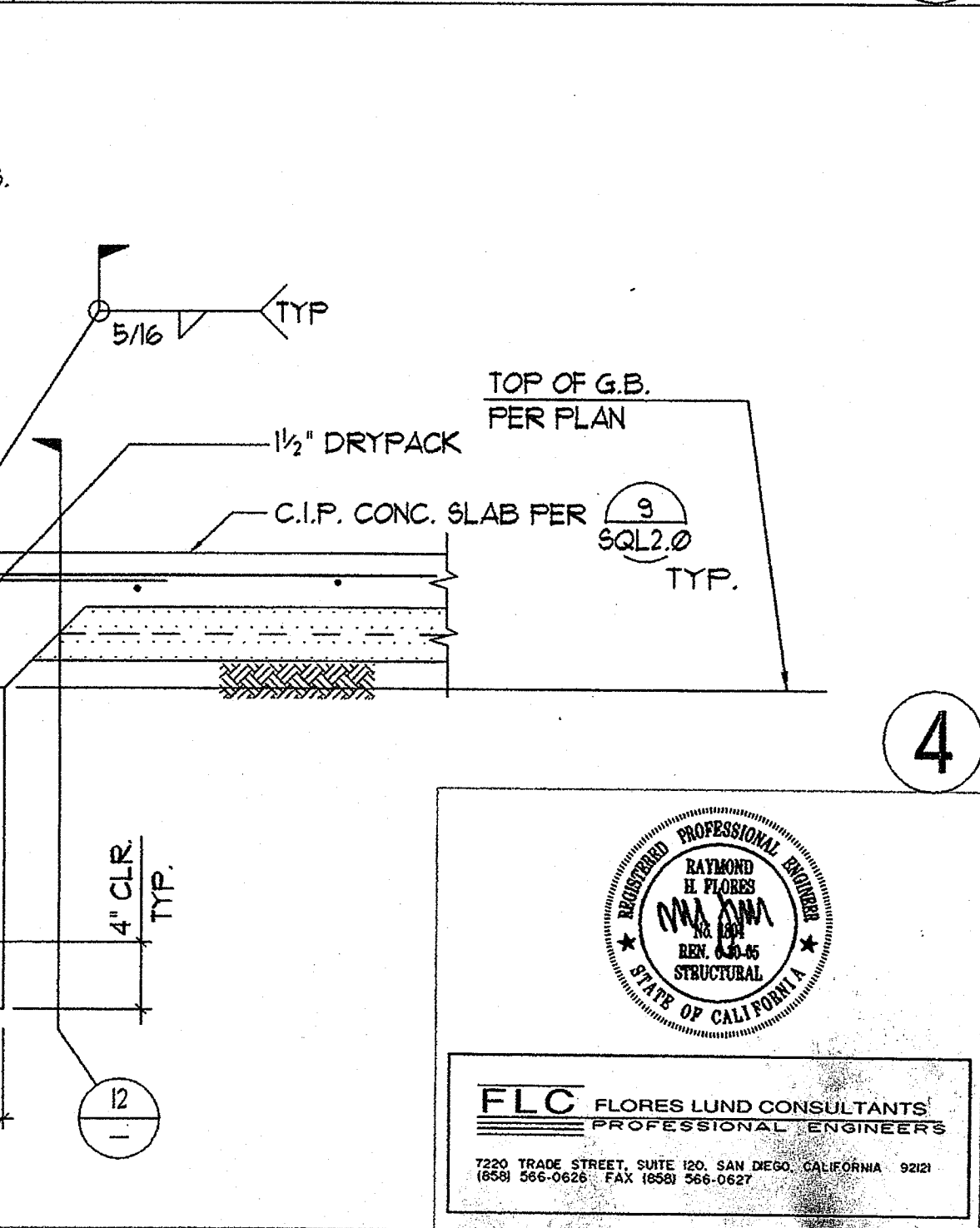
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2



3



1

NOTE: FOR INFORMATION NOT SHOWN OR CALLED OUT SEE 4/412 SQL4.0

NOTE: FOR INFORMATION NOT SHOWN OR CALLED OUT SEE 10 SQL4.0

ELEVATOR SHAFT MECHANICAL ROOM

12\"/>

CONT. BENT IE 3/8\"/>

HIP BM. PER PLAN

FOR OTHER NOTES REFER TO 4

DIAG. HSS PER ELEVATION 2

HSS COL. PER PLAN & ELEV.

WORK FT. 1 1/2\"/>

FIN. FLR. 1/2\"/>

STIRRUPS PER GRADE BM. SCHED. TYP.

4 - A.B.'s @ COL. W/O DIAG. BRACE

HSS COL. PER PLAN & ELEV.

RE TO MATCH WT WEB THICKNESS @ DIAG. ONLY

3\"/>

SIDENALK SLAB WHERE OCCURS PER ARCH'L DWGS.

GRADE BM. PER PLAN & SCHED.

BOLT HEAD OR NUT W/ WASHER TYP.

6 - 1/2\"/>

4 - 1/2\"/>

4 - 1/2\"/>

PER PLAN

4\"/>

12

RFI # 112-AS1 # 21

AT ELEVATOR STOP BEAM AT COL.

CONT. BM. PER PLAN ROOF

5 ELEVATOR 10 BUILDING, TYF 6 SQL5.1

HSS4X4X1/4 @ ELEV. TYP.

2nd FLOOR 6 TYP.

BM. PER PLAN

HSS4X4X1/4 @ BLDG. HSS4X4X1/4 @ ELEV. TYP.

1st FLOOR 6 TYP.

ADD'L 1-#8 x 20'-0\"/>

ADD'L 2-#8 x 20'-0\"/>

9 SQL5.0

W.B.M. PER PLAN

11 SQL5.0

W.B.M. PER PLAN

12 SQL5.0

CONC. STAIRS

10 SQL5.1

PLAN

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PROJECT NO. 758-000

PROJECT NOB. 025

P. T. N. 73569-9

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JEFFERSON MS NEW CONSTRUCTION

823 ACACIA STREET

OCEANSIDE, CA 92054

OCEANSIDE UNIFIED S.D.

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GROTH ARCHITECTS, INC. 3355 MISSION AVE. OCEANSIDE, CALIFORNIA 92054

PHONE 760-754-8191

FAX 760-754-8291

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JOHN SCOTT 6017

C-26609

4/30/2007

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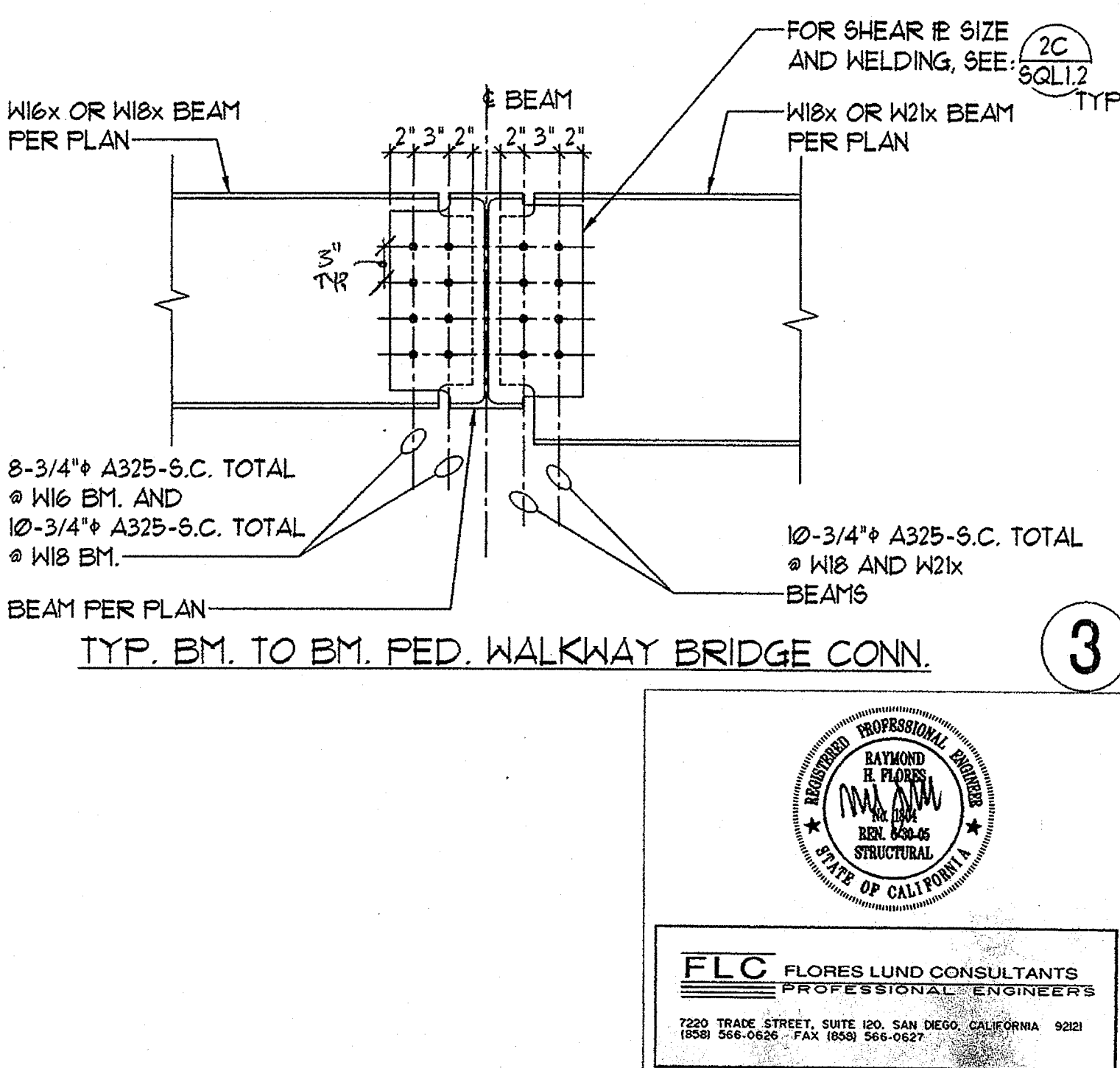
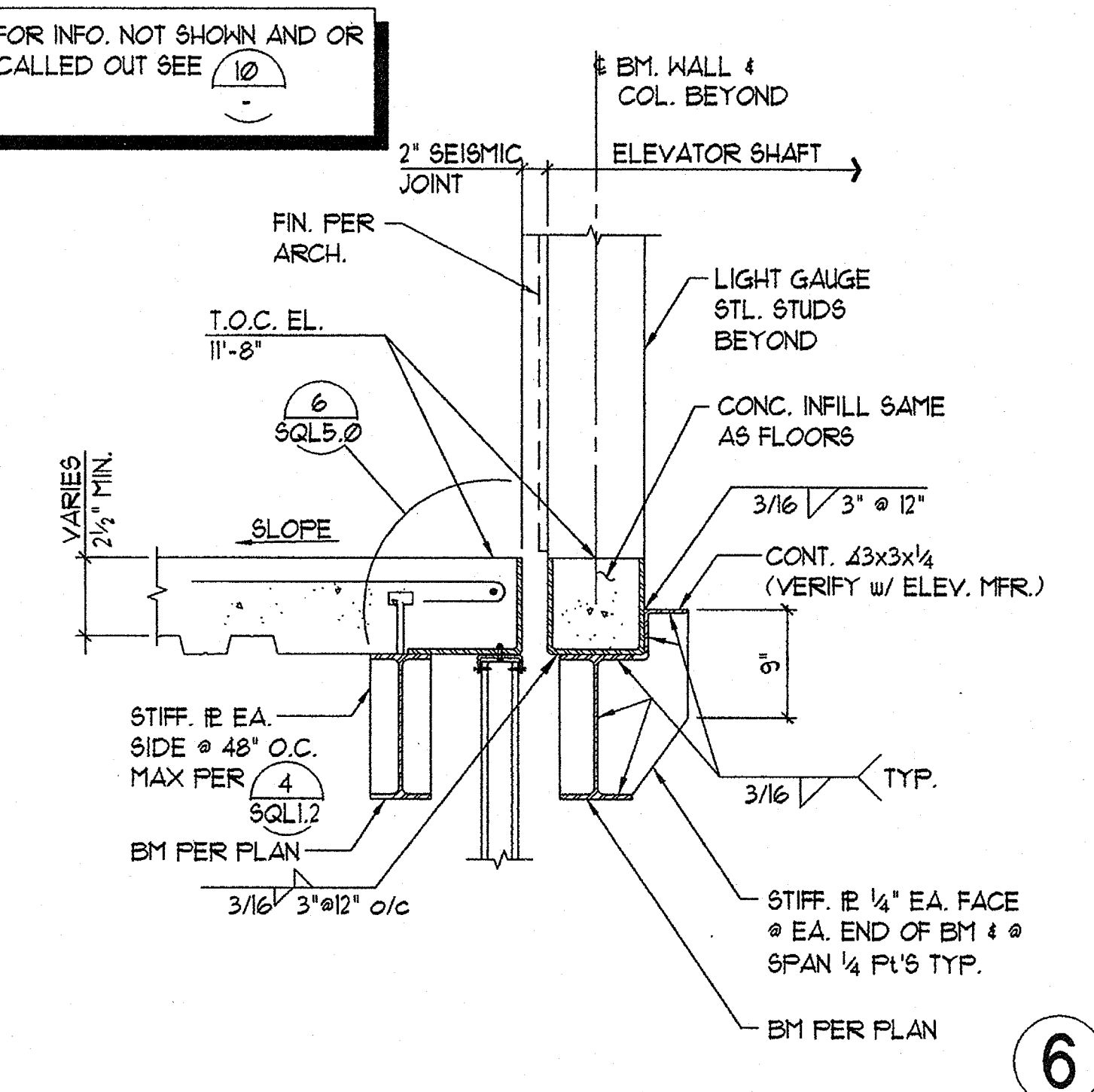
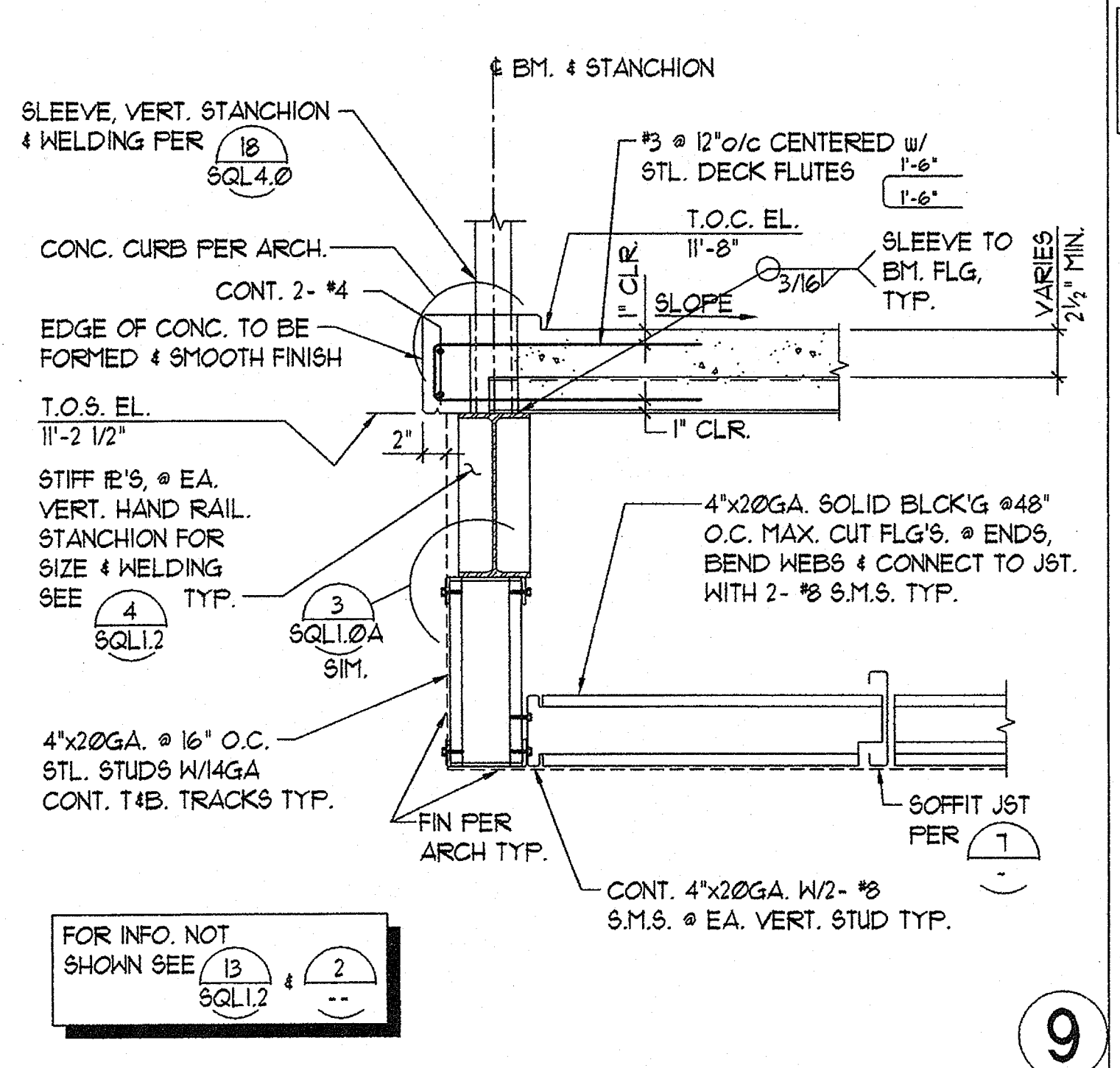
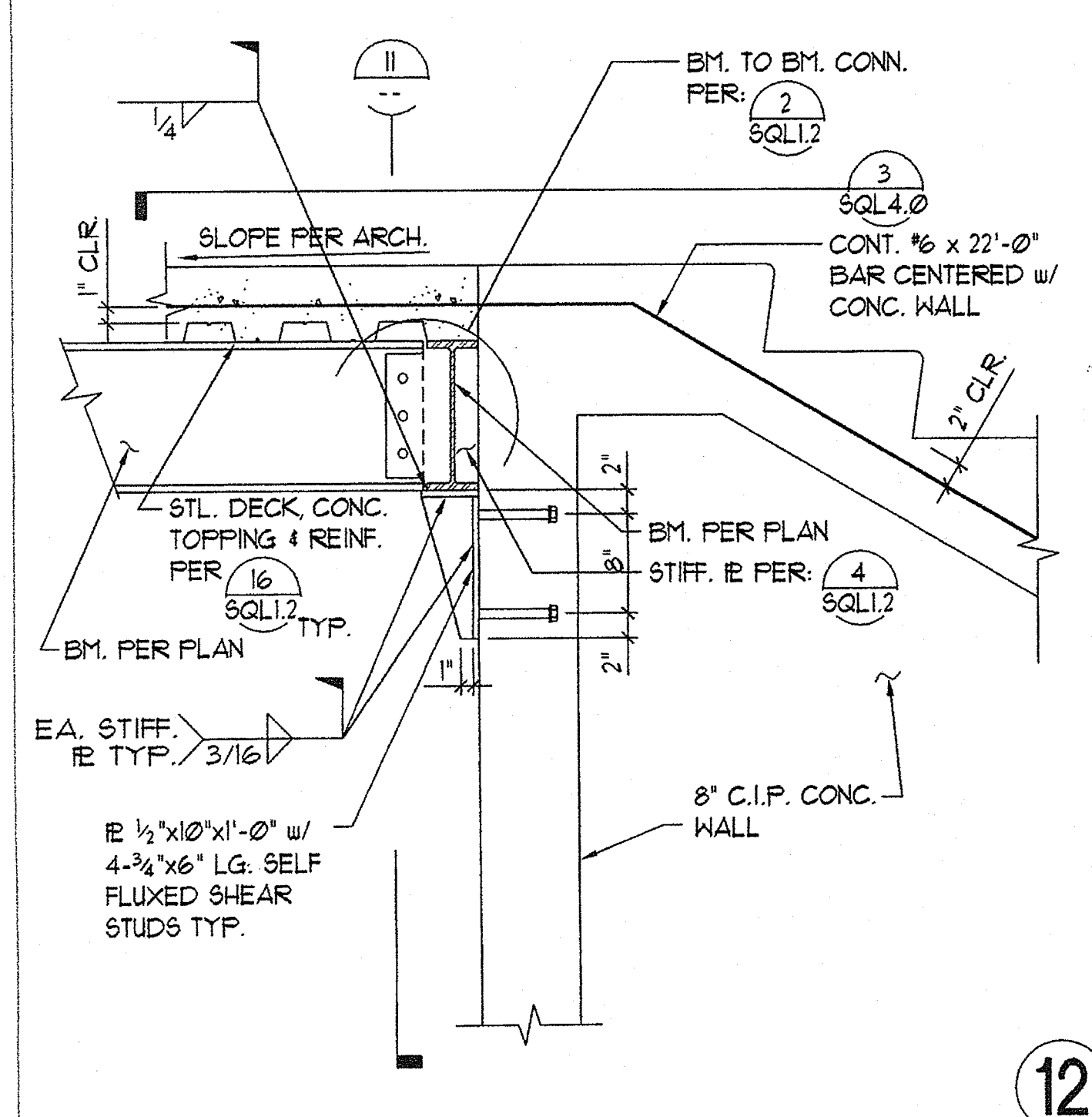
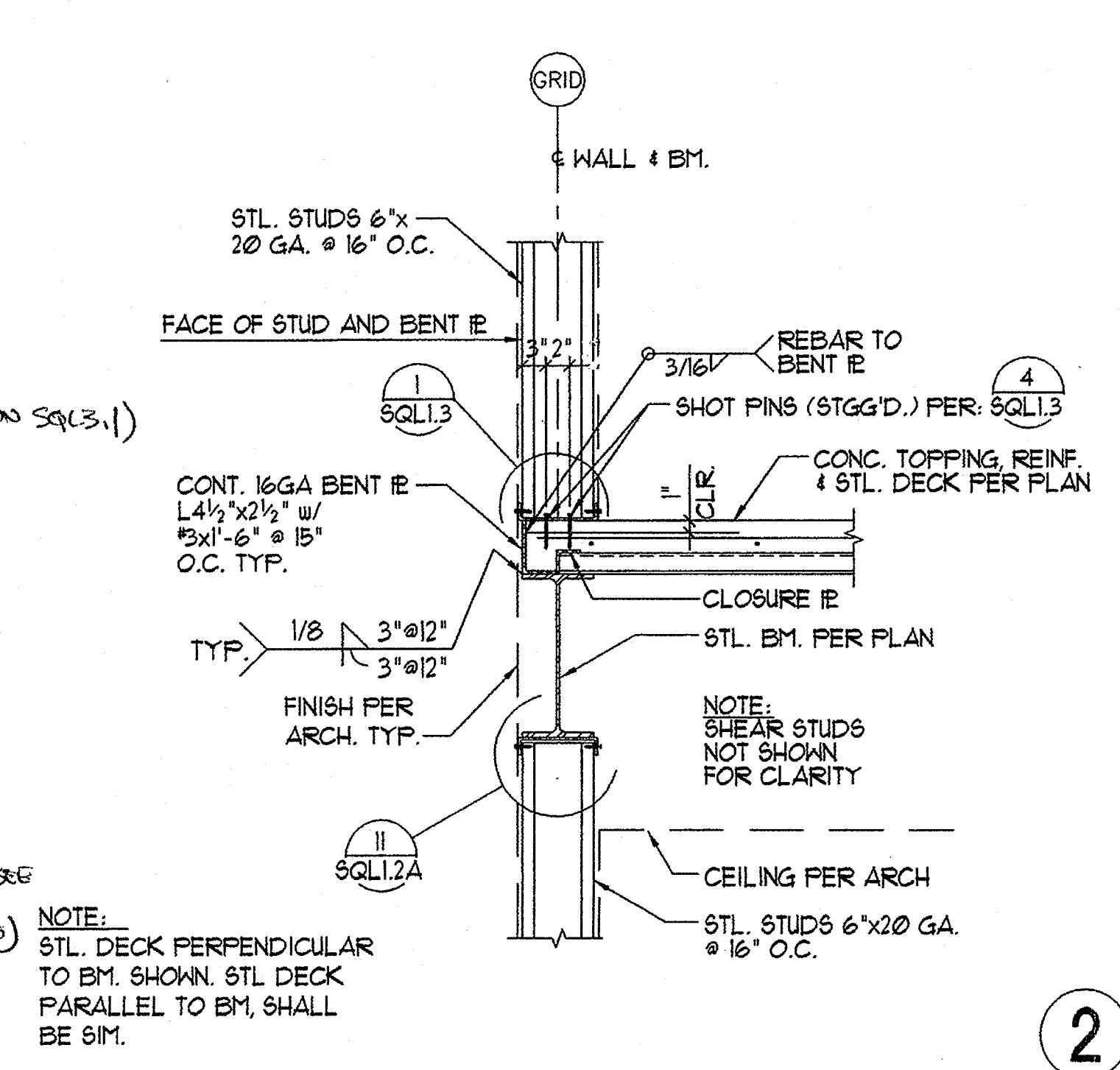
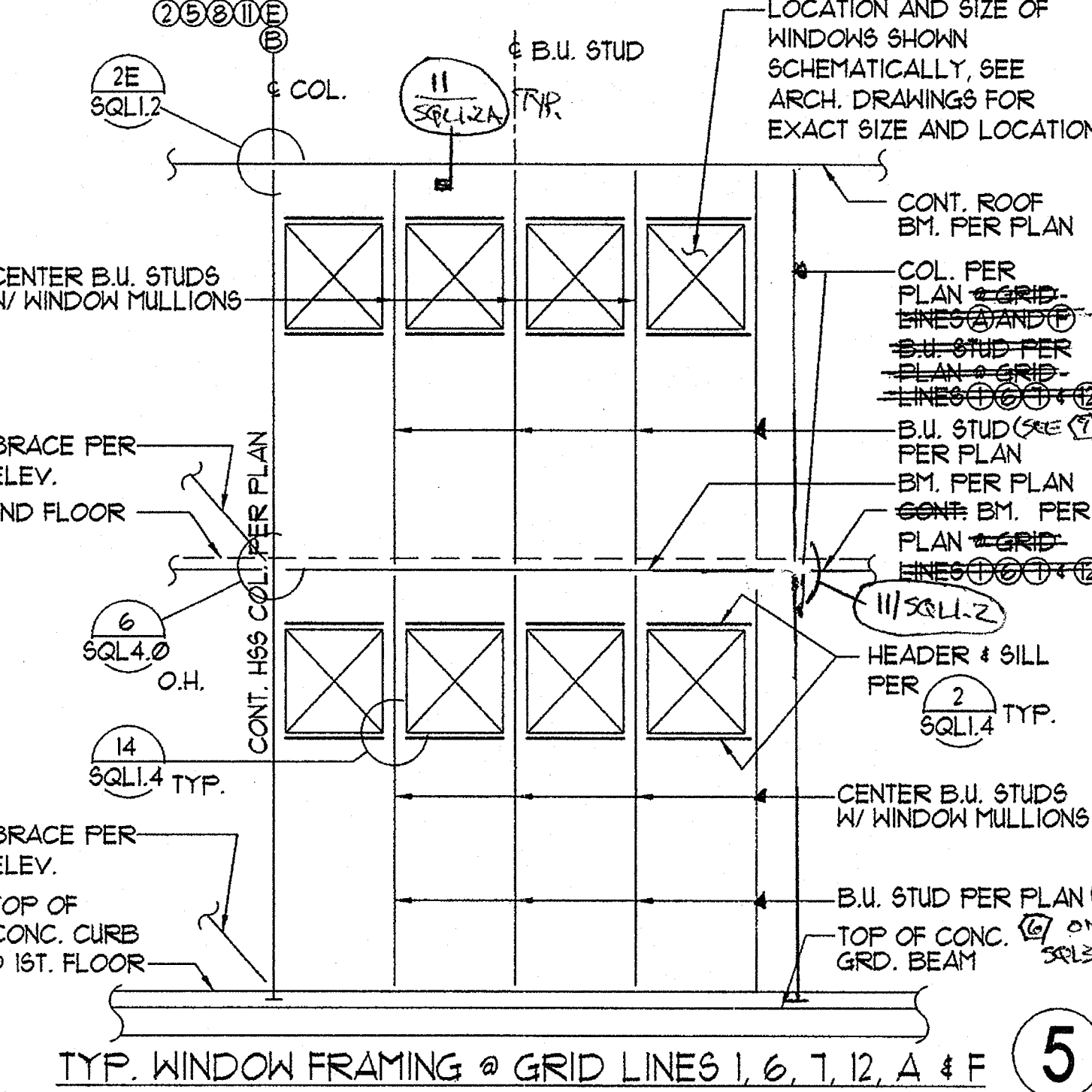
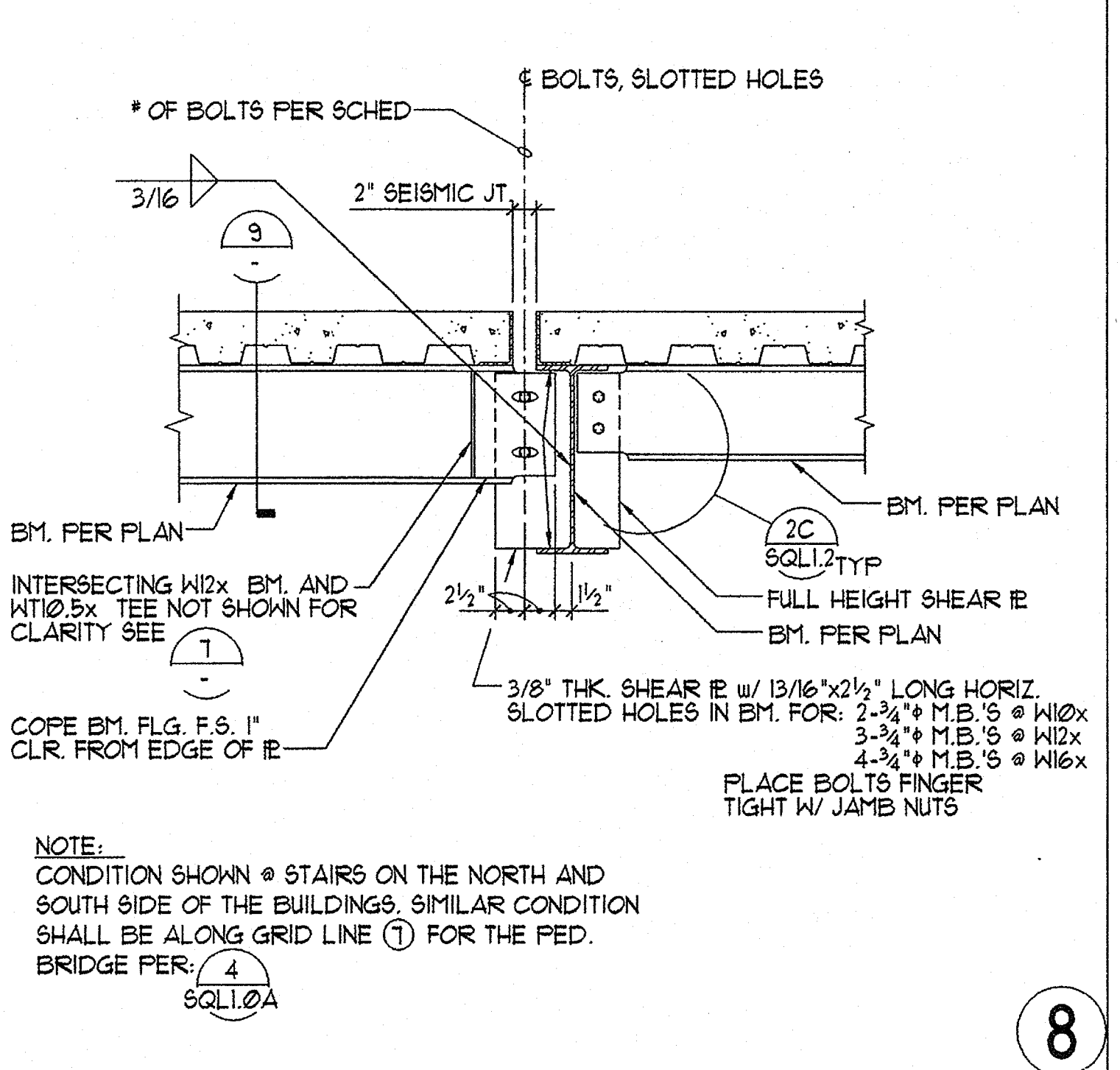
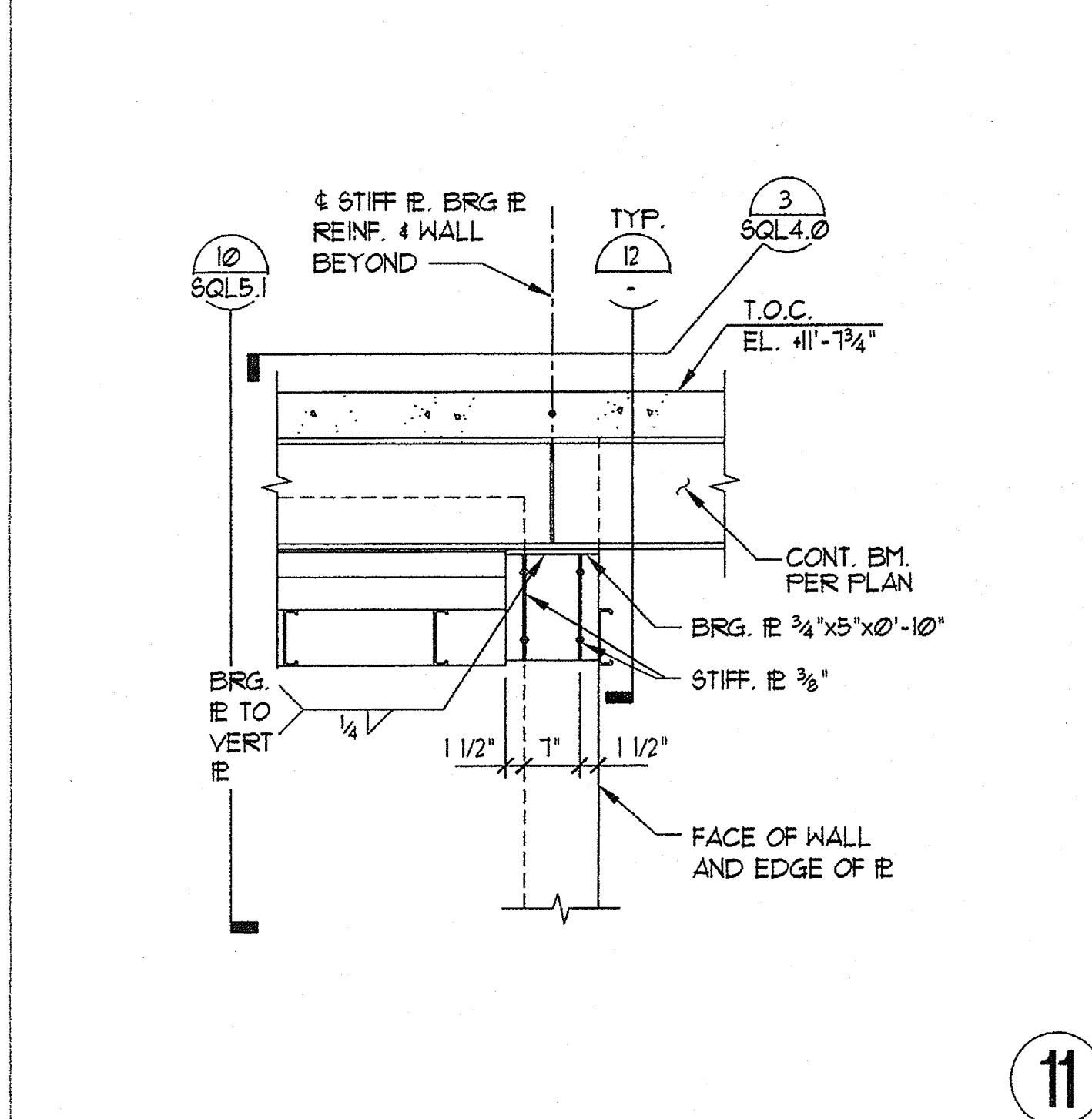
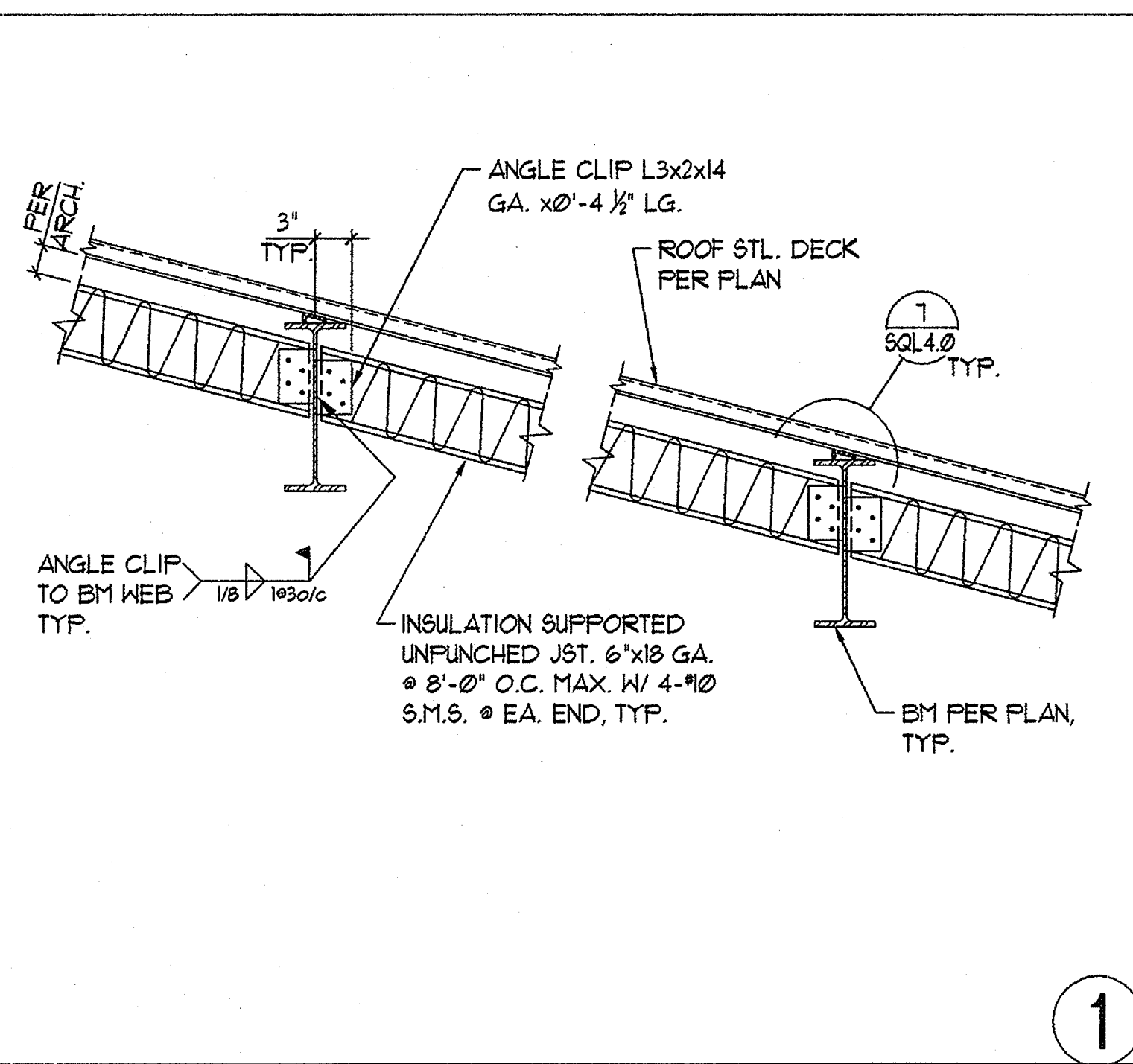
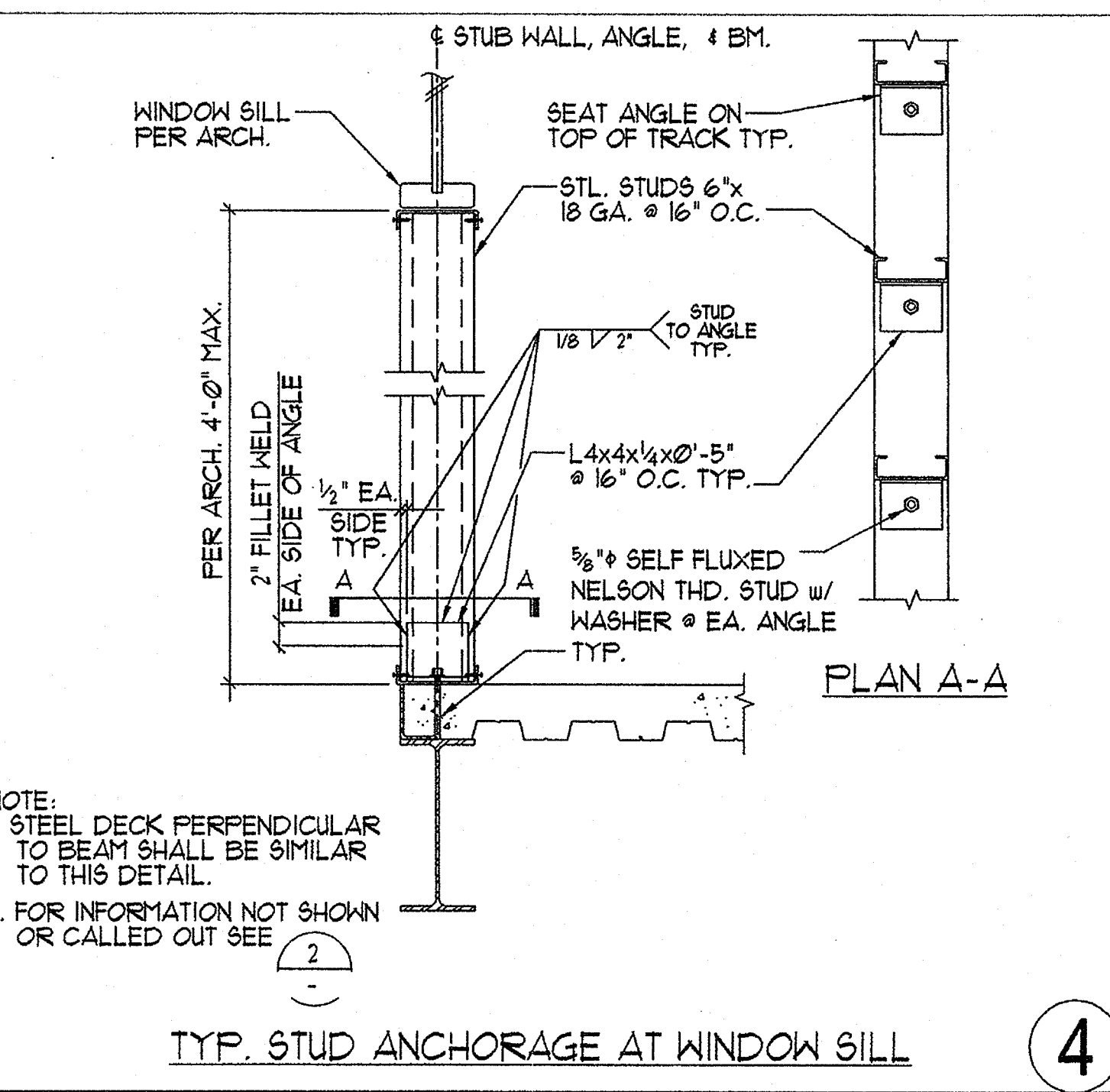
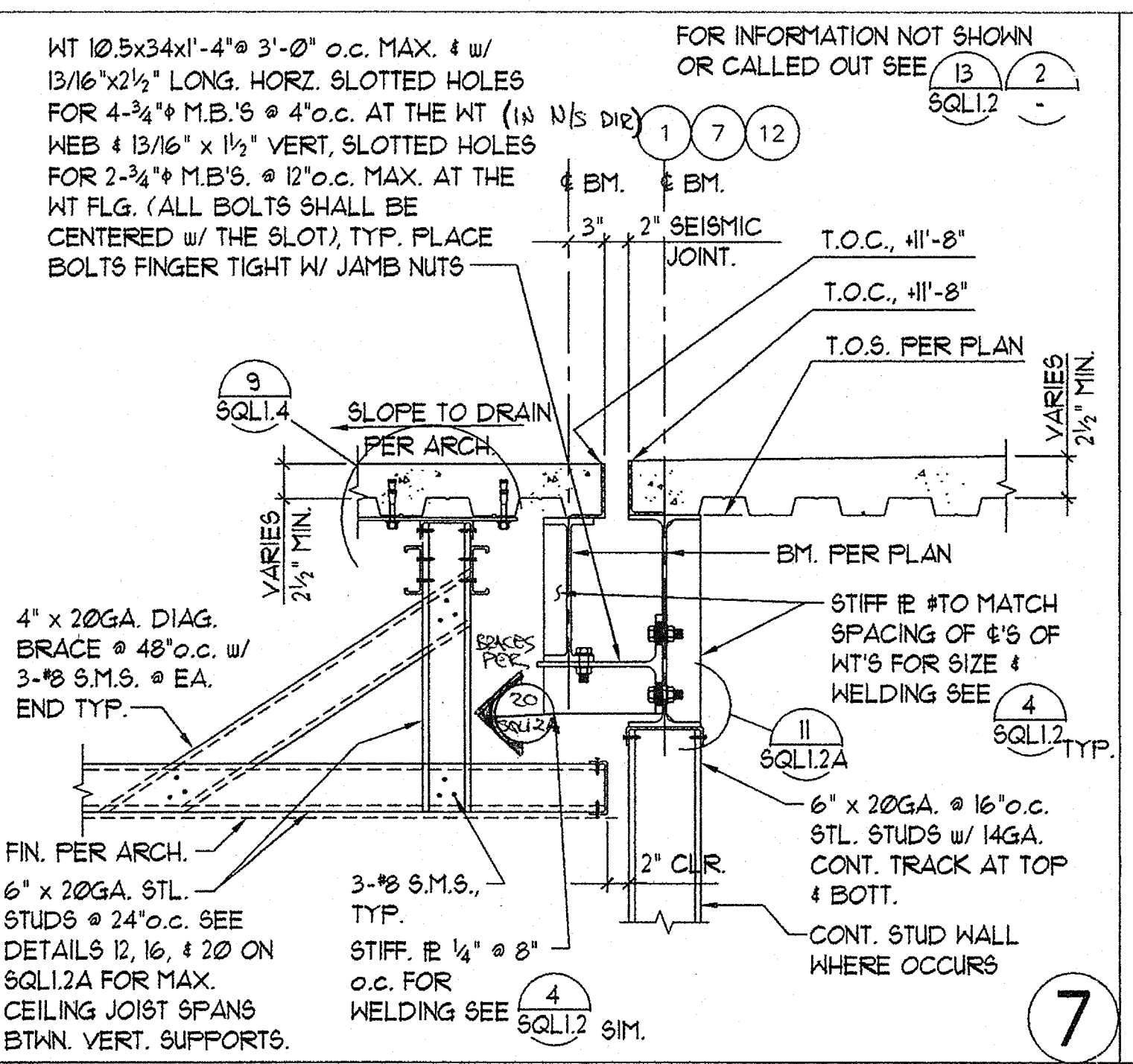
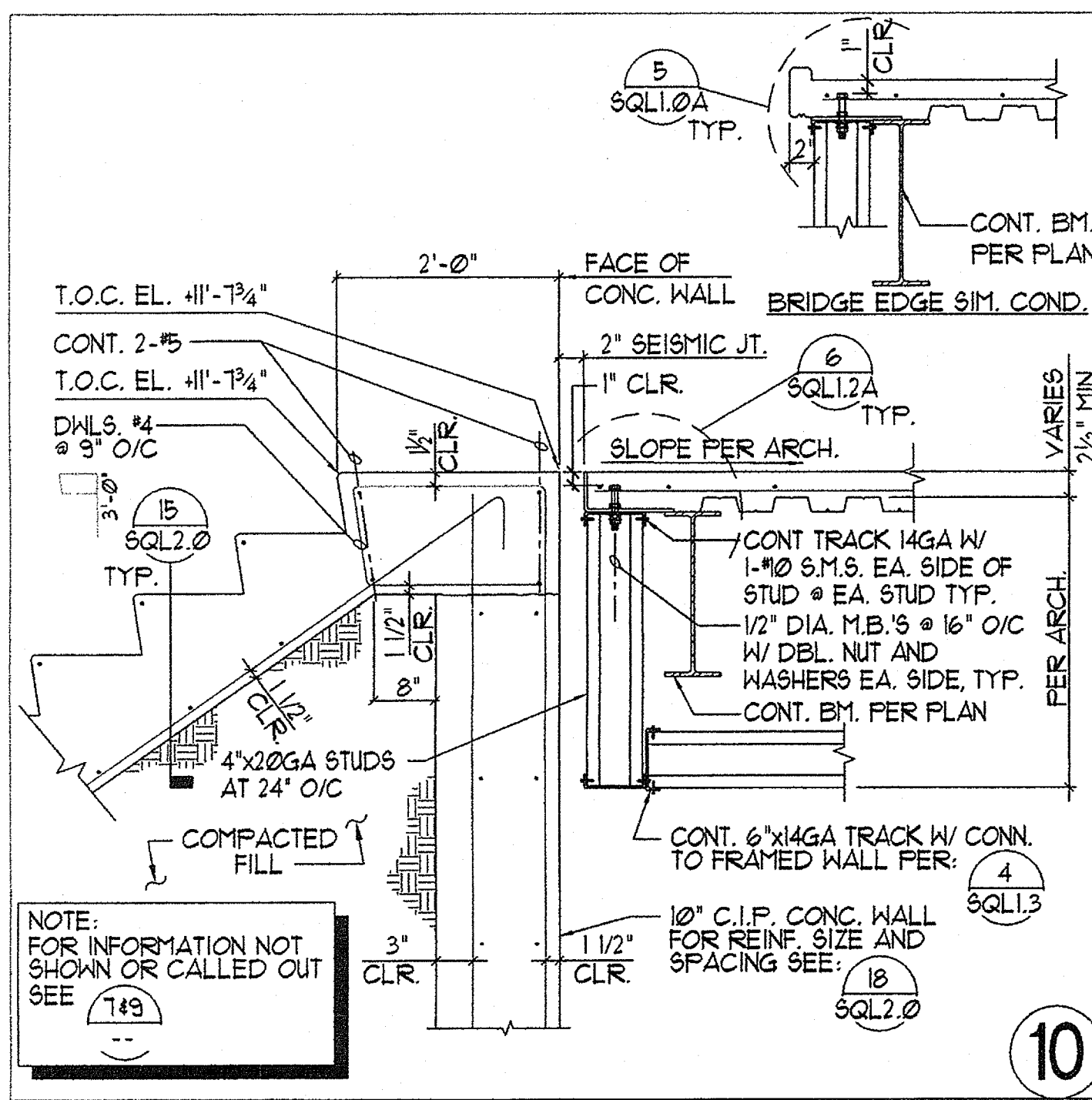
BRACE FRAME ELEVATION AND DETAILS

SQL4.0

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P. T. N. 73569-9

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JEFFERSON MS NEW CONSTRUCTION

823 ACACIA STREET

OCEANSIDE CA 92054

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3355 MISSION AVE SUITE 234 OCEANSIDE, CALIFORNIA 92054

PHONE 760-754-8191

FAX 760-754-8291

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C-26609

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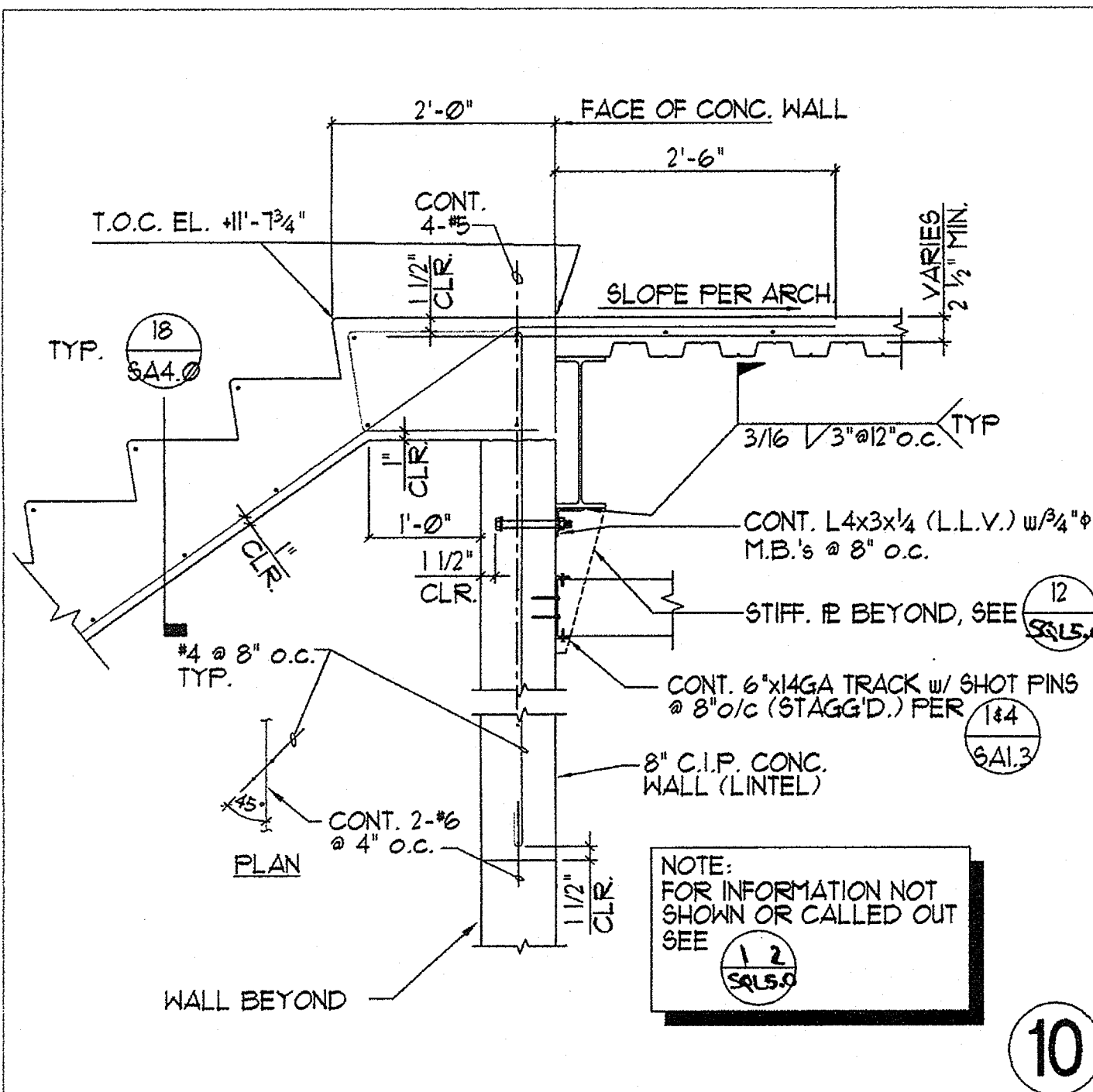
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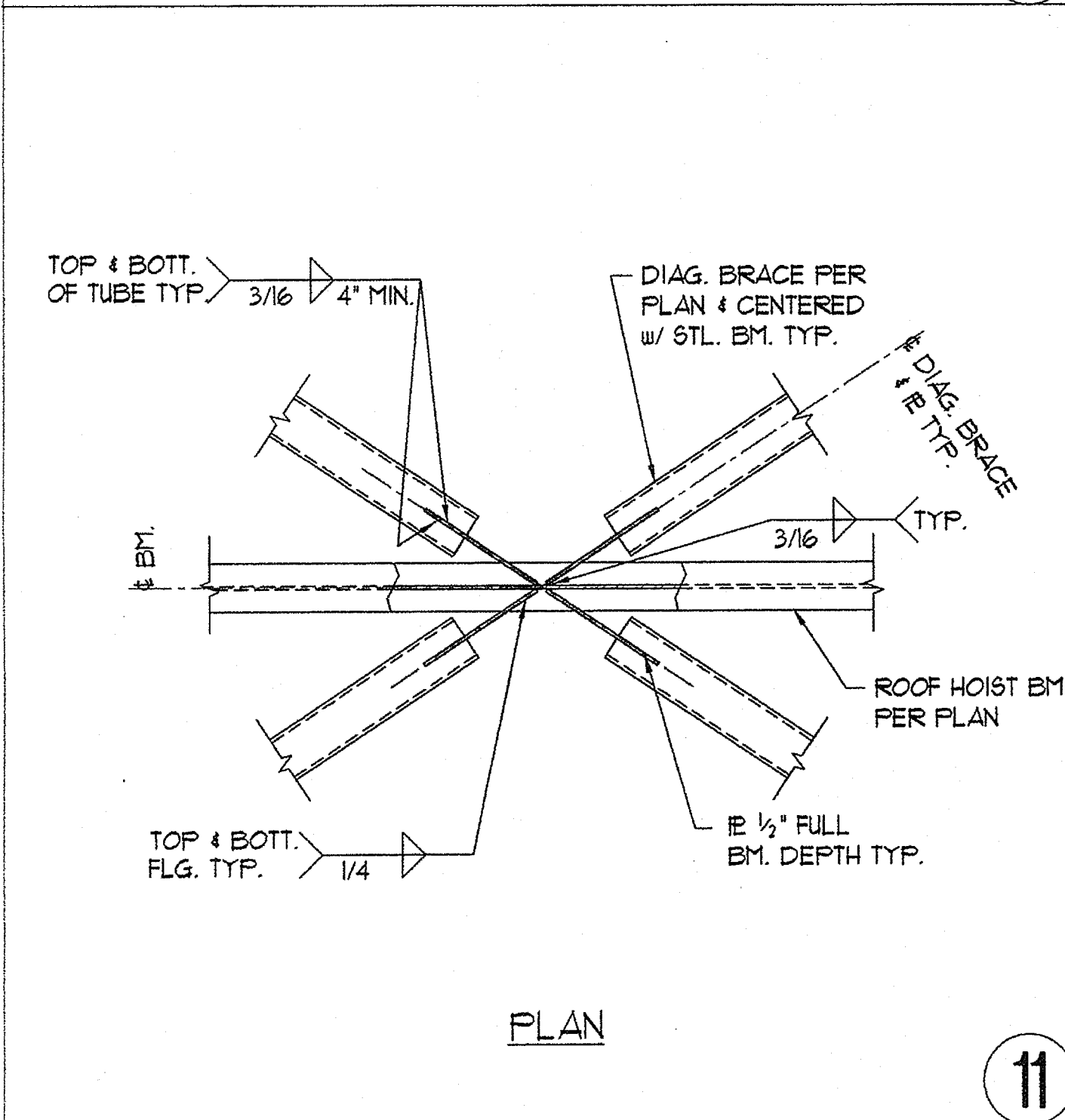
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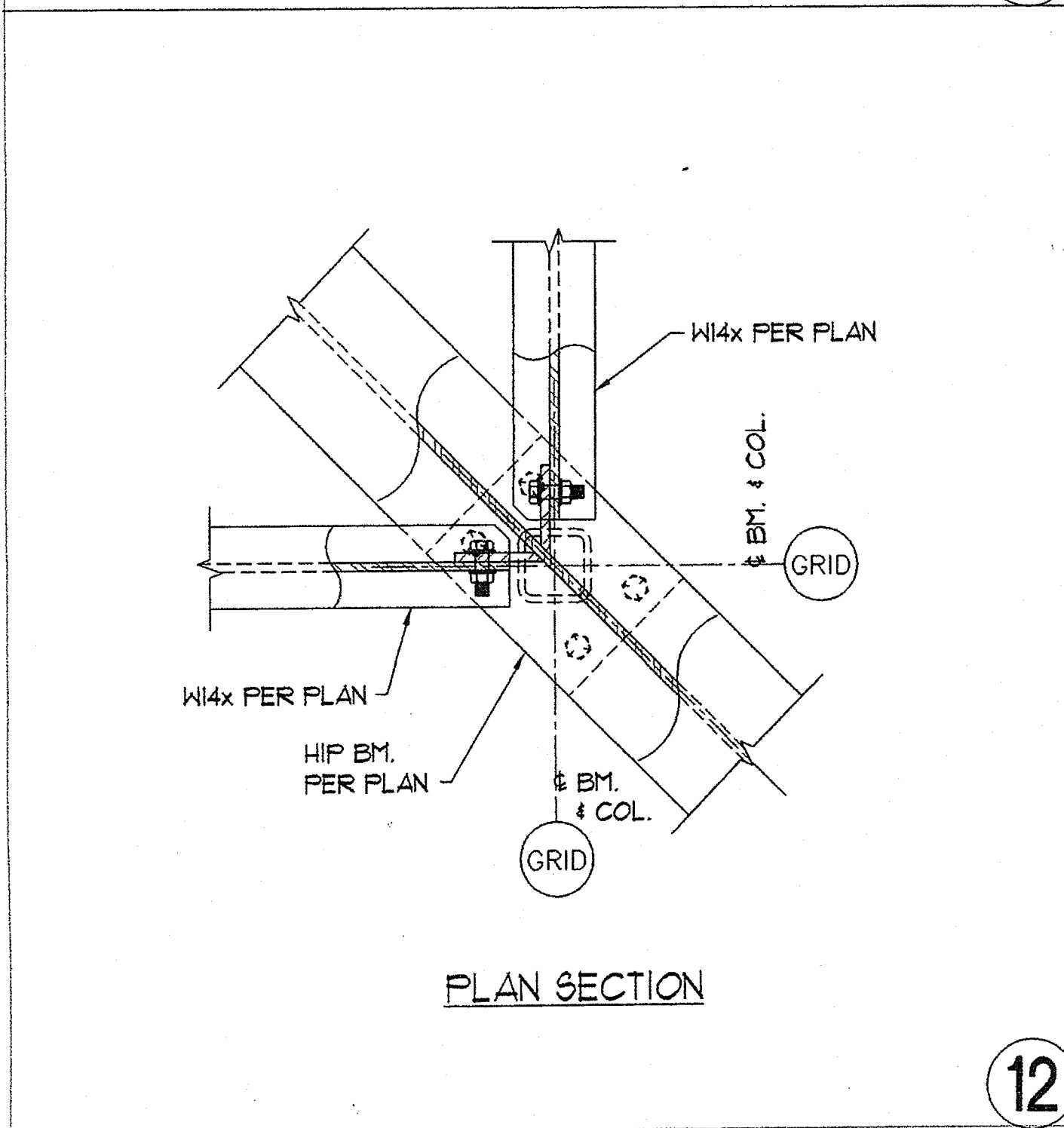
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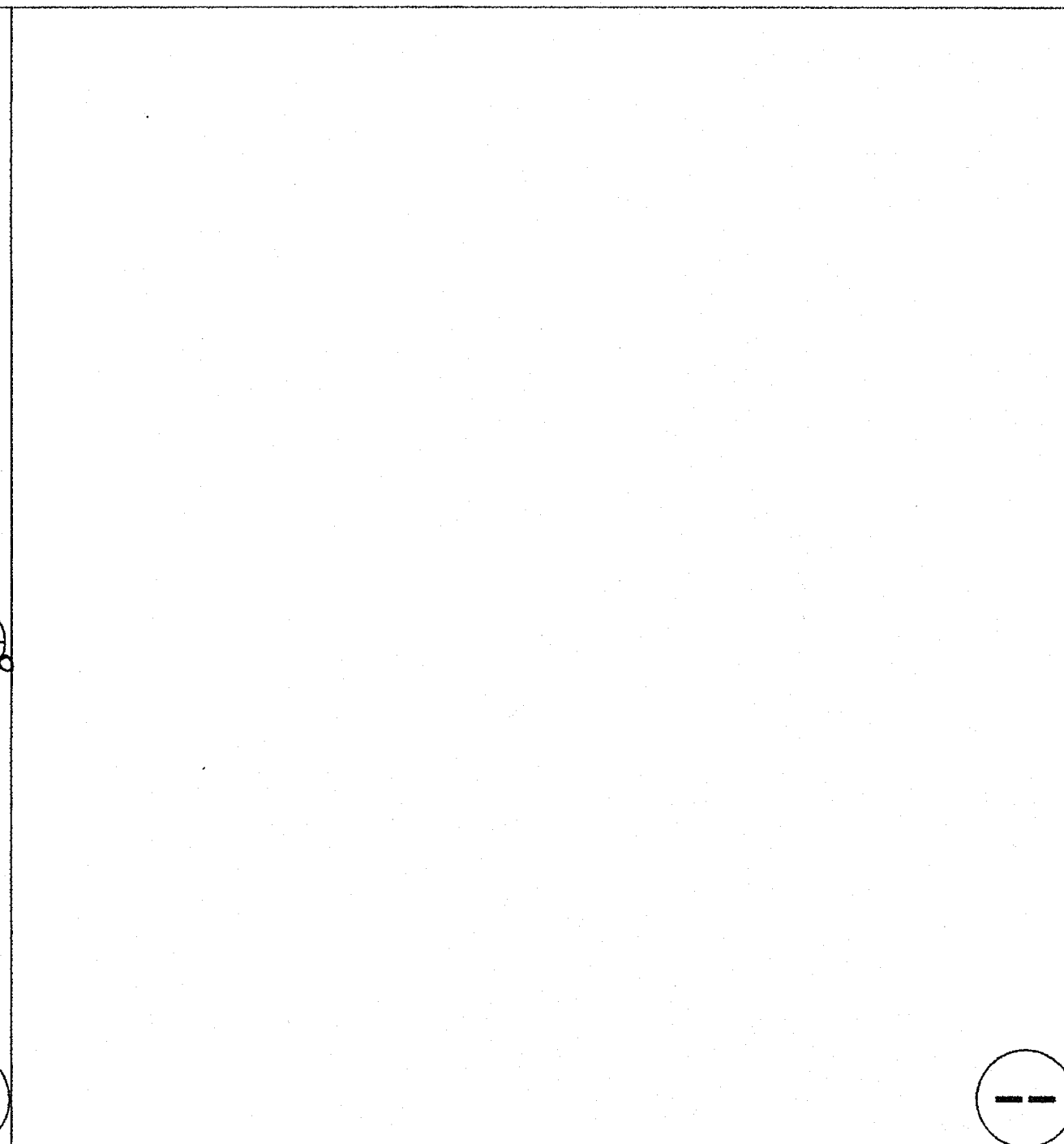
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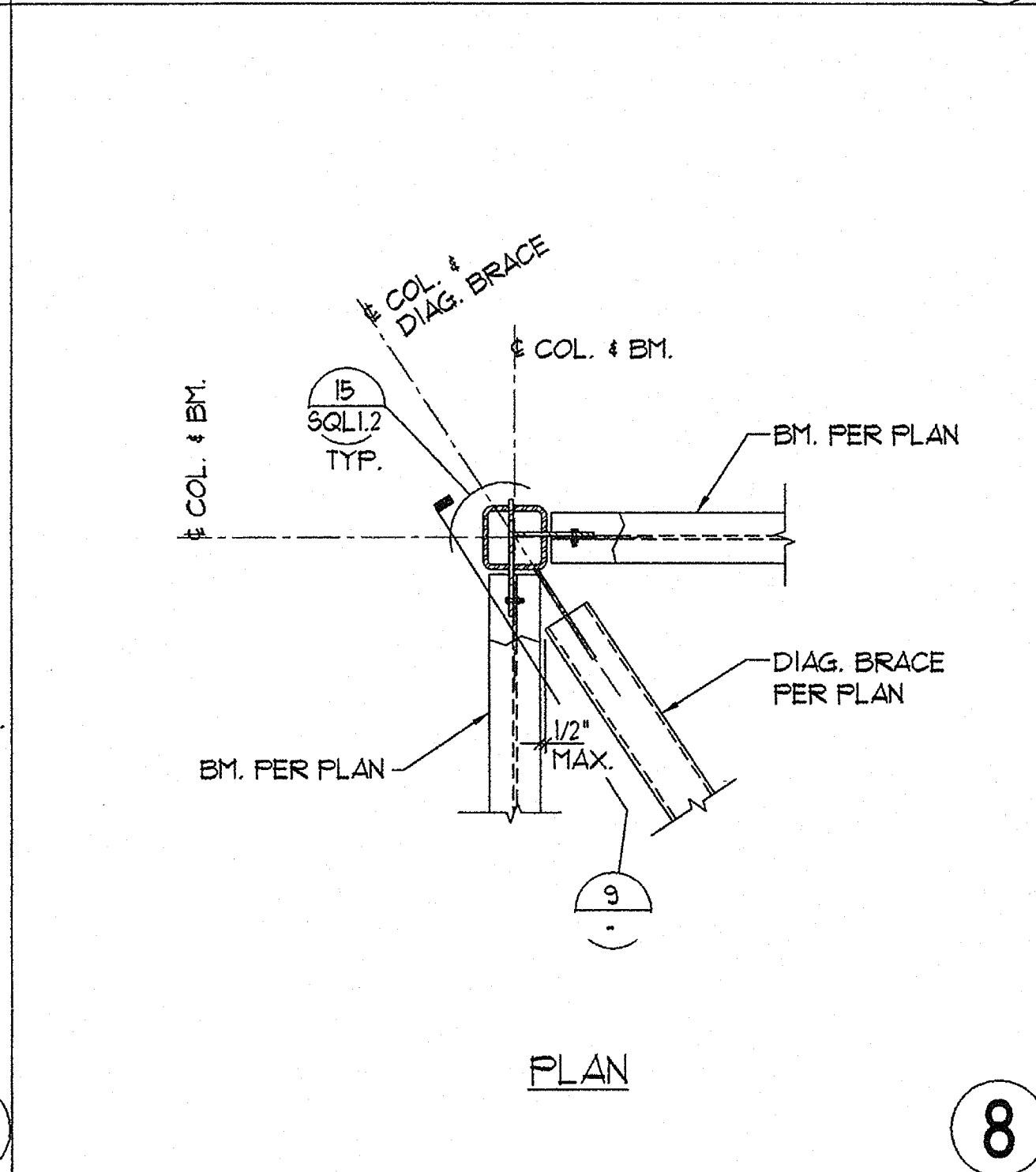
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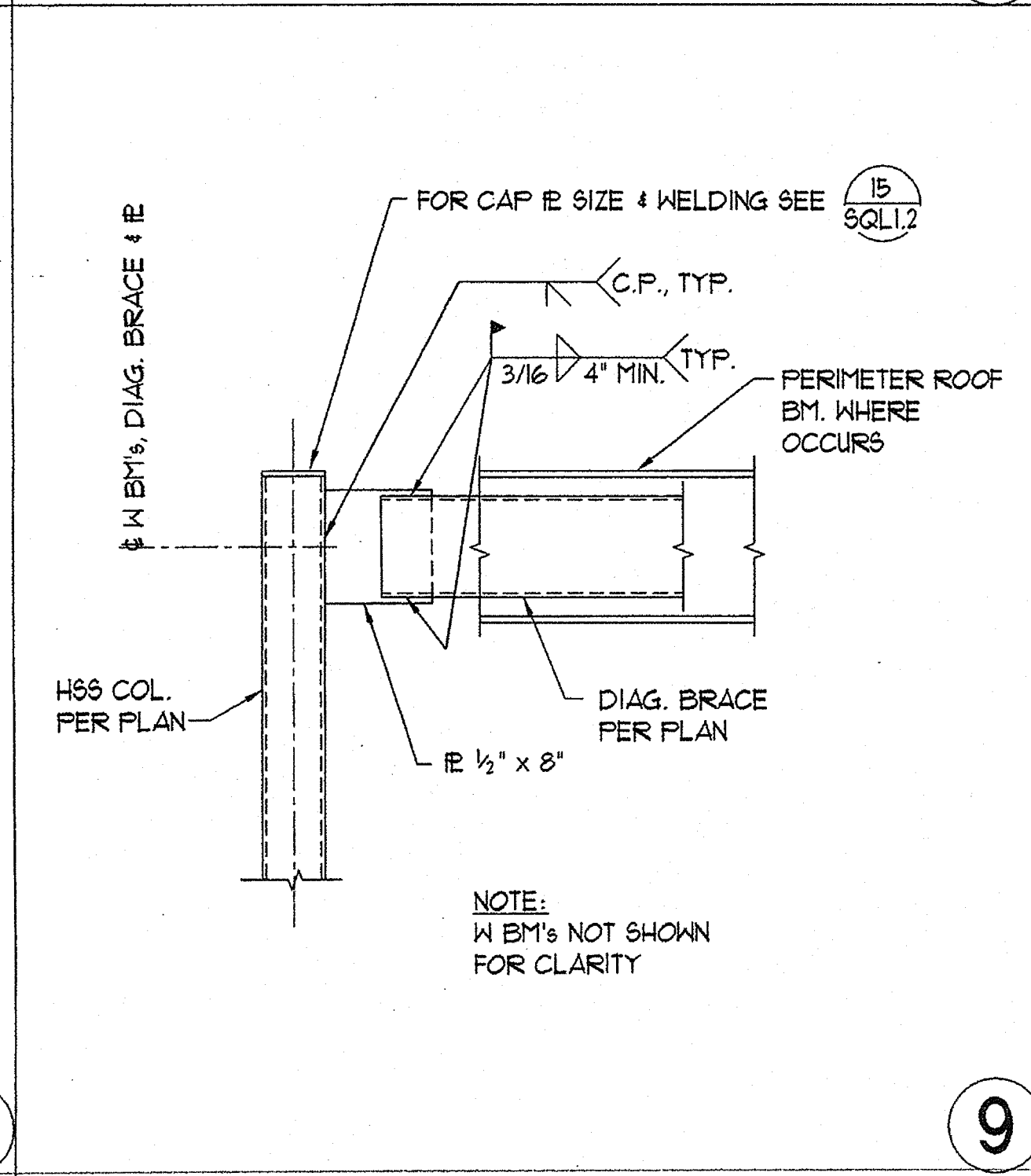
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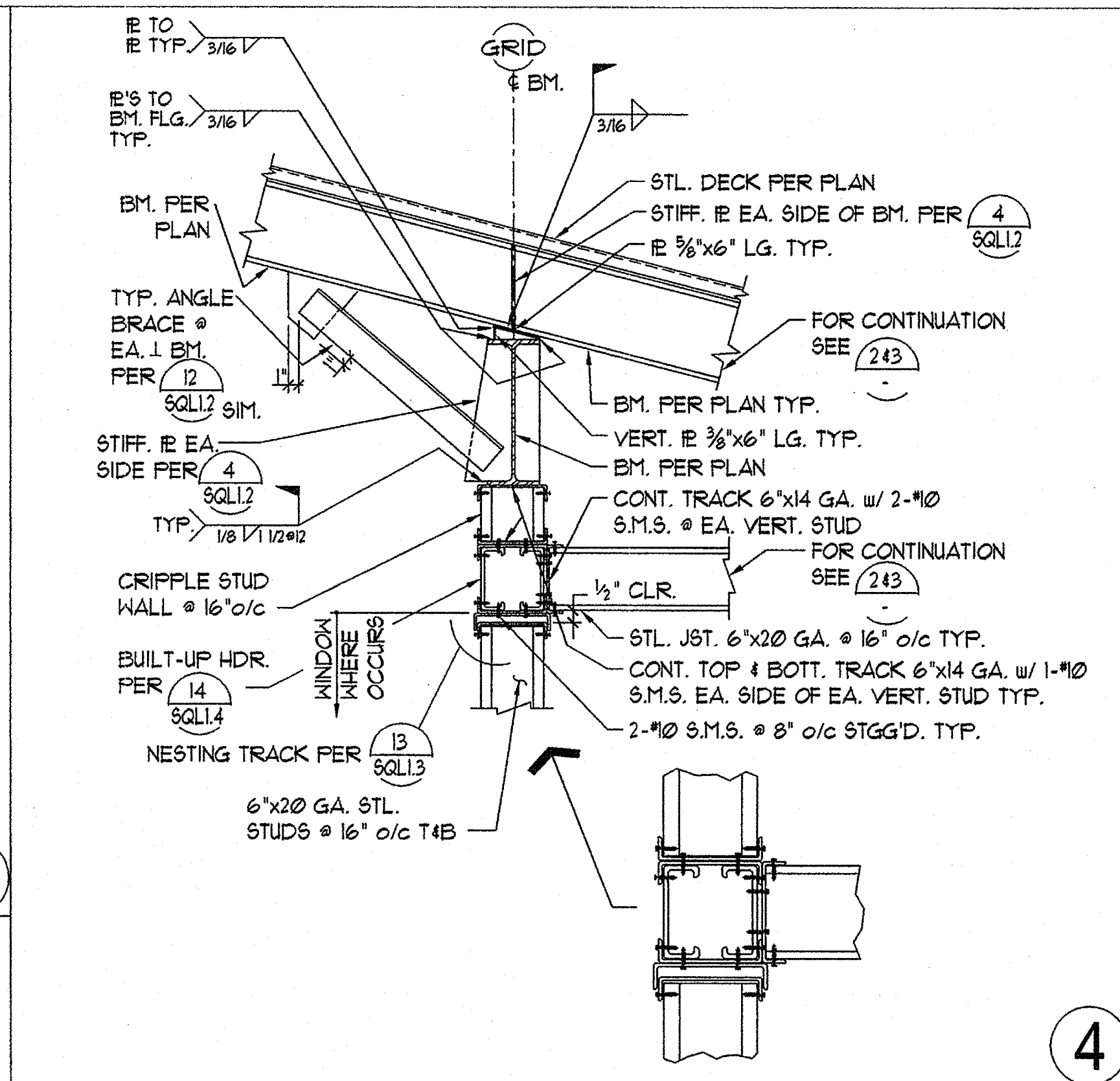
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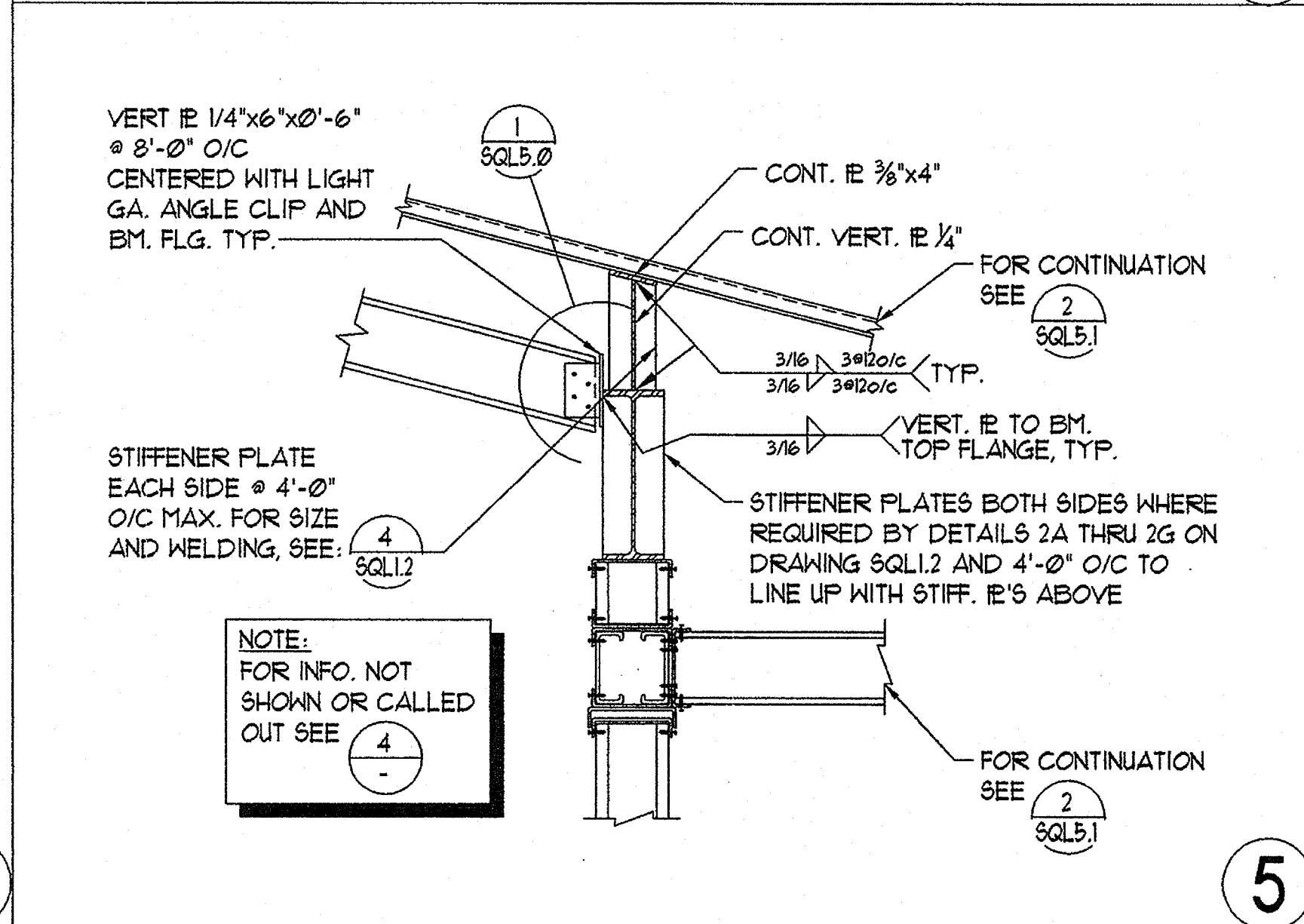
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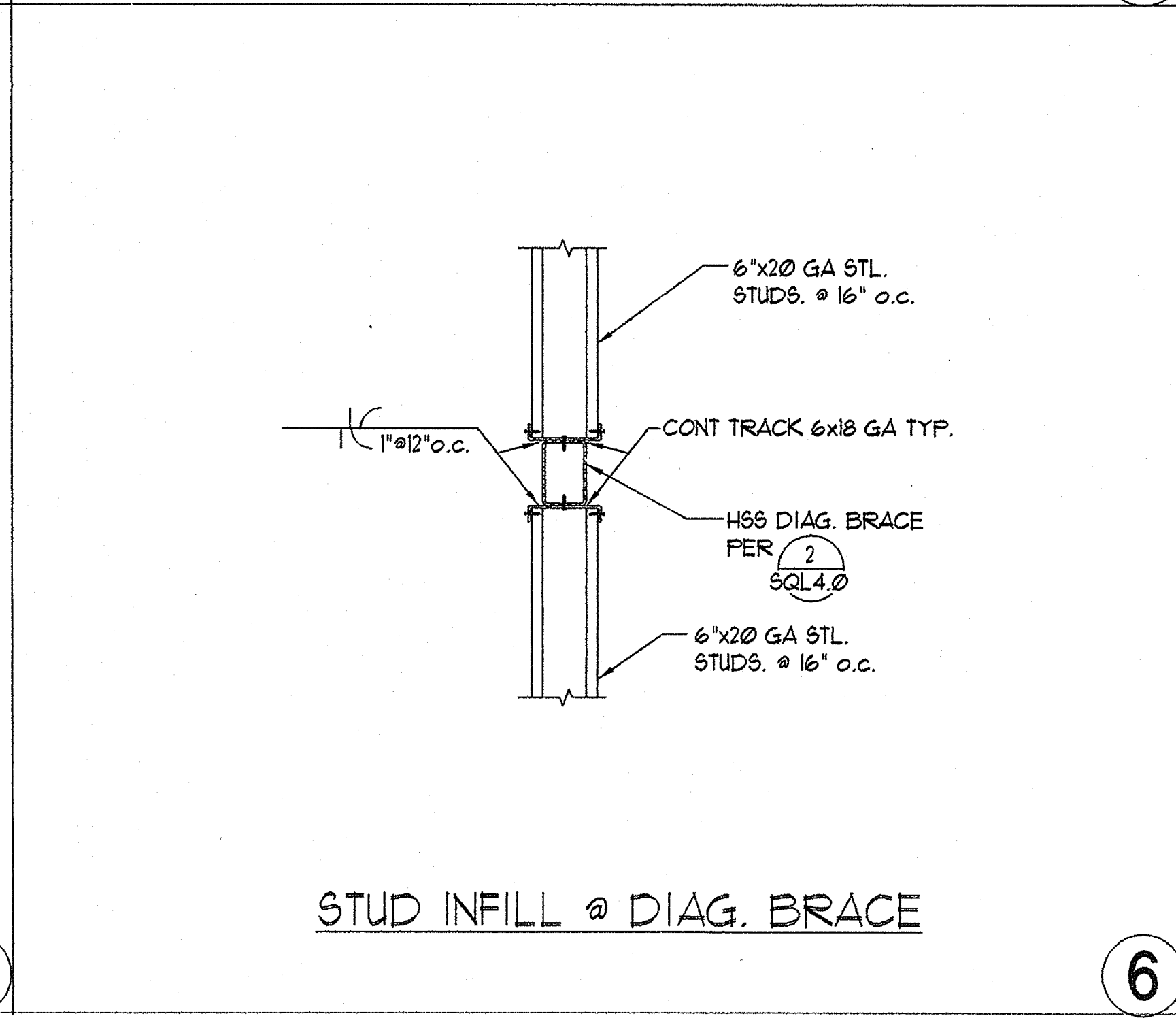
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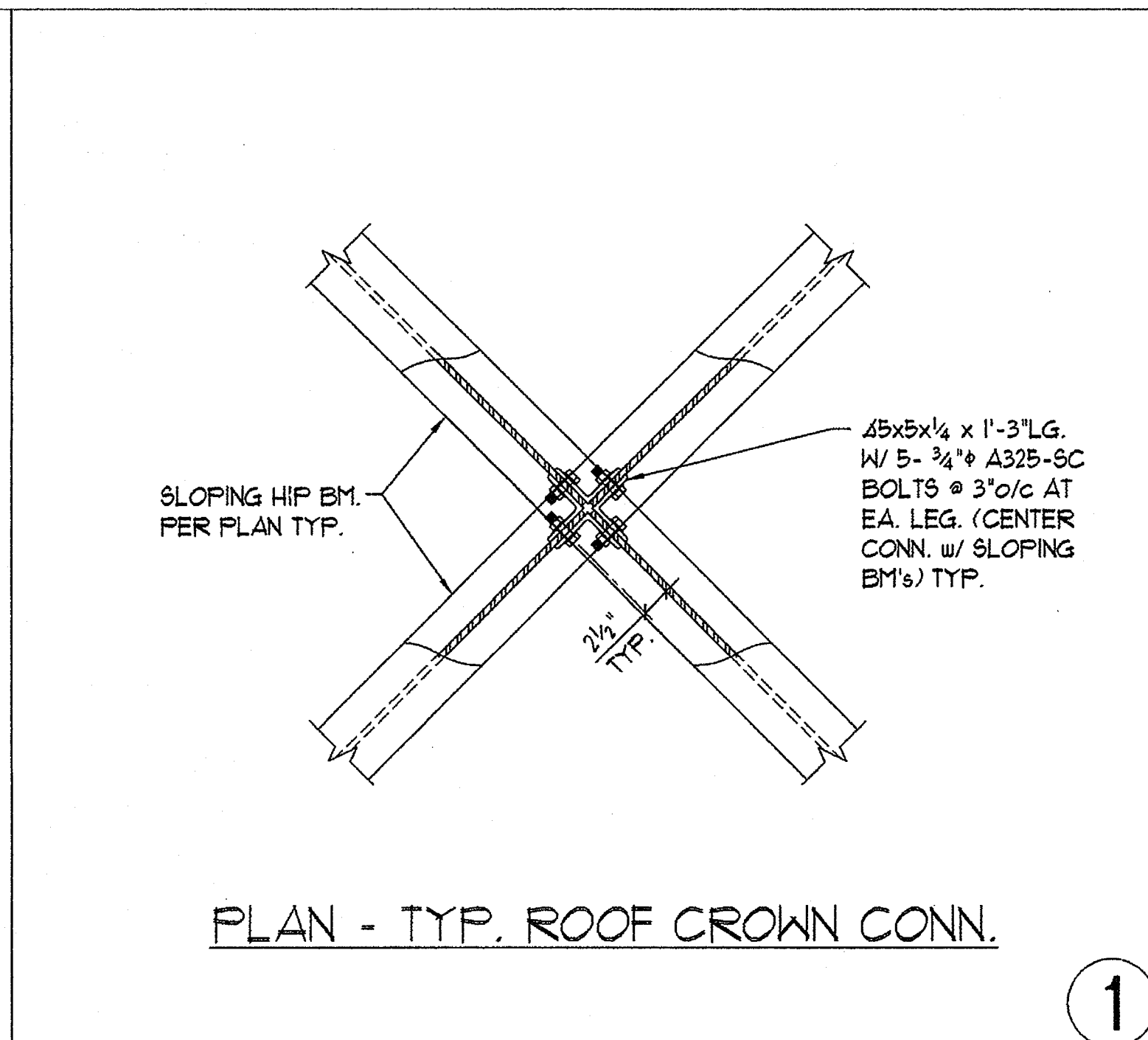
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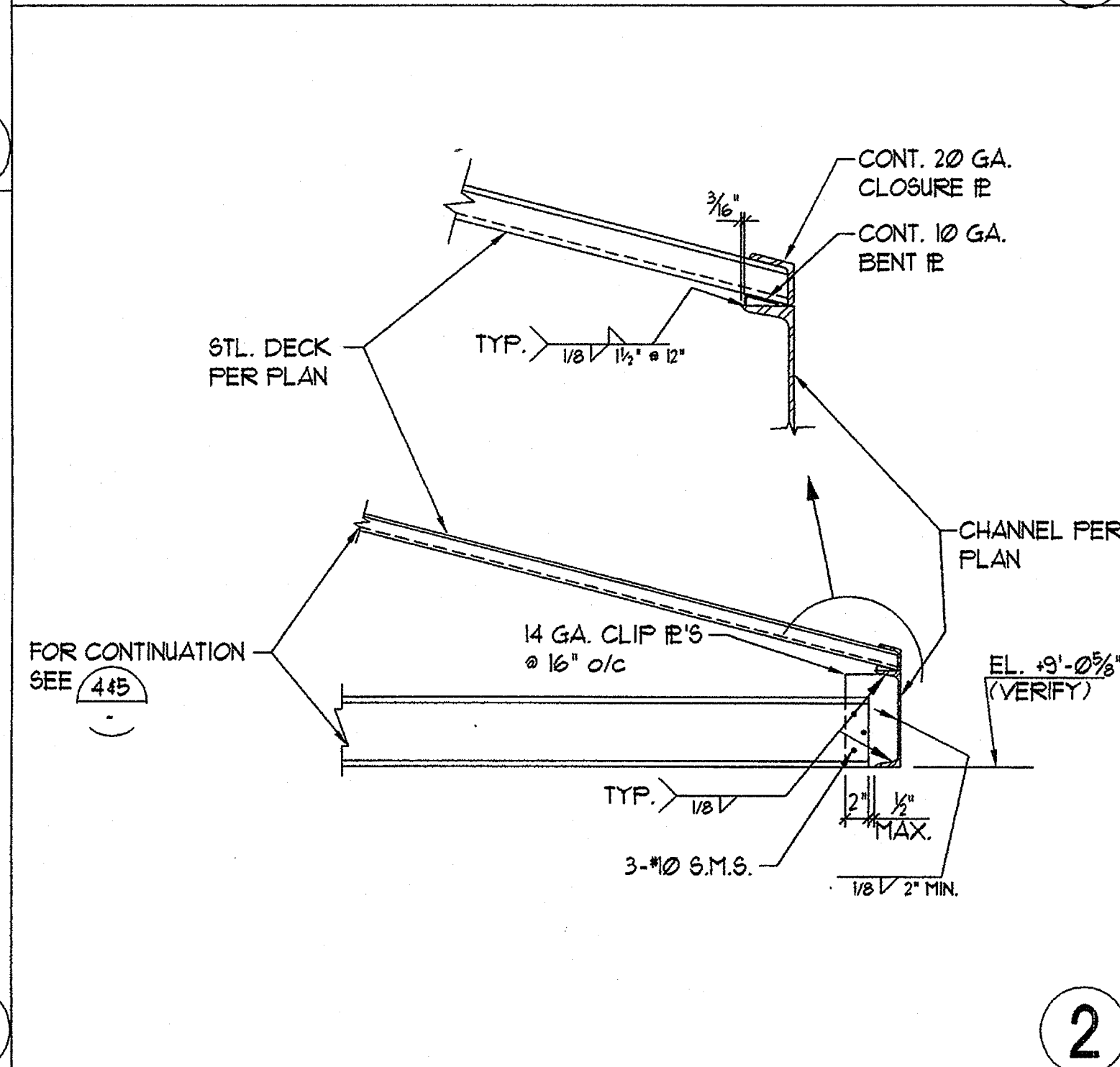
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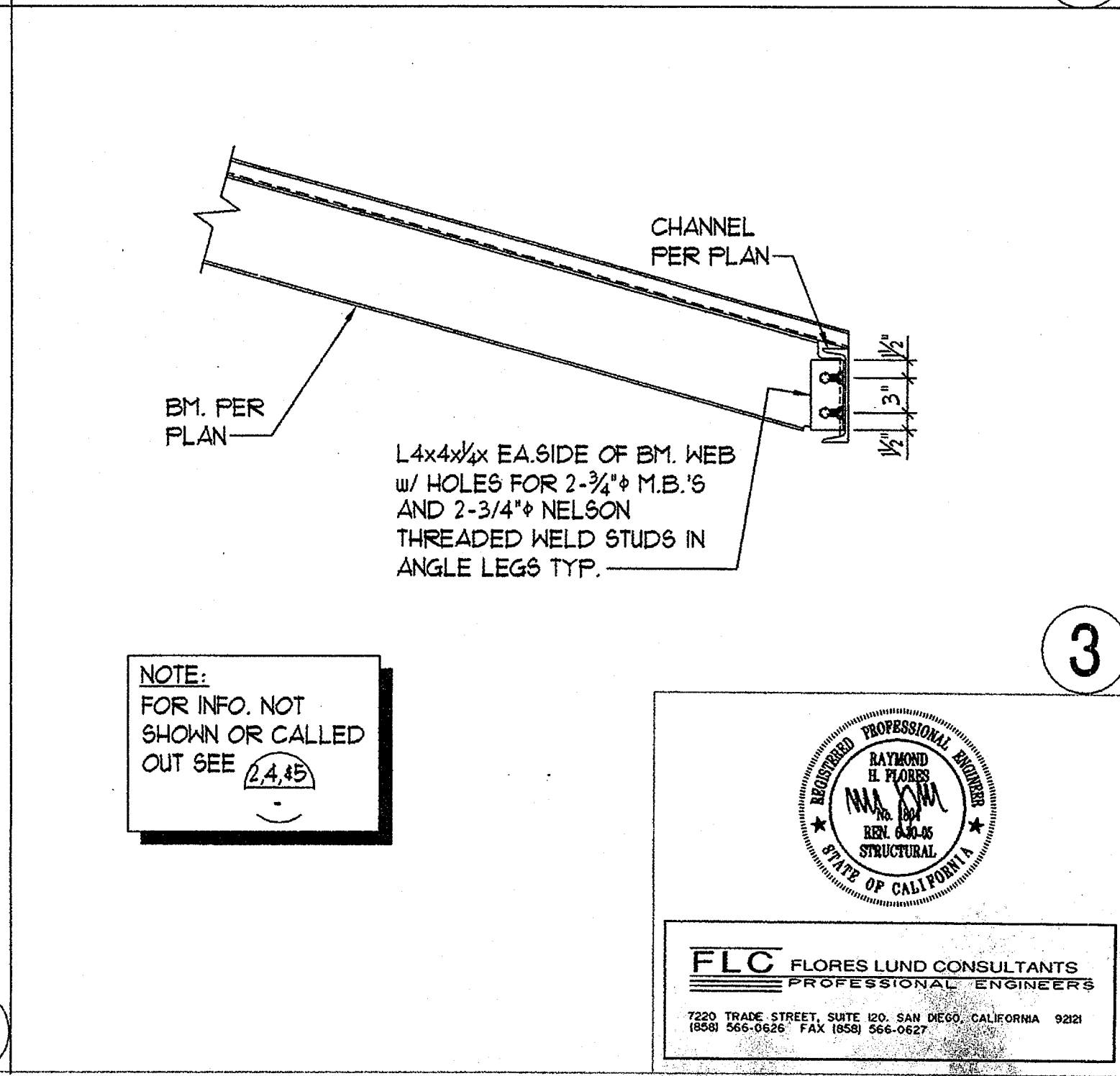
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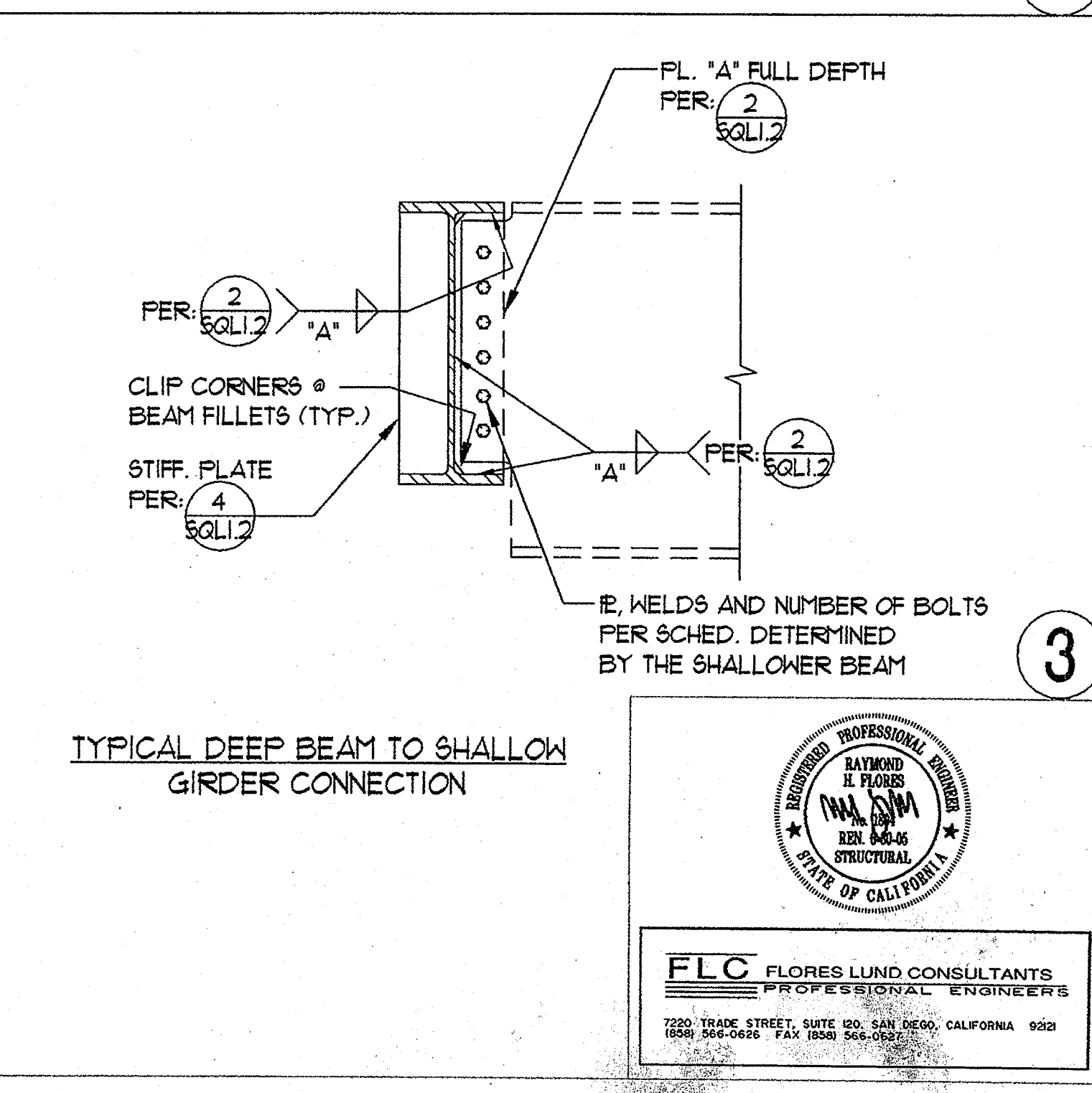
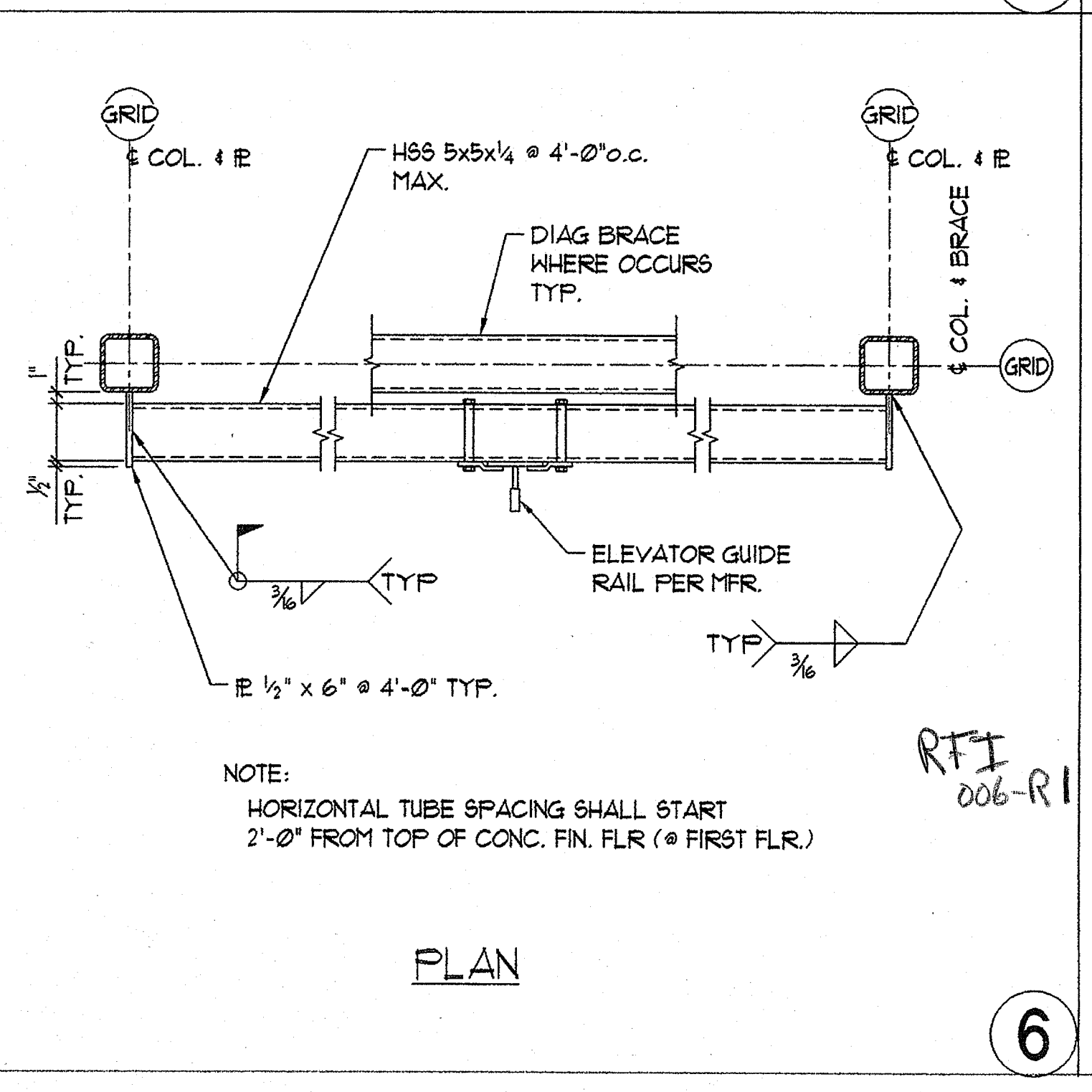
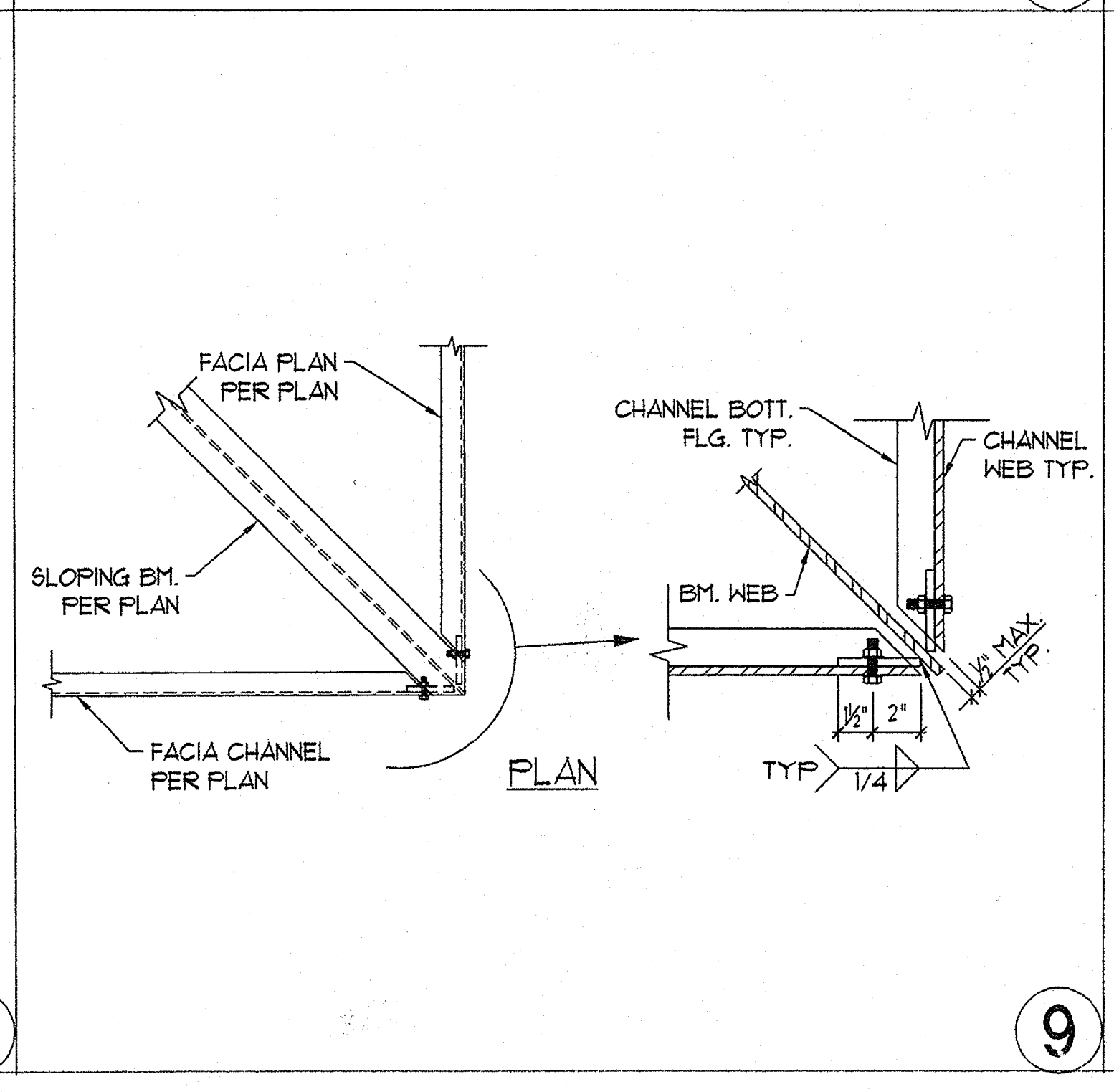
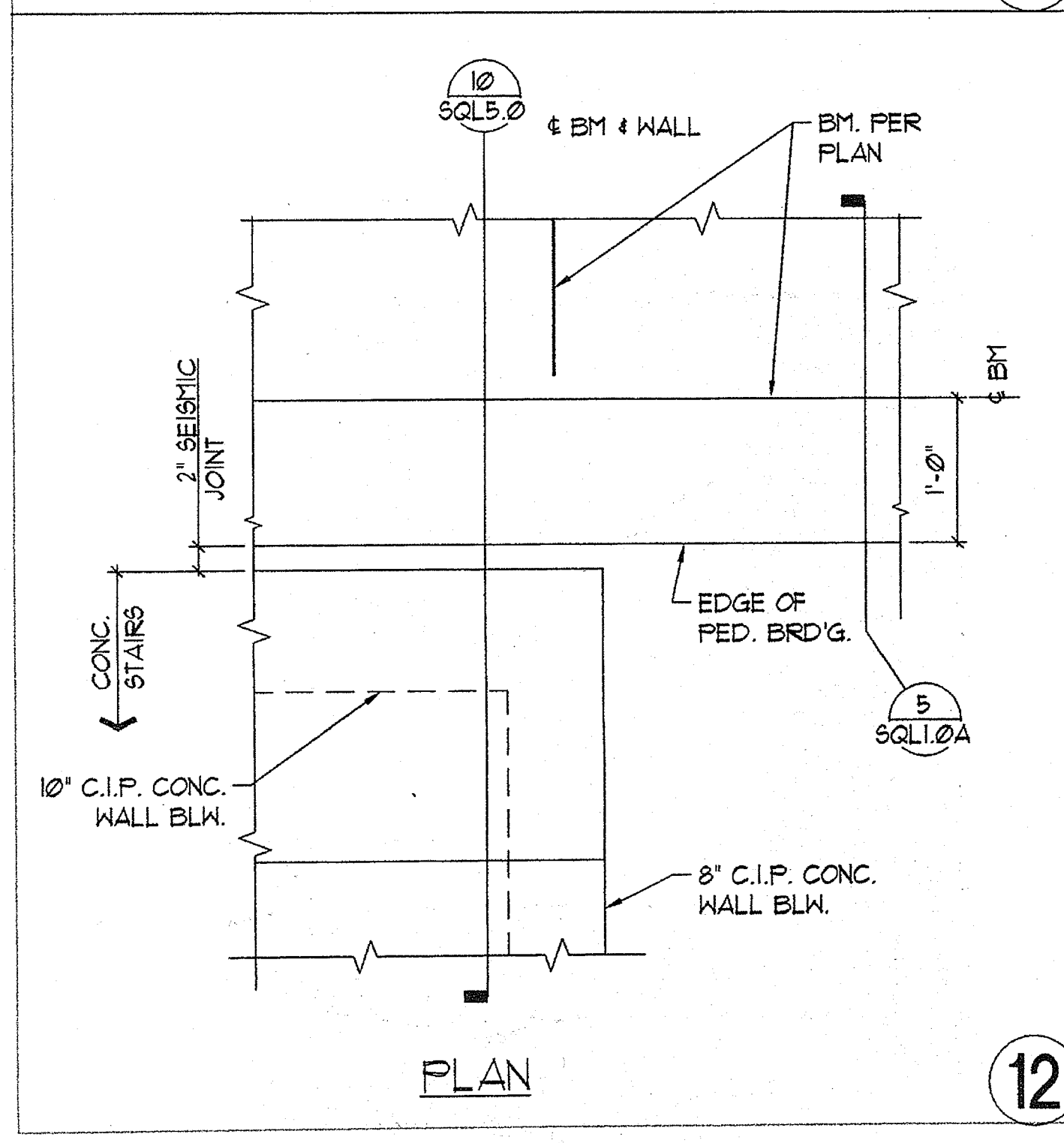
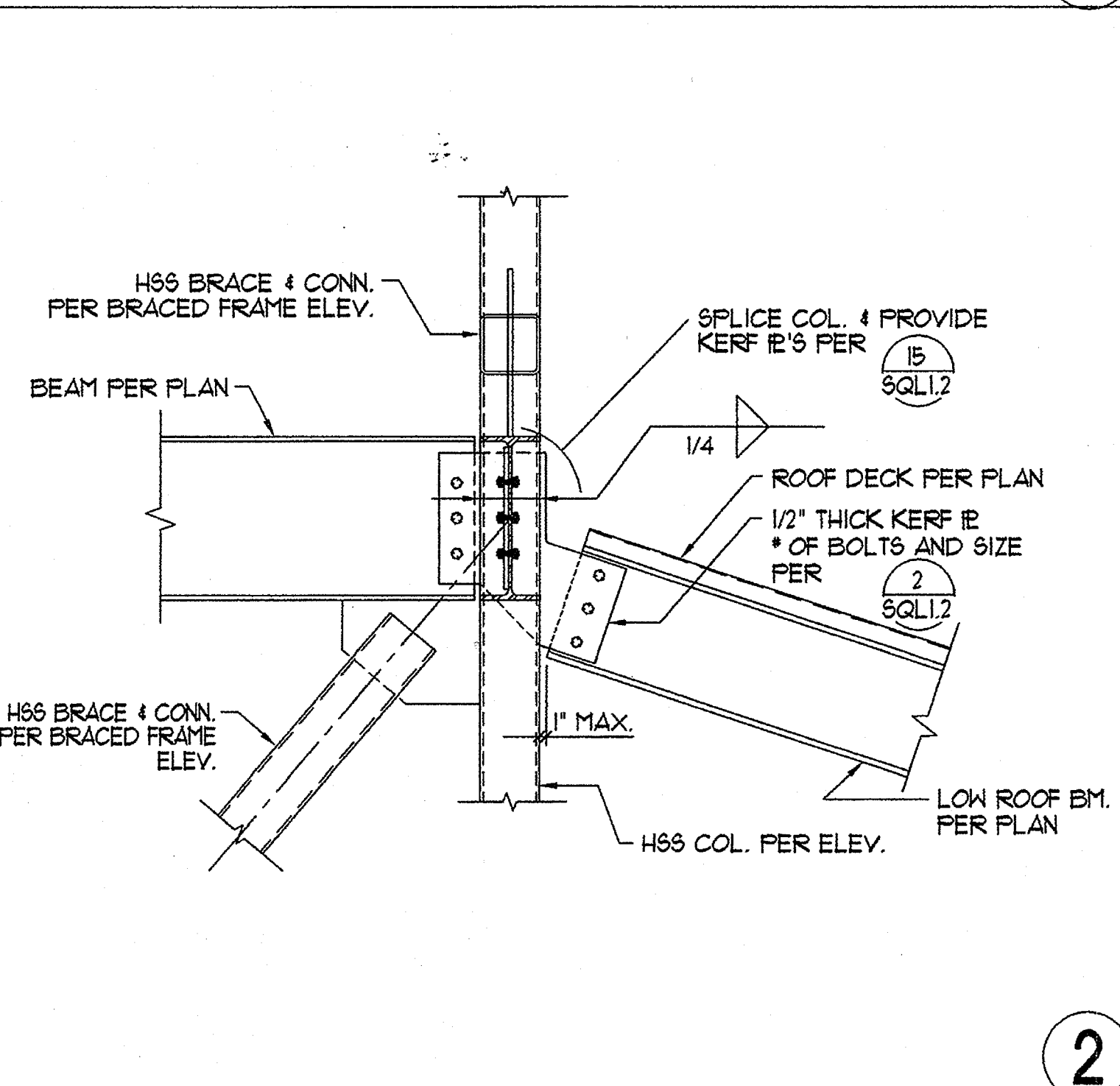
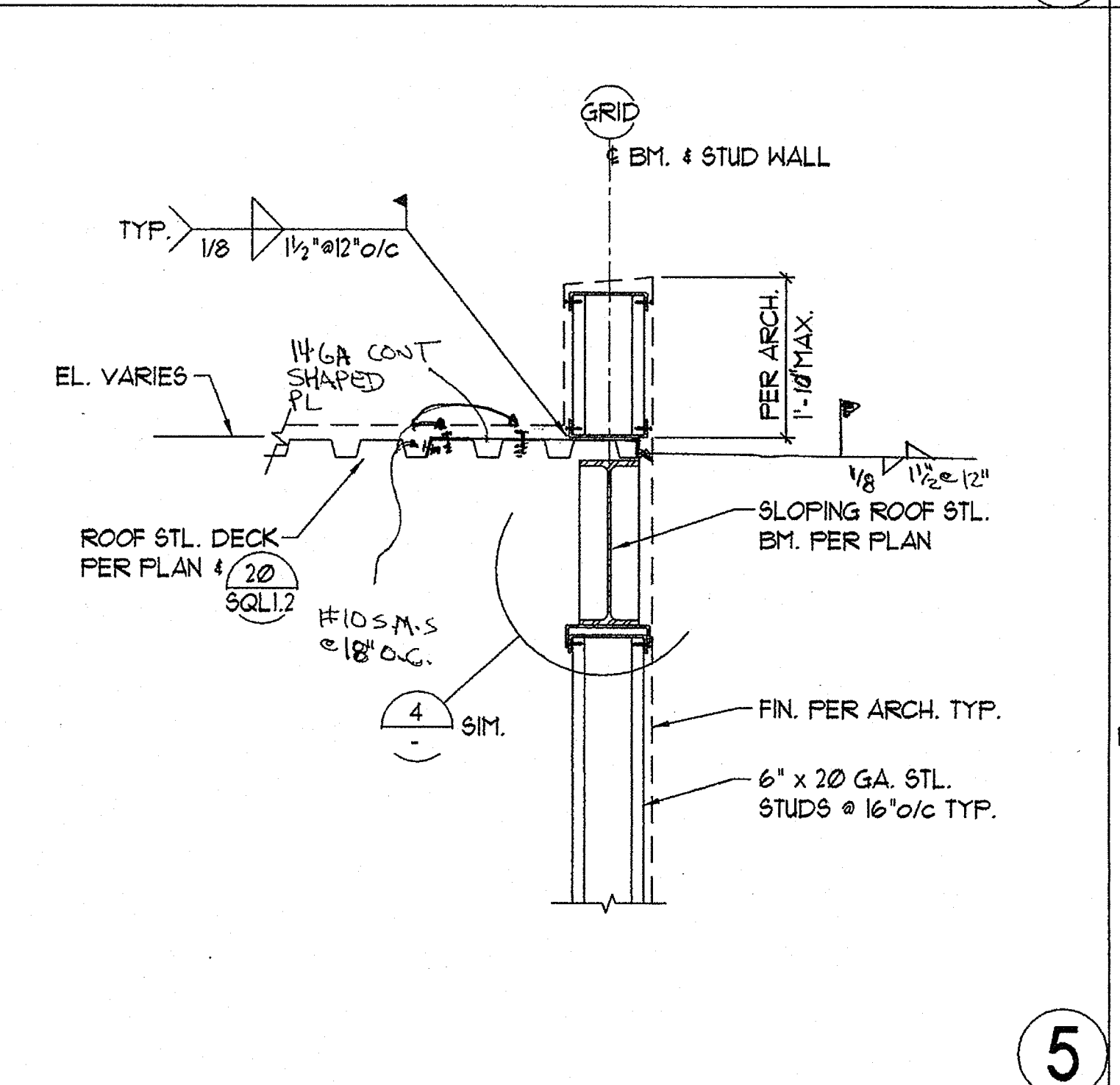
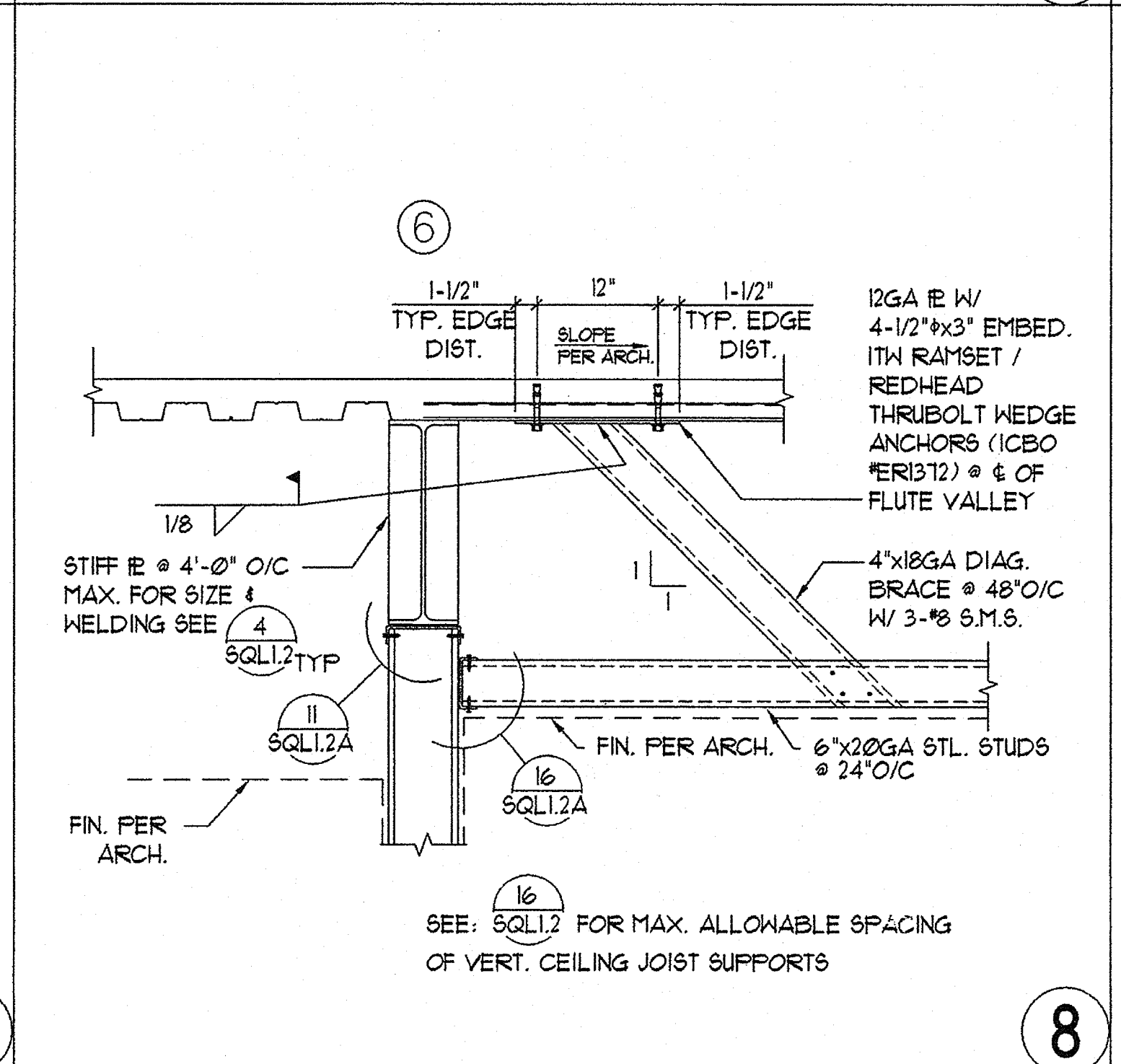
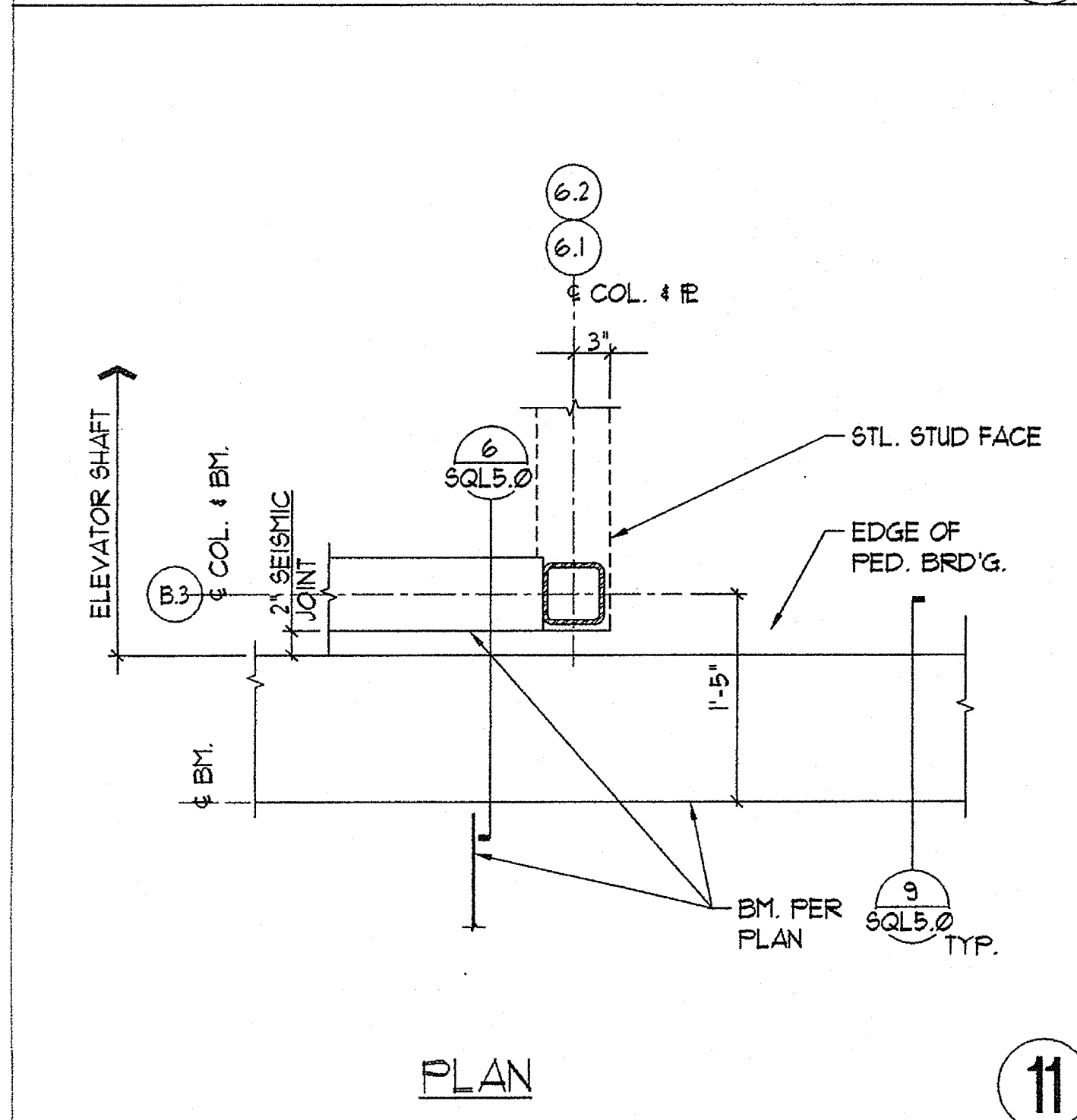
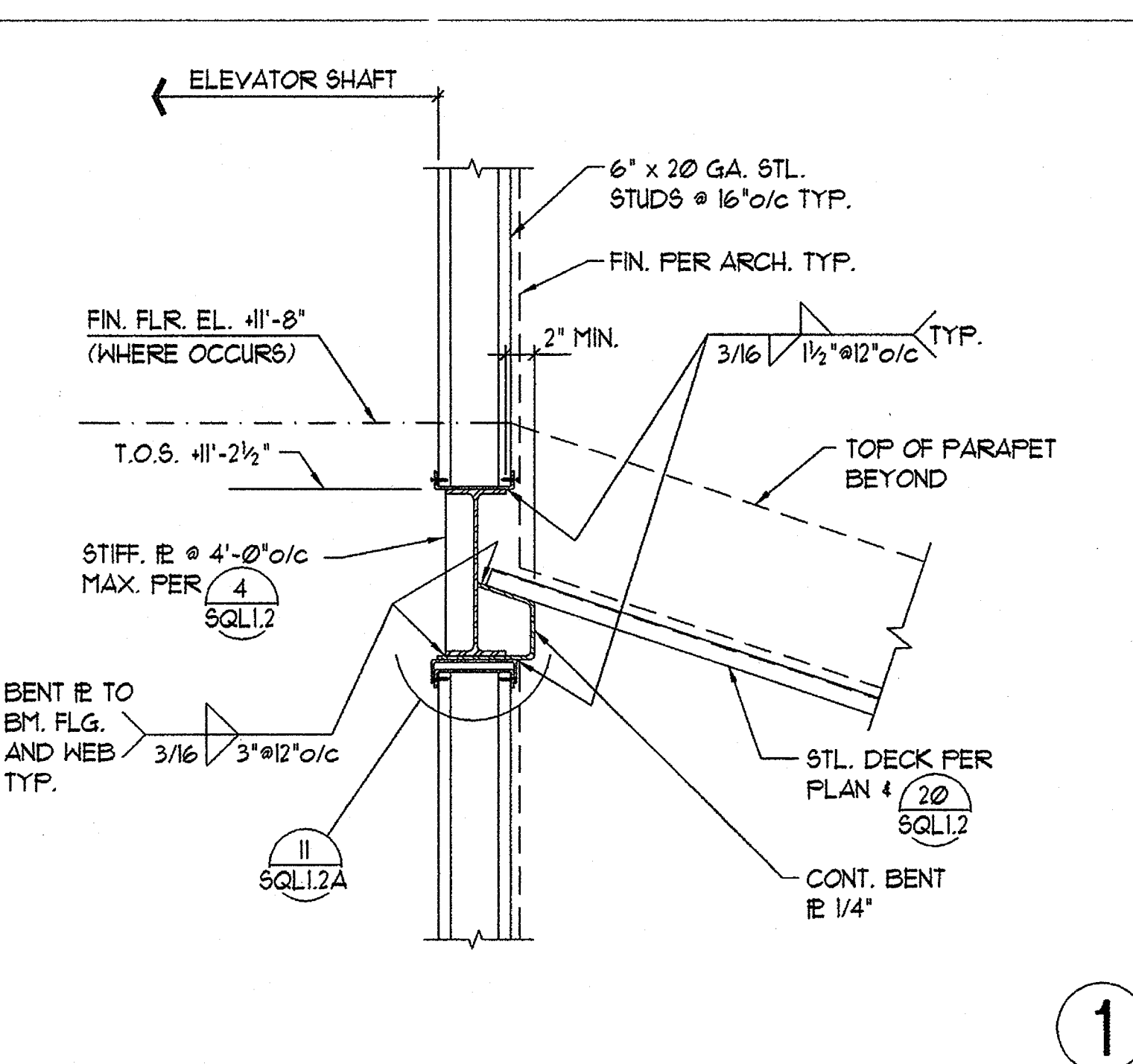
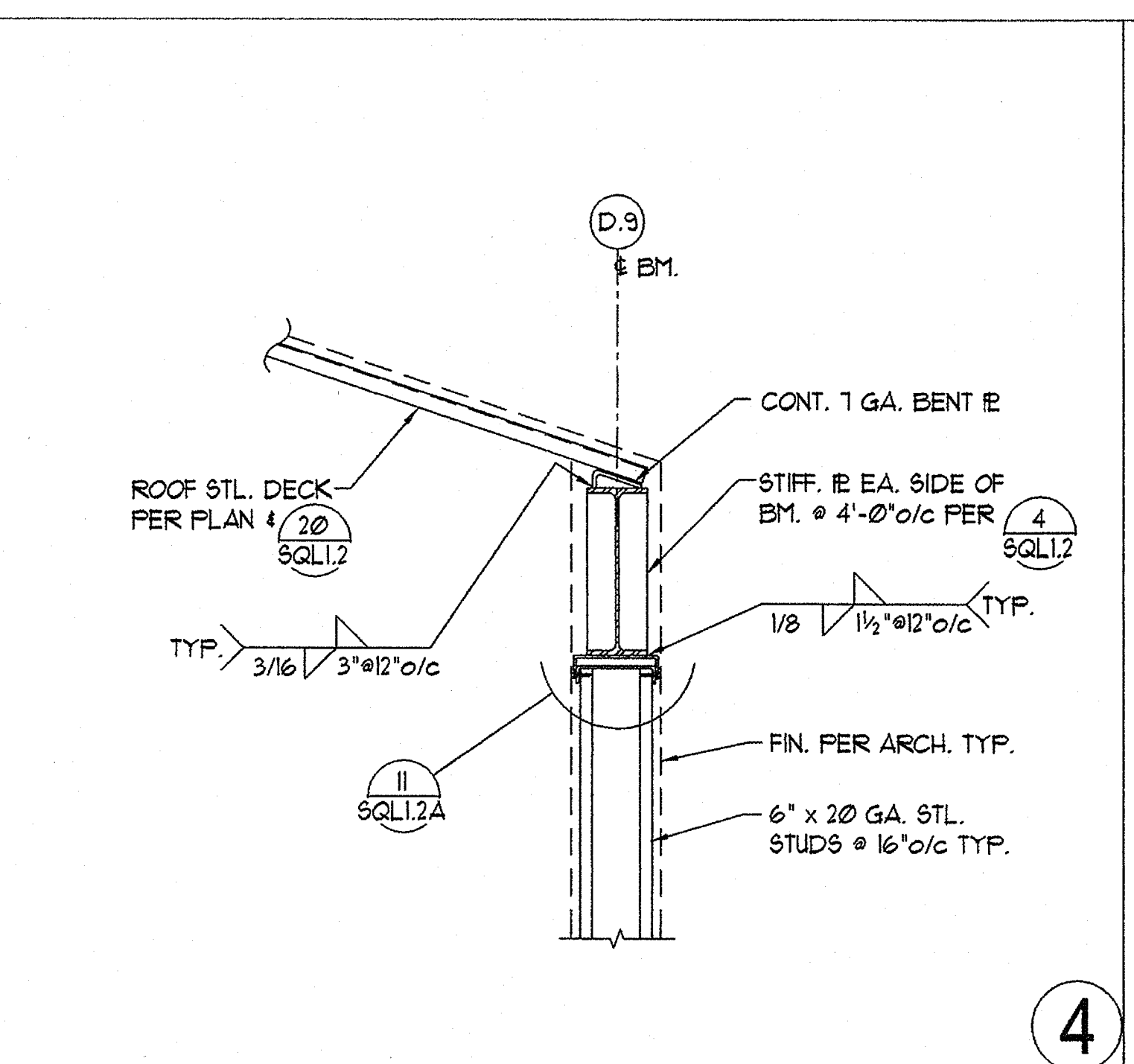
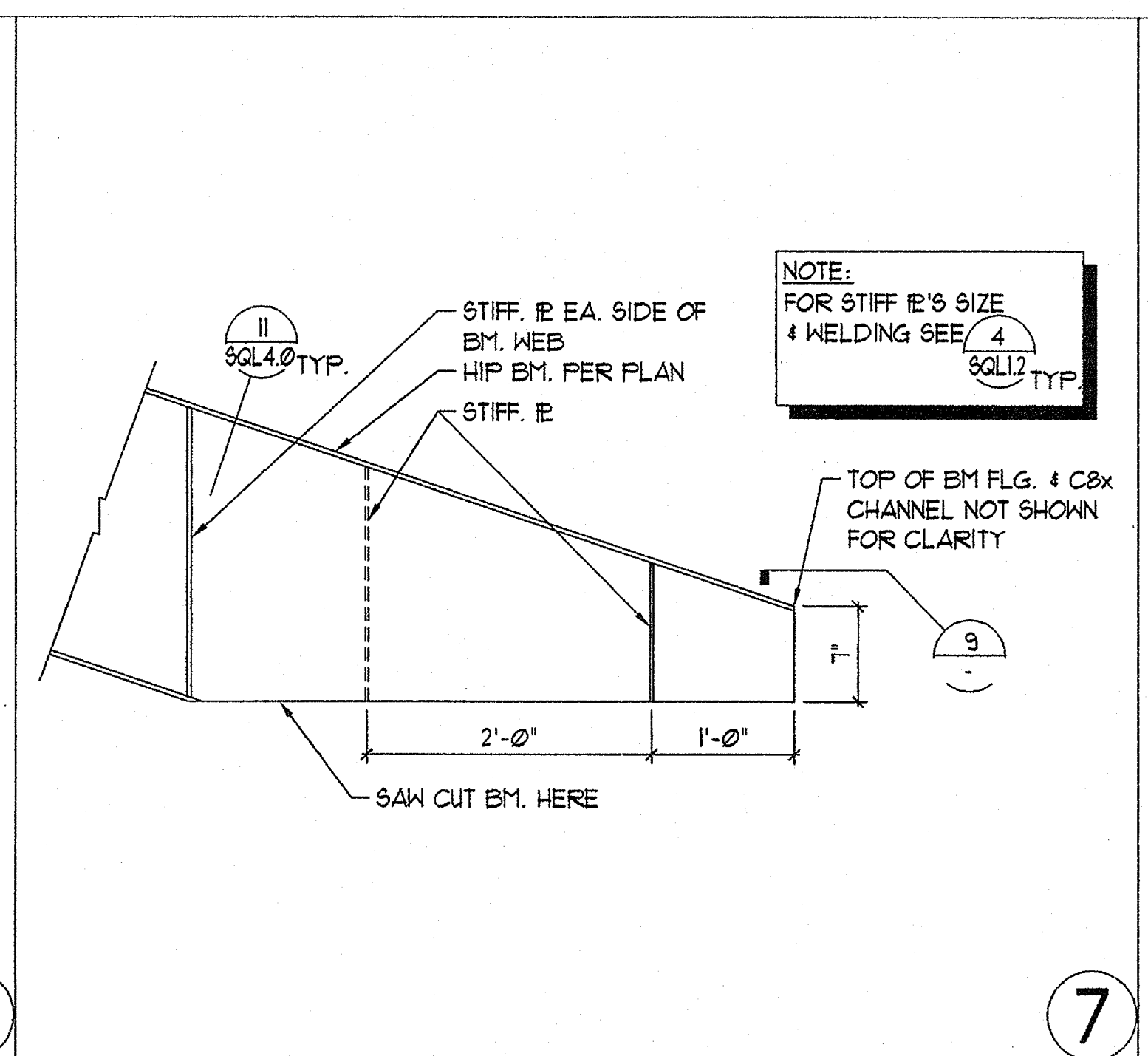
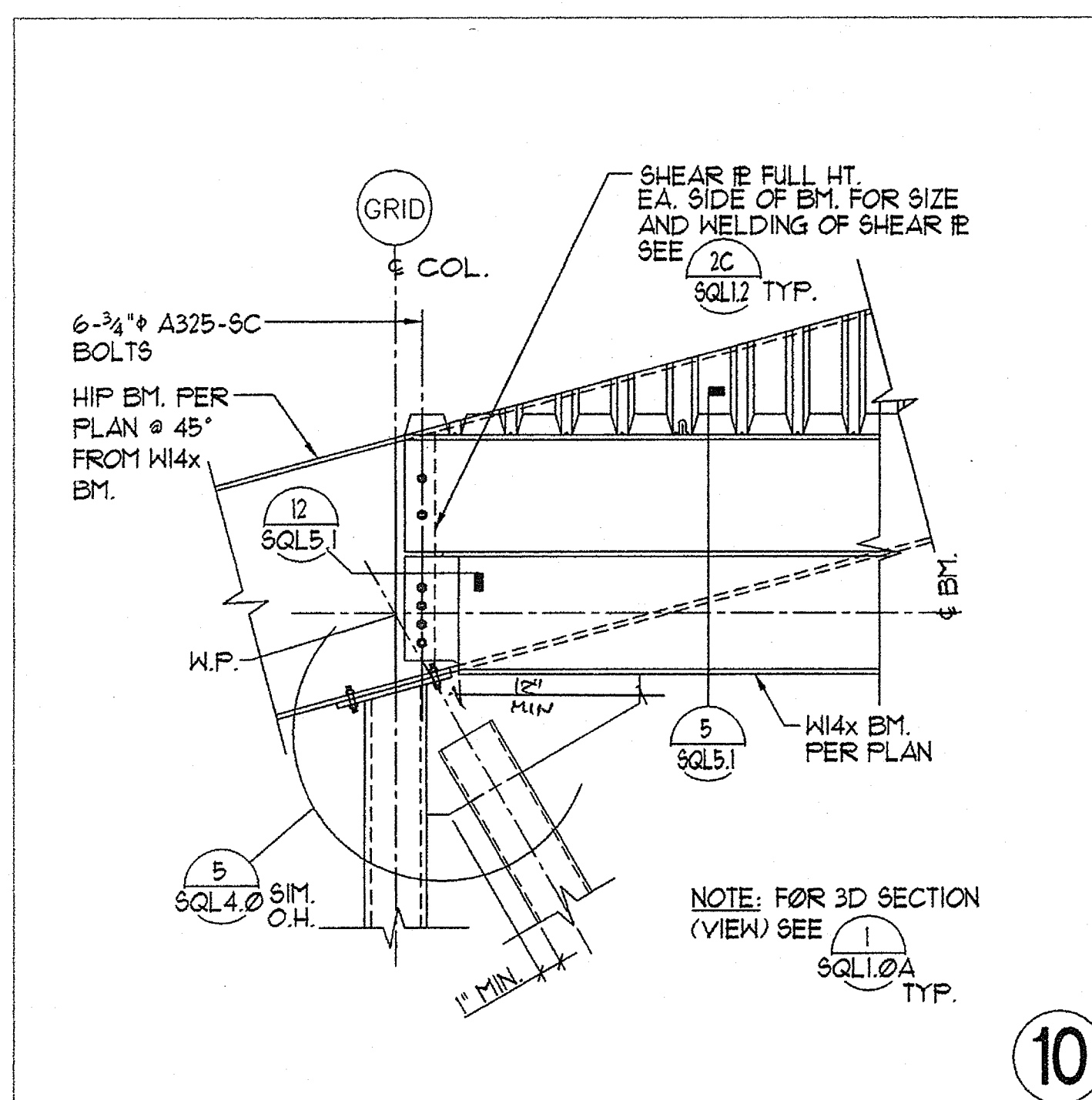
FLC FLORES LUND CONSULTANTS

PROFESSIONAL ENGINEERS

3355 MISSION AVE SUITE 234 OCEANSIDE, CALIFORNIA 92054

PHONE 760-754-8191

FAX 760-754-8291



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PROJECT NOS. 758-000

P. T. N. 73569-9

DATE

REVISIONS

JEFFERSON MS NEW CONSTRUCTION

823 ACACIA STREET
OCEANSIDE, CA 92054
OCEANSIDE UNIFIED S.D.

space art
function time

G-I

GROTH ARCHITECTS, INC. 3355 MISSION AVE. SUITE 234 OCEANSIDE, CALIFORNIA 92054 PHONE 760-754-8191 FAX 760-754-8291

DSA

IDENTIFICATION STAMP
DIV. OF THE STATE ARCHITECT
OFFICE OF REGULATION SERVICES
4-106494

AC: FLS: SS: [Signature]
DATE: MAR 28 2005

LICENSED ARCHITECT
JOHN SCOTT GROTH
C-26609
4/30/2007
STATE OF CALIFORNIA

SHEET TITLE

ROOF DETAILS

SQL5.2

FLC FLORES LUND CONSULTANTS
PROFESSIONAL ENGINEERS
7250 TRADE STREET, SUITE 200, SAN DIEGO, CALIFORNIA 92121
619-591-0076 FAX 619-591-0076

GENERAL NOTES

THE FOLLOWING GENERAL NOTES ARE A SUMMARY OF THE SPECIFICATIONS FOR THE CONVENIENCE OF THE CONTRACTOR. REFER TO THE SPECIFICATIONS AND ALL OTHER CONTRACT DOCUMENTS FOR ALL REQUIREMENTS.

GENERAL

- CONTRACTOR SHALL VERIFY ALL DIMENSIONS BEFORE STARTING WORK AND NOTIFY THE ARCHITECT IMMEDIATELY OF ANY DISCREPANCIES FOUND.
- SPECIFIC CODES AND DETAILS SHALL TAKE PRECEDENCE OVER THESE GENERAL NOTES AND THE TYPICAL DETAILS ON SN1.1 THROUGH SN1.3 IN CASE OF CONFLICT. NOTIFY ENGINEER FOR CLARIFICATION.
- WHERE NO CONSTRUCTION DETAILS ARE SHOWN OR NOTED FOR ANY PART OF THE WORK, THE DETAILS USED SHALL BE THE SAME AS FOR OTHER SIMILAR WORK, PROVIDED THAT PRIOR APPROVAL IS OBTAINED FROM THE ARCHITECT OR ENGINEER.
- THE DESIGN IS BASED ON TITLE 24, CODE OF REGULATIONS (C.C.R.) 2001 EDITION.
- NEITHER THE OWNER NOR THE ARCHITECT WILL ENFORCE SAFETY MEASURES OR REGULATIONS. THE CONTRACTOR SHALL DESIGN, CONSTRUCT AND MAINTAIN ALL SAFETY DEVICES, INCLUDING SIGNING AND ENGINERING, AND SHALL BE RESPONSIBLE FOR CONFORMING TO ALL LOCAL, STATE AND FEDERAL SAFETY AND HEALTH STANDARDS, LAWS AND REGULATIONS.

FOUNDATIONS:

A PRELIMINARY EXPLORATION OF THE SOILS UNDERLYING THIS SITE WAS MADE BY CONSTRUCTION TESTING & ENGINEERING INC. AND IS DESCRIBED IN A REPORT DATED JUNE 10, 2004.

THE FOLLOWING ARE RECOMMENDATIONS AND INTERPRETATIONS FROM THIS REPORT. FLORES LUND CONSULTANTS (FLC) CAN NOT BE HELD LIABLE FOR RECOMMENDATIONS PREPARED BY A DIFFERENT FIRM. (FLC'S EXPERIENCE IN THIS FIELD IS NIL).

- SITE PREPARATION**
BEFORE GRADING, THE SITE SHOULD BE CLEARED OF ANY EXISTING DEBRIS AND OTHER DELETERIOUS MATERIALS. IN AREAS TO RECEIVE STRUCTURES, OR DISTRESS-SENSITIVE IMPROVEMENTS, EXPANSIVE, SURFICIAL ERODED, DESICCATED, BURROWED, OR OTHERWISE LOOSE OR DISTURBED SOILS SHOULD BE REMOVED TO THE DEPTH OF COMPETENT MATERIALS. A DETERMINATION OF THE SUITABILITY OF THE EXPOSED SUBGRADES SHOULD BE MADE IN THE FIELD BY AN ENGINEER OR GEOLOGIST FROM THIS FIRM. ORGANIC AND OTHER DELETERIOUS MATERIALS NOT SUITABLE FOR STRUCTURAL BACKFILL SHOULD BE DISPOSED OF OFFSITE AT A LEGAL DISPOSAL SITE.
- SITE EXCAVATION**
BASED ON THE PRESENCE OF SHALLOW UNDOCUMENTED FILL OR TOPSOIL MATERIALS, AREAS BENEATH PROPOSED NEW STRUCTURES ARE TO BE EXCAVATED TO COMPETENT NATIVE MATERIALS AND TO A MINIMUM DEPTH OF 18 INCHES BELOW ALL PROPOSED FOUNDATIONS TO MINIMIZE EFFECTS OF DIFFERENTIAL SETTLEMENTS. THESE EXCAVATIONS CAN GENERALLY BE ACCOMPLISHED USING HEAVY-DUTY CONSTRUCTION EQUIPMENT. HOWEVER, LOCALIZED CEMENTED OR VERY HARD ZONES MAY BE ENCOUNTERED DURING THESE OPERATIONS. GRADING ACTIVITIES SHOULD BE CONTINUOUSLY MONITORED BY CTE. SUCH OBSERVATIONS ARE ESSENTIAL TO IDENTIFY FIELD CONDITIONS THAT DIFFER FROM THOSE IDENTIFIED DURING OUR SUBSURFACE INVESTIGATION AND ADJUST DESIGNS TO ACTUAL FIELD CONDITIONS ENCOUNTERED.
- FILL PLACEMENT AND COMPACTION**
AS STATED, AN ENGINEER OR GEOLOGIST FROM CTE SHOULD BE CALLED UPON TO VERIFY THAT THE PROPER SITE PREPARATION HAS OCCURRED BEFORE FILL PLACEMENT BEGINS. FOLLOWING THE REMOVAL OF LOOSE OR DISTURBED SOILS, AREAS TO RECEIVE FILLS OR CONCRETE OR SLABS ON GRADE SHOULD BE SCARIFIED, MOISTURE CONDITIONED TO ABOVE OPTIMUM MOISTURE CONTENT, AND PROPERLY COMPACTED. FILL AND BACKFILL SHOULD BE COMPACTED TO A MINIMUM RELATIVE COMPACTION OF 90 PERCENT AS EVALUATED BY ASTM D-1557 AT MOISTURE CONTENTS BETWEEN OPTIMUM AND TWO PERCENT ABOVE OPTIMUM. THE OPTIMUM LIFT THICKNESS FOR BACKFILL SOIL WILL BE DEPENDENT ON THE TYPE OF COMPACTION EQUIPMENT USED. GENERALLY BACKFILL SHOULD BE PLACED IN UNIFORM LIFTS NOT EXCEEDING EIGHT INCHES IN LOOSE THICKNESS. BACKFILL PLACEMENT AND COMPACTION SHOULD BE DONE IN OVERALL CONFORMANCE WITH GEOTECHNICAL RECOMMENDATIONS AND LOCAL ORDINANCES.
- FILL MATERIALS**
SOILS DERIVED FROM ON-SITE MATERIALS ARE CONSIDERED SUITABLE FOR REUSE ON THE SITE AREA AS FILL, PROVIDED THEY ARE SCREENED OF ORGANIC MATERIALS AND MATERIALS GREATER THAN THREE INCHES IN MAXIMUM DIMENSION. IMPORTED FILL BENEATH STRUCTURES, PAVEMENTS AND WALKS SHOULD HAVE AN EXPANSION INDEX LESS THAN OR EQUAL TO 30 (PER UBC 18-1-B) WITH LESS THAN 35 PERCENT PASSING THE NO. 200 SIEVE. IMPORTED FILL SOILS FOR USE IN STRUCTURAL OR SLOPE AREAS SHOULD BE EVALUATED BY THE SOILS ENGINEER TO DETERMINE STRENGTH CHARACTERISTICS BEFORE PLACEMENT ON THE SITE.
- THE ALLOWABLE SOIL BEARING PRESSURE IS 2,000 PSF FOR CONTINUOUS AND ISOLATED SPREAD FOOTINGS. FOOTINGS SHALL EXTEND A MINIMUM DEPTH OF 18 INCHES BELOW LOWEST ADJACENT SUBGRADE.
- FOOTING ELEVATIONS SHOWN ARE FOR BIDDING PURPOSES ONLY AND ARE ASSUMED TO BE IN SUITABLE BEARING MATERIALS. THE ACTUAL ADEQUACY OF THE BEARING MATERIAL SHALL BE DETERMINED BY A PROFESSIONAL, LICENSED GEOTECHNICAL ENGINEER REGISTERED IN THE STATE OF CALIFORNIA PRIOR TO PLACING OF REINFORCING OR POURING OF CONCRETE, AND FOOTING ELEVATIONS SHALL BE ADJUSTED, OR OTHER REMEDIAL ACTION TAKEN, AS DIRECTED BY THIS PROFESSIONAL AND APPROVED BY THE ARCHITECT, AND DSA.
- ALL ANCHORS SHALL BE TIED IN PLACE PRIOR TO CALLING FOR FOUNDATION INSPECTION.

STRUCTURAL STEEL

- STRUCTURAL WIDE FLANGE STEEL SHALL COMPLY WITH ASTM A 992 GRADE 50, EXCEPT FOR ANGLES, PLATES AND MISC. STEEL WITH ASTM A-36.
- MATERIALS AND WORKMANSHIP SHALL COMPLY WITH A.I.S.C. "SPECIFICATION FOR THE DESIGN, FABRICATION AND ERECTION OF STRUCTURAL STEEL FOR BUILDINGS".
- ALL WELDING SHALL BE BY THE ARC WELDING PROCESS USING E-70 ELECTRODES AND CERTIFIED WELDERS. THE USE OF E70T-4 IS PROHIBITED.
- CONNECTED MEMBERS SHALL BEAR ONLY UPON THE UNTHREADED PORTION OF BOLTS.
- ROUND HSS (HOLLOW STRUCTURAL SECTION) SHALL COMPLY WITH ASTM A-53, GRADE B (FY=35 KSI).
- RECTANGULAR HSS (HOLLOW STRUCTURAL SECTION) SHALL COMPLY WITH ASTM A 500, GRADE B (FY=46 KSI).
- HIGH STRENGTH BOLTS SHALL COMPLY WITH ASTM A-325, UNLESS OTHERWISE NOTED.
- MACHINE BOLTS SHALL COMPLY WITH ASTM A-307, UNLESS OTHERWISE NOTED.
- ALL FIELD & SHOP WELDING AND HIGH STRENGTH BOLTING SHALL BE CONTINUOUSLY INSPECTED BY A REGISTERED DEPUTY INSPECTOR.
- STEEL SHALL BE IDENTIFIED BY MILL CERTIFICATES.
- BOLT HOLES- STANDARD BOLT HOLES. FOR BEAM AND GIRDERS- 1/16" LARGER THAN BOLT SIZE. FOR BASE PLATES- 5/16" OVERSIZE BOLT HOLES. FOR BOLTS LESS THAN OR EQUAL TO 1" DIAMETER- 1/2" OVERSIZE BOLT HOLES FOR BOLTS GREATER THAN 1" DIAMETER.
- BASE PLATES: BEDDED ON 1 1/2" MIN. NON-SHRINK GROUT, U.O.N.
- FILLER METAL USED IN ARC WELDING SHALL BE IN ACCORDANCE WITH TABLE 4.1.1 OF "STRUCTURAL WELDING CODE - STEEL" (AWS D1.1-00). THE MIN. TENSILE STRENGTH FOR FILLER METAL SHALL BE 70 KSI.
- ALL FULL PENETRATION BUTT WELDS ARE REQUIRED TO HAVE ULTRASONIC TESTING (UT) PERFORMED BY A CERTIFIED TESTING INSPECTION LABORATORY.
- WELDS IDENTIFIED AS REQUIRING CONTINUOUS OR PERIODIC SPECIAL INSPECTION NEED NOT HAVE A SPECIAL INSPECTION WHEN WELDING IS DONE IN AN APPROVED FABRICATOR'S SHOP, HOWEVER, THE APPROVED FABRICATOR MUST SUBMIT A CERTIFICATE OF COMPLIANCE IN ACCORDANCE WITH U.B.C. SECTION 306 (f)
- STRUCTURAL STEEL SHOP DRAWINGS: TO BE REVIEWED BY THE ARCHITECT AND STRUCTURAL ENGINEER BEFORE FABRICATION.
- EXPOSED STRUCTURAL STEEL AND MISCELLANEOUS STEEL: HOT DIP GALVANIZED.
- ALL WELDS USED IN PRIMARY MEMBERS AND CONNECTIONS IN THE LATERAL FORCE-RESISTING SYSTEM SHALL BE MADE WITH A FILLER METAL THAT HAS A MINIMUM CHARRY V-NOTCH TOUGHNESS OF 20 ft.-lbs., AT MINUS 20 DEGREES F, AS DETERMINED BY AWS CLASSIFICATION OR MANUFACTURE CERTIFICATION. IN ADDITION, ALL WELDS DENOTED BY "C.V.N." MUST MEET THIS REQUIREMENT.

METAL DECKS

- METAL DECK SHALL BE STEEL AND SHALL BE "VERCO" PER PLAN. STEEL DECK OF OTHER MANUFACTURERS WILL BE ACCEPTABLE, PROVIDED THE DECK HAS PHYSICAL DIMENSIONS AND PHYSICAL PROPERTIES EQUAL TO OR GREATER THAN THE VERCO STEEL DECK, INCLUDING I.C.B.O. APPROVED DIAPHRAGM SHEAR VALUES.
- ALL METAL SHALL BE GALVANIZED.
- THE METAL DECK IS USED AS A DIAPHRAGM.
- METAL DECK SHALL BE PLACED IN THREE SPAN LENGTHS WHEREVER POSSIBLE.
- METAL DECK SHALL BE ERECTED SUCH THAT RIBS ARE NORMAL TO THE SUPPORTING MEMBERS.
- HOLES OR OTHER OPENINGS LARGER THAN 6" IN DIAMETER, IF NOT SHOWN ON STRUCTURAL DRAWINGS, SHALL BE APPROVED BY ENGINEER PRIOR TO CUTTING.

SPECIAL INSPECTION

SPECIAL INSPECTION BY SPECIAL INSPECTORS SATISFACTORY TO THE BUILDING OFFICIAL IS REQUIRED FOR THE FOLLOWING TYPES OF WORK IN CONFORMANCE WITH THE "CALIFORNIA BUILDING CODE (1997) UNIFORM BUILDING CODE AND REVISIONS INCLUDED IN THE TITLE 24) CHAPTER 17A.

- CONCRETE - SPECIAL INSPECTION IS REQUIRED FOR THE FOLLOWING:
 - CONCRETE PLACEMENT: DURING THE TAKING OF TEST SPECIMENS AND PLACING OF REINFORCED CONCRETE AND PNEUMATICALLY PLACED CONCRETE.
 - BOLTS INSTALLED IN CONCRETE: WHEN THE STRUCTURAL DESIGN REQUIRES AN INCREASED DESIGN VALUE, SPECIAL INSPECTION IS REQUIRED DURING THE INSTALLATION OF THE BOLTS AND PLACING OF THE CONCRETE AROUND SUCH BOLTS.
 - REINFORCING STEEL: DURING THE PLACING OF REINFORCING STEEL FOR ALL CONCRETE.
- MASONRY STRUCTURAL - SPECIAL INSPECTION IS REQUIRED FOR THE FOLLOWING:
 - DURING PREPARATION AND THE TAKING OF ANY REQUIRED PRISMS OR TEST SPECIMENS.
 - PLACING OF ALL MASONRY UNITS.
 - PLACEMENT OF REINFORCEMENT.
 - INSPECTION OF GROUT SPACE: IMMEDIATELY PRIOR TO CLOSING OF CLEANOUTS, AND DURING ALL GROUTING OPERATIONS.
- WELDING - SPECIAL INSPECTION IS REQUIRED FOR THE FOLLOWING:
 - DURING ALL FIELD WELDING.
 - DURING ALL STRUCTURAL WELDING INSPECTION OF ALL SHOP AND FIELD WELDING OPERATIONS, INCLUDING THE INSTALLATION OF AUTOMATIC END-WELDED STUD SHEAR CONNECTORS SHALL BE MADE BY A QUALIFIED WELDING INSPECTOR APPROVED BY DSA. SUCH INSPECTOR SHALL BE A PERSON TRAINED AND THOROUGHLY EXPERIENCED IN INSPECTING WELDING OPERATIONS. THE INSPECTOR'S ABILITY TO DISTINGUISH BETWEEN SOUND AND UNSOUND WELDING SHALL BE RELIABLY ESTABLISHED. THE MINIMUM REQUIREMENTS FOR A QUALIFIED WELDING INSPECTOR SHALL BE AS THOSE FOR AN AWS CERTIFIED WELDING INSPECTOR (CWI), AS DEFINED IN THE PROVISIONS OF THE 1992 EDITION OF AWS QCI, STANDARD AND GUIDE FOR QUALIFICATION AND CERTIFICATION OF WELDING INSPECTORS PUBLISHED BY THE AMERICAN WELDING SOCIETY. ALL WELDING INSPECTORS SHALL BE AS APPROVED BY DSA.
 - DURING ALL STRUCTURAL WELDING OF REINFORCING STEEL.
 - INSTALLATION AND INSPECTION OF HIGH STRENGTH BOLTS SHALL BE IN ACCORDANCE WITH THE SPECIFICATION FOR STRUCTURAL JOINTS USING ASTM A-325 BOLTS OF THE RESEARCH COUNCIL OF THE ENGINEERING FOUNDATION, NOVEMBER 13, 1985. 2727 (f), IR 27-7 & UBC STANDARD SECTION 27.705 (c).

STRUCTURAL OBSERVATION (AS REQUIRED BY SECTION 1702A OF THE C.B.C.)

- THE DESIGN PROFESSIONAL RESPONSIBLE FOR THE STRUCTURAL DESIGN SHALL SUBMIT A STATEMENT IN WRITING TO DSA, STATING THAT DURING THE CONSTRUCTION OF THIS STRUCTURE, SITE VISITS HAVE BEEN PERFORMED TO OBSERVE GENERAL COMPLIANCE WITH THE APPROVED STRUCTURAL PLANS, SPECIFICATIONS AND CHANGE ORDERS. FURTHER, THE STATEMENT SHALL, IN DETAIL, NOTE HOW ANY DEFICIENCIES HAVE BEEN CORRECTED.
- AT A MINIMUM, THIS STRUCTURE REQUIRES SITE OBSERVATIONS BEFORE THE FOLLOWING HAS STARTED:
 - POURING THE INITIAL FOOTINGS.
 - DURING THE START OF ERECTION OF THE ROUGH STRUCTURE.
 - AFTER COMPLETION OF ALL THE ROUGH FRAMING.

REINFORCED CONCRETE - CONTINUED

- WALLS AND COLUMNS SHALL BE DOWELED FROM SUPPORTS WITH BARS OF THE SAME SIZE AND SPACING.
- SPLICE CONTINUOUS REINFORCING IN ACCORDANCE WITH THE TYPICAL DETAIL 5 ON SHEET SN1.1 STAGGER SPLICE ALL REINFORCING.
- PROVIDE MINIMUM EMBEDMENT OF REINFORCING IN CONFORMANCE WITH THE TYPICAL DETAIL 1 ON SHEET SN1.1
- REFER TO THE ARCHITECTURAL, MECHANICAL AND ELECTRICAL DRAWINGS FOR LOCATIONS OF PIPES, DUCTS, VENTS AND SIMILAR OPENINGS.
- REINFORCING, ANCHOR BOLTS AND ALL OTHER EMBEDDED ITEMS SHALL BE SECURELY HELD IN POSITION AND SHALL BE INSPECTED PRIOR TO PLACING CONCRETE.
- CHAMFER: 3/4" ON ALL EXPOSED CORNERS.
- ALL ANCHOR BOLTS SHALL BE TIED IN PLACE PRIOR TO PLACING CONCRETE.
- NO FIELD BENDING OF REINFORCEMENT WILL BE PERMITTED UNLESS APPROVED BY DSA AND THE ENGINEER.

REINFORCED MASONRY (SPECIAL INSPECTION REQ'D PER U.B.C.)

- CONCRETE BLOCK UNITS (CMU) SHALL BE MEDIUM WEIGHT, OPEN ENDED UNITS SOLID GROUTED AND COMPLY WITH ASTM C90, GRADE 'N', AND SHALL ALSO COMPLY WITH U.B.C. STANDARD NO. 24-4 FOR f'm = 1500 PSI. LAY BLOCK IN RUNNING BOND. PROVIDE DEEP CUT BOND BEAM UNITS WHERE HORIZONTAL REINFORCEMENT OCCURS.
- MORTAR SHALL HAVE A MINIMUM COMPRESSIVE STRENGTH OF 1800 PSI AFTER 28 DAYS. RECOMMENDED MIXING PROPORTIONS BY VOLUME:
PORTLAND CEMENT: 1 PART
SAND (BASED ON DAMP LOOSE CONDITION): NOT LESS THAN 2 1/4 AND NOT MORE THAN 3 TIMES THE SUM OF THE SEPARATE VOLUMES OF CEMENTITIOUS MATERIALS.
LIME: OVER 1/4 TO 1/2 PART BY VOLUME.
- COARSE GROUT SHALL HAVE A MINIMUM COMPRESSIVE STRENGTH OF 2000 PSI AFTER 28 DAYS. RECOMMENDED MIXING PROPORTIONS BY VOLUME:
PORTLAND CEMENT: 1 PART
SAND (BASED ON DAMP LOOSE CONDITION): 2 1/4 TO 3 TIMES THE SUM OF THE SEPARATE VOLUMES OF THE CEMENTITIOUS MATERIALS.
PEA GRAVEL (BASED ON DAMP LOOSE CONDITION): 1 TO 2 TIMES THE SUM OF THE VOLUMES OF THE CEMENTITIOUS MATERIALS.
- ALL REINFORCING STEEL SHALL COMPLY WITH ASTM A615, GRADE 60 FOR NO. 4 & LARGER, OTHERWISE GRADE 40. WELDED REINFORCEMENT SHALL COMPLY WITH ASTM, A706 OR VERIFIED EQUIVALENT.
- UNLESS OTHERWISE DETAILED: LAP #4 AND #5 REINFORCEMENT BARS A MINIMUM OF 48 DIAMETERS, LAP #6 AND LARGER REINFORCEMENT BARS A MINIMUM OF 60 DIAMETERS. STAGGER ALL HORIZONTAL LAP SPLICES.
- UNLESS OTHERWISE DETAILED: FOR ALL HORIZONTAL REINFORCING, PROVIDE A STANDARD 90 DEGREE HOOK AT THE END OF ALL WALLS AND ADJACENT TO OPENINGS. HOOK THE REINFORCING INTO THE INTERSECTING WALL OR HOOK INTO THE LAST VERTICAL CELL WHERE NO INTERSECTING WALL OCCURS.
- PROVIDE 3/4" MINIMUM OR ONE BAR DIAMETER, WHICHEVER IS GREATER, GROUT BETWEEN THE FACE OF A REBAR AND ANY INTERIOR SURFACE OF THE MASONRY.
- VERTICAL REINFORCEMENT IN MASONRY WALLS SHALL BE SPACED A MAXIMUM OF 16" O/C.
- NO PIPES, CONDUITS OR ANY FORAING MATERIAL SHALL BE PLACED ON THE CMU WALL CELLS UNLESS APPROVED PRIOR TO THE START OF ANY MASONRY WORK OR IT IS SPECIFICALLY DETAILED. FAILURE TO COMPLY WITH THIS, WILL BE A CAUSE FOR REMOVAL, AT CONTRACTORS COST.
- ALL VERTICAL REINFORCING SHALL BE IN ONE PIECE FROM FOOTING TO ROOF, OR FROM FOOTING TO FLOOR AND FROM FLOOR TO ROOF AND SHALL BE SECURED AGAINST DISPLACEMENT PRIOR TO GROUTING BY WIRE POSITIONERS OR OTHER SUITABLE DEVICES NOT EXCEEDING 200 BAR DIAMETER INTERVALS NOR 10"-0" MAXIMUM ON CENTER. SPLICING OF REINFORCEMENT WILL NOT BE ACCEPTED UNLESS APPROVED PRIOR TO THE START OF ANY MASONRY WORK.
- ALL CELLS SHALL BE GROUTED SOLID. GROUT LIFTS SHALL NOT EXCEED 4'-0" IN HEIGHT. CLEANOUT HOLES SHALL BE PROVIDED AT THE BASE OF ALL GROUTED CELLS FOR GROUT LIFTS OVER 5'-0" HIGH. "GROUT-AID" SHALL BE ADDED TO THE MIX FOR GROUT LIFTS OVER 5'-0" HIGH.
- GROUT CONSTRUCTION JOINTS SHALL BE FORMED BY STOPPING THE POUR 1 1/2" BELOW THE UPPERMOST UNIT.
- GROUT SHALL BE CONSOLIDATED BEFORE LOSS OF PLASTICITY USING MECHANICAL VIBRATING EQUIPMENT TO MINIMIZE VOIDS DUE TO WATER LOSS. RODDING WILL NOT BE ACCEPTED.
- MATERIAL QUALITY CONTROL: PRIOR TO DELIVERY OF MASONRY MATERIALS TO THE JOB SITE, DELIVER TO THE ARCHITECT A LETTER CERTIFYING THAT ALL SUCH MATERIALS TO BE DELIVERED TO THE JOB SITE ARE IN STRICT CONFORMANCE WITH THE PROVISIONS OF THIS SECTION.
- PROVIDE SPECIAL INSPECTION AS PER SECTION 1701A.5.7 OF THE C.B.C. FOR ALL MASONRY WORK.
- ALL MASONRY WORK SHALL HAVE QUALITY ASSURANCE AND SHALL COMPLY WITH THE C.B.C. SEC. 2105A

REINFORCING STEEL

- DETAILS OF REINFORCEMENT SHALL BE IN ACCORDANCE WITH CHAPTER 7 OF "AMERICAN CONCRETE INSTITUTE" 318 UNLESS OTHERWISE NOTED. REINFORCING STEEL DETAILING, BENDING AND PLACING SHALL BE IN ACCORDANCE WITH THE CONCRETE REINFORCING STEEL INSTITUTE "MANUAL OF STANDARD PRACTICE", LATEST EDITION.
- WELDING OF REINFORCING STEEL, IF PERMITTED BY THE ARCHITECT, SHALL BE IN ACCORDANCE WITH THE "STRUCTURAL WELDING CODE - REINFORCING STEEL" OF THE AMERICAN WELDING SOCIETY, AWS D1-4, AND SHALL BE PERFORMED BY WELDERS QUALIFIED UNDER THE PROCEDURES CONTAINED THEREIN.
- ALL REINFORCING STEEL SHOP DRAWINGS SHALL BE REVIEWED BY THE STRUCTURAL ENGINEER AND THE ARCHITECT PRIOR TO FABRICATION.
- ALL REINFORCING STEEL SHALL CONFORM TO ASTM A-615, GRADE 60 FOR NO. 4 AND LARGER, OTHERWISE GRADE 40. WELDED REINFORCING WHERE PERMITTED SHALL CONFORM TO ASTM 706, OR A VERIFIED AND APPROVED EQUIVALENT.
- WALLS AND COLUMNS SHALL BE DOWELED FROM SUPPORTS WITH BARS OF THE SAME SIZE AND SPACING.
- SPACER TIES: PROVIDE A MINIMUM OF #3 TIES AT 24 INCHES IN ALL BEAMS AND FOOTINGS.
- SPLICE MINIMUM REINFORCING IN ACCORDANCE WITH THE TYPICAL DETAIL 5 ON SHEET SN1.1
- PROVIDE MINIMUM EMBEDMENT OF REINFORCING IN CONFORMANCE WITH THE TYPICAL DETAIL 1 ON SHEET SN1.1
- BAR SUPPORTS SHALL BE PROVIDED IN ACCORDANCE WITH THE PROVISIONS OF "BAR SUPPORT SPECIFICATION" BY THE CONCRETE REINFORCING STEEL INSTITUTE (CRSI).
- ALL REINFORCING STEEL AND OTHER EMBEDDED ITEMS SHALL BE SECURELY HELD IN POSITION AND SHALL BE INSPECTED PRIOR TO PLACING CONCRETE AND OR GROUT.
- NO FIELD BENDING OF REINFORCEMENT WILL BE PERMITTED UNLESS APPROVED BY DSA AND THE ENGINEER.

DESIGN CRITERIA

- "CALIFORNIA CODE OF REGULATIONS" (C.C.R.), TITLE 24, 2001 EDITION AND STANDARDS REFERENCED THEREIN.
- RECOMMENDED LATERAL FORCE REQUIREMENTS AND COMMENTARY" BY THE STRUCTURAL ENGINEERS ASSOCIATION OF CALIFORNIA, 1999 EDITION.
- DESIGN LOADS:
 - LIVE LOADS
ROOF.....16 PSF @ SLOPING ROOF (REDUCIBLE)
20 PSF @ FLAT ROOF (REDUCIBLE)
 - WIND (PER UBC)
LESS THAN 40 FEET ABOVE GROUND..... 17.4 PSF (HRZ.)
9.4 PSF (UPWARD)
BASIC WIND SPEED..... 70 MPH
EXPOSURE CATEGORY = C
 - SEISMIC -
SEISMIC ZONE 4, Z=0.4 I = 1.0
SEISMIC SOURCE TYPE "B" R = 4.5 (MASONRY SHEAR WALLS)
Nv = 1.1 Vmax = 2.5 Ca
Na = 1.0
SOIL PROFILE TYPE; Sd = 0.244 W (ULTIMATE STRENGTH)
Cv = 0.64(Nv) = 0.70 = 0.174 W (WORKING STRESS)
Ca = 0.44(Na) = 0.44

SLAB-ON-GRADE

- THE PURPOSE OF THESE NOTES IS TO ACHIEVE THE BEST POSSIBLE FLOOR FINISH UTILIZING THE EXPERIENCE OF THE CONTRACTOR SINCE THE CONTRACTOR'S MEANS AND METHODS SIGNIFICANTLY AFFECT THE QUALITY AND THEREFORE THE SUCCESS OR FAILURE OF THE DESIGN.
- THE CONTRACTOR SHALL SUBMIT A POUR SEQUENCE SCHEDULE FOR REVIEW BY THE ARCHITECT AND STRUCTURAL ENGINEER PRIOR TO CASTING ANY SLAB-ON-GRADES. THE SUBMITTAL MUST CONTAIN THE FOLLOWING: AMOUNT OF CEMENT, STRENGTH OF CONCRETE, AGGREGATE SIZE, SLUMP AMOUNT AND THE CONTRACTOR'S ENDORSEMENT THAT HE CAN PRODUCE A SUCCESSFUL SLAB-ON-GRADE.
- IF A PUMP MIX IS PROPOSED, IT SHOULD BE PROPORTIONED TO MINIMIZE SHRINKAGE IN ADDITION TO CONFORMING TO ALL OTHER REQUIREMENTS.
- AS A GUIDELINE TO THE CONTRACTOR, THE SLAB-ON-GRADE SHALL BE CAST IN SQUARE OR RECTANGULAR SECTIONS APPROXIMATELY 400 SQUARE FEET MAXIMUM IN AREA WITH MAXIMUM DISTANCE OF 20 FEET BETWEEN CONSTRUCTION OR WEAKENED JOINTS.
- AS A FURTHER GUIDELINE TO THE CONTRACTOR, THE DRAWINGS MAY CONTAIN SUGGESTED LOCATIONS FOR CONSTRUCTION JOINTS (C.J.) AND WEAKENED JOINTS (W.J.).
- REFER TO DETAIL 7 ON SHEET SN1.1 FOR WEAKENED JOINT (W.J.) AND CONSTRUCTION JOINT (C.J.) DETAIL.

REINFORCED CONCRETE

- ALL CONCRETE WORK SHALL CONFORM TO THE "REQUIREMENTS FOR REINFORCED CONCRETE" (ACI 318) AND THE "SPECIFICATIONS FOR STRUCTURAL CONCRETE FOR BUILDINGS" (ACI 301), LATEST APPROVED EDITIONS, WITH MODIFICATIONS AS NOTED IN THE DRAWINGS, SPECIFICATIONS, AND TITLE 24.
- ALL REINFORCING DETAILING SHALL CONFORM TO THE CONCRETE REINFORCING STEEL INSTITUTE "MANUAL OF STANDARD PRACTICE" LATEST EDITION.
- CONTINUOUS INSPECTION BY AN D.S.A. APPROVED INSPECTOR IS REQUIRED FOR ALL STRUCTURAL REINFORCED CONCRETE WORK.
- WELDING OF REINFORCING STEEL, IF PERMITTED BY THE D.S.A. AND THE STRUCTURAL ENGINEER, SHALL BE IN ACCORDANCE WITH THE "STRUCTURAL WELDING CODE - REINFORCING STEEL" OF THE AMERICAN WELDING SOCIETY, AWS D1.4, AND SHALL BE PERFORMED BY WELDERS QUALIFIED UNDER THE PROCEDURES CONTAINED THEREIN.
- ALL REINFORCING STEEL SHOP DRAWINGS SHALL BE REVIEWED BY THE ARCHITECT AND STRUCTURAL ENGINEER PRIOR TO FABRICATION.
- CONCRETE STRENGTHS: THE CONCRETE STRENGTHS SHOWN IN THE FOLLOWING TABLE ARE MINIMUM COMPRESSIVE STRENGTH AT 28 DAYS. THE AGGREGATES SHOWN ARE THE MAXIMUM SIZE (INCHES) AND THE SLUMP SHOWN IS THE MAXIMUM (INCHES). THE COMPRESSIVE STRENGTH OF CONCRETE SHALL BE PROPORTIONAL AS REQUIRED IN C.C.R. TITLE 24, SECTION 1905A.2 FOR EITHER METHOD A, B, OR C.

ITEM OF CONSTRUCTION (145 P.C.F. AVE.)	STRENGTH	AGGREGATE	SLUMP
A. FOUNDATION SYSTEM	3,000 PSI	1 1/2"	4"
B. SLAB ON GRADE	3,000 PSI	1"	3"
C. MISC. PATCHING	3,000 PSI	3/8"	3"

- BATCH PLANT INSPECTION MAY BE WAIVED PROVIDED THE CONCRETE PLANT COMPLIES FULLY WITH THE REQUIREMENTS OF U.B.C. STANDARD NO.28-13 AND HAS BEEN CERTIFIED BY AN AGENCY ACCEPTABLE TO D.S.A. TO COMPLY WITH THE REQUIREMENTS OF THE "NATIONAL READY MIXED CONCRETE ASSOCIATION". THE PLANT MUST BE EQUIPPED WITH AN AUTOMATIC BATCHER IN WHICH THE TOTAL BATCHING CYCLE, EXCEPT FOR THE MEASURING AND INTRODUCTION OF AN ADMIXTURE, IS COMPLETED BY ACTIVATING A SINGLE STARTER DEVICE.

WHEN BATCH PLANT INSPECTION IS WAIVED, THE FOLLOWING REQUIREMENTS SHALL APPLY AND SHALL BE DESCRIBED IN THE CONTRACT SPECIFICATIONS: APPROVED INSPECTOR OF THE TESTING LABORATORY SHALL CHECK THE FIRST BATCHING AT THE START OF WORK AND FURNISH MIX PROPORTIONS TO THE LICENSED WEIGHMASTER. LICENSED WEIGHMASTER TO POSITIVELY IDENTIFY MATERIALS AS TO QUANTITY AND CERTIFY TO EACH LOAD BY A TICKET.

TICKETS SHALL BE TRANSMITTED TO THE PROJECT INSPECTOR BY A TRUCK DRIVER WITH LOAD IDENTIFIED THEREON. INSPECTOR WILL NOT ACCEPT THE LOAD WITHOUT A LOAD TICKET IDENTIFYING THE MIX AND WILL KEEP A DAILY RECORD OF PLACEMENTS, IDENTIFYING EACH TRUCK, ITS LOAD AND TIME OF RECEIPT, AND APPROXIMATE LOCATION OF DEPOSIT IN THE STRUCTURE. WILL TRANSMIT A COPY OF THE DAILY RECORD TO THE ENFORCEMENT AGENCY.

AT THE END OF THE PROJECT, THE WEIGHMASTER SHALL FURNISH AN AFFIDAVIT TO THE ENFORCEMENT AGENCY ON FORM SSS 411-8 CERTIFYING THAT ALL CONCRETE FURNISHED CONFORMS IN EVERY PARTICULAR TO PROPORTIONS ESTABLISHED BY MIX DESIGNS.

- THE EVALUATION AND ACCEPTANCE OF CONCRETE SHALL CONFORM TO THE REQUIREMENTS OF C.B.C. TITLE 24 SECTION 1905A.6.
- ALL REINFORCING STEEL SHALL CONFORM TO ASTM A615, GRADE 60 FOR NO. 4 AND LARGER, OTHERWISE GRADE 40.
- MINIMUM PROTECTIVE CONCRETE COVERAGE OF REINFORCING:
 - ON EARTH SIDE WHEN PLACED AGAINST EARTH.....3 IN.
 - ON EARTH SIDE WHEN FORMED.....2 IN.
 - EXTERIOR WALL STEEL ABOVE GRADE.....1 1/2IN.
 - INTERIOR WALL STEEL AND SUPPORTED SLABS.....1 IN.
 - TIED COLUMNS (TO TIES) ABOVE GRADE.....1 1/2 IN.
 - BEAMS (TO STIRRUPS) ABOVE GRADE.....1 1/2 IN.
- NO PIPES OR DUCTS SHALL BE PLACED IN CONCRETE COLUMNS, WALLS OR SLABS UNLESS SPECIFICALLY DETAILED OR UNLESS SLEEVES ARE PROVIDED IN ACCORDANCE WITH THE TYPICAL DETAIL 13 ON SHEET SN1.1

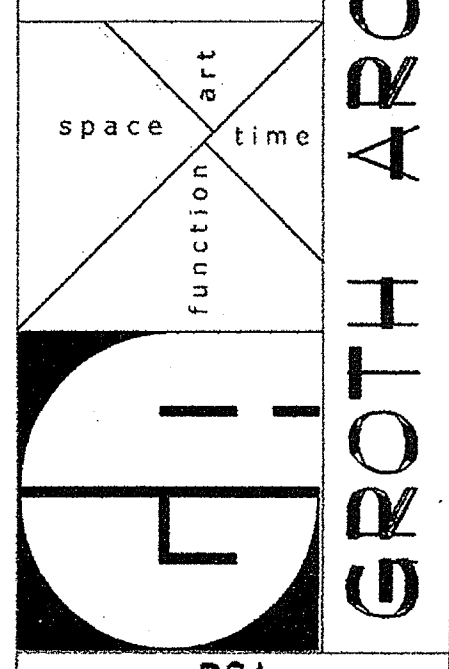
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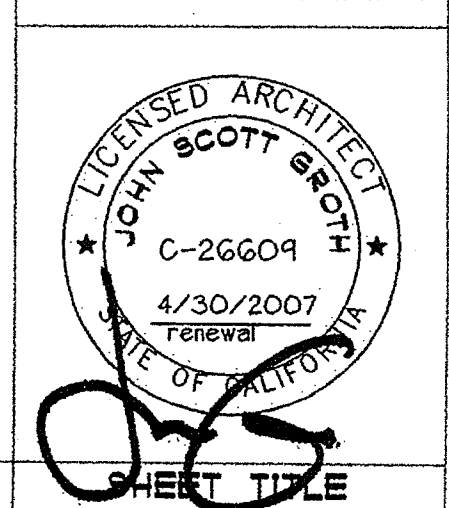
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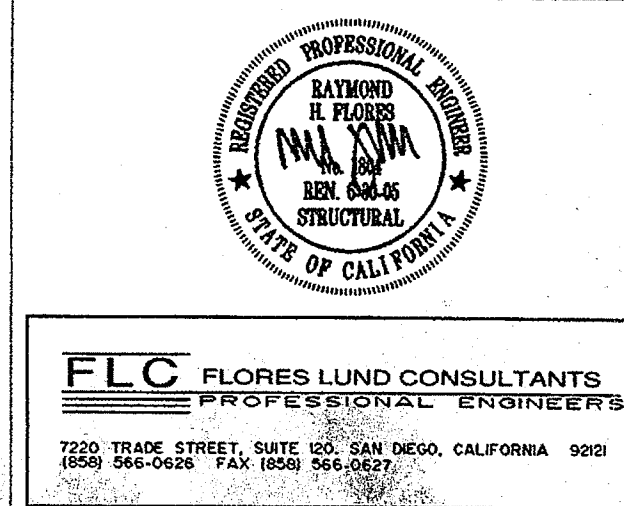
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SN1.0



FLC FLORES LUND CONSULTANTS
PROFESSIONAL ENGINEERS
7220 TRADE STREET, SUITE 200, SAN DIEGO, CALIFORNIA 92121
(619) 566-0629 FAX: (619) 566-0627

GENERAL NOTES (CONT.)

POWDER DRIVEN SHOT PINS (LOW VELOCITY)

1. QUALIFICATION FOR USE OF ALL POWER ACTUATED TOOLS MUST MEET ANSI A10.3 STANDARD AS REQUIRED BY THE MANUFACTURER AND ALL OSHA REQUIREMENTS.
2. TESTING THE OPERATOR, TOOL, AND FASTENER SHALL BE PRE-QUALIFIED BY THE PROJECT INSPECTOR. HE SHALL OBSERVE THE TESTING OF THE FIRST 10 FASTENER INSTALLATIONS. A TEST "PULL-OUT" LOAD OF NOT LESS THAN TWICE THE DESIGN LOAD SHALL BE APPLIED TO THE PIN IN SUCH A MANNER AS NOT TO RESIST THE SPALLING TENDENCY OF THE CONCRETE SURROUNDING THE PIN. THEREAFTER, RANDOM TESTS UNDER THE PROJECT INSPECTOR'S SUPERVISION SHALL BE MADE OF APPROXIMATELY 1 IN 10 PINS. IF ANY PIN FAILS TESTING, TEST ALL PINS OF THE SAME CATEGORY NOT PREVIOUSLY TESTED UNTIL TWENTY (20) CONSECUTIVE PASS, THEN RESUME THE INITIAL TESTING FREQUENCY.

COLD-FORMED METAL

1. PROVIDE METAL STUDS AND ACCESSORIES AS INDICATED ON THE DRAWINGS, AS SPECIFIED HEREIN, AND AS NEEDED FOR A COMPLETE AND PROPER INSTALLATION.
2. CONTRACTOR SHALL PROVIDE EFFECTIVE, FULL TIME QUALITY CONTROL OVER ALL FABRICATION AND ERECTION COMPLYING WITH THE PERTINENT CODES AND REGULATIONS OF GOVERNMENT AGENCIES (DSA) HAVING JURISDICTION.
3. ALL PRODUCTS TO BE MANUFACTURED BY THE CURRENT MEMBERS OF THE STEEL STUD MANUFACTURERS ASSOCIATION AND THE ICBO REPORT ER-4943P, INCLUDING THE SUPPLEMENTAL ICBO SUBMITTAL AND THE PRODUCT TECHNICAL INFORMATION.
4. ALL GALVANIZED STUDS AND JOISTS SHALL BE FORMED FROM STEEL THAT CORRESPONDS TO THE MINIMUM REQUIREMENTS OF 1996 A.I.S.I. STANDARDS.
5. ALL STRUCTURAL MEMBERS SHALL BE DESIGNED IN ACCORDANCE WITH THE AMERICAN IRON AND STEEL INSTITUTE (AISI) "SPECIFICATION FOR THE DESIGN OF COLD-FORMED STEEL STRUCTURAL MEMBERS" 1996 EDITION.
6. ALL LIGHT GAGE STEEL MEMBERS: STUDS AND TRACKS SHALL BE FORMED FROM STEEL HAVING A MINIMUM 33,000 PSI YIELD POINT (ASTM A653 SS GRADE 33 OR ASTM A1011 SS GRADE 33) FOR THICKNESSES OF 0.0179 INCH THROUGH 0.0451 INCH, AND A MINIMUM 50,000 PSI YIELD POINT (ASTM A653 SS GRADE 50, CLASS 1 OR 3, OR ASTM A1011 SS GRADE 50) FOR THICKNESSES OF 0.0538 INCH THROUGH 0.1180 INCH.
7. PROVIDE ALL ACCESSORIES INCLUDING, BUT NOT LIMITED TO, TRACKS, CLIPS, WEB STIFFENERS, ANCHORS, FASTENING DEVICES, RESILIENT CLIPS, AND OTHER ACCESSORIES REQUIRED FOR A COMPLETE AND PROPER INSTALLATION, AND AS RECOMMENDED BY THE MANUFACTURER AND APPROVED BY DSA FOR THE STEEL MEMBERS USED.
8. FASTENING OF COMPONENTS SHALL BE WITH SELF-DRILLING SCREWS OR WELDING. SCREWS OR WELDS SHALL BE OF SUFFICIENT SIZE TO INSURE THE STRENGTH OF THE CONNECTION. ALL WELDS OF GALVANIZED STEEL SHALL BE TOUCHED UP WITH A ZINC-RICH PAINT. ALL WELDS OF CARBON SHEET STEEL SHALL BE TOUCHED UP WITH PAINT. WIRE TYING OF COMPONENTS SHALL NOT BE PERMITTED.

EXPANSION BOLTS OR EPOXY-TYPE ANCHORS IN CONCRETE (HARD ROCK OR LIGHTWEIGHT) PER 2001 CBC, TITLE 24, CHAPTER 19A, SEC. 1923A.3.5, AND IR 19-1

1. ALL EXPANSION ANCHORS SHOULD MEET THE MINIMUM DEPTH OF EMBEDMENT CRITERIA ESTABLISHED BY THE ICBO REPORT.
2. INSTALLED ANCHORS ARE TO BE IN ACCORDANCE WITH THE REQUIREMENTS GIVEN IN THE ICBO RESEARCH COMMITTEE RECOMMENDATIONS FOR THE SPECIFIC ANCHOR, OR AS REQUIRED BY THE MANUFACTURER FOR THE ANCHORS SUBMITTED WITHOUT ICBO RESEARCH COMMITTEE RECOMMENDATIONS.
3. WHEN EXPANSION ANCHORS ARE INSTALLED INTO CONCRETE SUPPORTED BY A METAL DECK, THE ANCHORS SHALL BE CENTERED ON THE "LOW" FLUTE OF THE DECKING WHERE PRACTICABLE. THE DECK SHOULD HAVE A MINIMUM THICKNESS OF 20 GAGE, WITH FLUTES AS WIDE AS POSSIBLE.
4. WHEN INSTALLED FROM THE BOTTOM, EMBEDMENT SHALL BE 1 1/2" ABOVE THE TOP OF THE DECKING FLUTE (EXCEPT 1/4" DIA. AND 5/16" DIA. ANCHORS FOR CEILINGS). THE EFFECTIVE DEPTH OF EMBEDMENT SHOULD BE CONSIDERED TO BE 1/3 OF THE METAL DECK HEIGHT PLUS THE 1 1/2" NOTED ABOVE. THERE SHALL BE A ONE INCH (0"-1") MINIMUM CLEAR DISTANCE FROM THE TOP OF CONCRETE TO END OF BOLT.
5. WHEN INSTALLED FROM THE TOP OF CONCRETE, THE DEPTH OF EMBEDMENT SHALL BE THE DEPTH OF CONCRETE ABOVE THE TOP OF THE FLUTE PLUS ONE THIRD (1/3) THE FLUTE HEIGHT. A MINIMUM ONE INCH (0"-1") CLEAR DISTANCE FROM THE STEEL DECK TO THE BOTTOM OF THE ANCHOR MUST BE MAINTAINED.
6. HOLES SHALL BE CLEAN AND FREE FROM DUST IMMEDIATELY PRIOR TO INSTALLATION OF THE ANCHOR.
7. TEST LOADS, JOB SITE TESTING FOR VERIFYING SATISFACTORY INSTALLATION AND WORKMANSHIP IS REQUIRED.
8. THE TEST LOAD MAY BE APPLIED BY ANY METHOD THAT WILL EFFECTIVELY MEASURE THE TENSION IN THE ANCHOR (REF. ASTM E488-90), SUCH AS DIRECT PULL WITH A HYDRAULIC JACK, CALIBRATED SPRING LOADING DEVICES, OR A CALIBRATED TORQUE WRENCH.
9. ANCHORS IN WHICH TORQUE IS USED TO EXPAND THE ANCHOR WITHOUT APPLYING TENSION TO THE BOLT (TORQUE CONTROLLED ANCHOR) REQUIRES SUFFICIENT DATA FROM EITHER THE MANUFACTURER, OR FROM INDEPENDENT TESTING, TO ESTABLISH APPROPRIATE TORQUE TEST VALUES. THE TEST VALUES MUST BE SHOWN ON THE CONTRACT DOCUMENTS.
10. THE FOLLOWING CRITERIA APPLY FOR THE ACCEPTANCE OF INSTALLED ANCHORS:
 - A. HYDRAULIC RAM METHOD: THE ANCHOR MUST NOT EXHIBIT OBSERVABLE MOVEMENT AT THE APPLICABLE TEST LOAD. FOR WEDGE AND SLEEVE TYPE ANCHORS, A PRACTICAL WAY TO DETERMINE OBSERVABLE MOVEMENT IS THAT THE WASHER UNDER THE NUT BECOMES LOOSE. DROP-IN ANCHORS ARE ONLY TO BE TESTED WITH THIS METHOD.
 - B. TORQUE WRENCH METHOD: THE APPLICABLE TEST TORQUE FOR WEDGE OR SLEEVE-TYPE ANCHORS, MUST BE REACHED WITHIN THE FOLLOWING LIMITS, ONE-HALF (1/2) TURN OF THE NUT; ONE-QUARTER (1/4) TURN OF THE NUT FOR THE 3/8" DIA. SLEEVE ANCHOR ONLY.
 - C. IF ANY ANCHOR FAILS TESTING, ALL ANCHORS OF THE SAME CATEGORY, NOT PREVIOUSLY TESTED, SHALL BE TESTED UNTIL 20 CONSECUTIVE ANCHORS PASS THE TEST REQUIREMENTS. THE INITIAL TESTING FREQUENCY SHALL THEN BE RESUMED.

TEST VALUES HARDROCK OR LIGHTWEIGHT CONCRETE						
ANCHOR DIA. (in)	WEDGE		SLEEVE		SHELL	
	LOAD (lbs)	TORQUE (ft-lbs)	LOAD (lbs)	TORQUE (ft-lbs)	LOAD (lbs)	TORQUE (ft-lbs)
1/4	800	10	400	4	1000	-
5/16	-	-	400	5	1400	-
3/8	1100	25	700	10	1800	-
1/2	2000	50	900	20	2700	-
5/8	2300	80	1100	45	3700	-
3/4	3700	150	1400	90	5400	-
1	5800	250	-	-	-	-

NOTES

1. ANCHOR DIA. REFERS TO THE THREAD SIZE FOR THE WEDGE AND SHELL CATEGORIES AND TO THE ANCHOR OUTSIDE DIA. FOR THE SLEEVE CATEGORY.
2. APPLY PROOF TEST LOADS TO WEDGE & SLEEVE ANCHORS WITHOUT REMOVING THE NUT IF POSSIBLE. IF NOT, REMOVE NUT AND INSTALL A THREADED COUPLER TO THE SAME TIGHTNESS OF THE ORIGINAL NUT USING A TORQUE WRENCH & APPLY LOAD.
3. FOR SLEEVE/SHELL INTERNALLY THREADED CATEGORIES, VERIFY THAT THE ANCHOR IS NOT PREVENTED FROM WITHDRAWING BY A BASEPLATE OR OTHER FIXTURES. IF RESTRAINT IS FOUND, LOOSEN AND SHIM OR REMOVE FIXTURE(S) PRIOR TO TESTING.
4. REACTION LOADS FROM TEST FIXTURES MAY BE APPLIED CLOSE TO THE ANCHOR BEING TESTED, PROVIDED THE ANCHOR IS NOT RESTRAINED FROM WITHDRAWING BY THE FIXTURE(S).
5. SHELL TYPE ANCHORS SHOULD BE TESTED AS FOLLOWS:
VISUALLY INSPECT 25% FOR FULL EXPANSION AS EVIDENCED BY THE LOCATION OF THE EXPANSION PLUG IN THE ANCHOR BODY. PLUG LOCATION OF A FULLY EXPANDED ANCHOR SHOULD BE AS RECOMMENDED BY THE MANUFACTURER, OR, IN THE ABSENCE OF SUCH RECOMMENDATION, AS DETERMINED ON THE JOB SITE FOLLOWING THE MANUFACTURER'S INSTALLATION INSTRUCTIONS, AND;
PROOF LOAD 5% AS INDICATED IN THE TABLE ABOVE, BUT NOT LESS THAN THREE ANCHORS PER DAY FOR EACH DIFFERENT PERSON OR CREW INSTALLING ANCHORS, OR; TEST 50% OF THE INSTALLED ANCHORS PER 1923A.3.5
6. TEST EQUIPMENT IS TO BE CALIBRATED BY AN APPROVED TESTING LABORATORY IN ACCORDANCE WITH STANDARD RECOGNIZED PROCEDURES.
7. TORQUE TEST VALUES FOR SHELL TYPE ANCHORS ARE OMITTED DUE TO LACK OF DATA. TORQUE TESTING CAN OCCUR ON AN INDIVIDUAL BASIS WHEN TEST PROCEDURES ARE SUBMITTED AND APPROVED BY THE ENFORCEMENT AGENCY. TABULATED VALUES MAY BE FORTHCOMING ONCE THE ENFORCEMENT AGENCY HAS MORE DATA TO EVALUATE THE FEASIBILITY OF STANDARD TORQUE VALUES.
8. TESTING SHOULD OCCUR 24 HOURS MINIMUM AFTER INSTALLATION OF THE SUBJECT ANCHORS.

FLC FLORES LUND CONSULTANTS
PROFESSIONAL ENGINEERS
7250 TRADE STREET, SUITE 100, SAN DIEGO, CALIFORNIA 92121
619-596-0626 FAX 619-596-0427

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805-754-8191
805-754-8291

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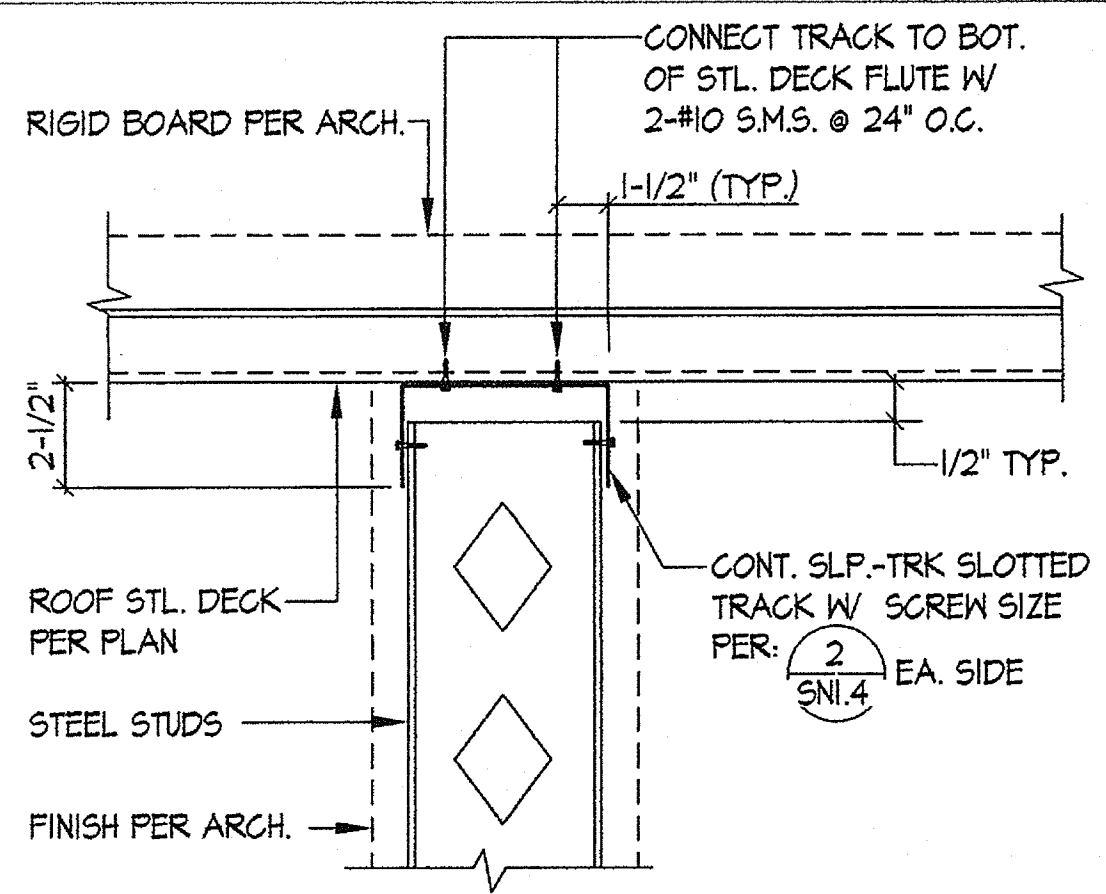
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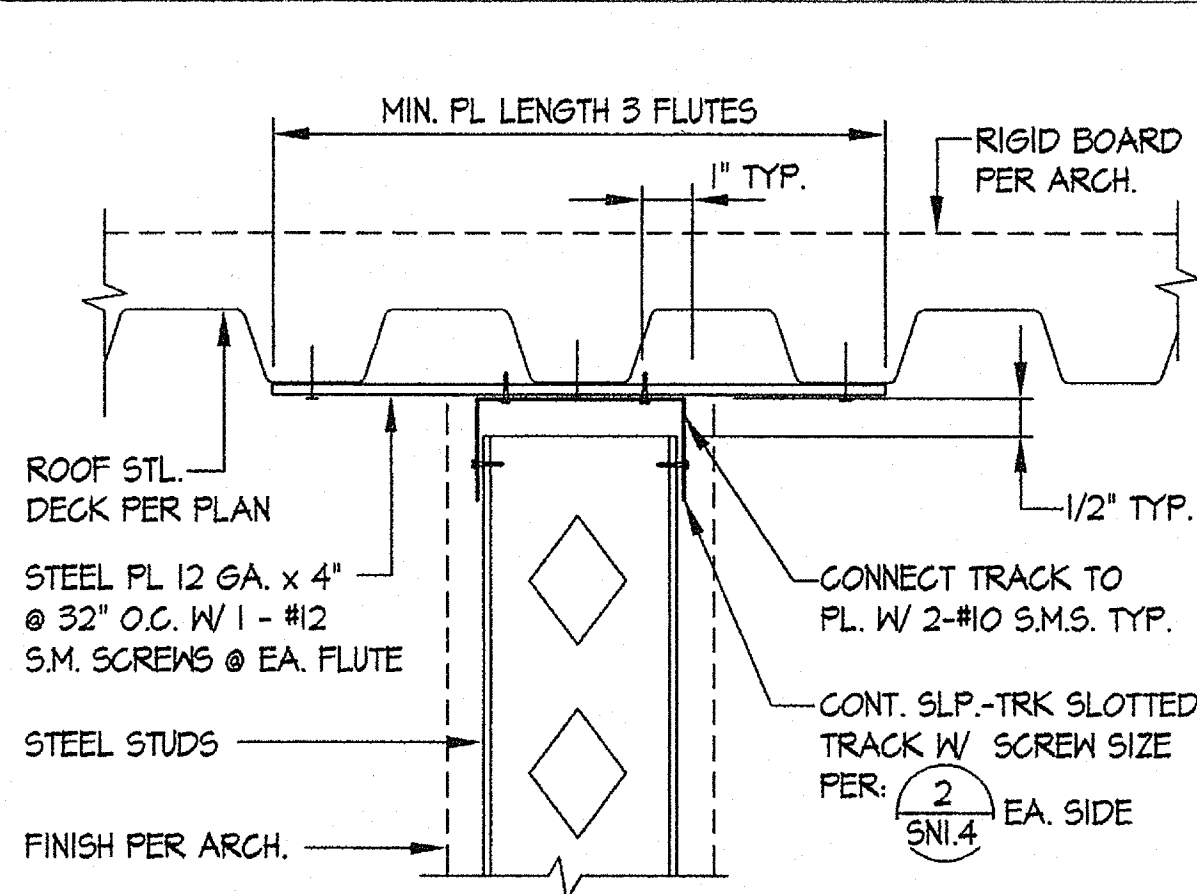
GENERAL NOTES

SN1.0A

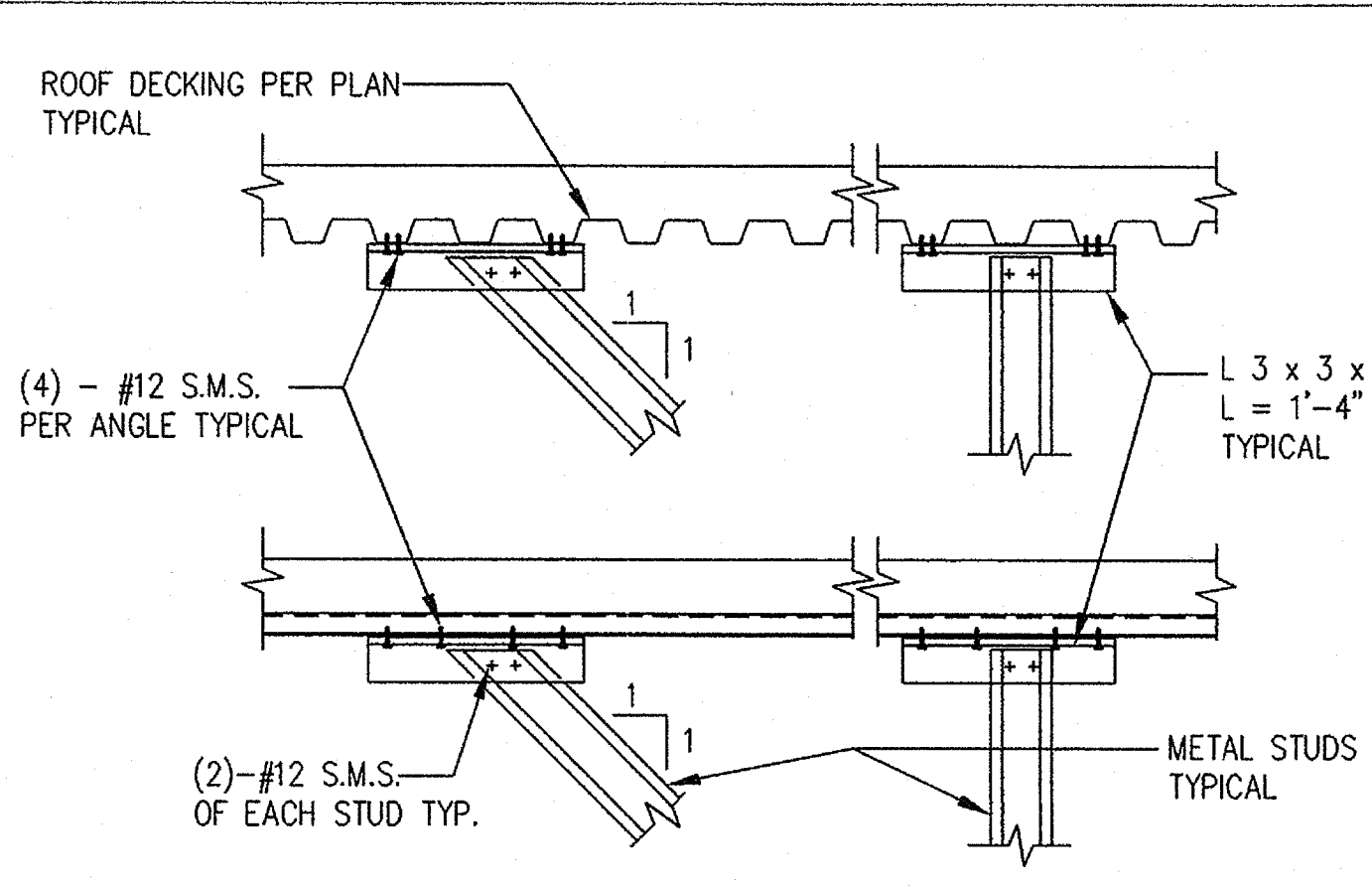
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FAX 760-754-8291
SUITE 234
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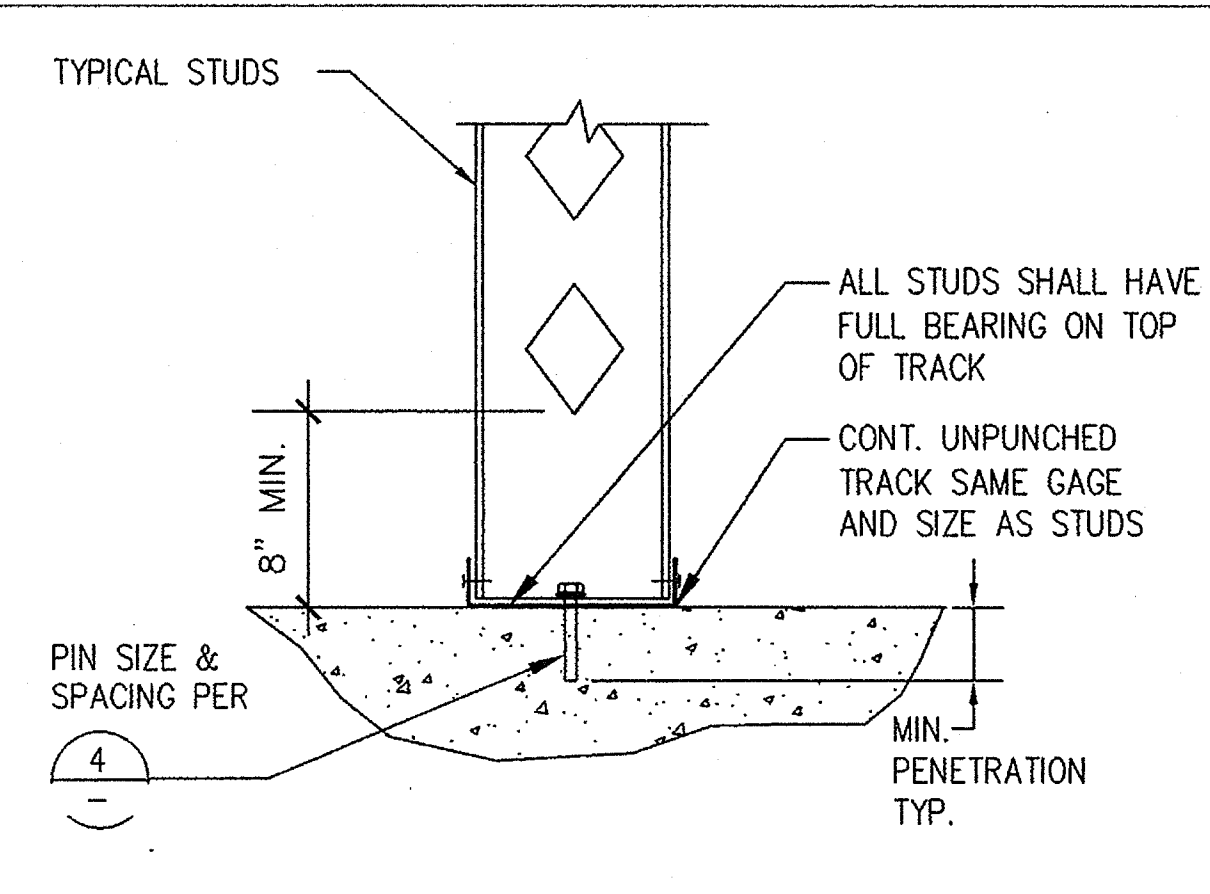
TYP. NON-BRG. STUD PERP. TO ROOF DECK **13**



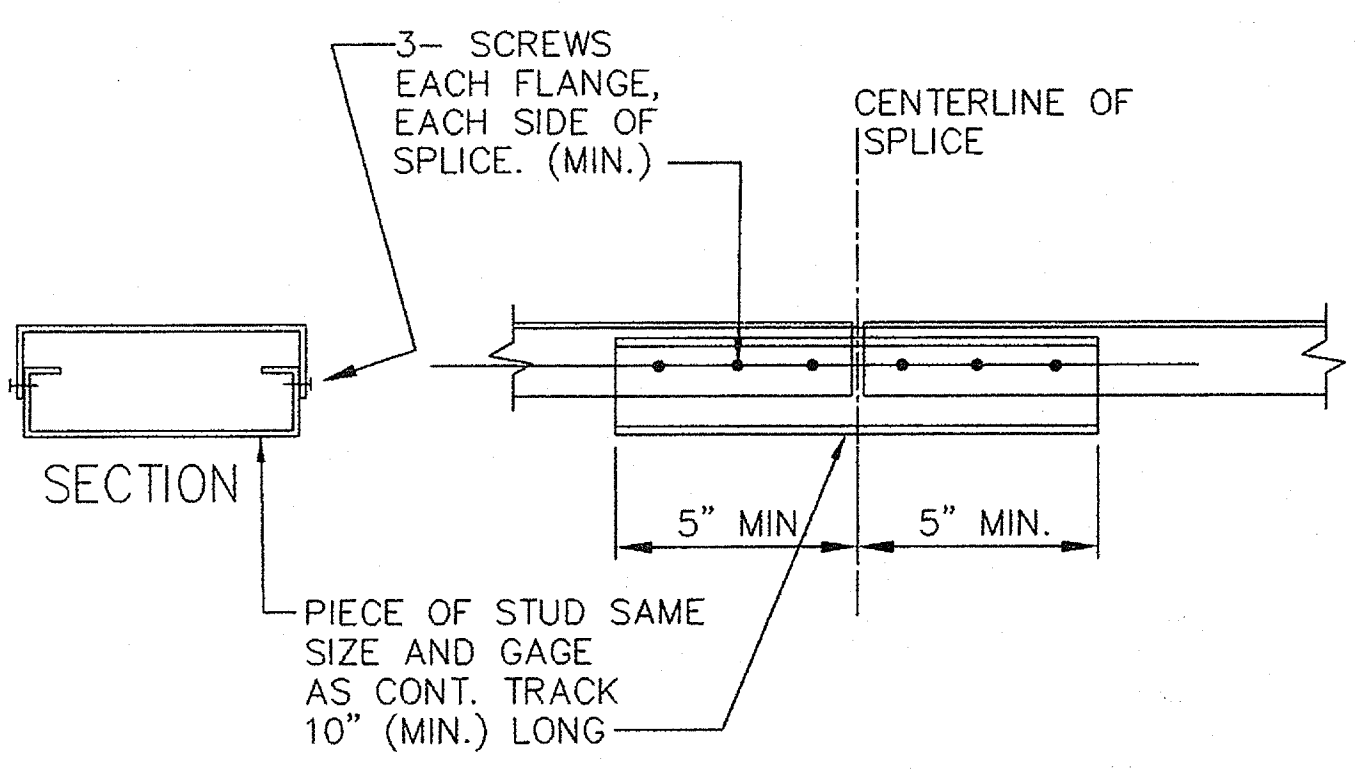
TYP. NON-BRG. STUD PARALLEL TO ROOF DECK **9**



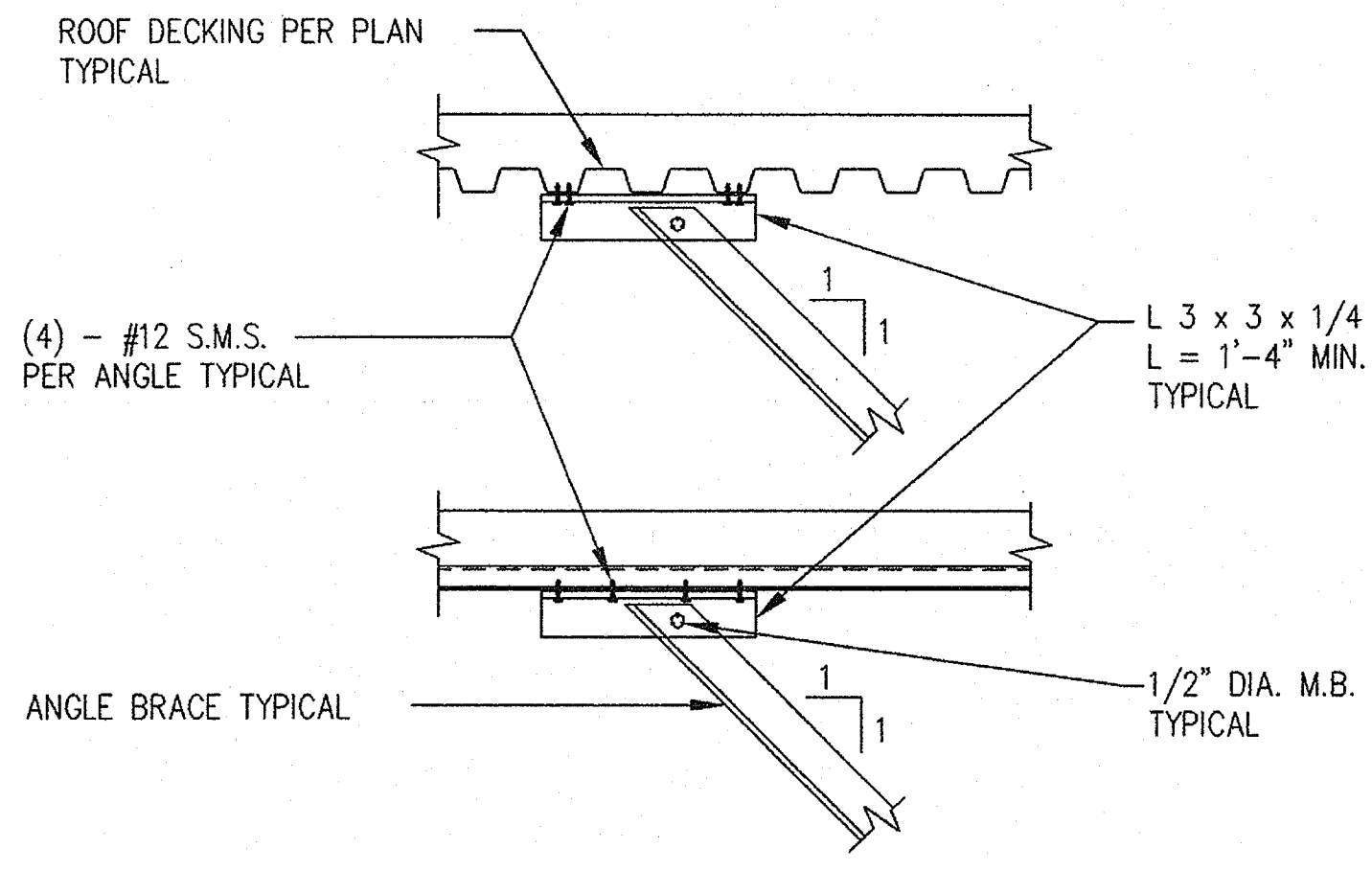
TYP. STUD BRACING AT ROOF **5**



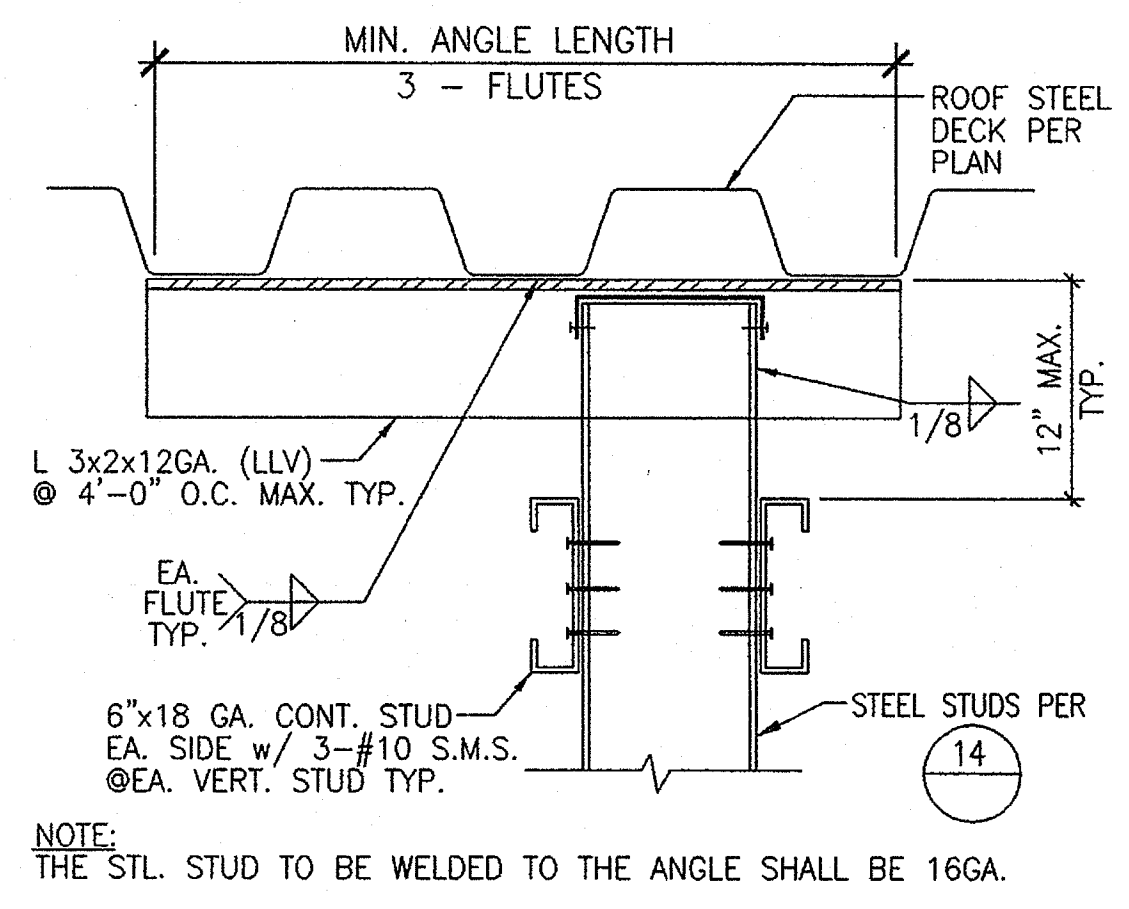
BOTTOM OF WALL AT CONC. **1**



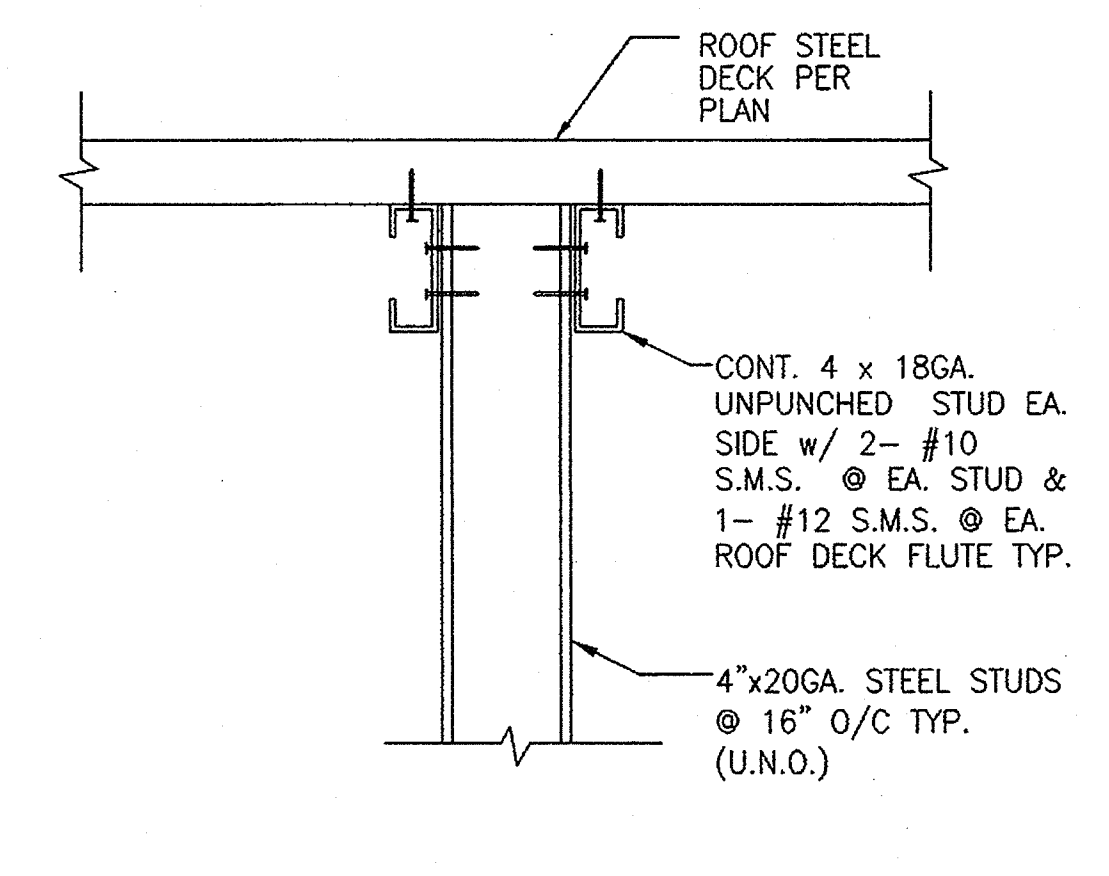
TYP. TOP STEEL TRACK SPLICE **14**



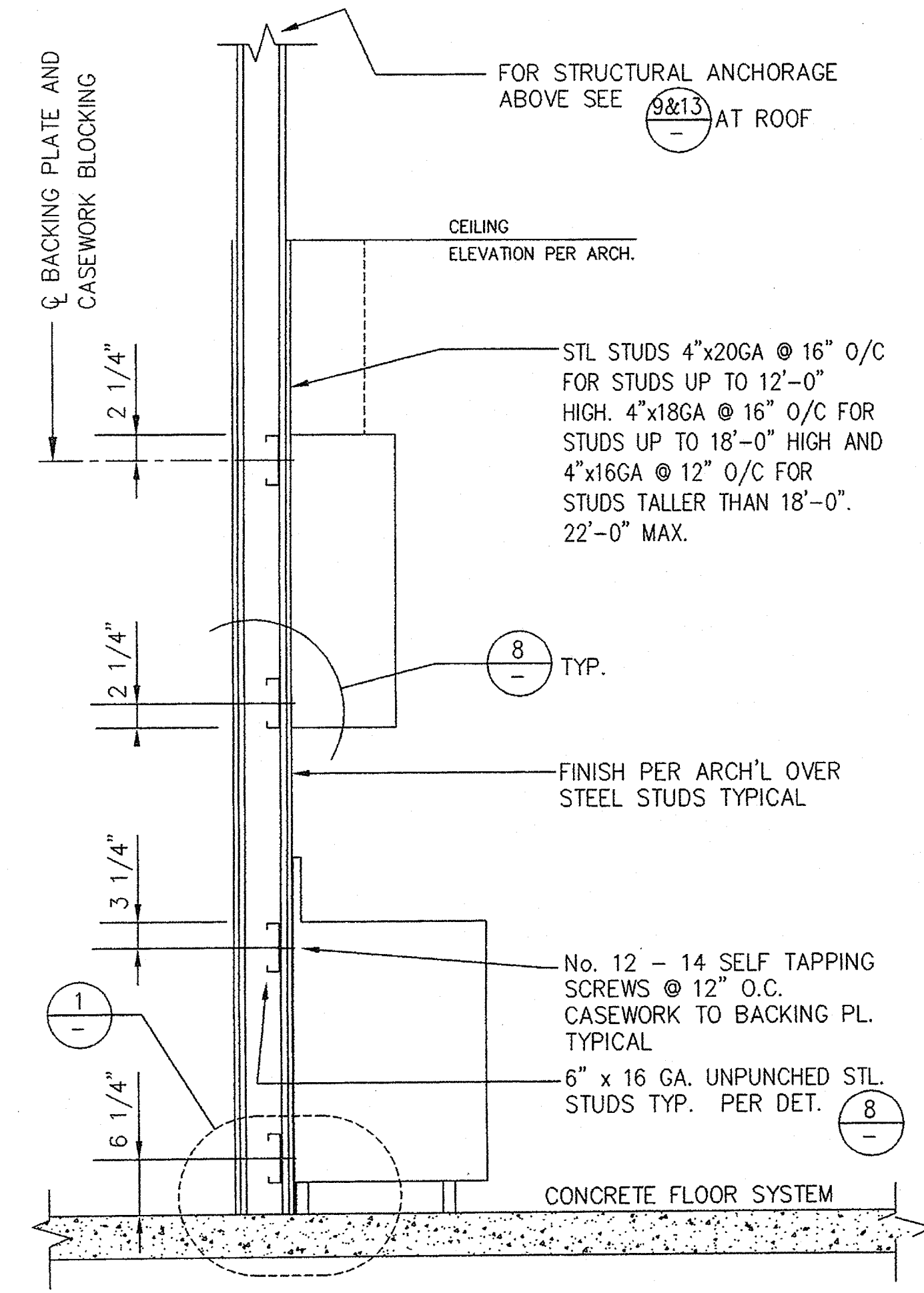
TYP. ANGLE BRACING AT ROOF **10**



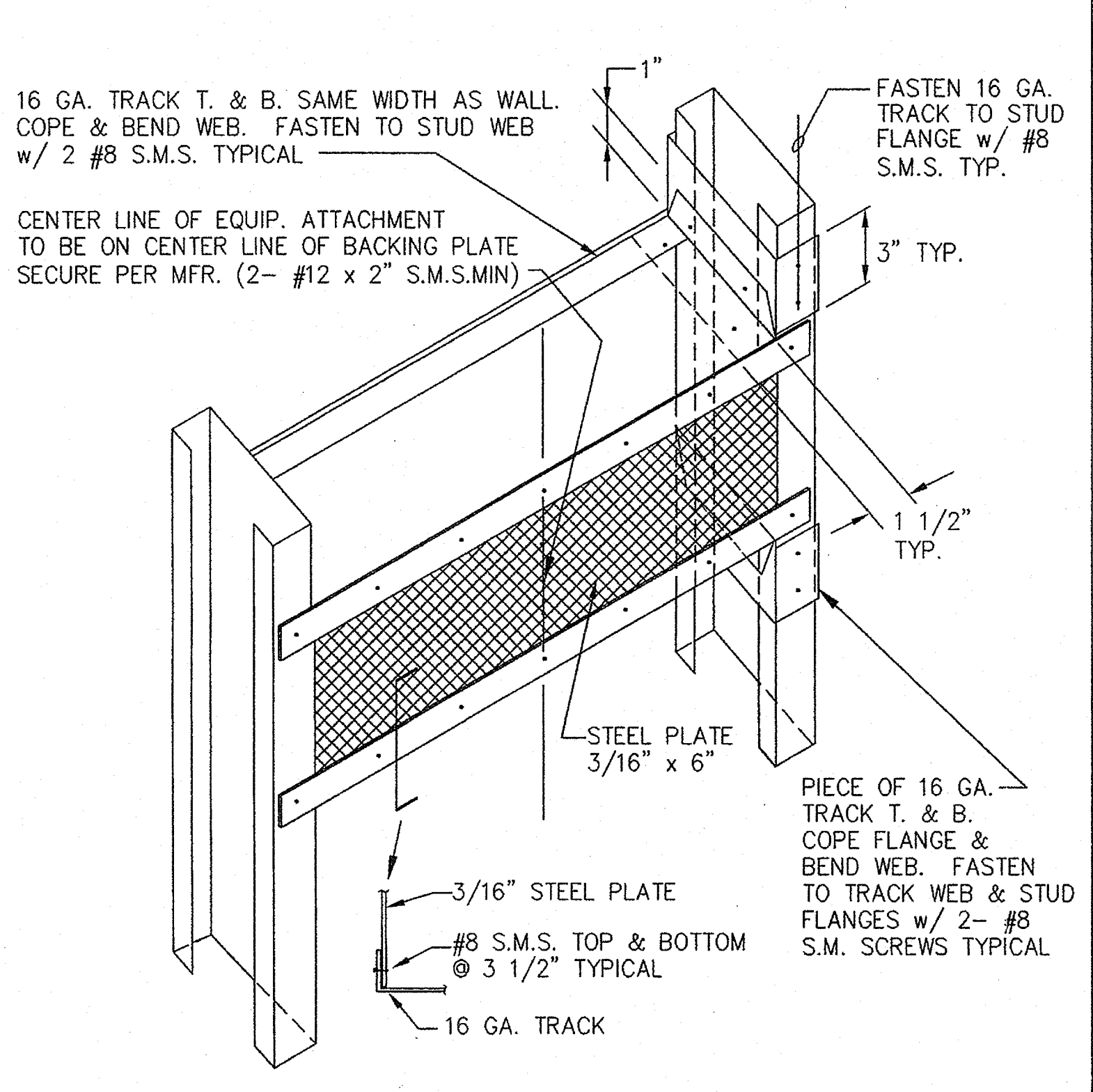
HUNG STUDS - PARALLEL TO ROOF STEEL DECK FLUTES **6**



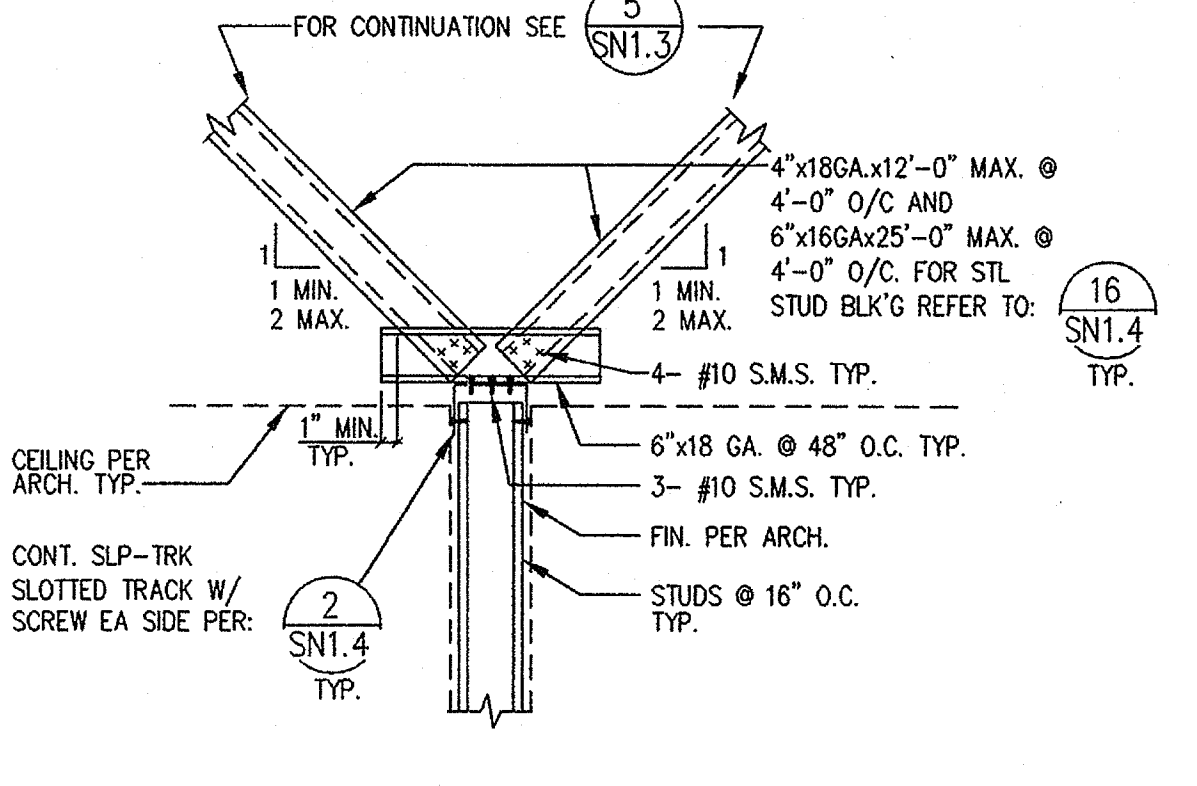
HUNG STUDS - PERP. TO ROOF STEEL DECK FLUTES **2**



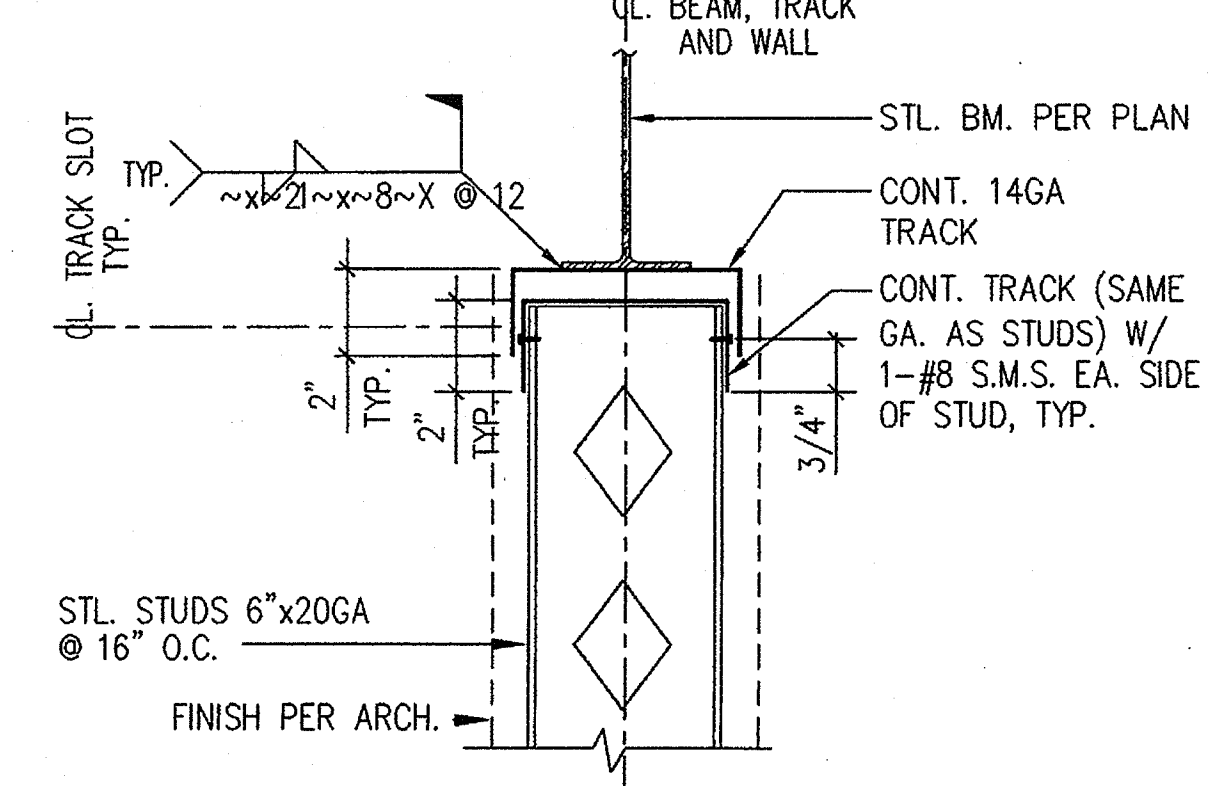
TYP. CASEWORK SUPPORT **16**



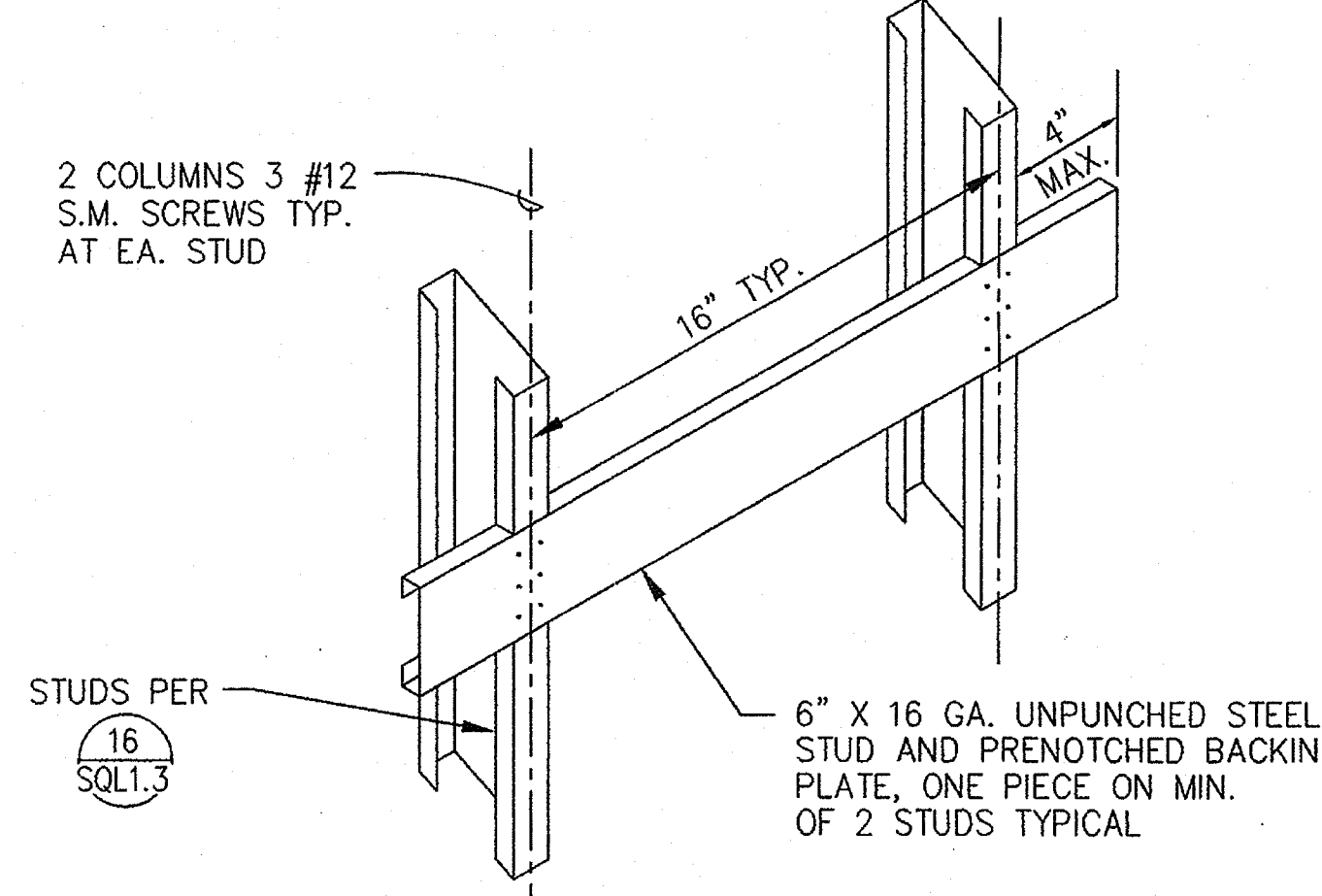
MECHANICAL, ELECTRICAL EQUIPMENT BACKING PLATE **12**



TYP. PARTITION CONN. @ CEILING LEVEL FOR EXACT LOCATION, SEE ARCH. **7**



TYP. NON-BEARING STUD **3**



BACKING PLATE **8**

POWDER DRIVEN FASTENERS IN CONC.

MINIMUM SHANK DIAMETER	MINIMUM PENETRATION	TRACK DEPTH	TRACK GAGE	MAXIMUM SPACING
0.170"	1 1/4"	4" & 6"	20	32"
0.170"	1 1/2"	4" & 6"	18	32"
0.170"	1 1/2"	4" & 6"	16	24"

- NOTES:
- FASTENERS SHALL BE PER ITW RAMSET/ RED HEAD ACTUATED FASTENERS. LATEST I.C.B.O. REPORT No. 1639, (TYP.)
 - FOR TOOL QUALIFICATION AND FASTENER TESTING SEE "POWDER DRIVEN SHOT PINS" NOTES ON DWG. SQL1.0A

FASTENER SCHEDULE

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JEFFERSON MS NEW CONSTRUCTION
823 ACACIA STREET
OCEANSIDE, CA 92054
OCEANSIDE UNIFIED S.D.

GROTH ARCHITECTS, INC. 3355 MISSION AVE. SUITE 234 OCEANSIDE, CALIFORNIA 92054
PHONE 760-754-8191
FAX 760-754-8291

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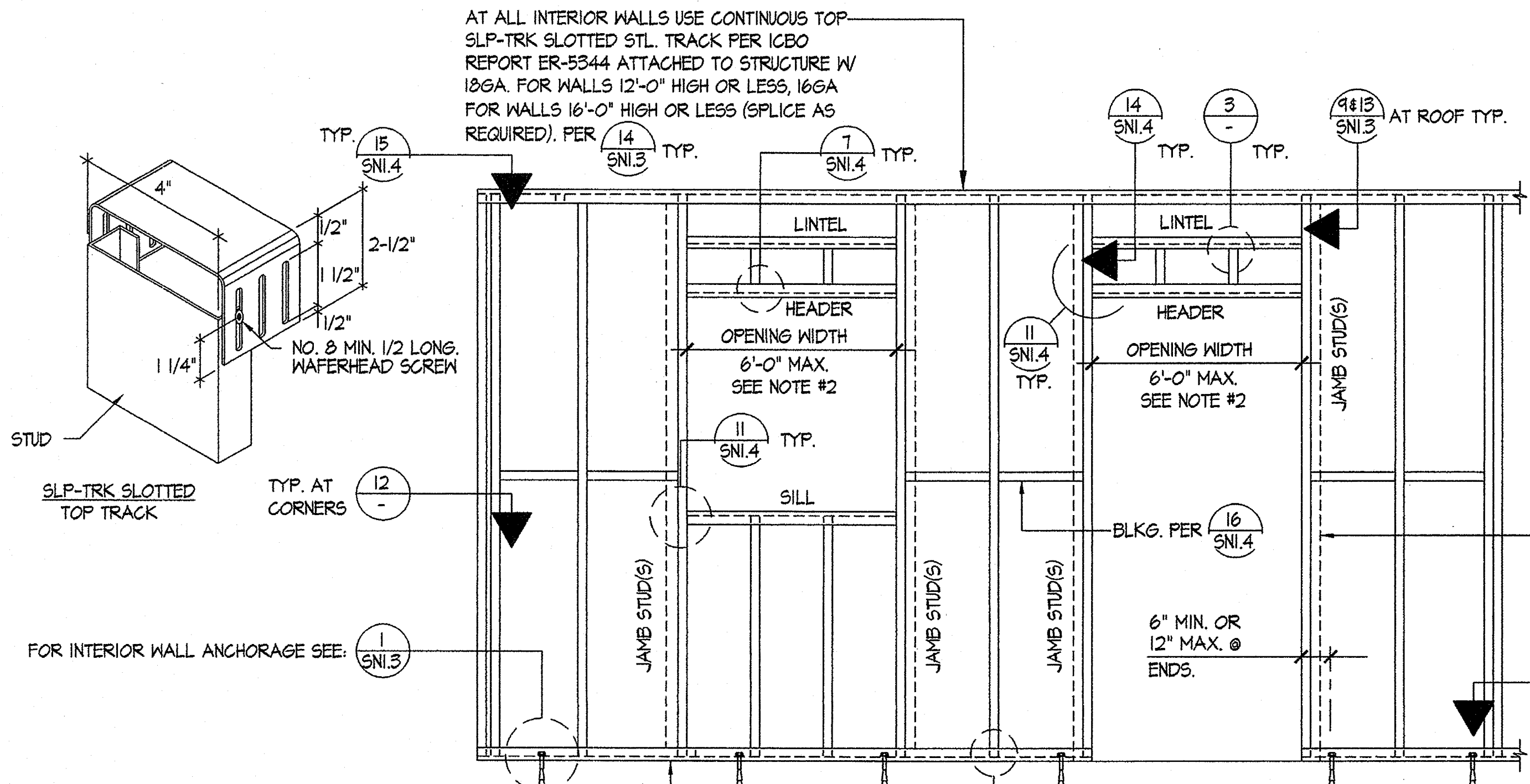
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FLC FLORES LUND CONSULTANTS
PROFESSIONAL ENGINEERS
7230 TRADE STREET, SUITE 100, SAN DIEGO, CALIFORNIA 92121
(619) 560-0626 FAX (619) 566-0627

SN1.3



AT ALL INTERIOR WALLS USE CONTINUOUS TOP-SLP-TRK SLOTTED STL. TRACK PER ICBO REPORT ER-5344 ATTACHED TO STRUCTURE W/ 18GA. FOR WALLS 12'-0" HIGH OR LESS, 16GA FOR WALLS 16'-0" HIGH OR LESS (SPLICE AS REQUIRED). PER 14 SNI.3 TYP.

FOR INTERIOR WALL ANCHORAGE SEE: 1 SNI.3

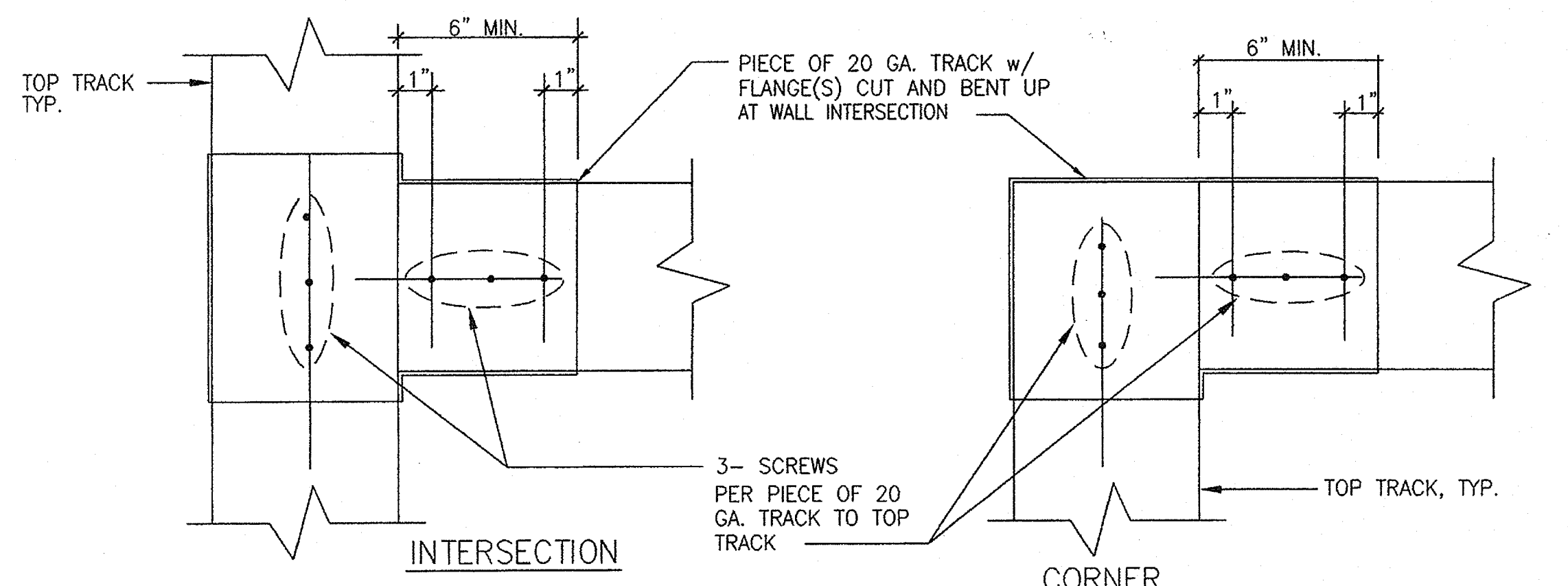
CONT. UN-PUNCHED TRACK, SAME GAGE AND SIZE AS STUDS

DASHED LINE INDICATES DBL. JAMB STUD WHERE REQUIRED. SEE NOTE 6.

NOTES:

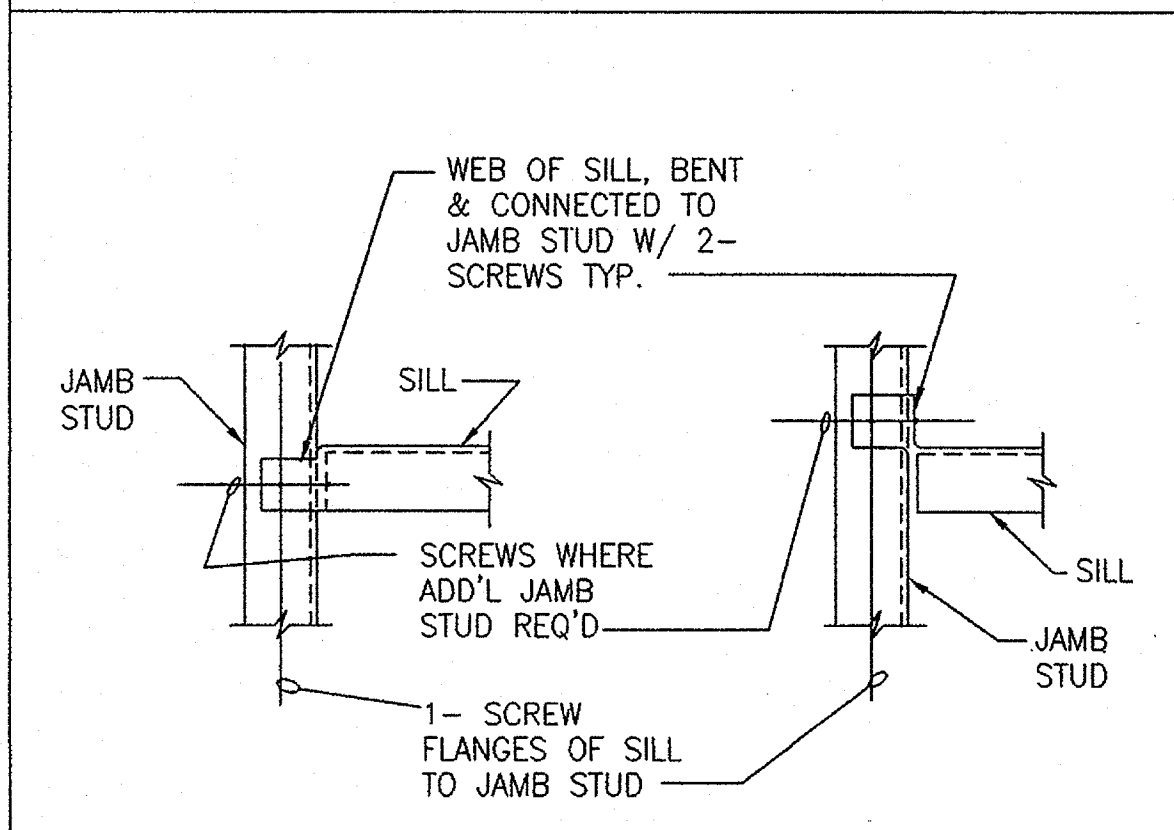
1. WALL TOP & BOTT. TRACK SAME WIDTH AS STUDS
2. FOR OPENINGS GREATER THAN 6'-0" HEADER AND WINDOW SILL TRACK SHALL BE REPLACED BY LINTEL PER 14 SNI.4, 2.A.
3. USE #8 S.M.S. FOR 20 GA. & 18 GA. COMPONENTS
4. USE #12 S.M.S. FOR 16 GA. COMPONENTS
5. SCREW SIZE IS BASED ON THINEST STEEL GA. TO BE CONNECTED
6. PROVIDE DBL. JAMB STUDS FOR WALLS TALLER THAN 10'-0" AND/OR AT OPENINGS BIGGER THAN 6'-0" (SEE NOTE 2 ABOVE) BACK TO BACK CONN. SHALL BE PER DETAIL 14 SNI.4, 2.A.
7. THE TOP SLP-TRK SLOTTED STL. TRACK SHALL BE PER ICBO REPORT ER-5344
8. FOR STUD SIZES AND SPACING SEE: 16 SNI.3 TYP. AND FOR BOTT. TRACK SIZE SEE: 2 SNI.4 TYP.

TYPICAL INTERIOR WALL OPENING FRAMING



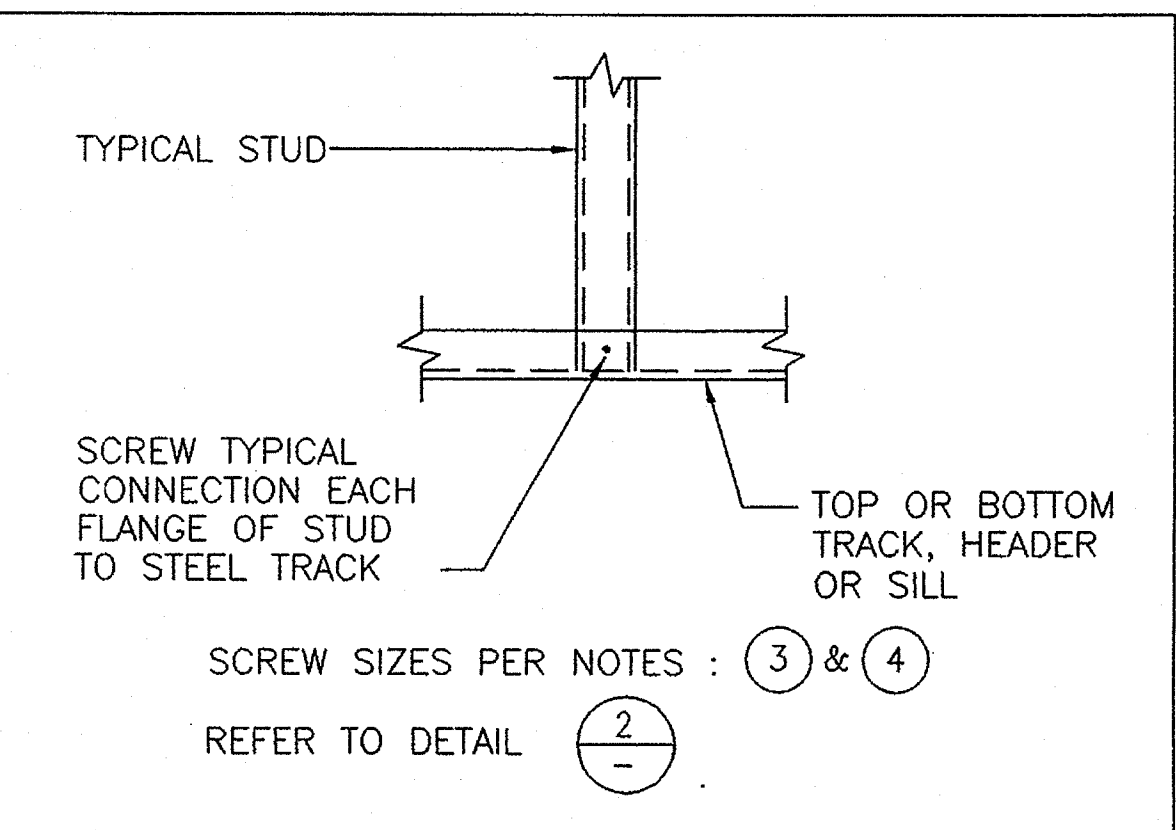
PLAN AT TOP TRACKS

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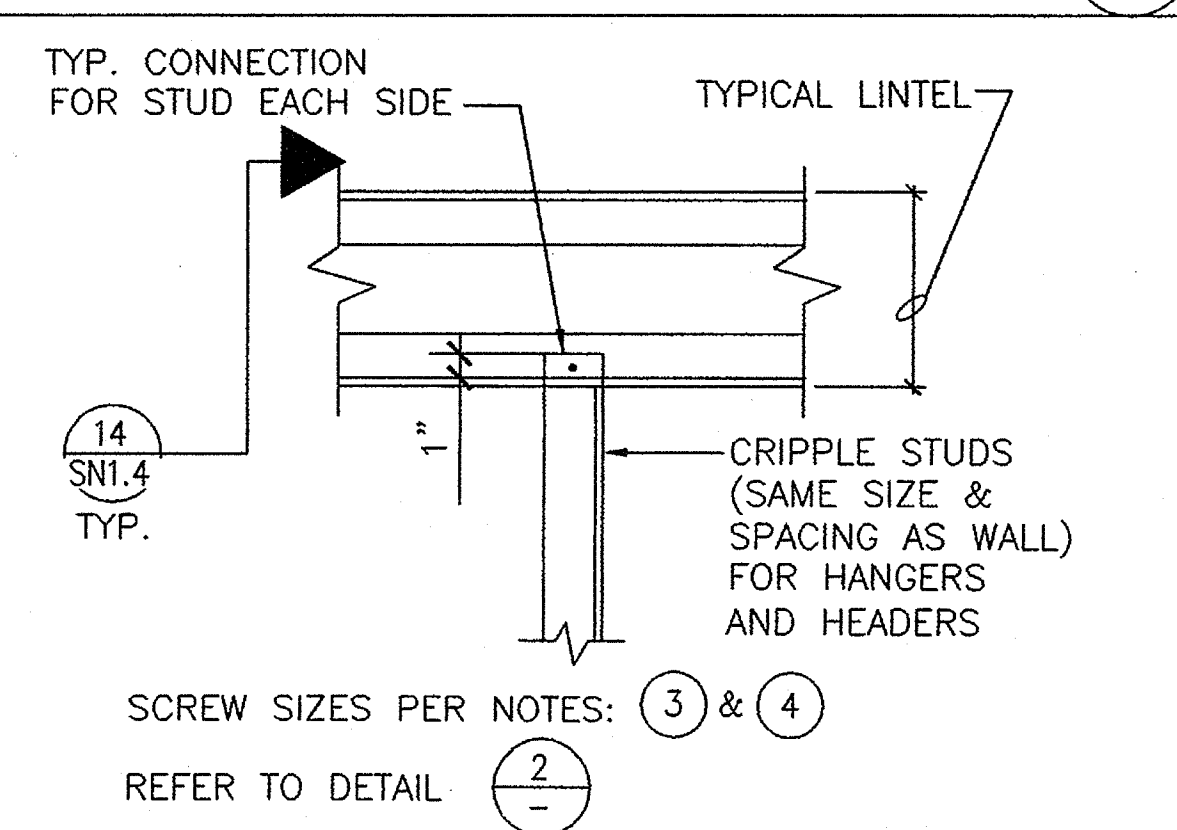
TYPICAL DETAIL

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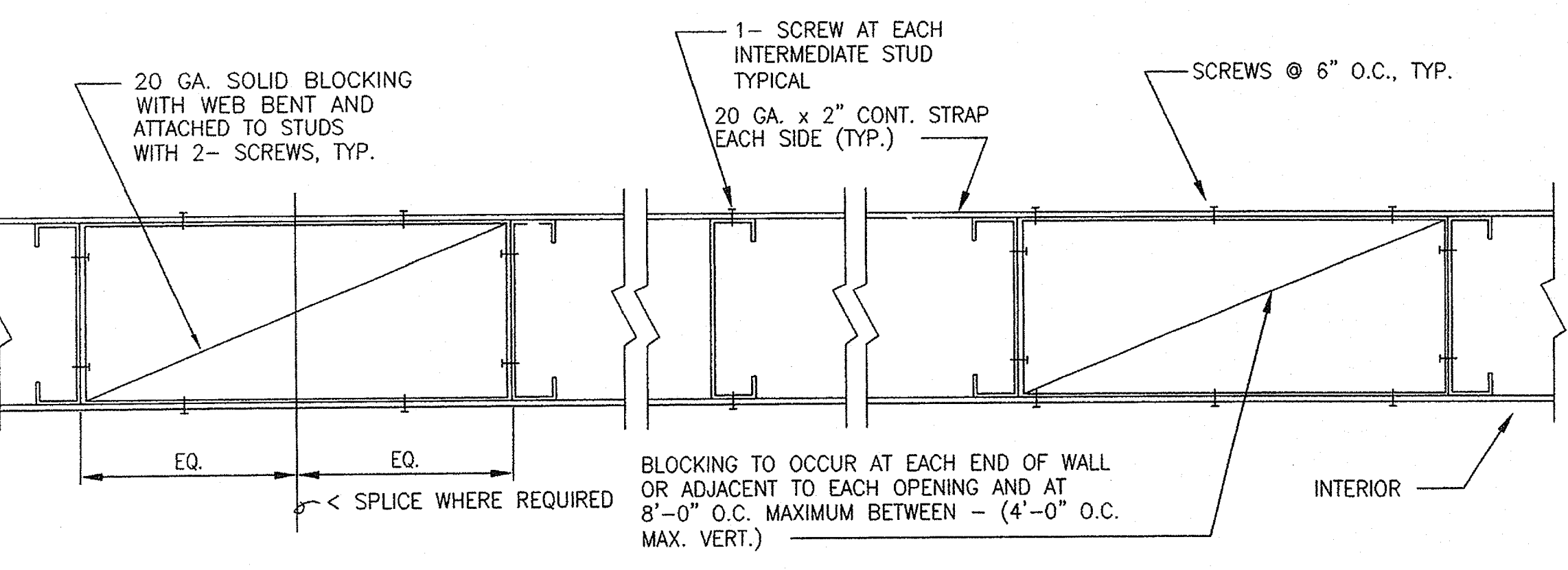
TYPICAL STUD CONNECTION

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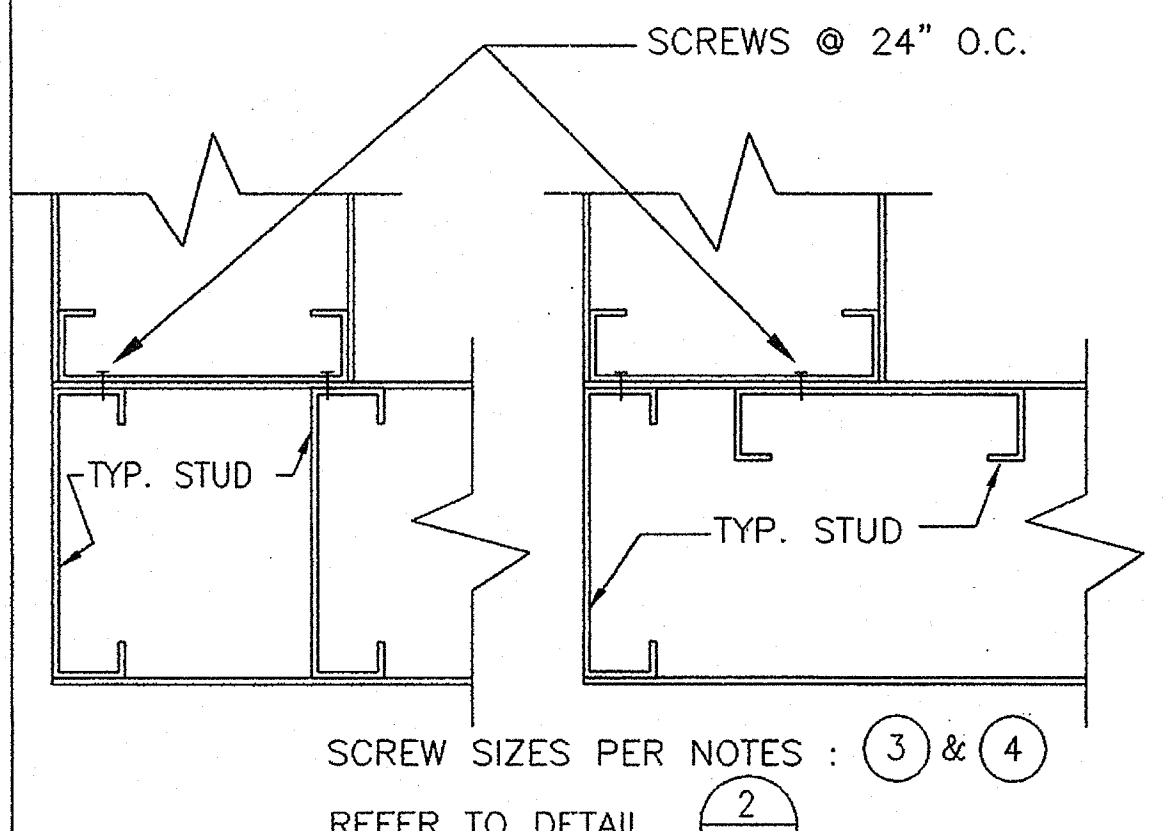
STUD CONNECTION TO LINTEL

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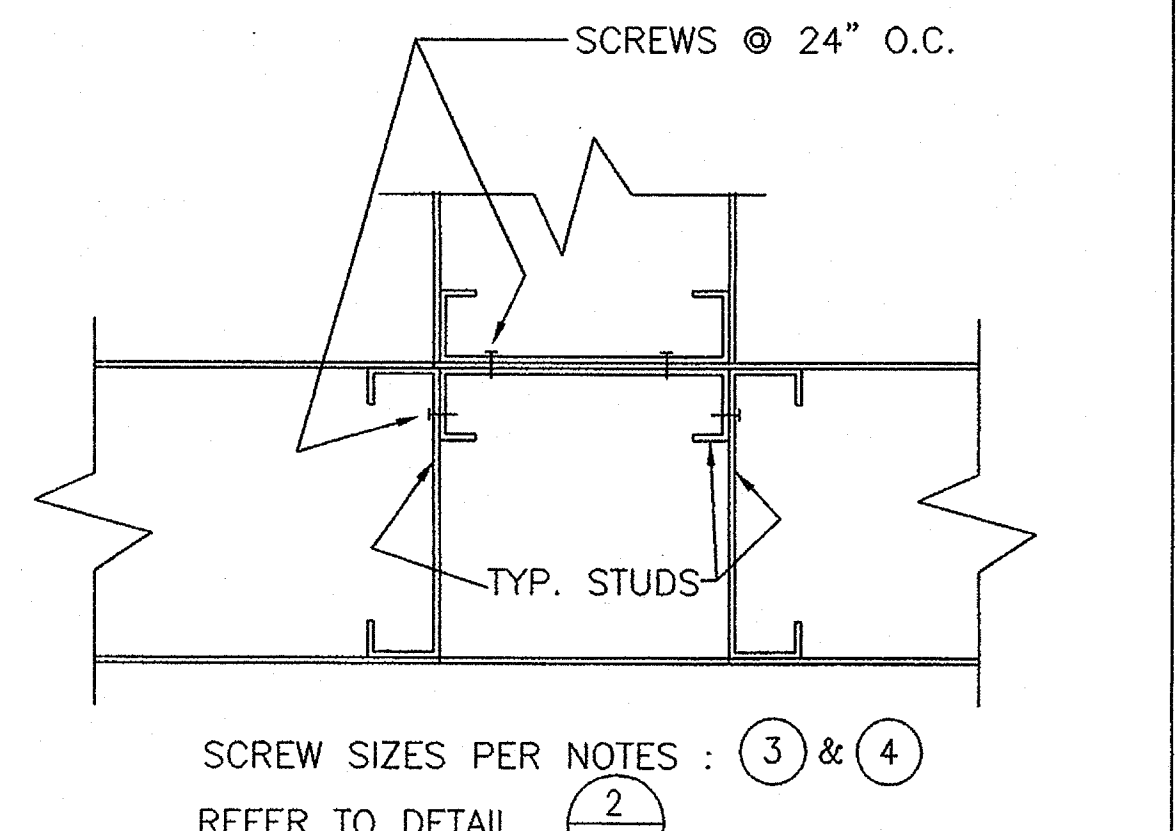
TYPICAL WALL BRIDGING OR BLOCKING

16



TYPICAL CORNER PLAN

12



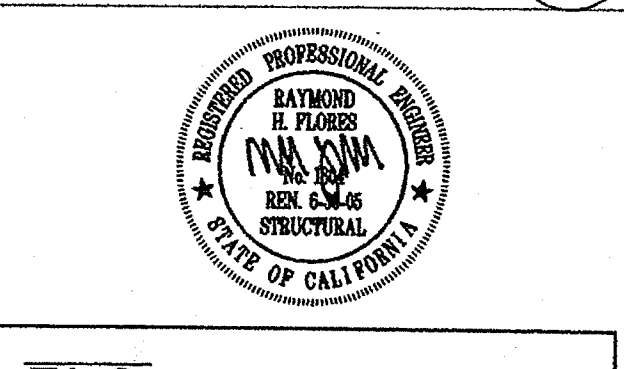
TYPICAL INTERSECTION PLAN

8

NOTES:

1. ALL WEDGE TYPE ANCHORS SHALL BE ITW RAMSET/REDHEAD EXPANSION BOLT PER LATEST ICBO REPORT #ER-1372. FOR TESTING SEE GENERAL NOTES, DWG. S1.0A
2. POWER DRIVEN PINS SHALL BE ITW RAMSET / REDHEAD PER LATEST ICBO REPORT #1639 SEE NOTES ON 4 SNI.3

4



7220 TRADE STREET, SUITE 100, SAN DIEGO, CALIFORNIA 92121
619 544-0629 FAX 619 544-0627

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OCEANSIDE, CA 92054

OCEANSIDE UNIFIED S.D.

3355 MISSION AVE. SUITE 234 OCEANSIDE, CALIFORNIA 92054

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JOHN SCOTT GROTH

C-26609

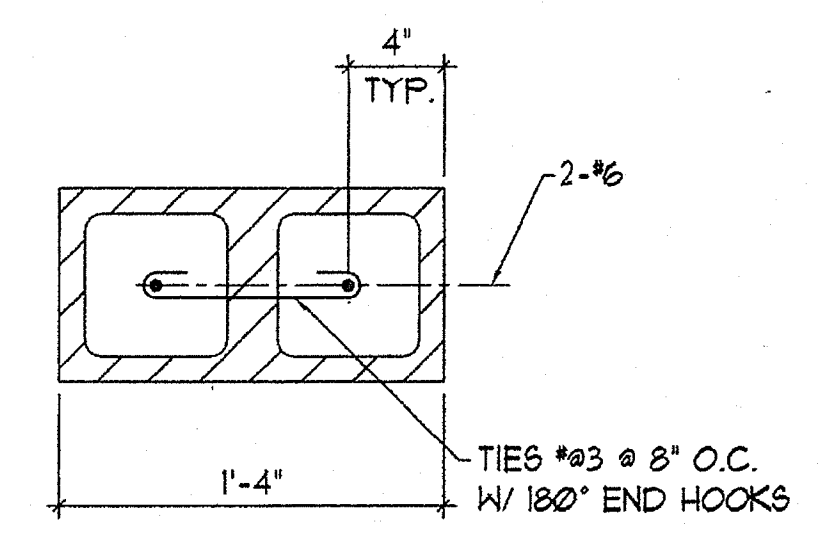
4/30/2007

STATE OF CALIFORNIA

SHEET TITLE

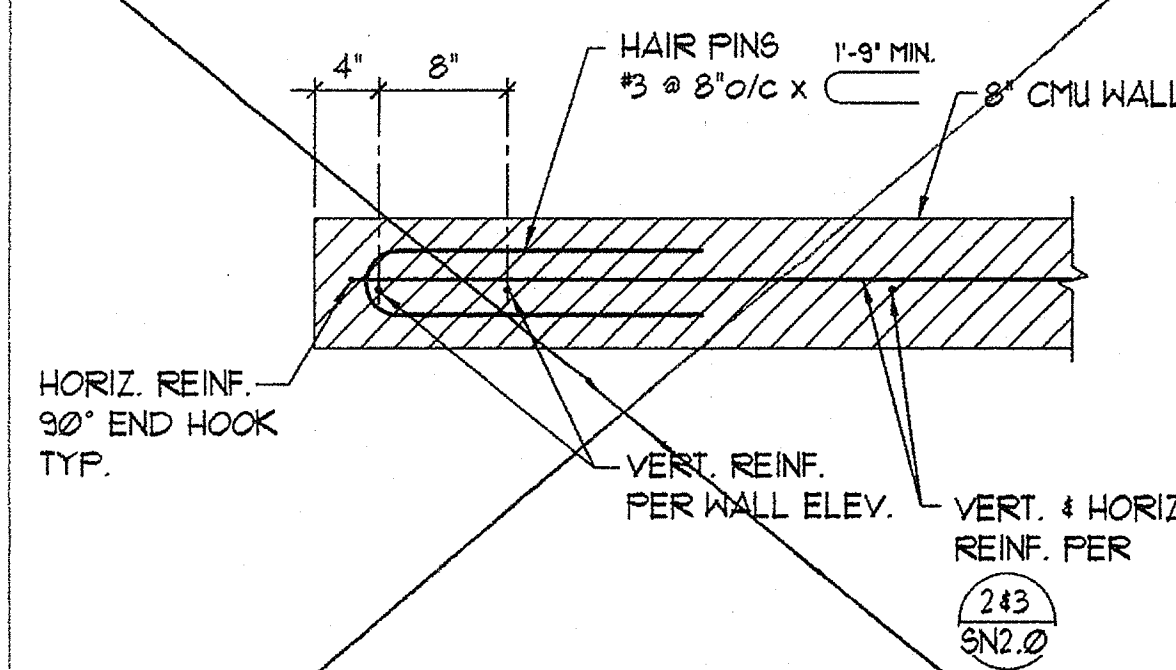
TYPICAL DETAILS

SN1.4



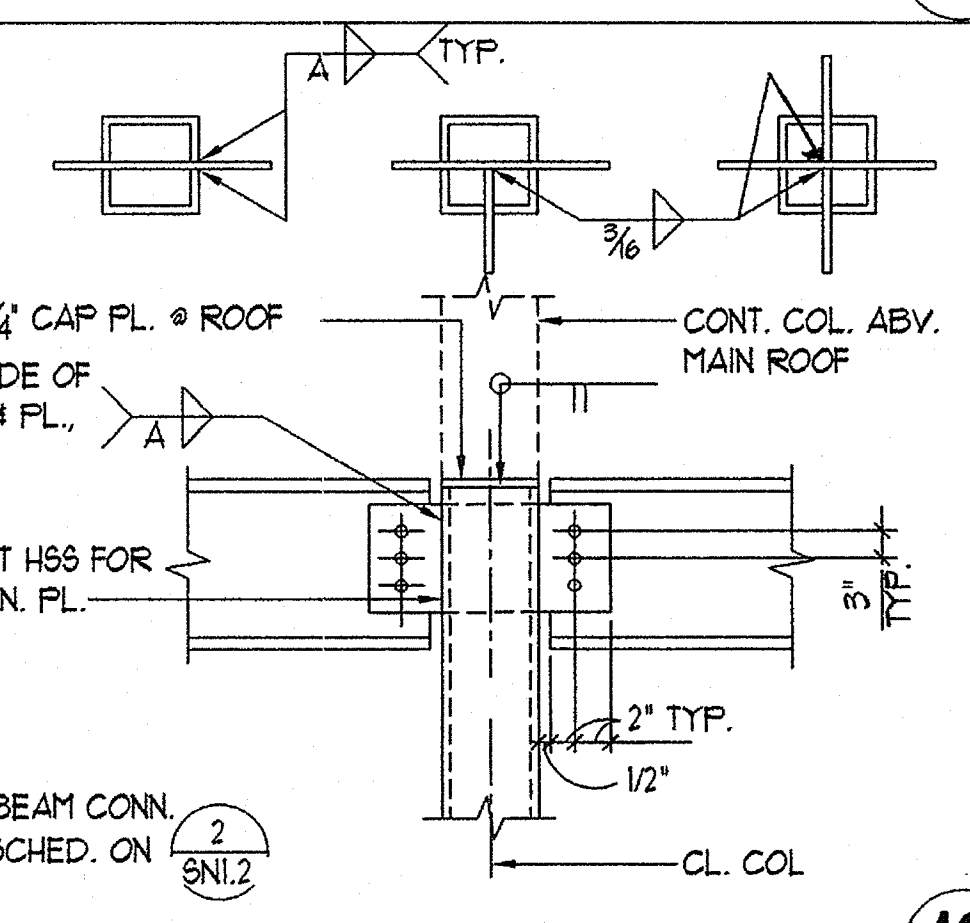
PLAN- INTERMEDIATE CMU WINDOW JAMB

17



PLAN NOT USED

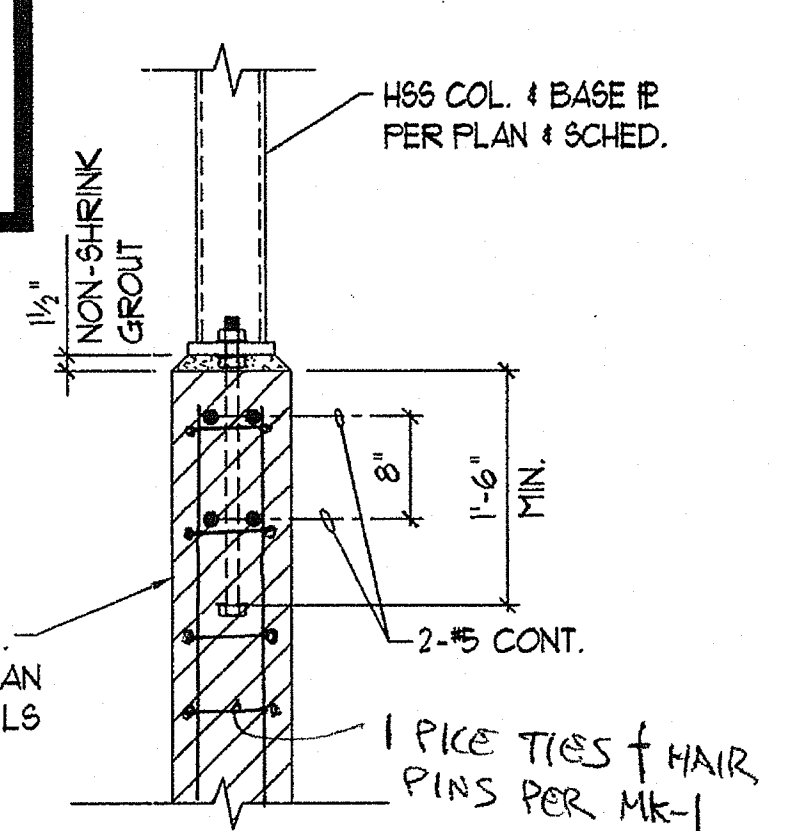
18



BEAM TO H66 COL.

10

NOTE: FOR INFO. NOT SHOWN OR CALLED OUT SEE (1) TYP. (55 @)

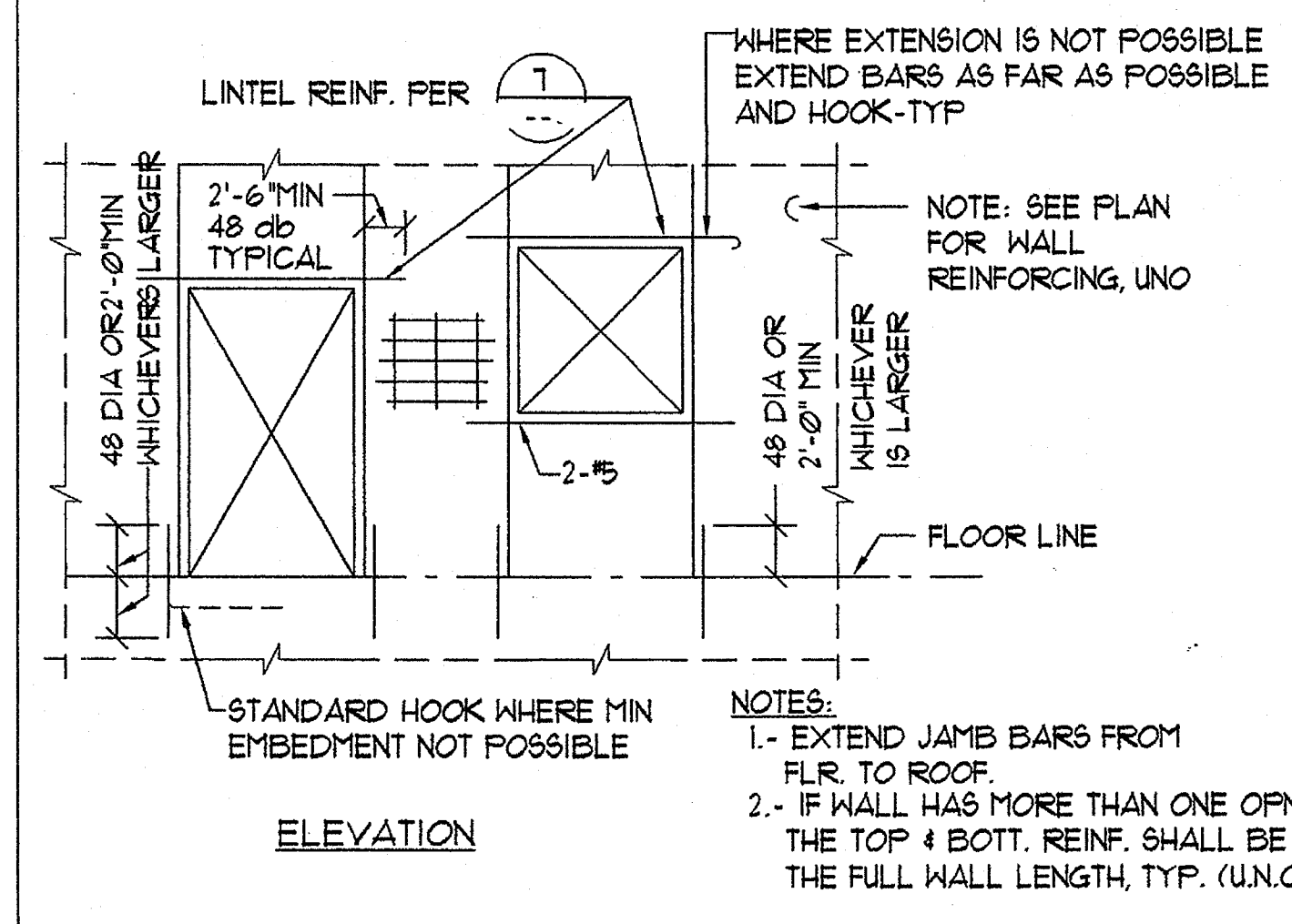


TYP. WALL/COLUMN CONNECTION

11

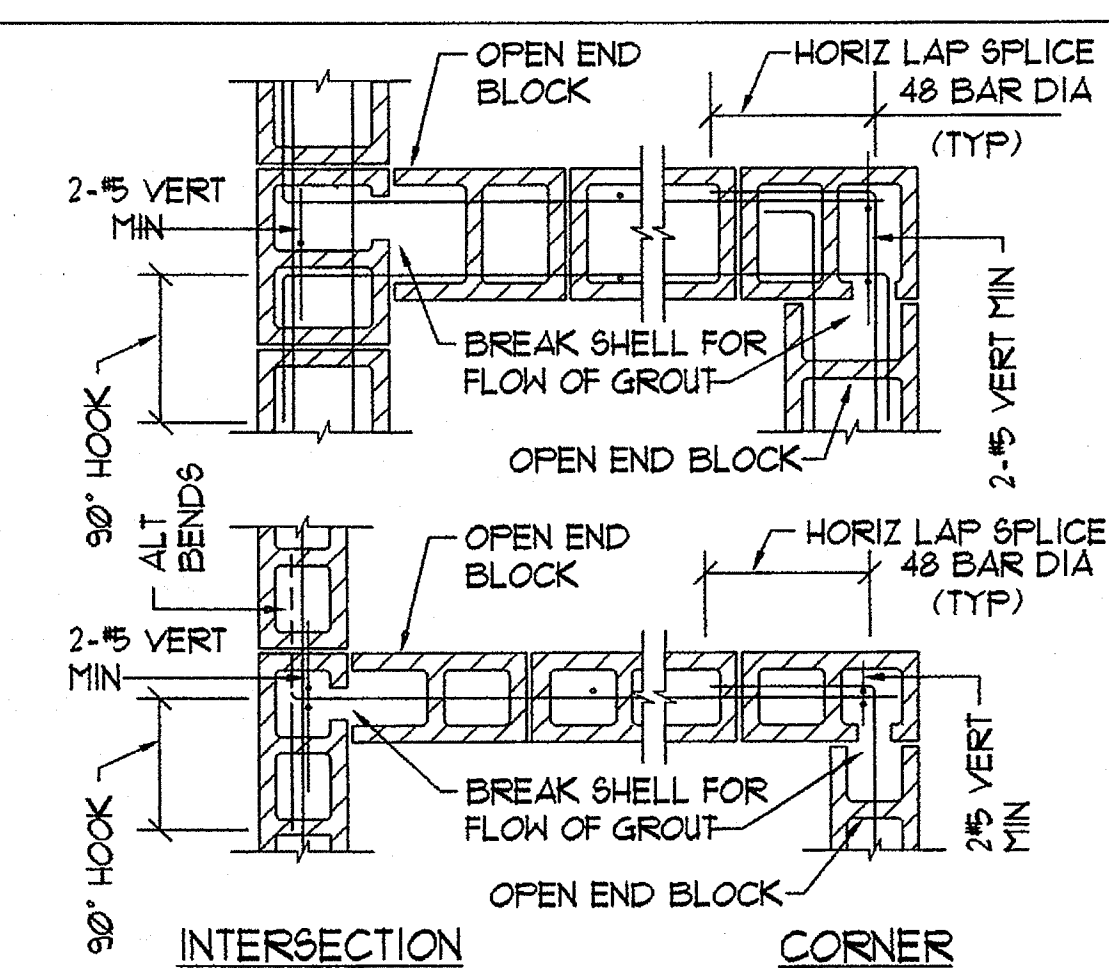
PLAN- TYP. WALL REINF. @ ROOF STL. BM.

15



MIN. REINFORCING AT MASONRY WALL OPENINGS

1

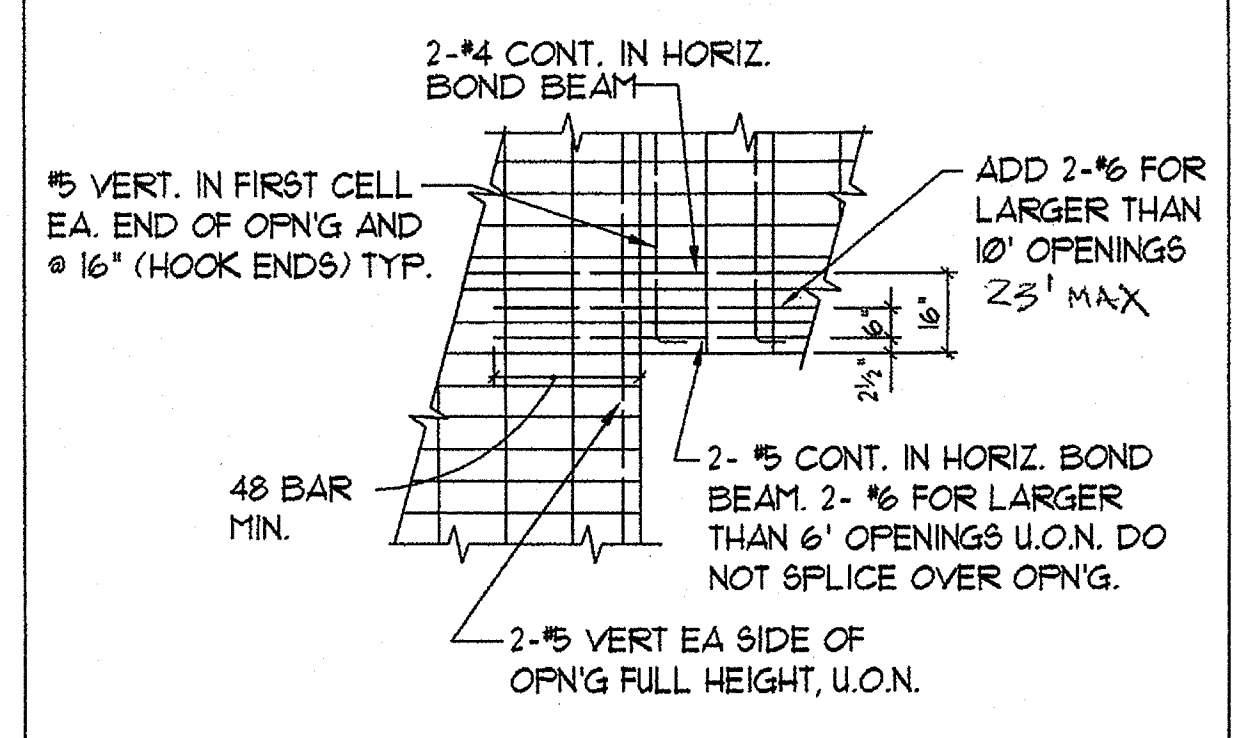


MINIMUM MASONRY REINFORCING DETAILS

2

NOTES:
1. WHERE VERT. OR HORIZ. BARS ARE SHOWN IN THIS DETAIL, PROVIDE BARS OF SAME SIZE AND DETAIL AS SHOWN ON DRAWINGS FOR WALLS. WHERE SIZE IS NOT SHOWN, USE AT LEAST #5 VERTS. AND #4 HORIZ.
2. AT CONTRACTORS OPTION, CORNER BARS MAY BE USED TO LAP (48 DIA. MIN.) W/ HORIZ. REINF. AS FOLLOWS:

CONCRETE MASONRY WALL INTERSECTIONS

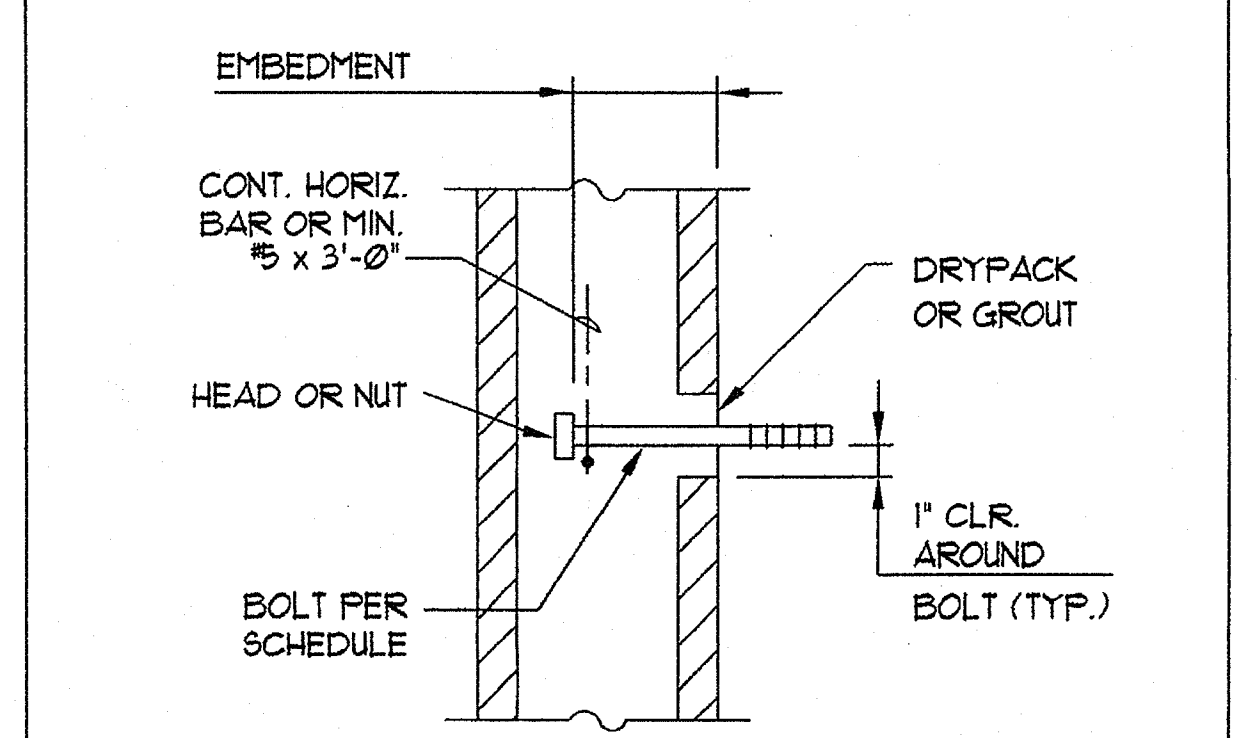
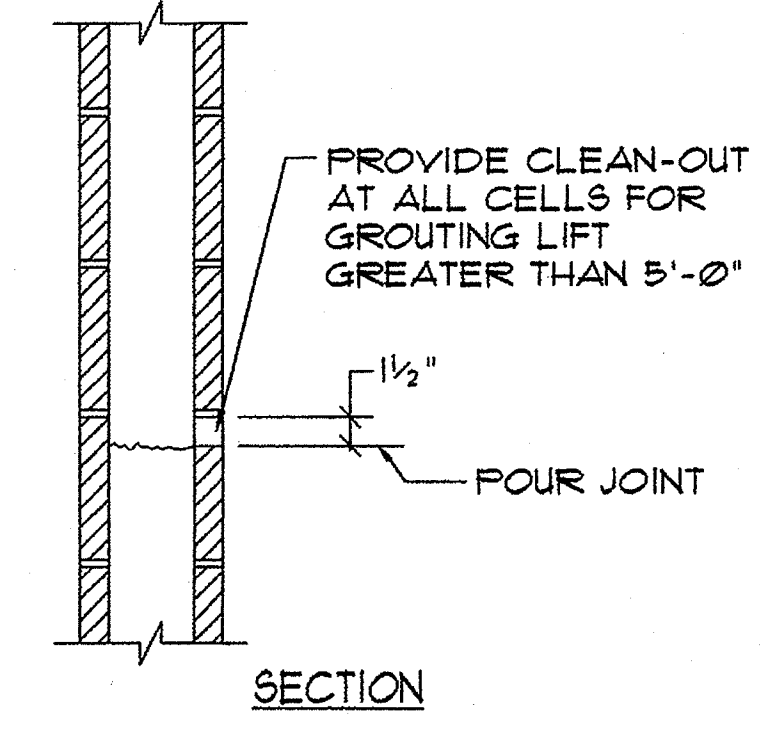


TYP. MASONRY LINTELS

7

VERTICAL CONSTRUCTION JOINT IN MASONRY WALL

3

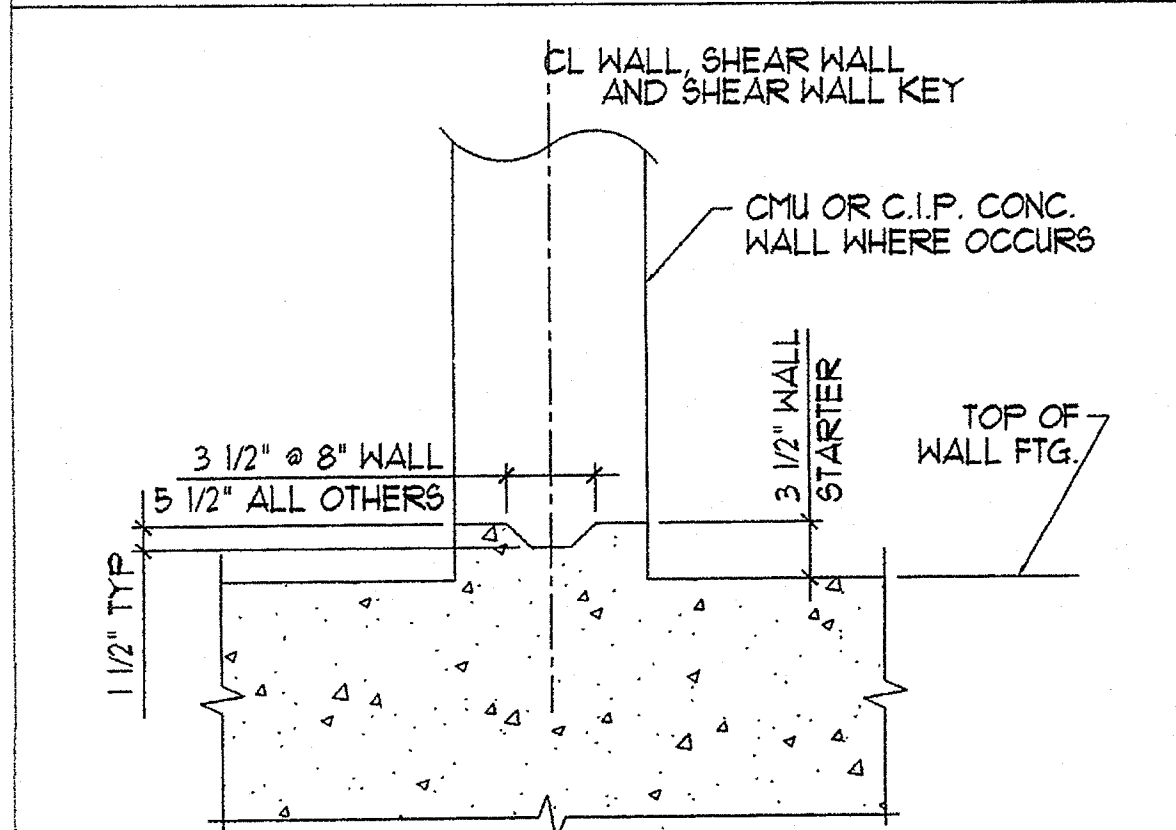


MIN. EMBED. FOR ALL BOLTS

BOLT SIZE	EMBEDMENT
5/8"	5"

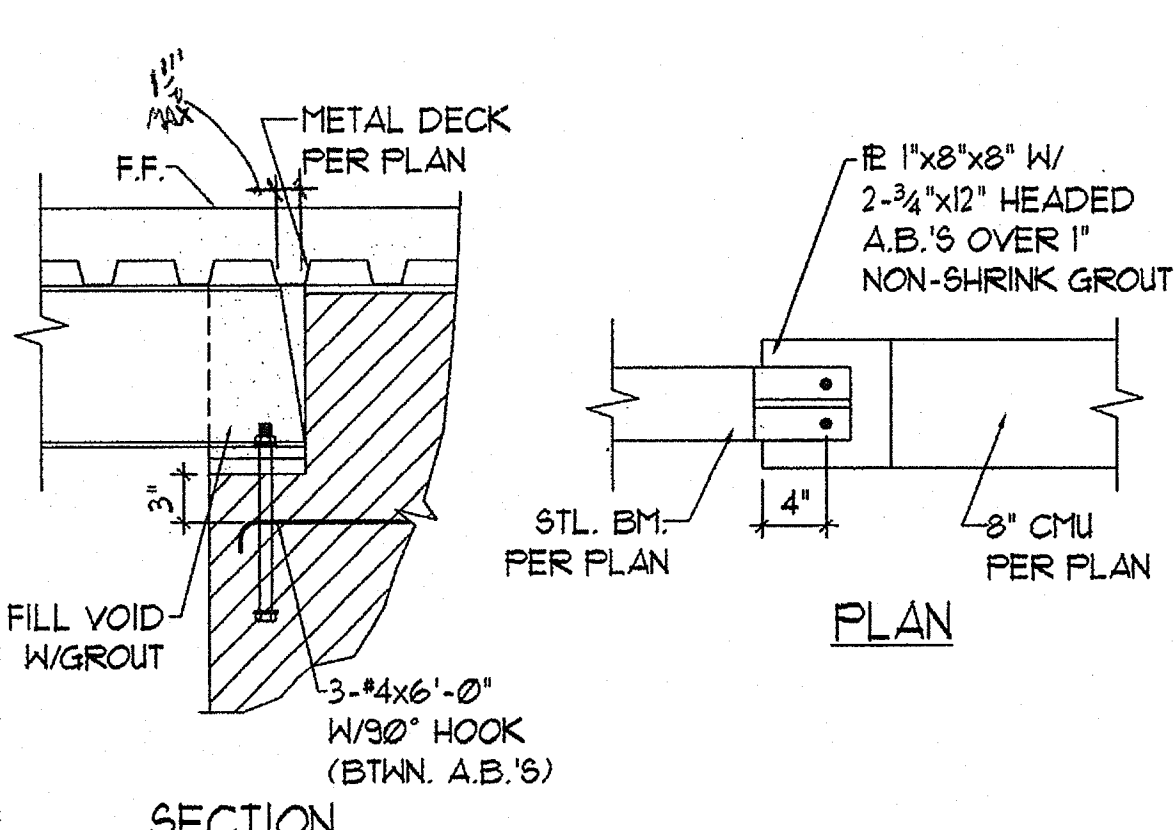
BOLT EMBED. @ CMU WALLS

8



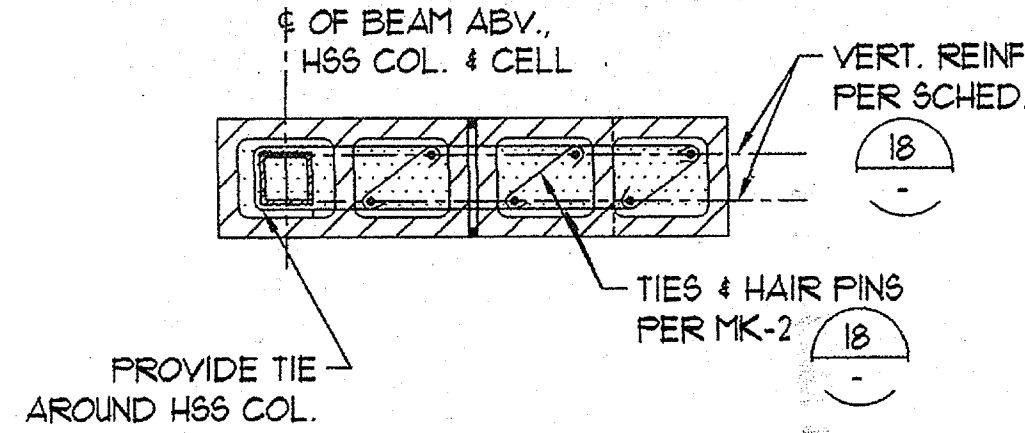
NOTE: FOR STARTER WALL FORMIG AND INFORMATION NOT SHOWN OR CALLED OUT SEE (10) (3N1)

20

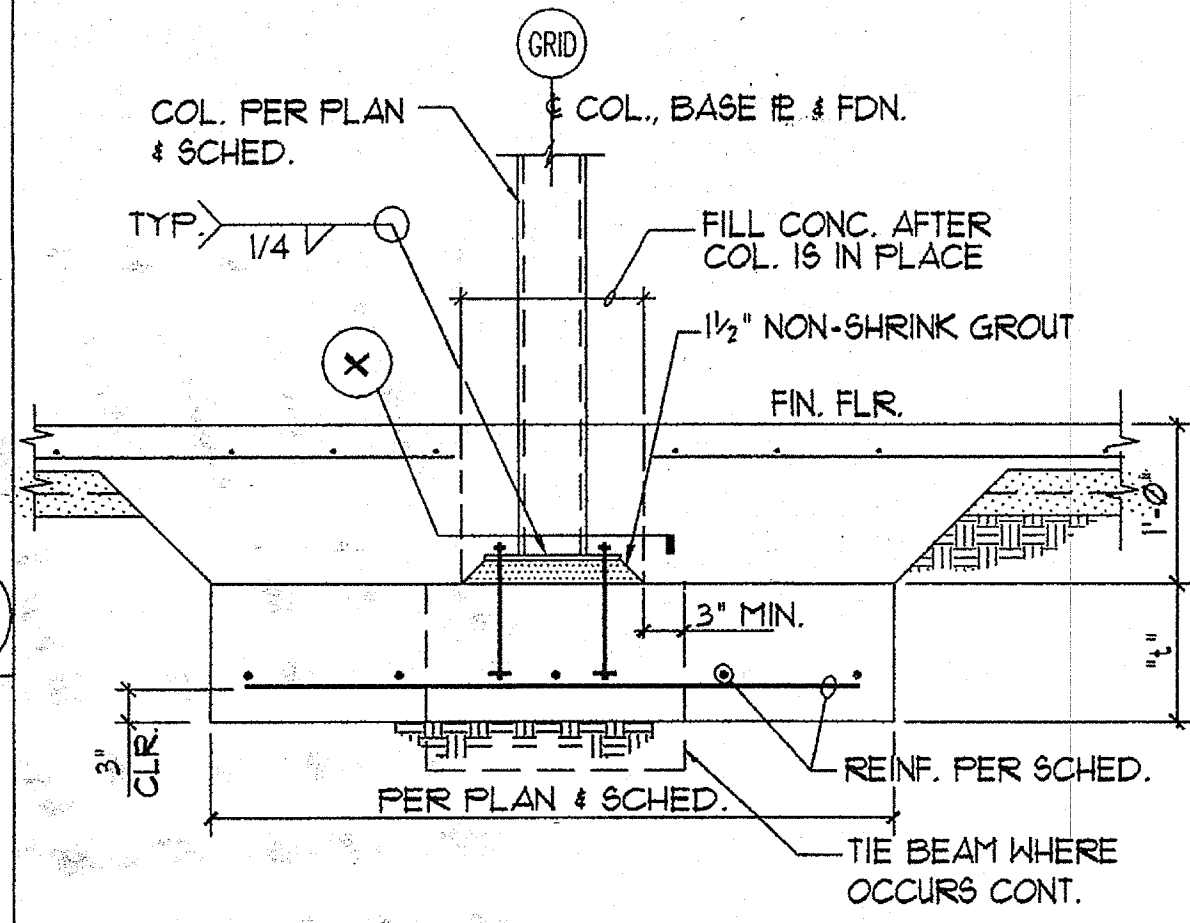


TYP. WALL/BEAM FLUSH CONNECTION

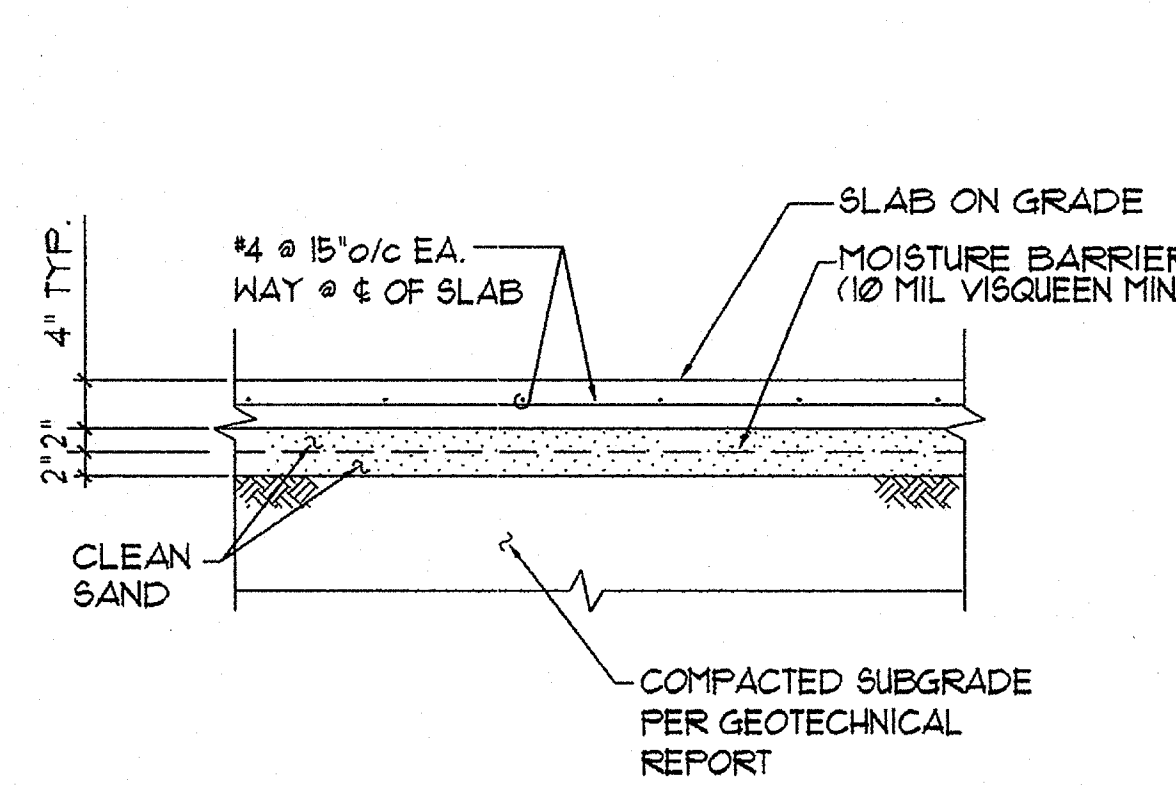
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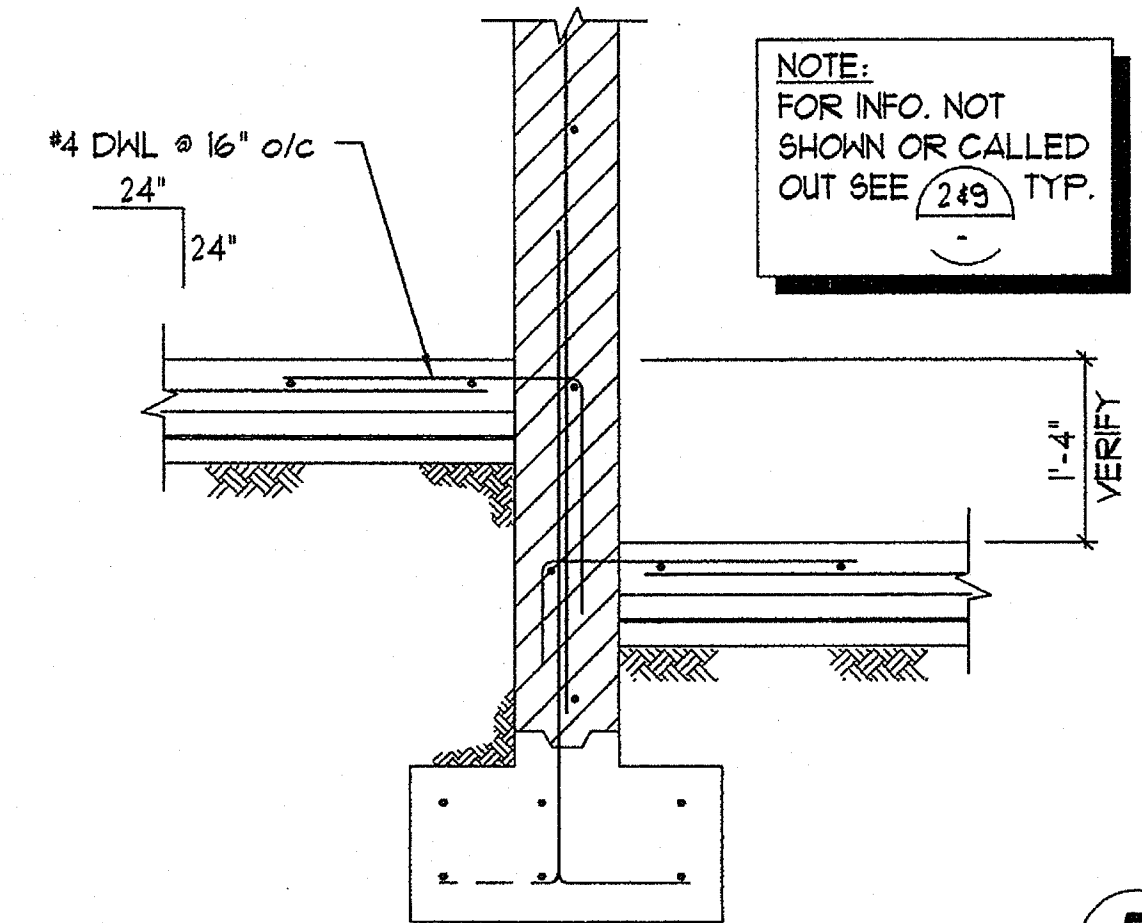
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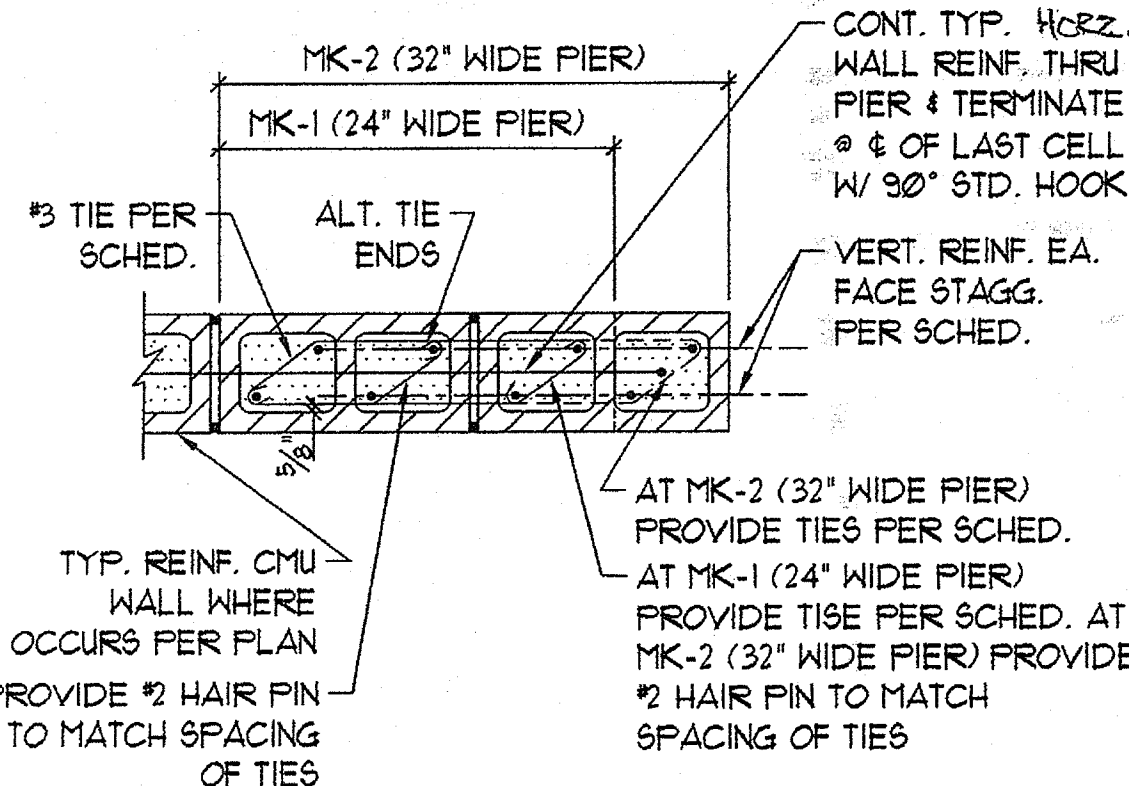
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5



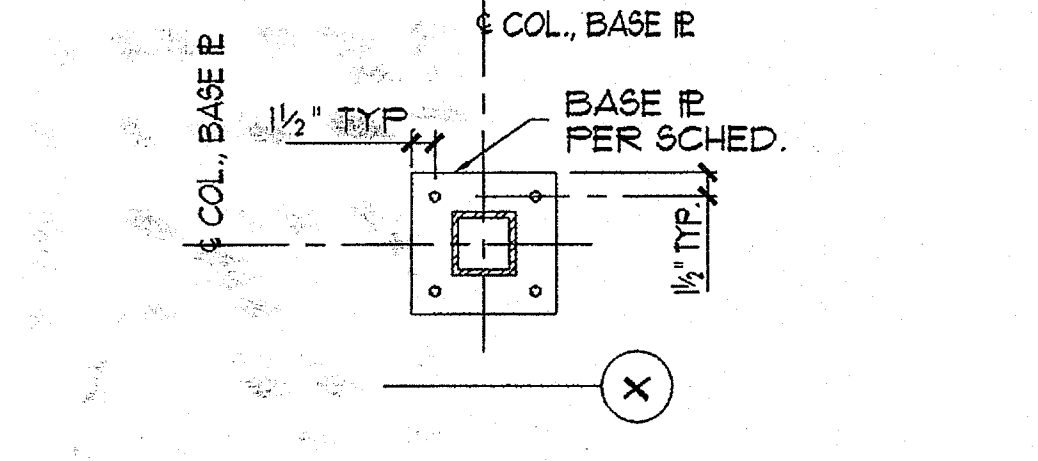
1



BOUNDARY ELEMENT REINFORCEMENT SCHEDULE				
BOUNDARY ELEMENT MARK	LENGTH	VERT. REINF. EA. FACE	TIES	REMARKS
MK1	24"	3-#5	#3@8" O/C	
MK2	32"	4-#5	#3@8" O/C	

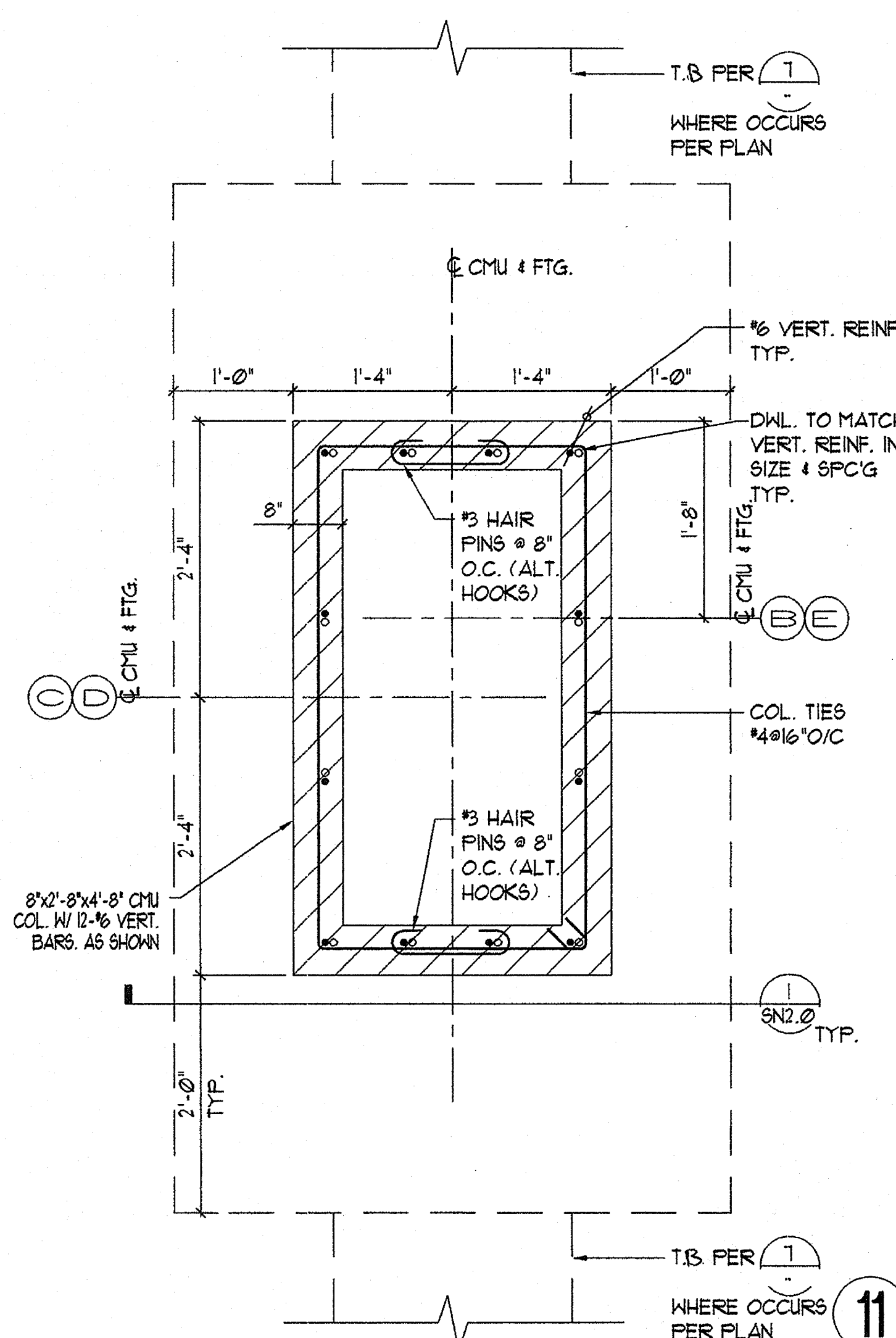
NOTES
1. PROVIDE VERT. REINF. FULL HEIGHT OF WALL.
2. EXTEND TIES 24" ABOVE OPENING, MIN.

18

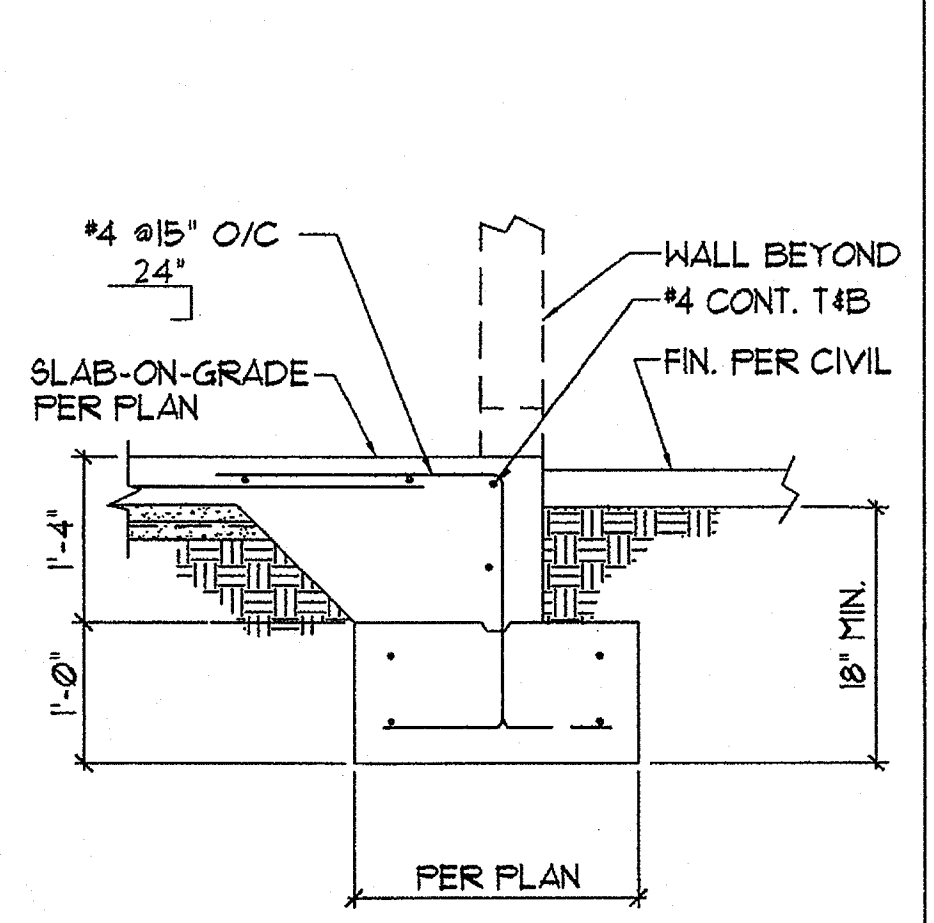


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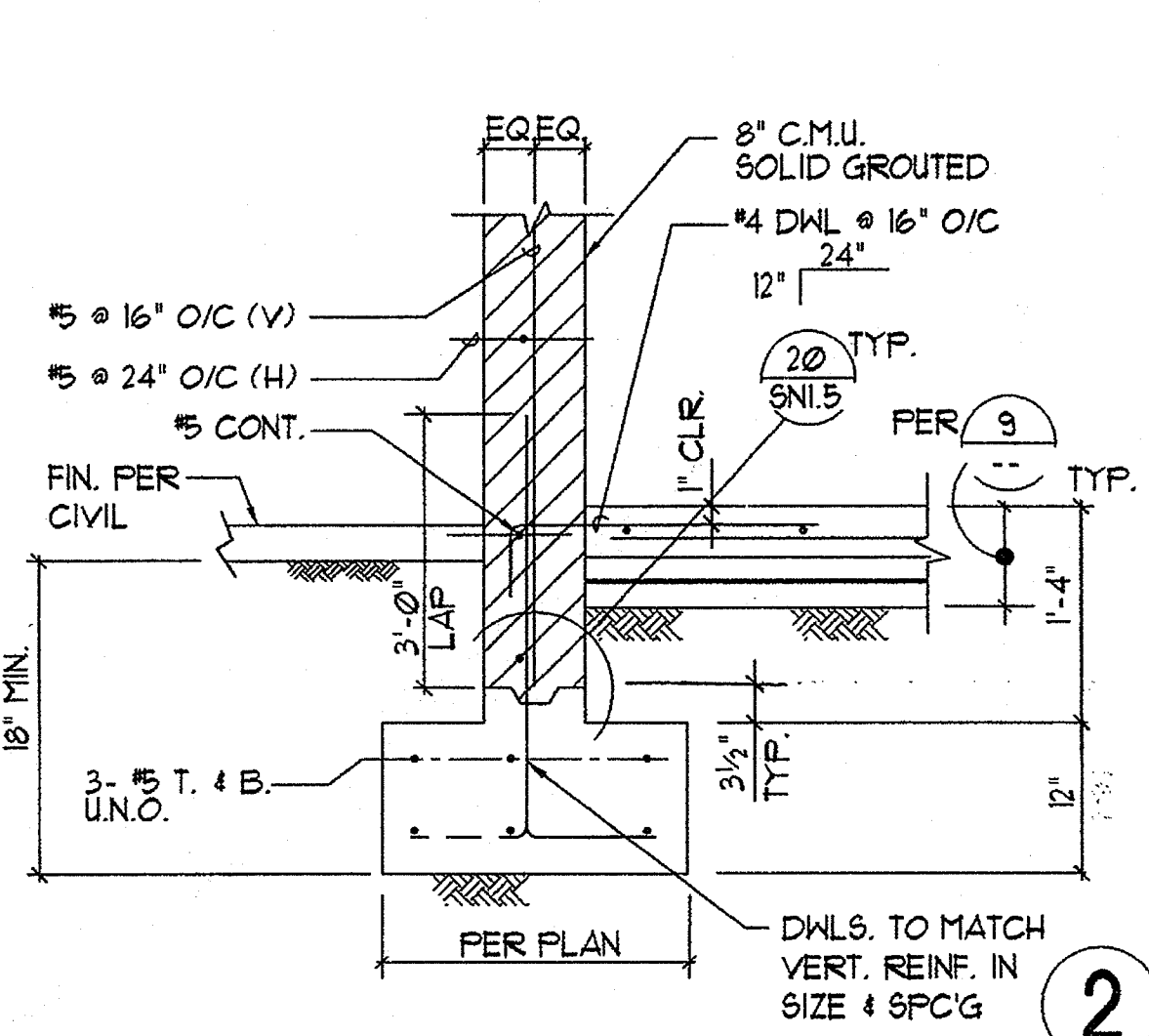
COLUMN SCHEDULE			
MARK	SIZE	TYP. BASE IR AND A.B.'S U.N.O.	REMARKS
C4	HSS 4x4x1/4	IR 3/4"x6"x12" w/ 2-3/4"x8" EMBED. A.B.'S	
C3	HSS 3x3x5/16		SEE 16/SN2.0



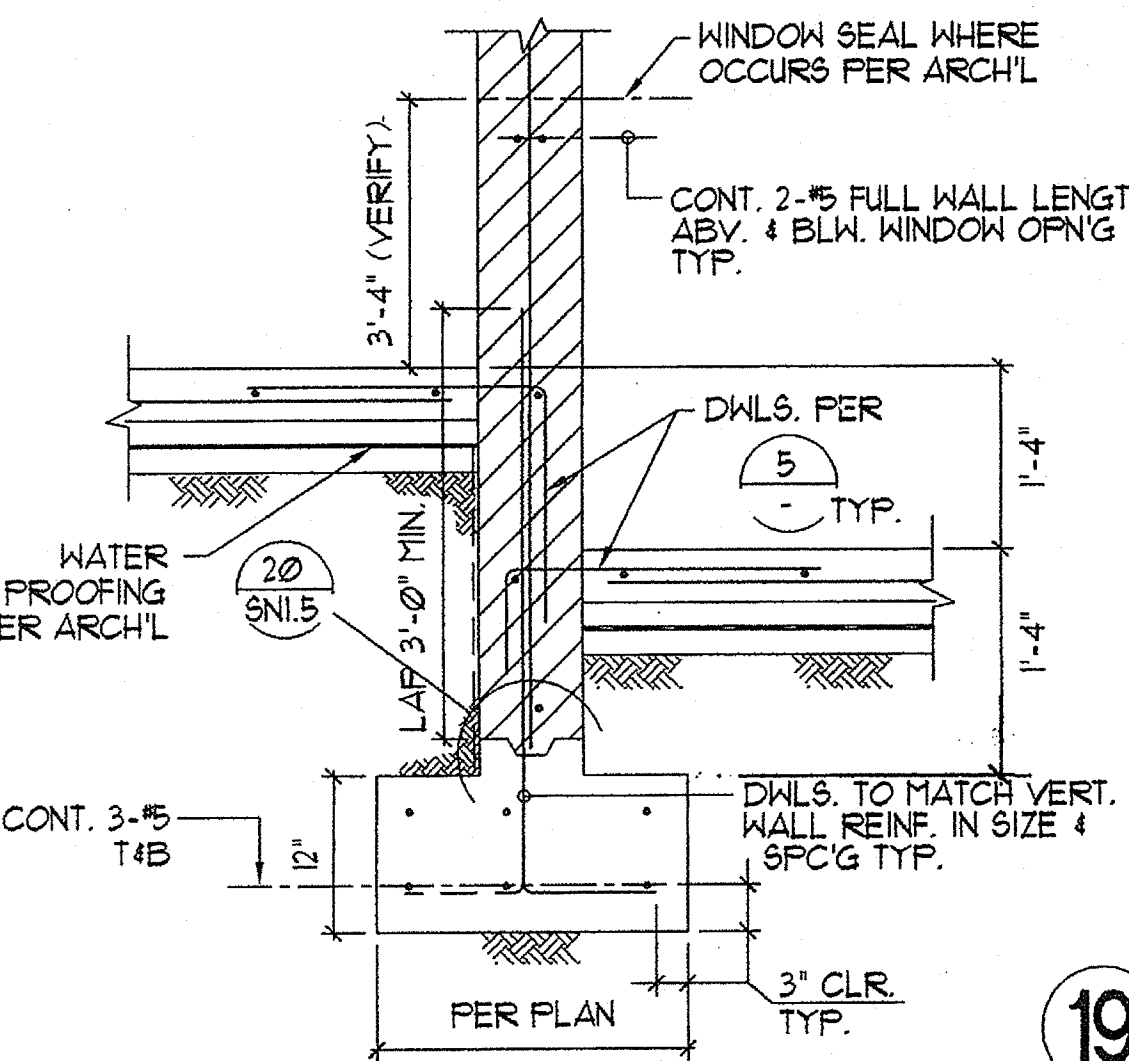
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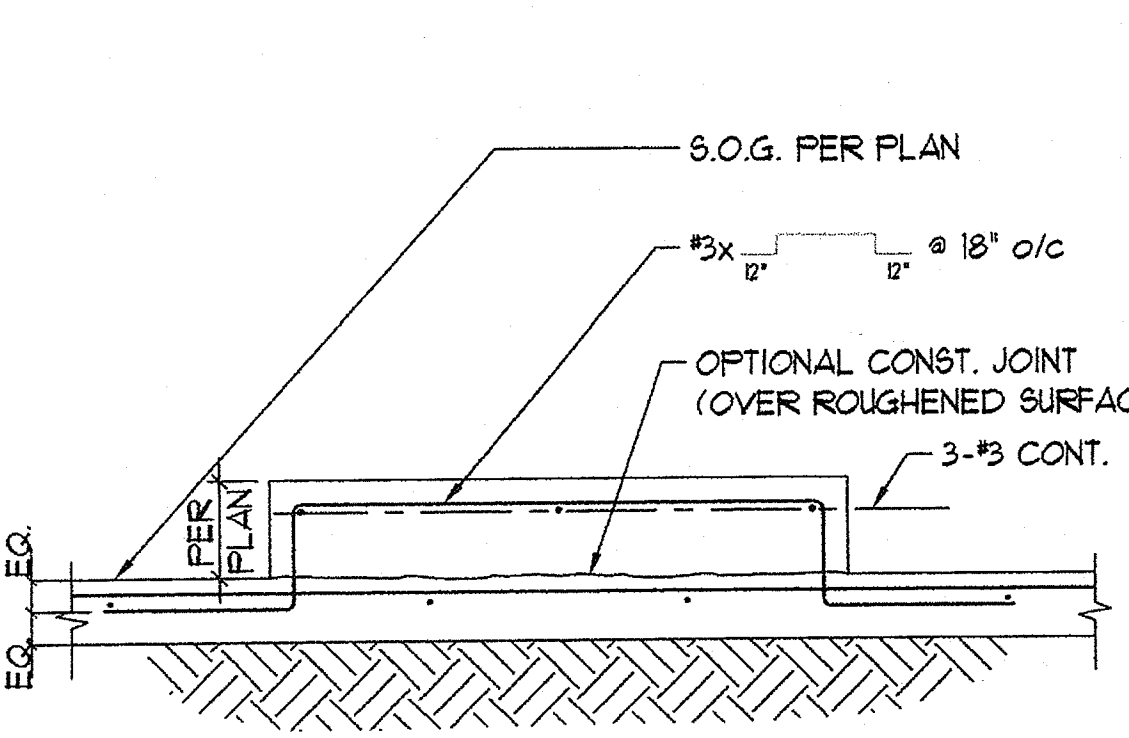
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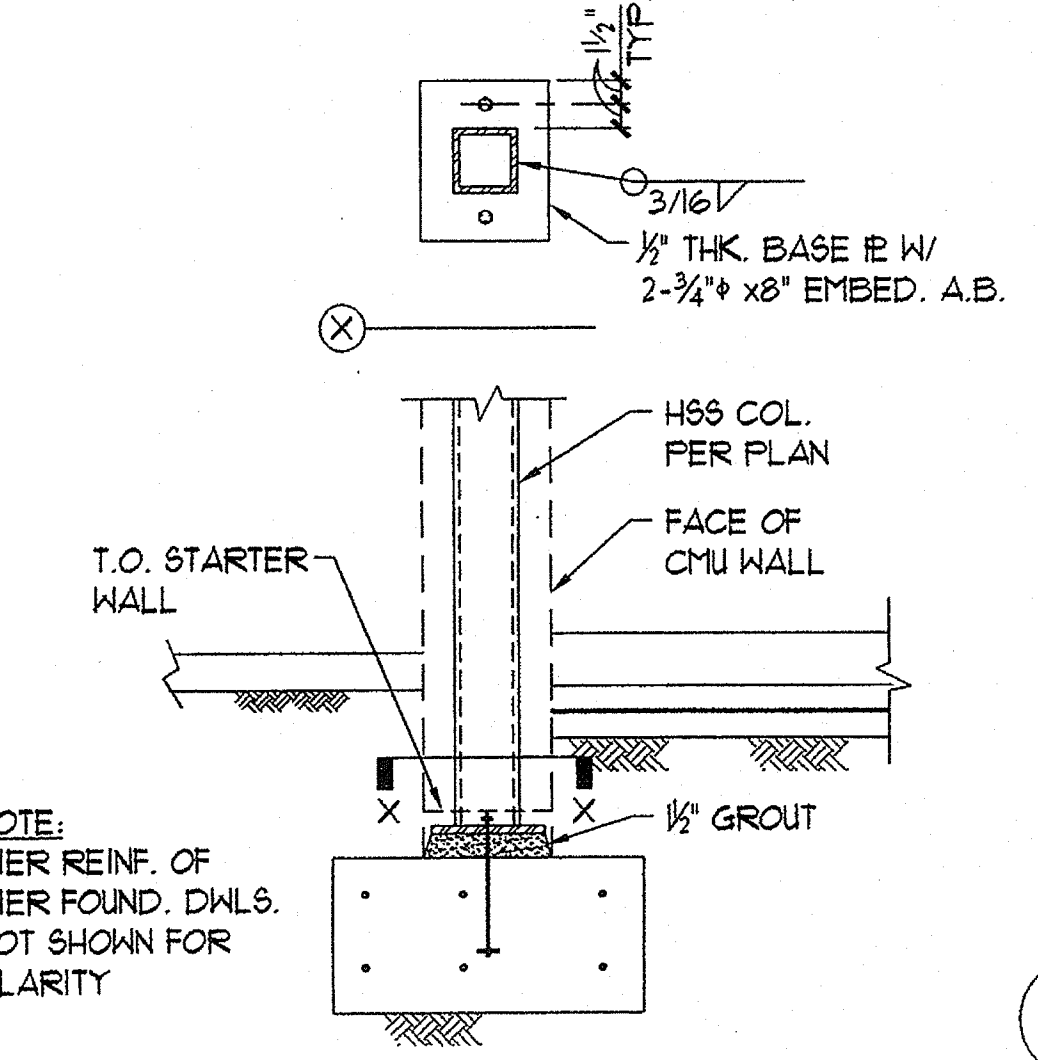
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19



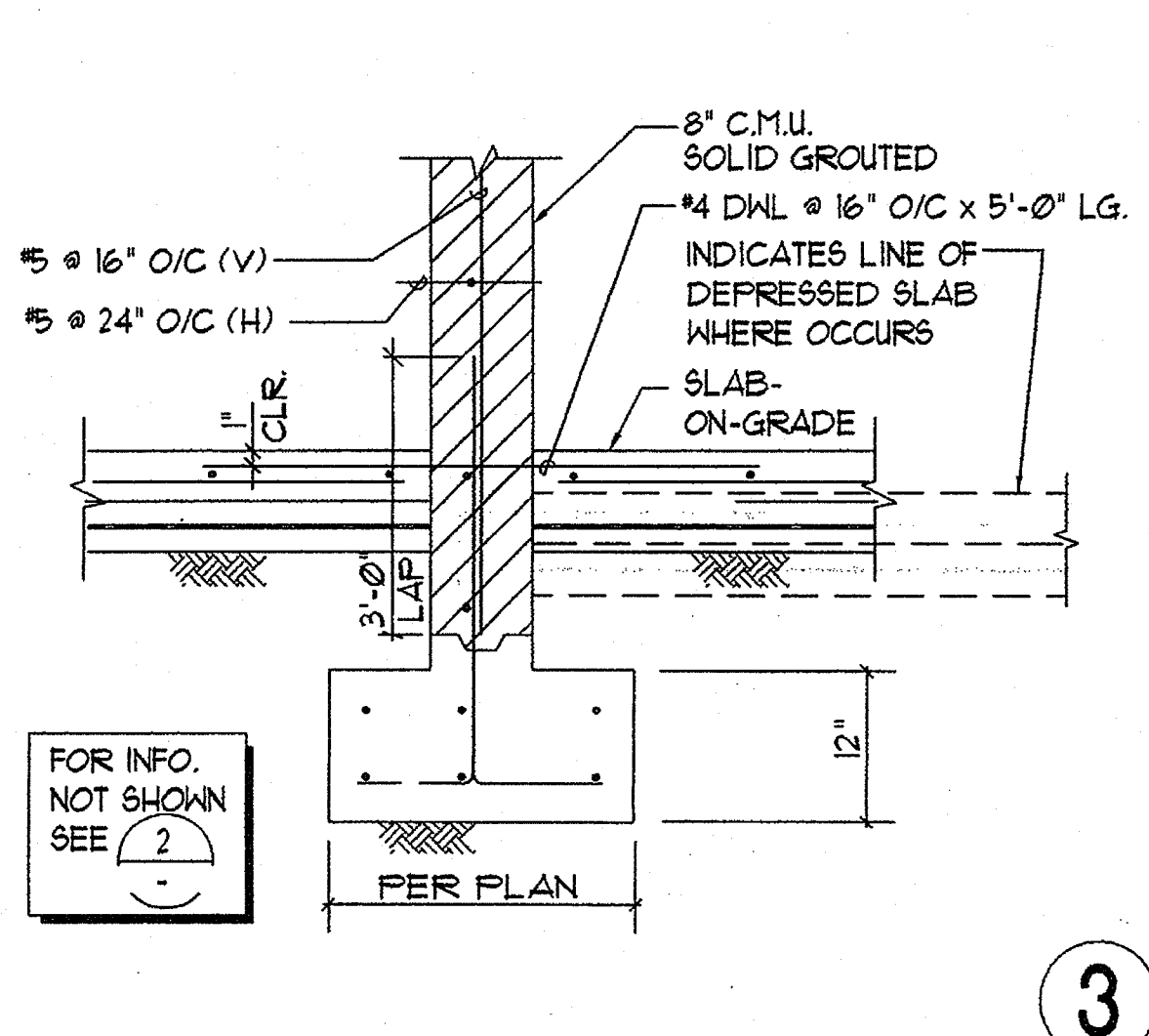
20



16



7



3

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JOHN SCOTT BOHL
C-26609
4/30/2007
renewed
STATE OF CALIFORNIA

SHEET TITLE

FOUNDATION DETAILS

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PROFESSIONAL ENGINEERS
7250 TRADE STREET, SUITE 100, SAN DIEGO, CALIFORNIA 92121
619 594-0628 FAX 619 594-0627

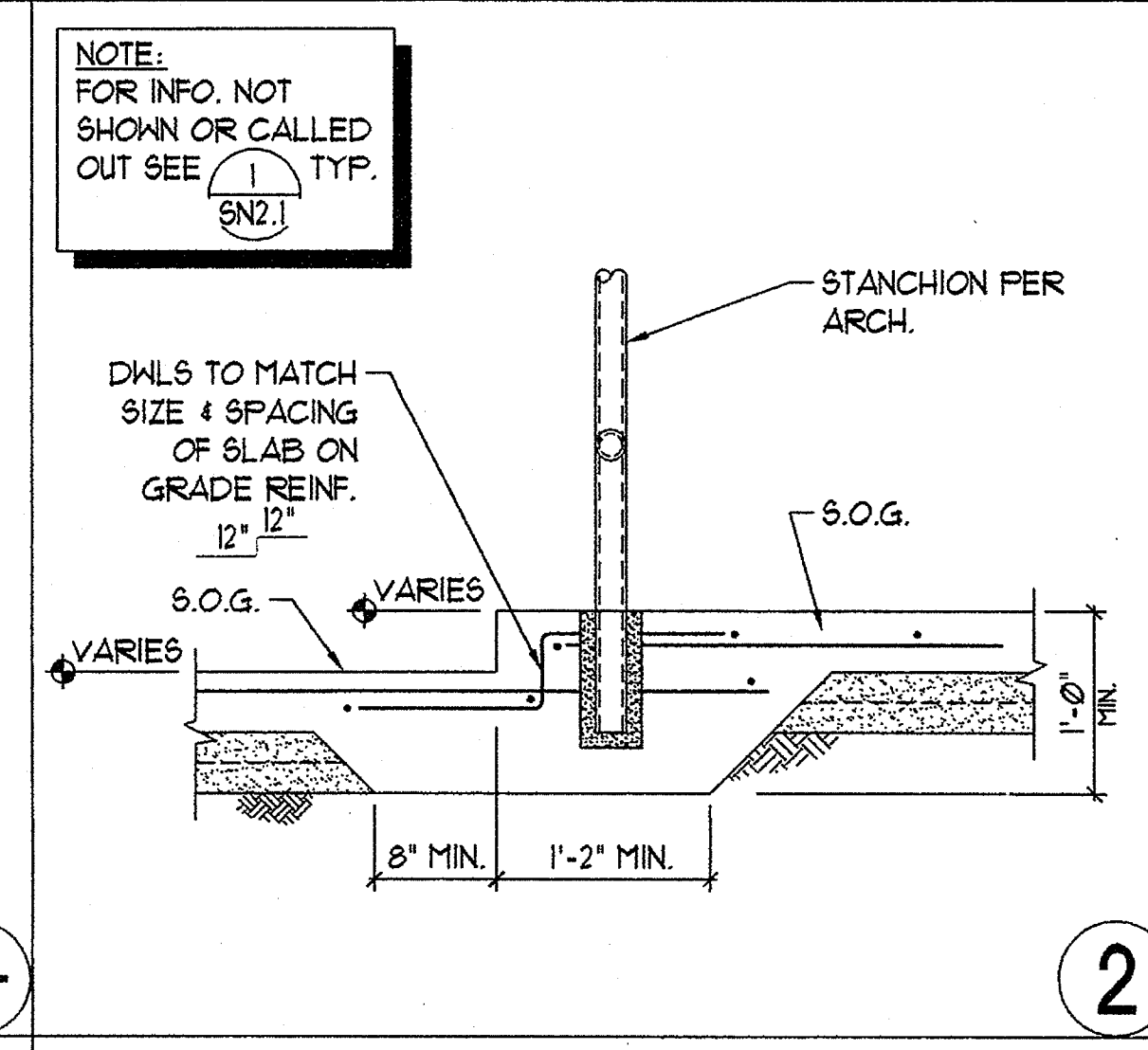
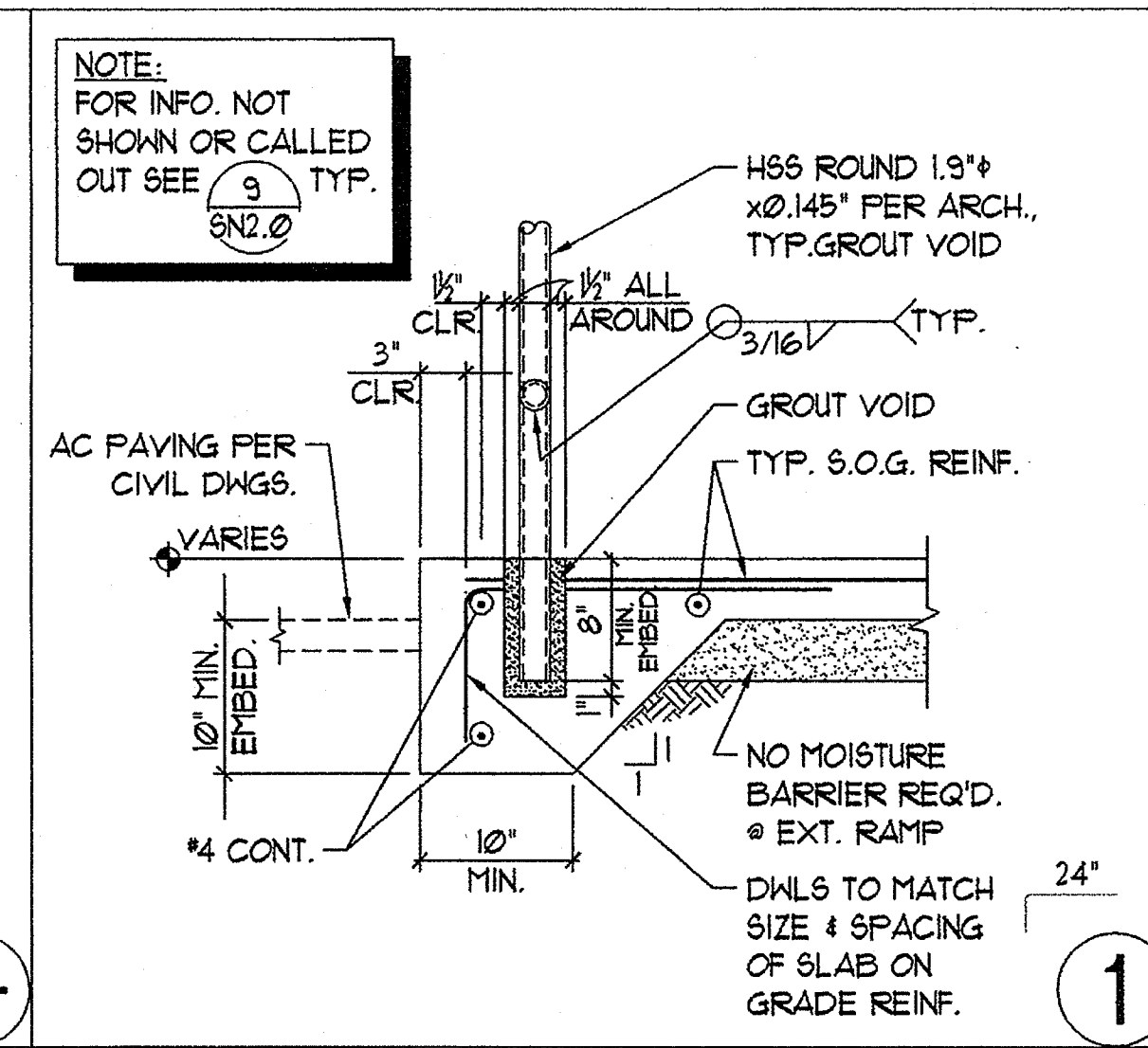
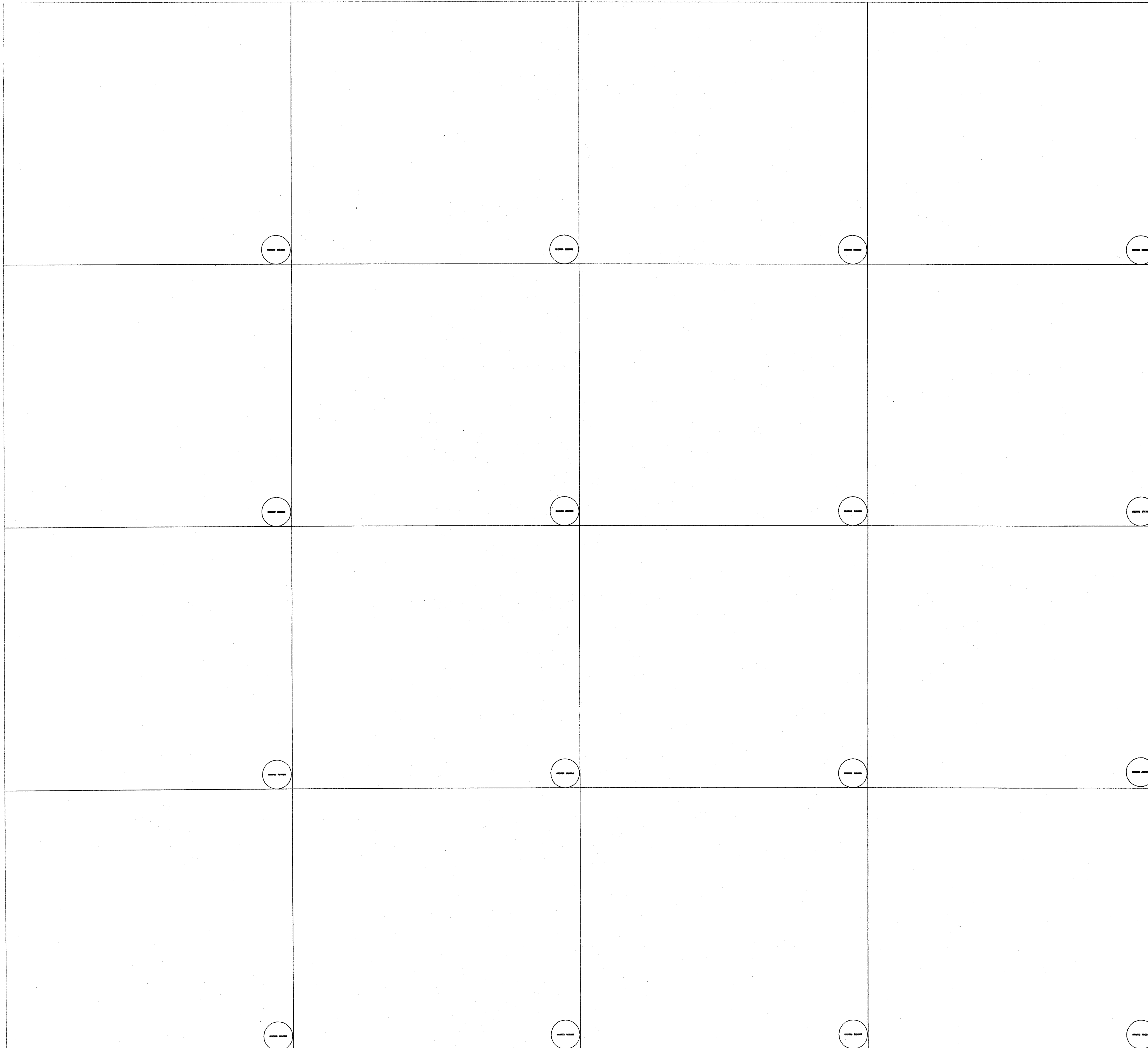
SN2.0

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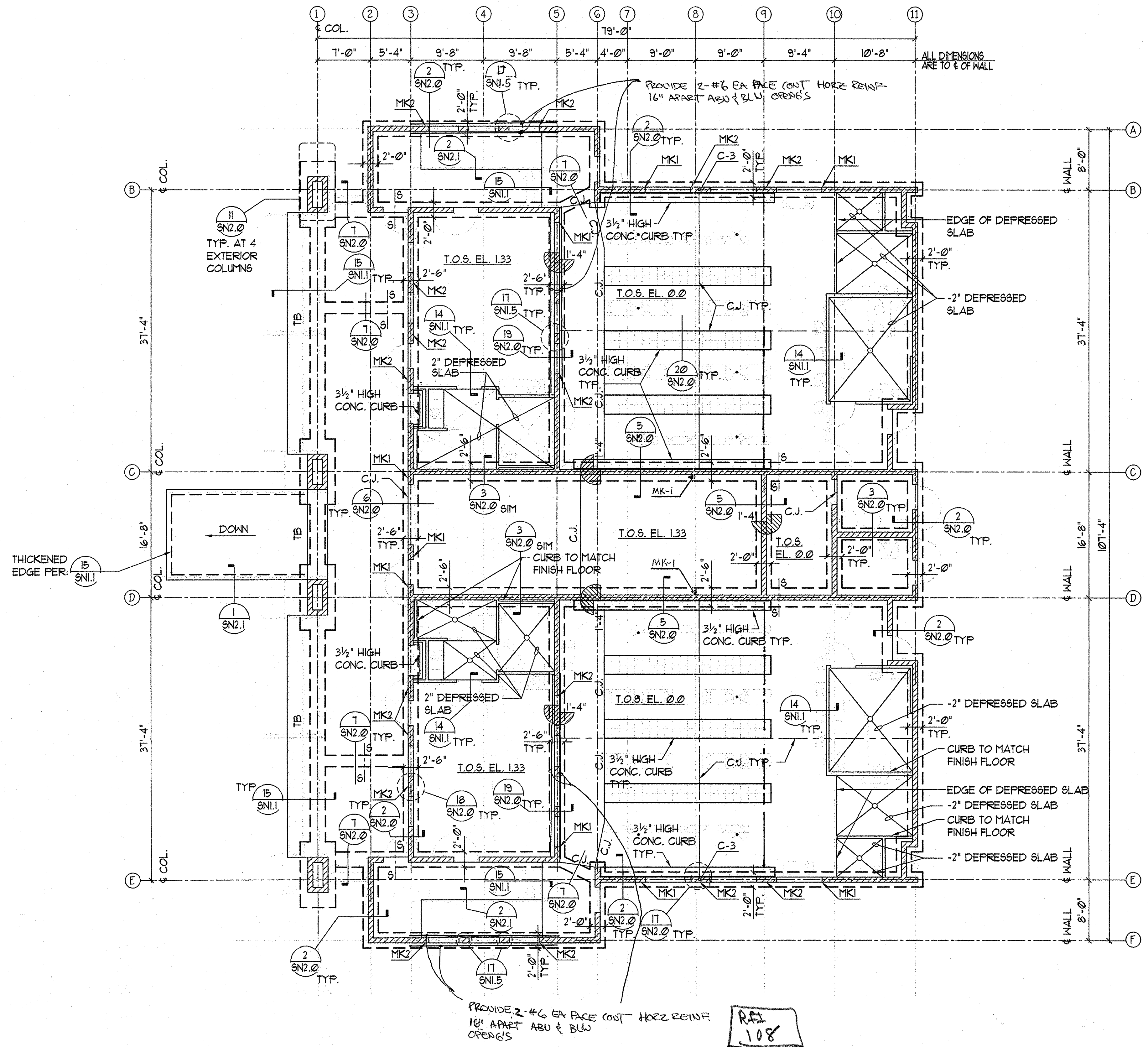
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 4/30/2007
 Renewal
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 7220 TRADE STREET, SUITE 100, SAN DIEGO, CALIFORNIA 92121
 (619) 566-0265 FAX (619) 566-0267

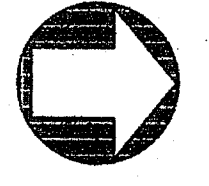
FOUNDATION DETAILS

SN2.1



SHOWER/LOCKER BLDG. - FOUNDATION PLAN

1/8" = 1'-0"



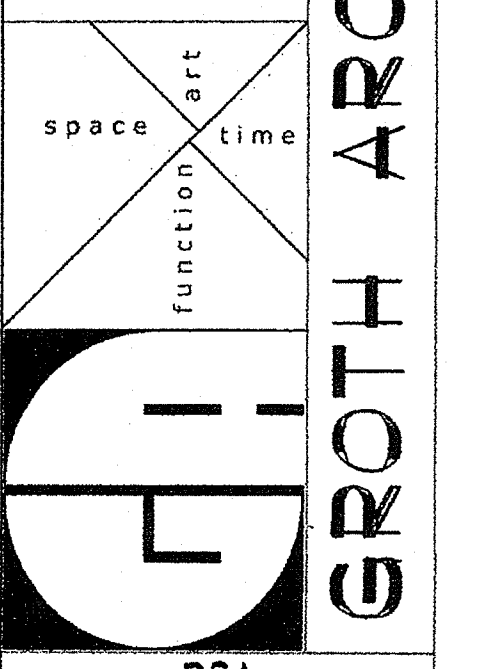
NOTES:

- TOP OF CONCRETE SLAB-ON-GRADE SHALL BE PER CIVIL DWG'S = DATUM ELEVATION. SEE ARCH. DRAWINGS FOR RAMP LANDING ELEVATIONS.
- FOR TYP. CONCRETE SLAB-ON-GRADE SIZE AND REINFORCEMENT SEE (9) SN2.0 TYP.
- GB-1 DENOTES GRADE BEAM PER SCHED. (8) SN2.0 TYP.
- C.J. DENOTES CONSTRUCTION JOINT PER (7) SN1.1 TYP.
- Col DENOTES COL. SIZE PER SCHED. (14) SN1.1 TYP.
- T.O.S. DENOTES TOP OF SLAB ELEVATION. TOP OF FDN'S AT INTERSECTING WALLS AND/OR FTGS. SHALL BE AT THE SAME ELEVATION. STEP FTG'S AS REQ'D PER (8) SN1.1 IN ORDER TO ACCOMPLISH THIS.
- MK DENOTES SPECIAL PIER REINFORCEMENT PER (18) SN2.0 TYP.
- (6) DENOTES APPROXIMATE LOCATION OF OR STEP IN FOOTING PER (8) SN1.1 TYP.

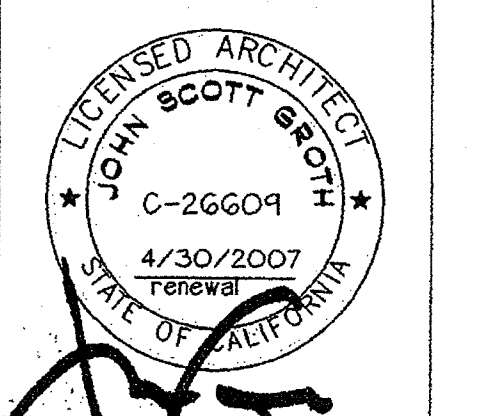
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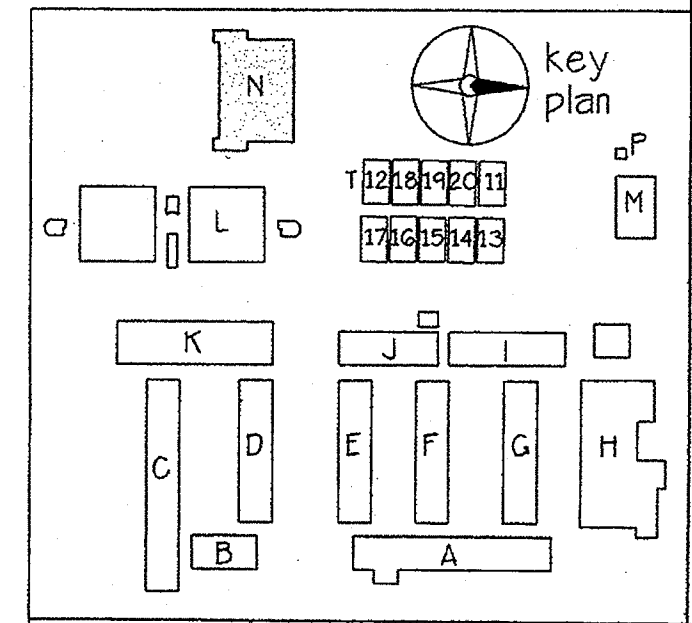


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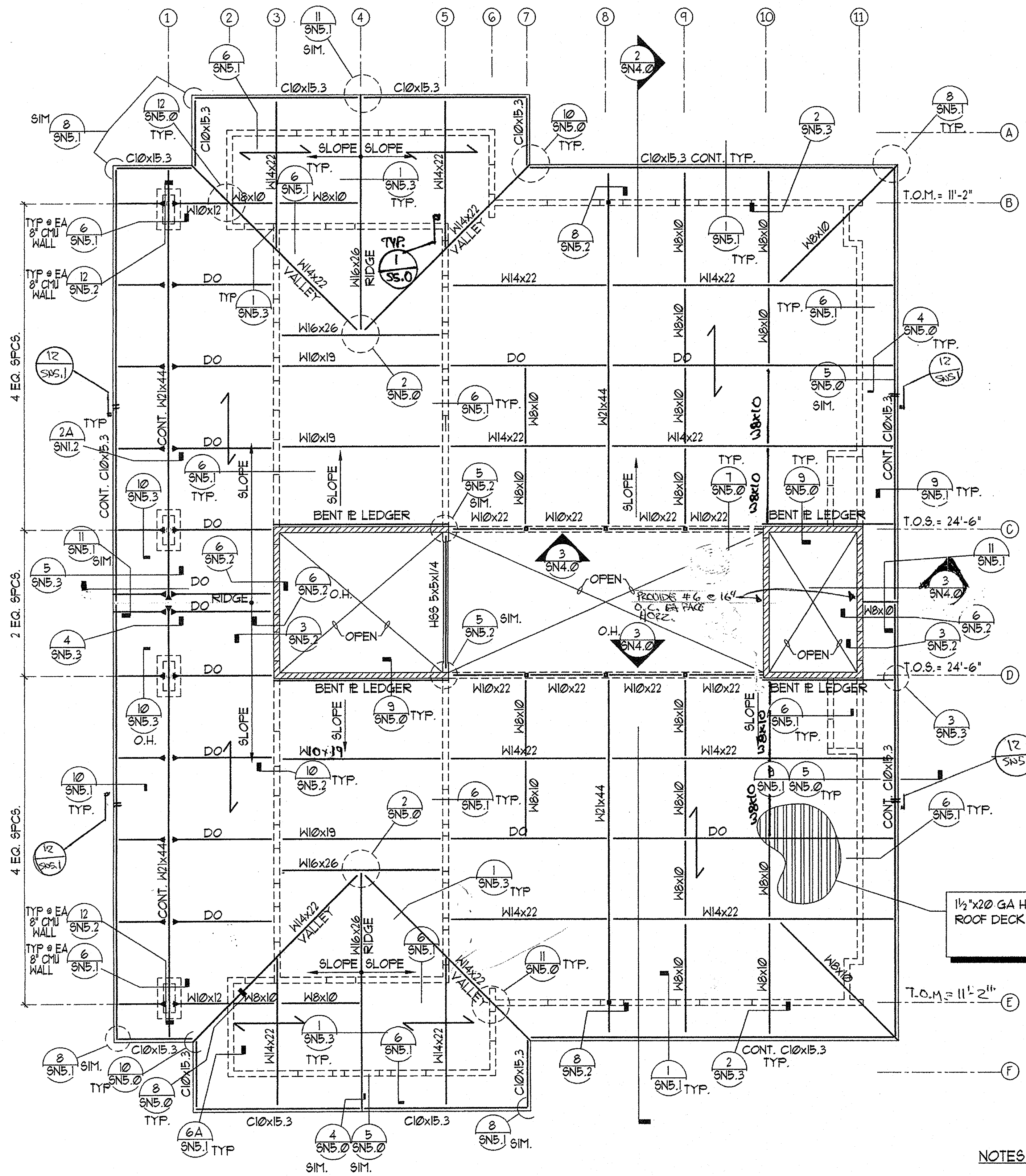
SHOWER/LOCKER BUILDING FOUNDATION PLAN

SN3.0



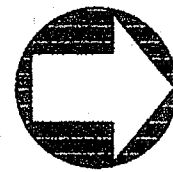
REGISTERED PROFESSIONAL ENGINEER
 RAYMOND F. FLORES
 10000
 800 544-8343
 STRUCTURAL
 STATE OF CALIFORNIA

FLC FLORES LUND CONSULTANTS
 PROFESSIONAL ENGINEERS
 7230 TRADE STREET, SUITE 500, SAN DIEGO, CALIFORNIA 92121
 (619) 544-0044 FAX (619) 544-0027



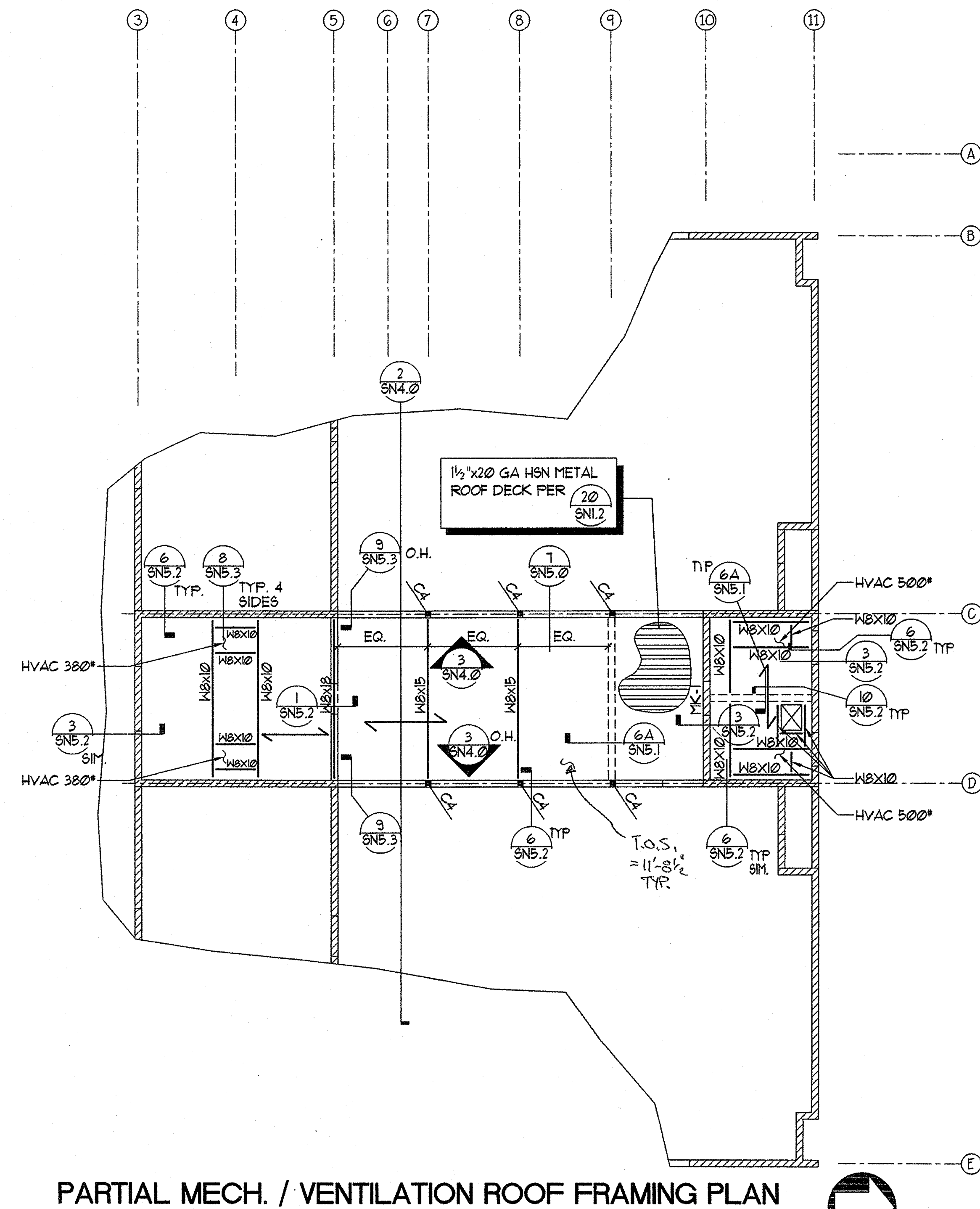
SHOWER/LOCKER BLDG. - MAIN ROOF FRAMING PLAN

1/8" = 1'-0"



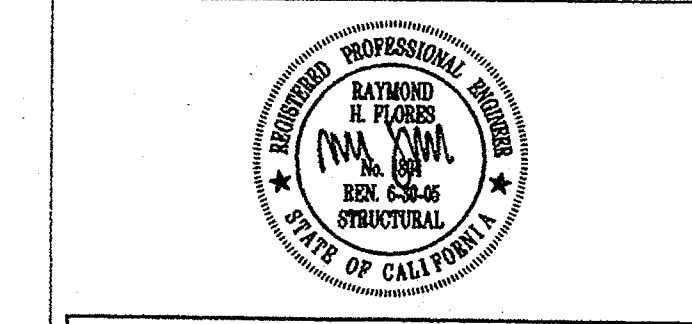
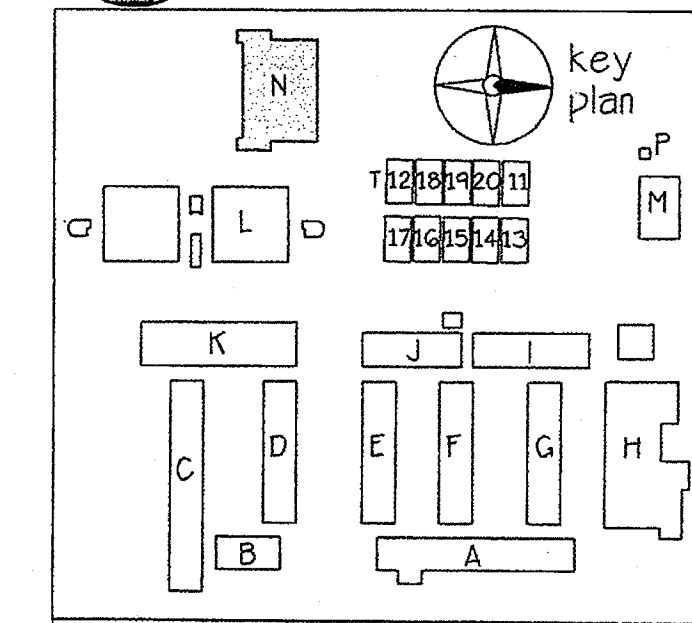
NOTES:

1. DENOTES DIRECTION OF THE STL. DECK FLUTES
2. FOR STL DECK SIZE & WELDING SEE (20) SNI.2
3. FOR TYP. STL. FRAMING SEE (2) SNI.2
4. DENOTES BM. MOMENT CONNECTION PER (2A) U.N.O.
5. DENOTES COL. SIZE PER SCHED (14) SNI.2
6. FOR REINFORCING OF ROOF DECK @ TYP. OPENINGS NOT SHOWN ON PLAN SEE (34T) SNI.2
7. CMU WALLS MUST BE CUT TO MATCH SLOPE OF ROOF



PARTIAL MECH. / VENTILATION ROOF FRAMING PLAN

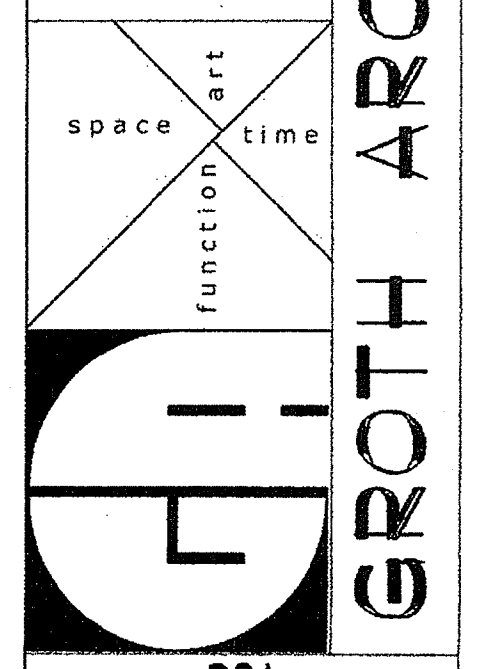
1/8" = 1'-0"



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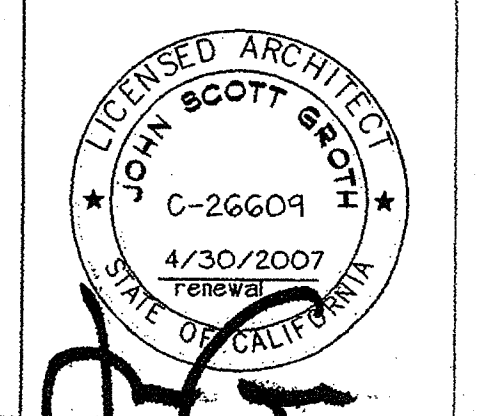
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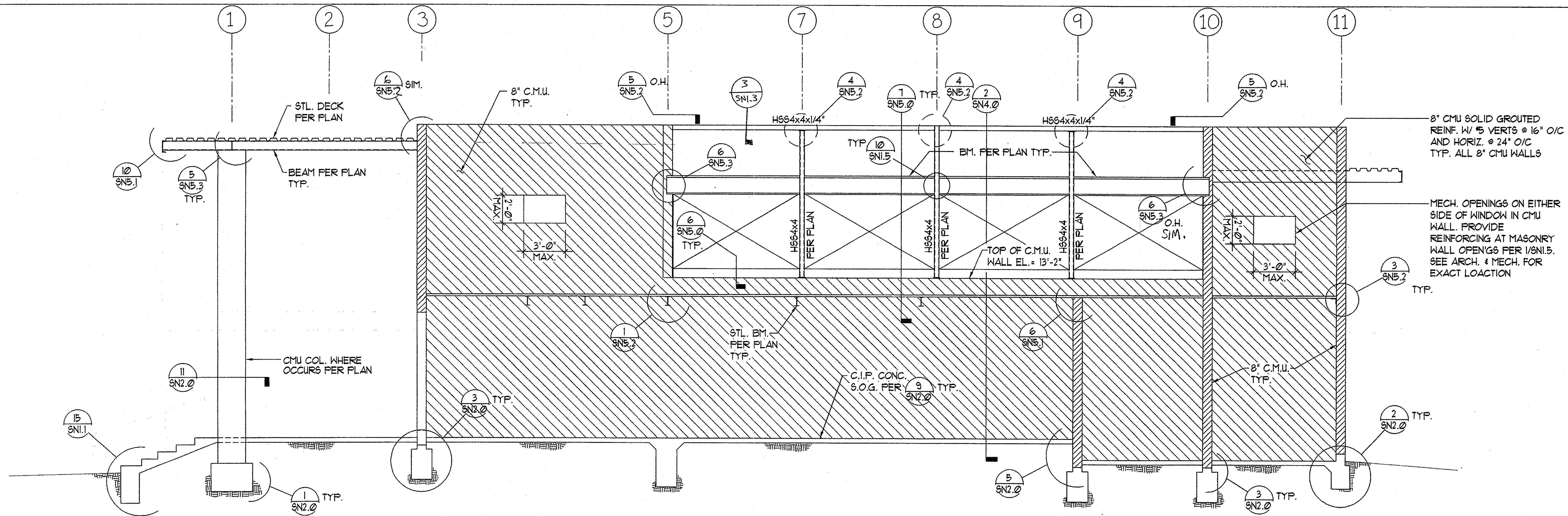
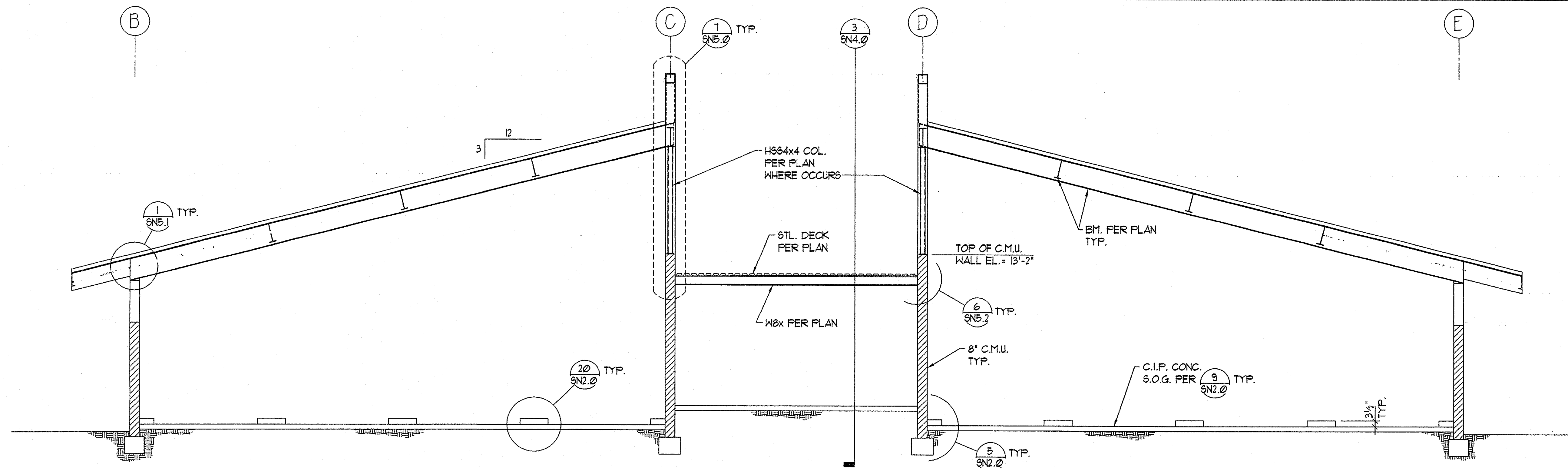
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SHOWER/LOCKER BUILDING ROOF FRAMING PLAN

SN3.1

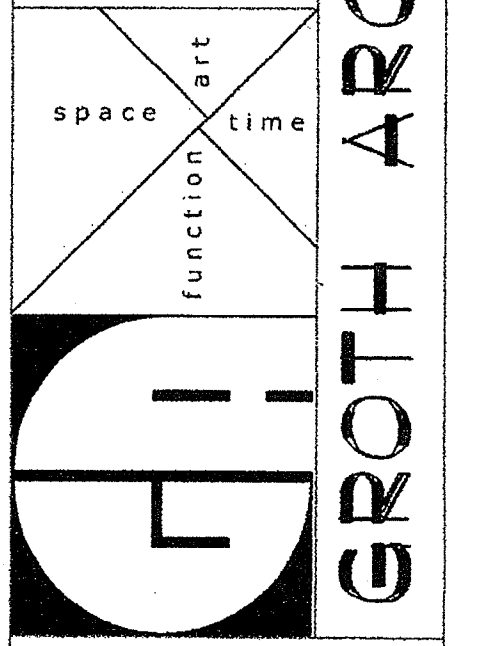
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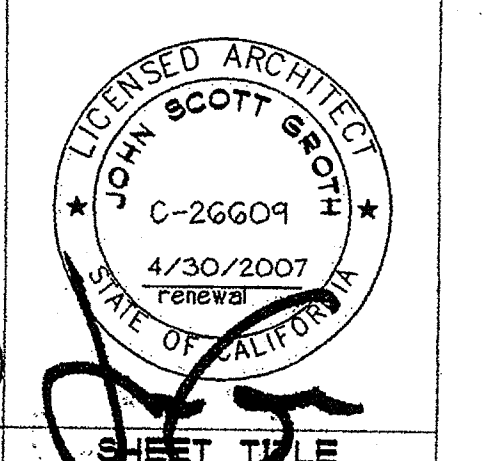
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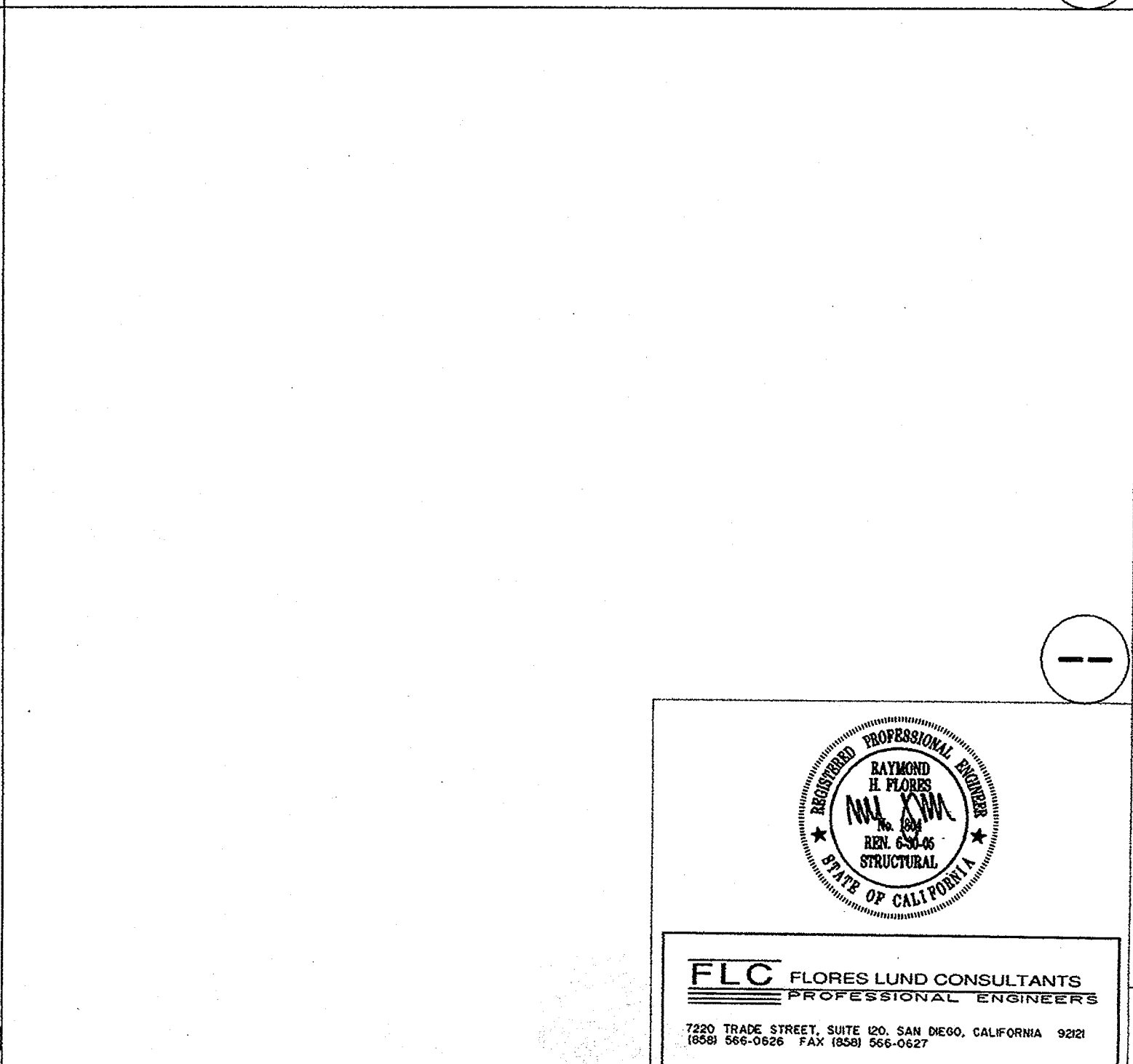
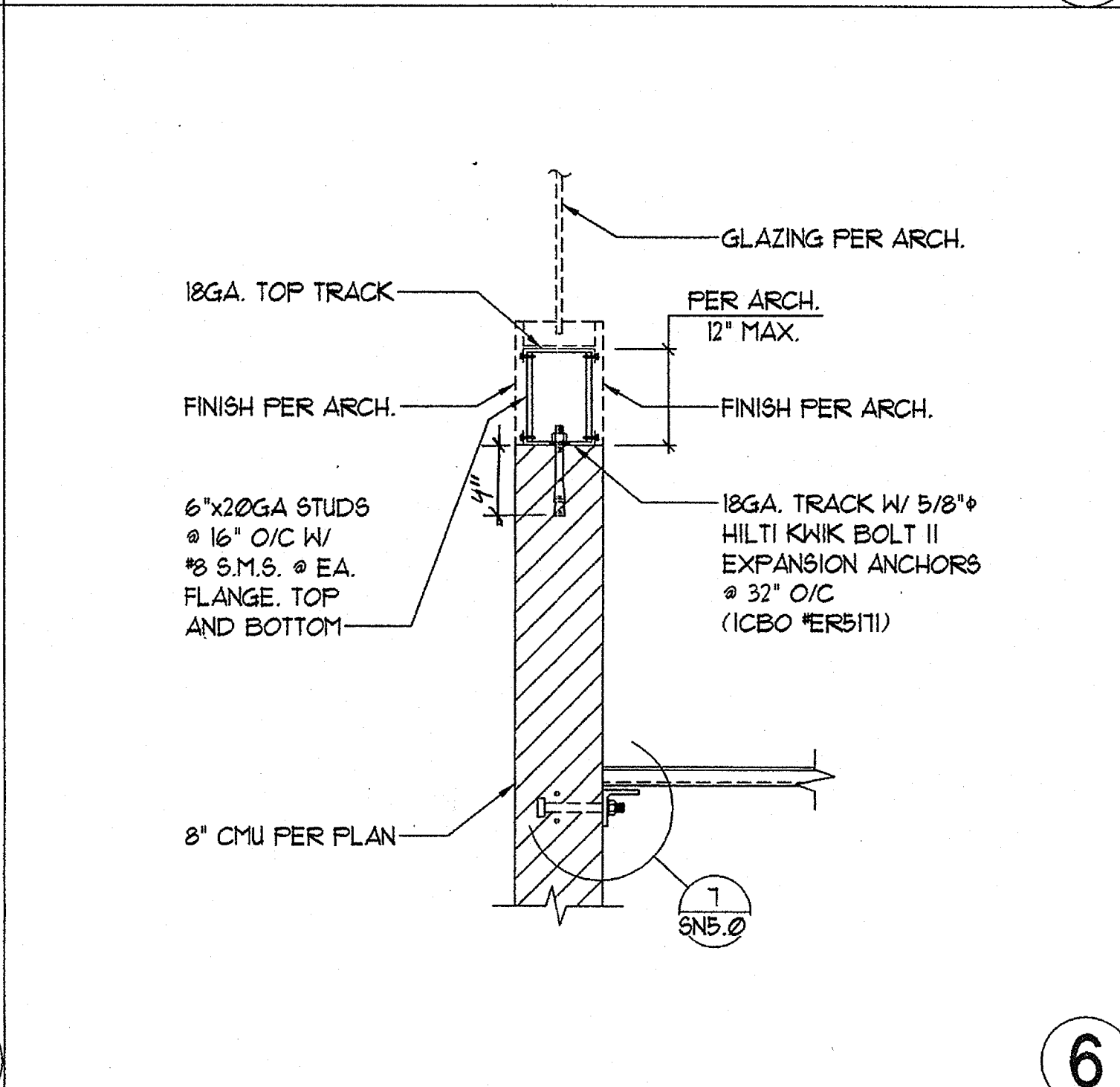
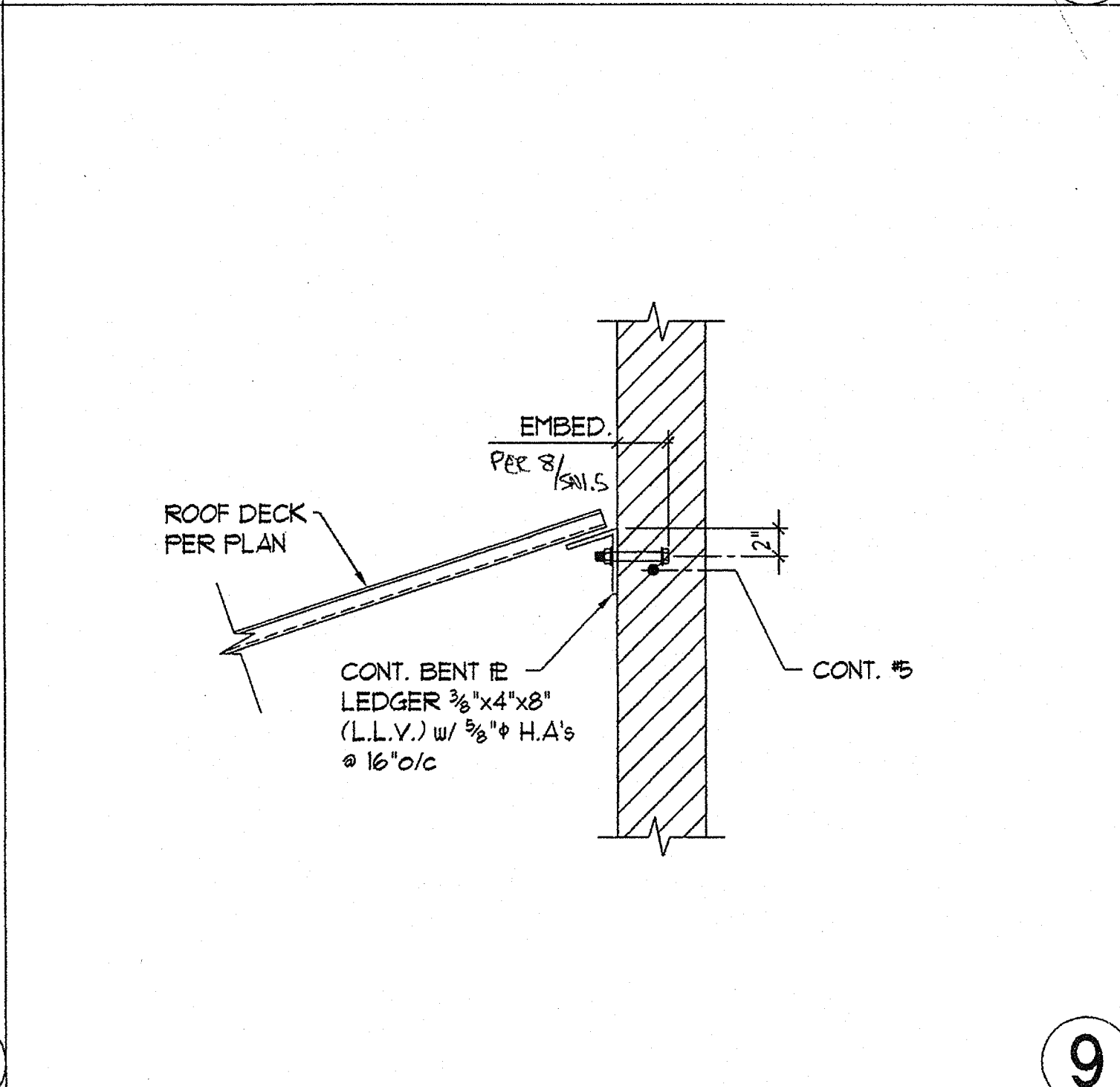
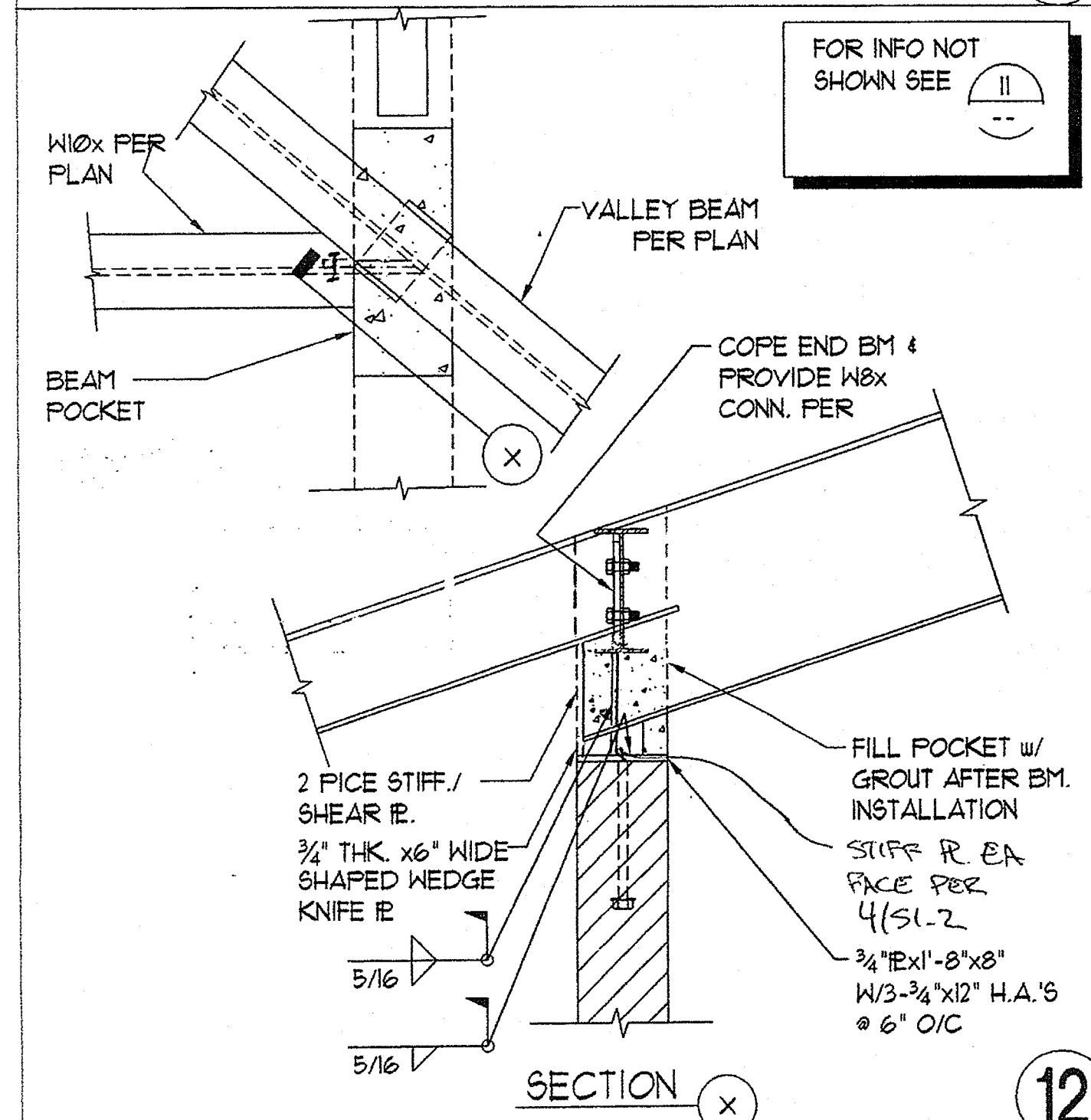
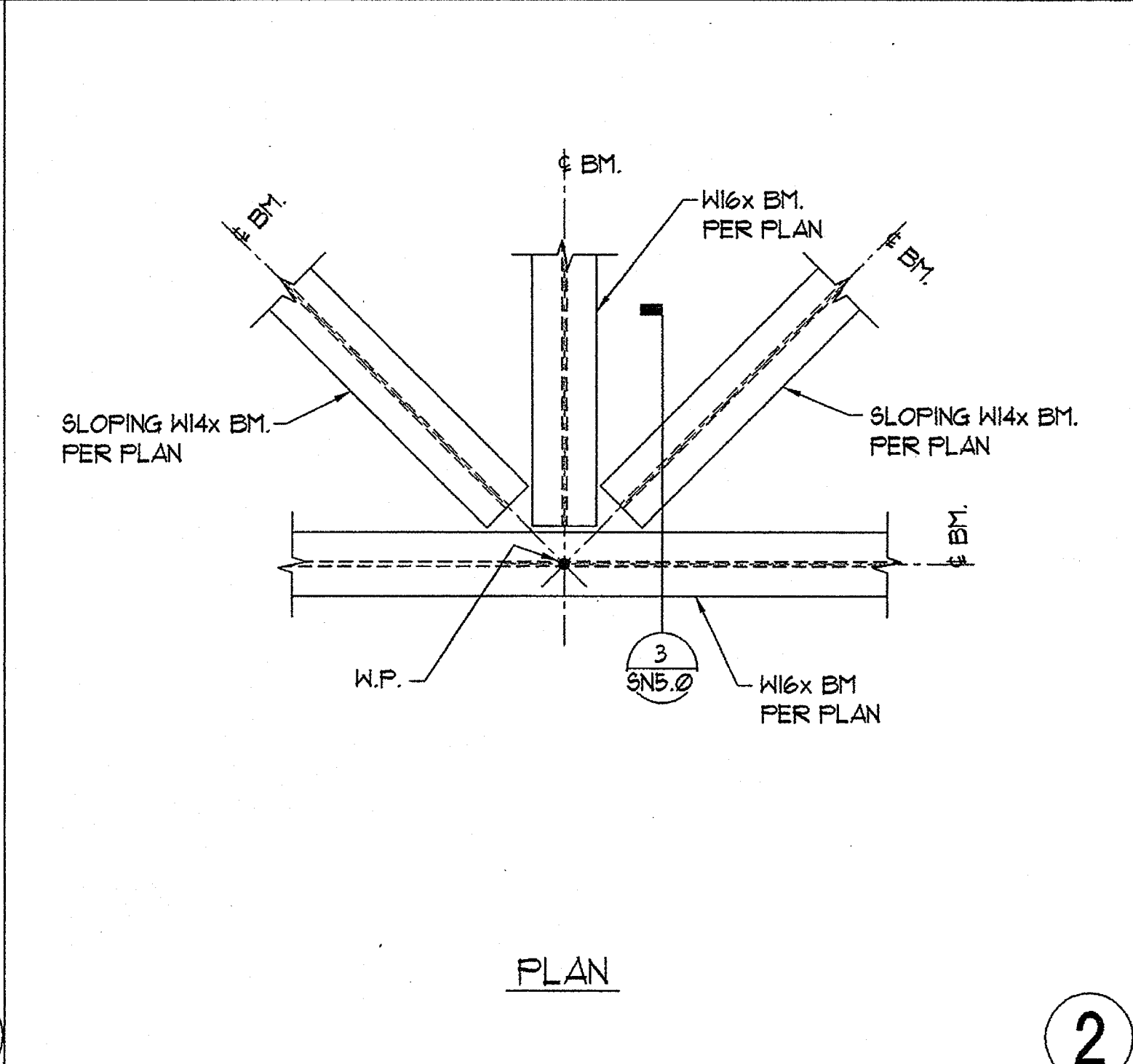
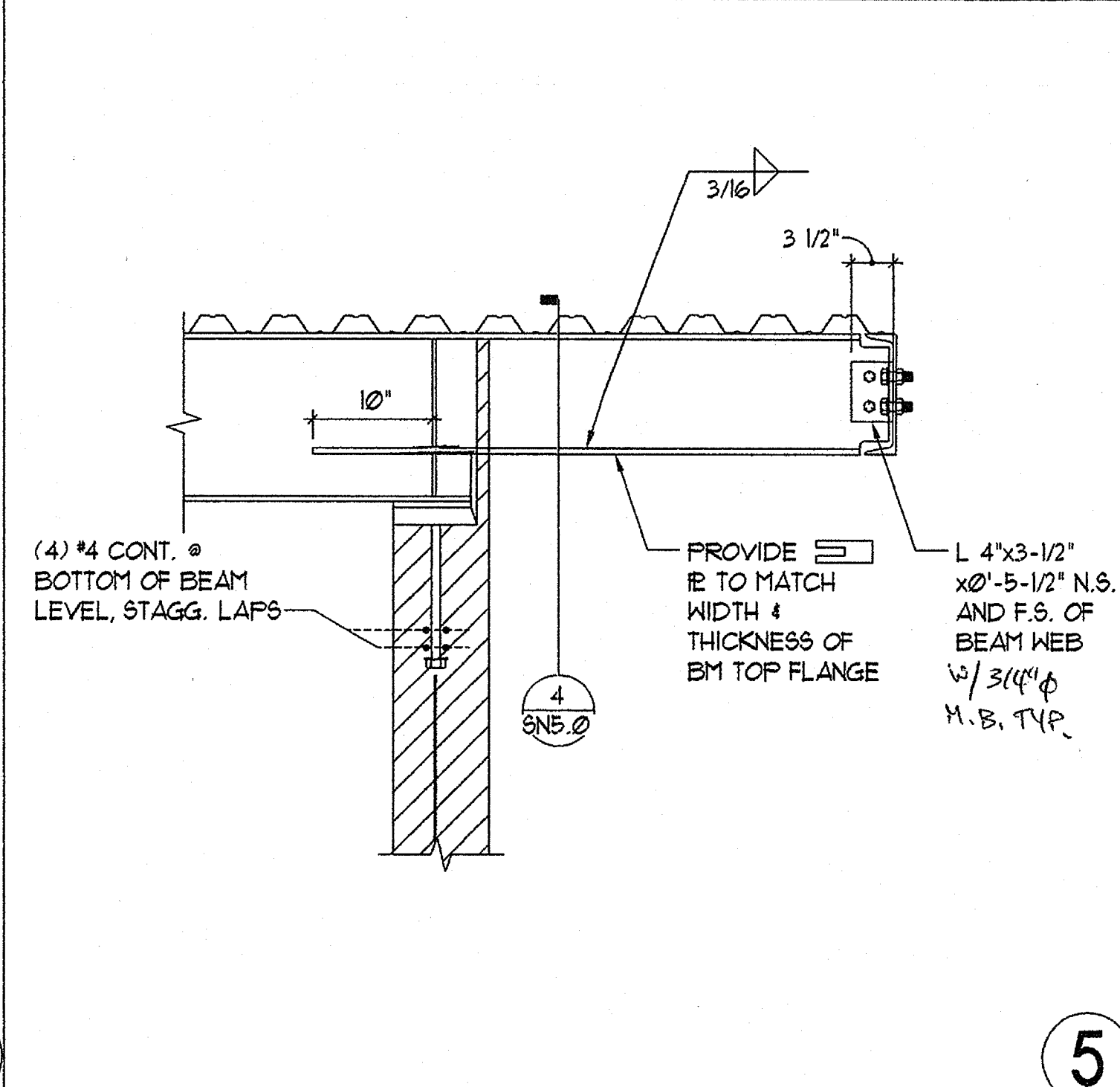
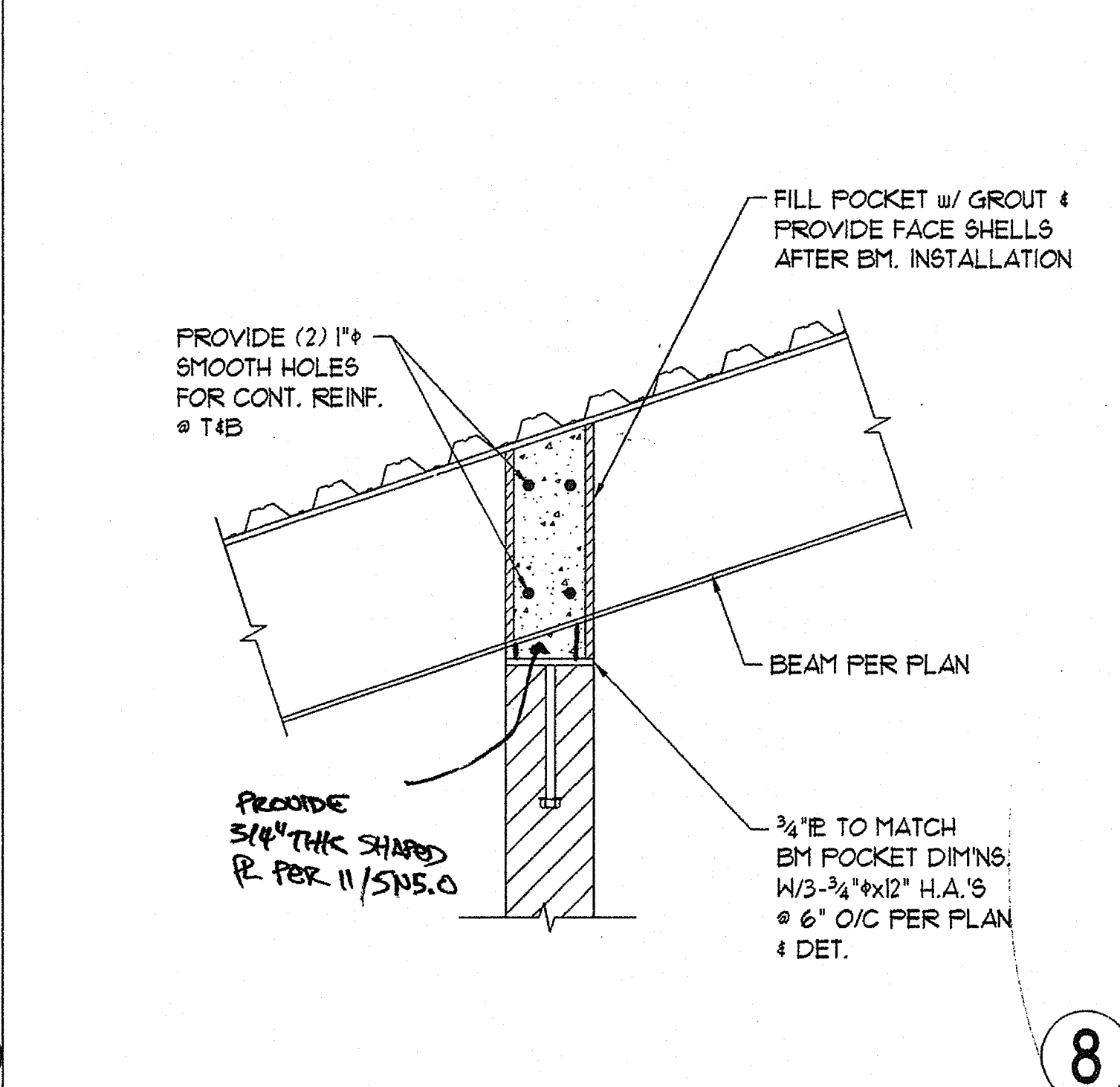
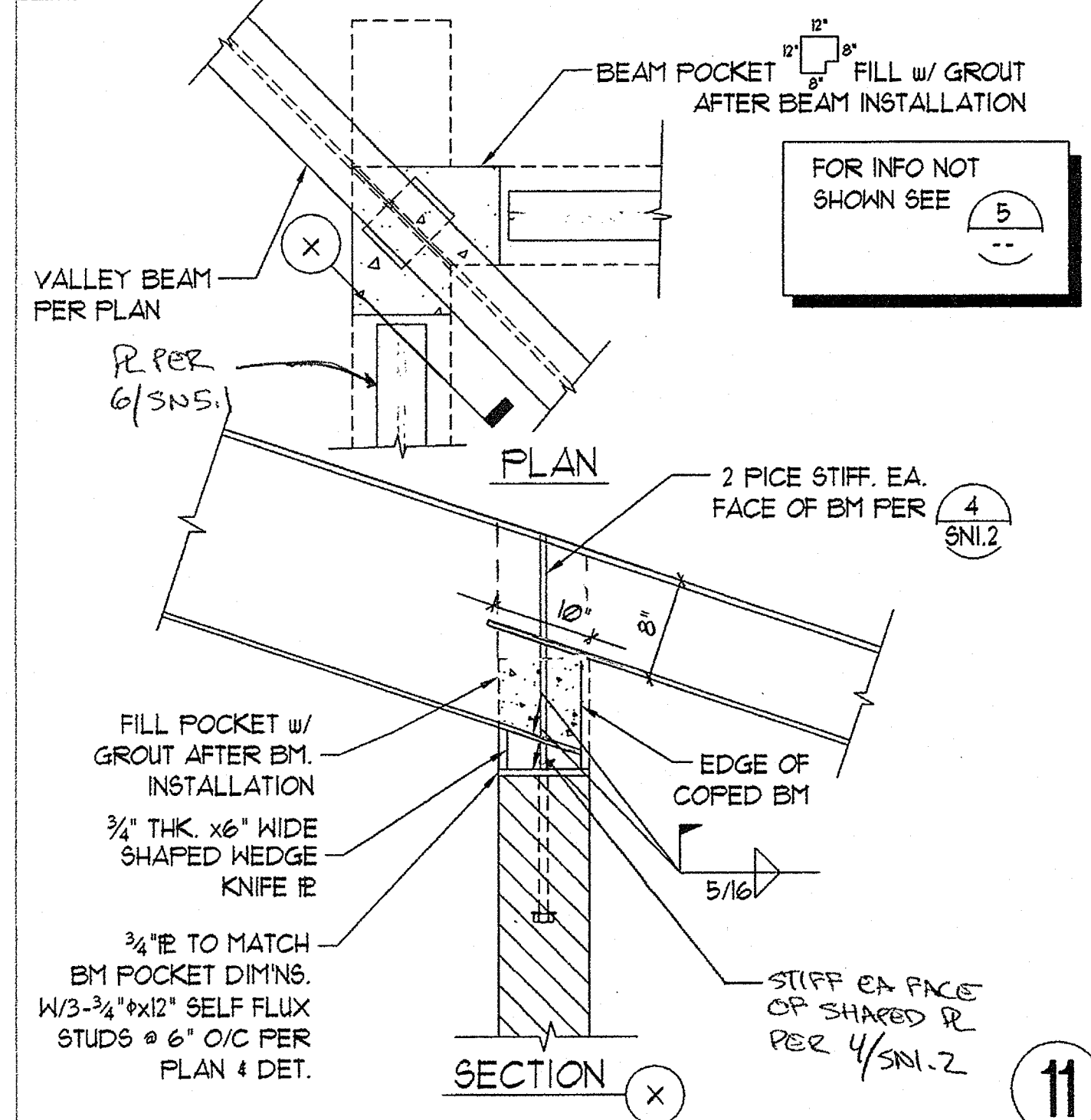
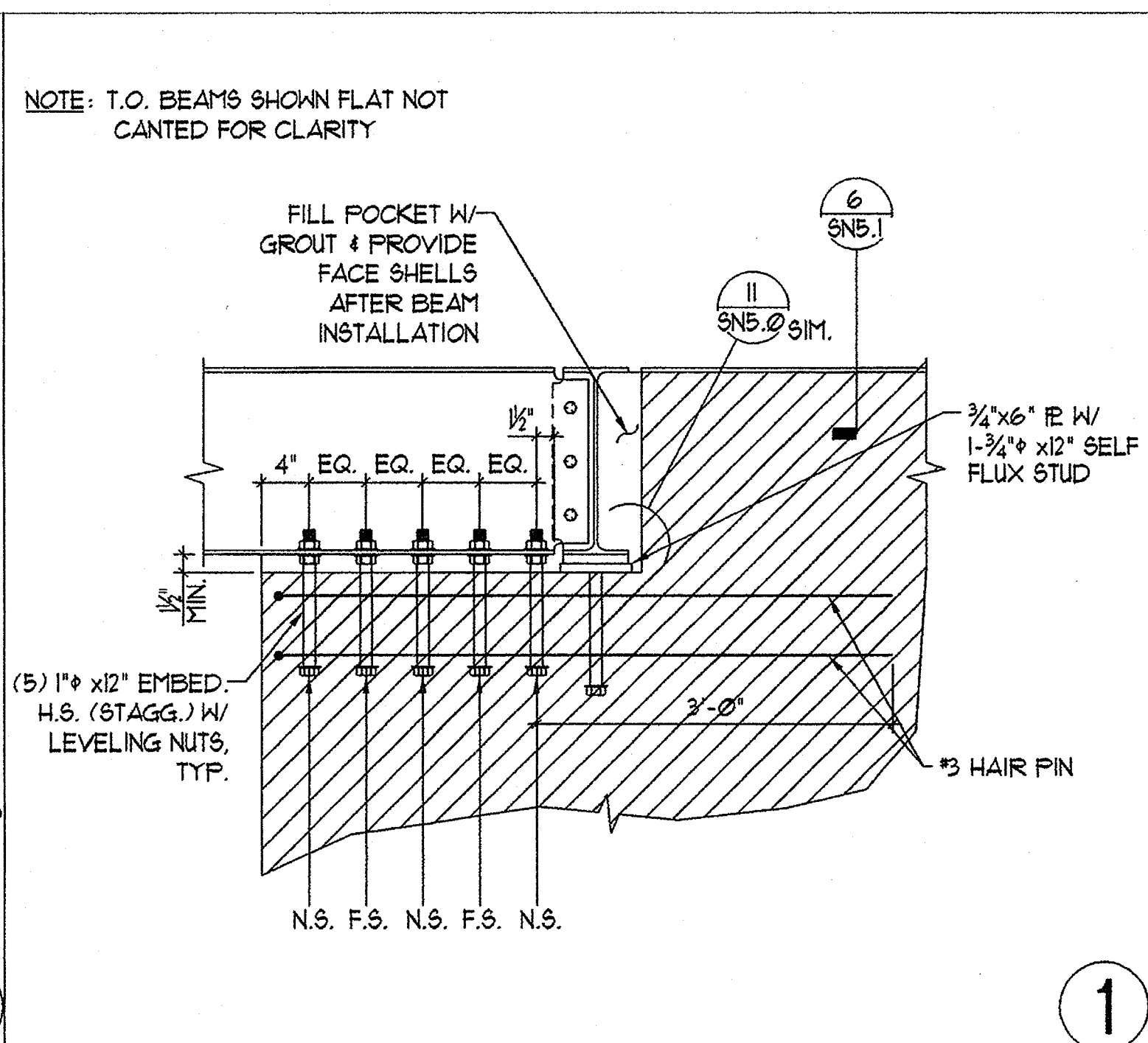
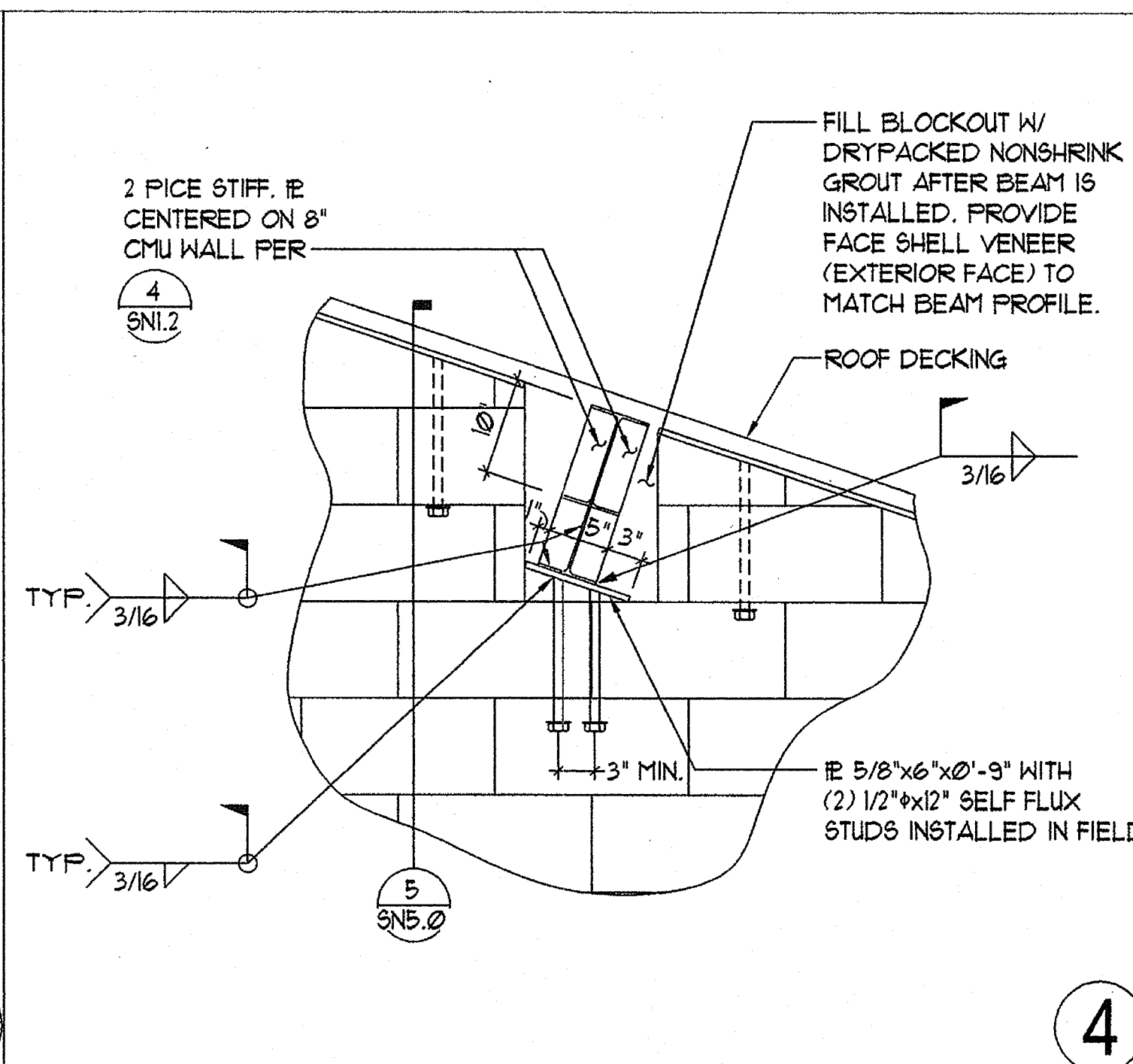
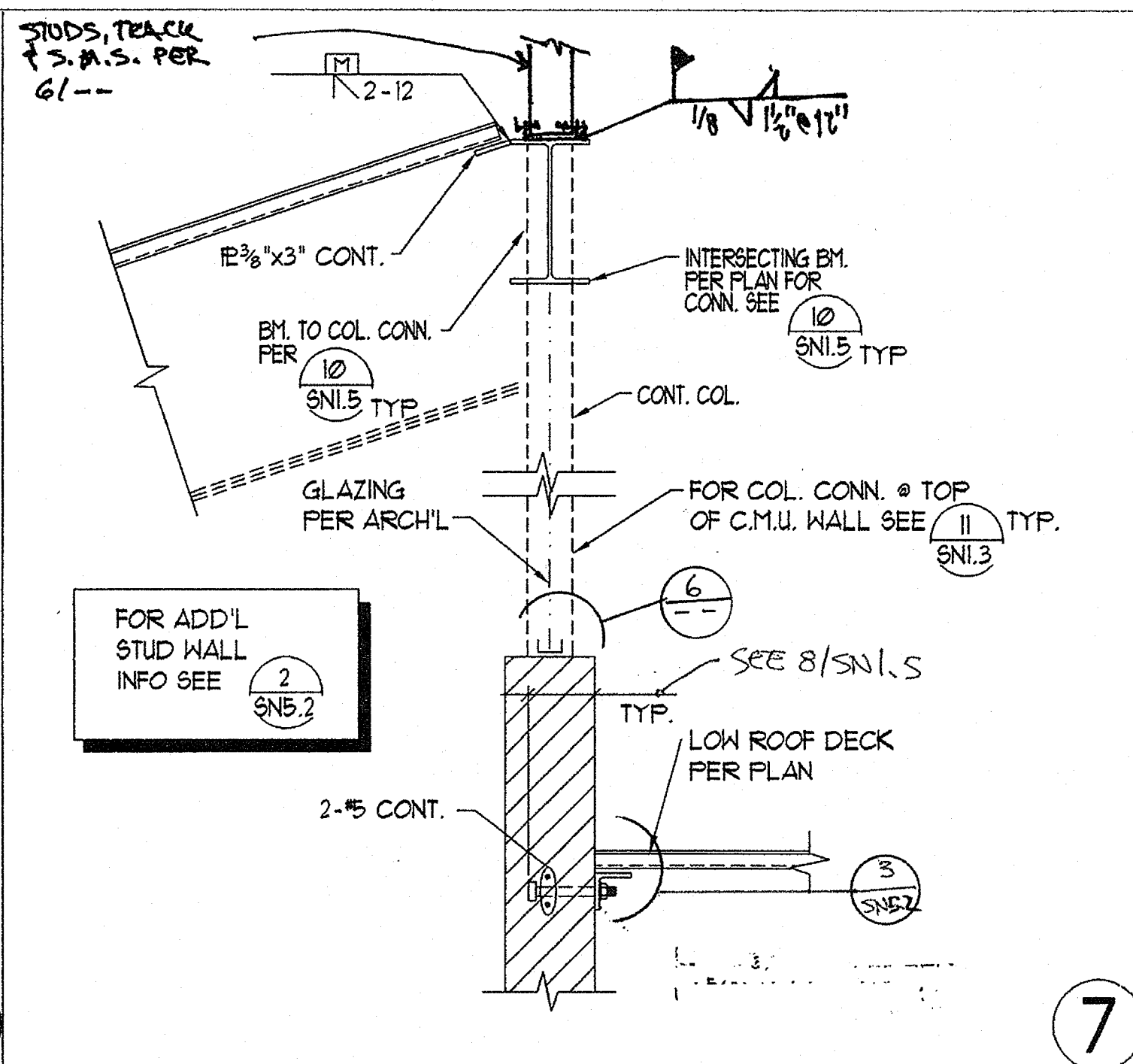
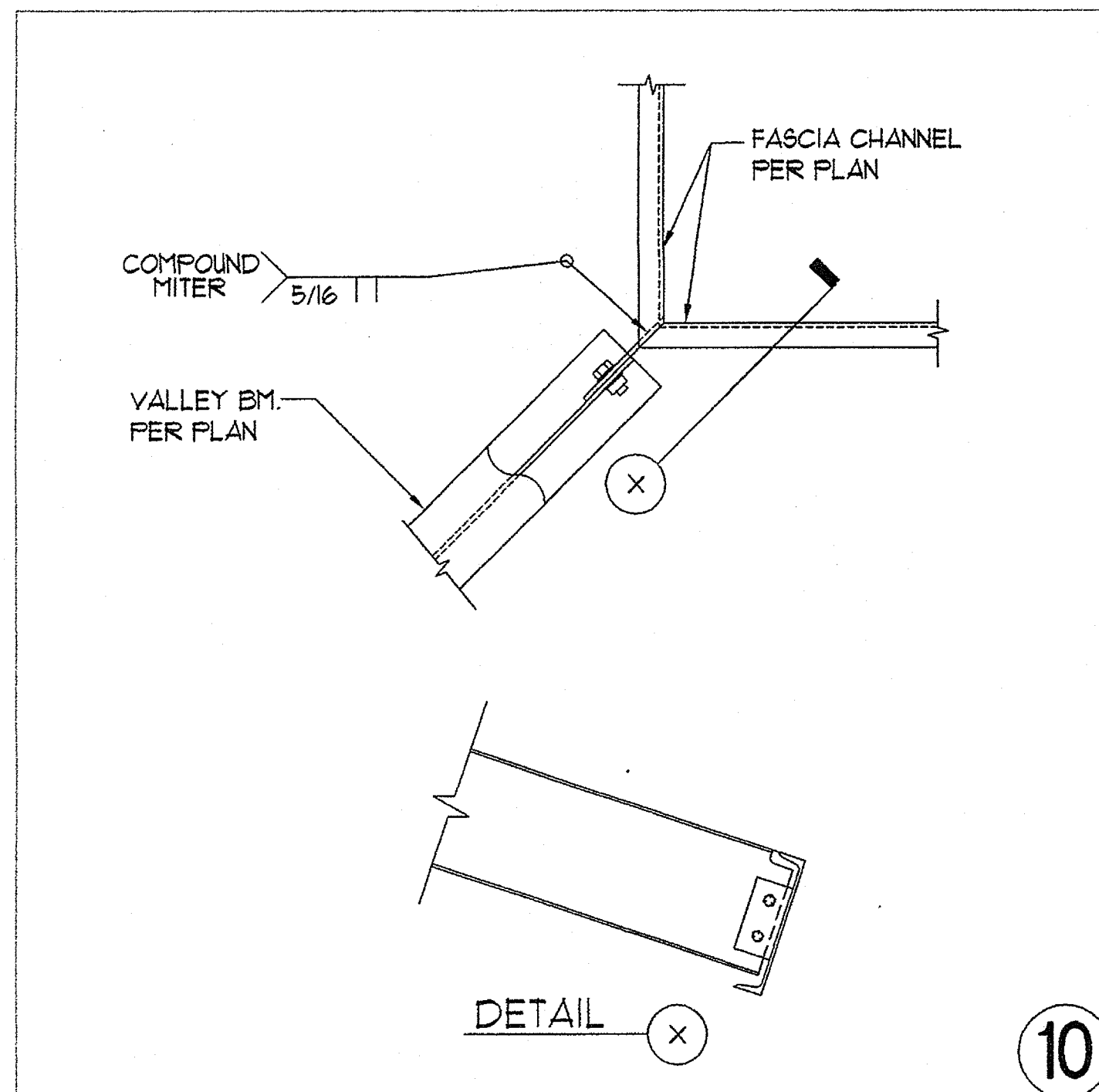


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 7220 TRAM STREET, SUITE 200, SAN DIEGO, CALIFORNIA 92121
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P. T. N. 73569-9

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REVISIONS

JEFFERSON MS NEW CONSTRUCTION

823 ACACIA STREET

OCEANSIDE, CA 92054

OCEANSIDE UNIFIED S.D.

3355 MISSION AVE. SUITE 234

OCEANSIDE, CALIFORNIA 92054

PHONE 760-754-8191

FAX 760-754-8291

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OFFICE OF REGULATION SERVICES

4-106494

AC FLS SS

DATE MAR 28 2005

LICENSED ARCHITECT

JOHN SCOTT GROTH

C-26609

4/30/2007

STATE OF CALIFORNIA

CHEST TITLE

ROOF DETAILS

SN5.0

FLC FLORES LUND CONSULTANTS

PROFESSIONAL ENGINEERS

7220 TRADE STREET, SUITE 500, SAN DIEGO, CALIFORNIA 92121

TEL: 619-444-0422 FAX: 619-444-0427

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All drawings, notes, and specifications shall be read in conjunction with the project contract documents. The contractor shall be responsible for obtaining all necessary permits and for compliance with all applicable laws, codes, and regulations. The architect shall not be responsible for construction methods or for the safety of the construction. The architect shall not be responsible for the construction of the project.

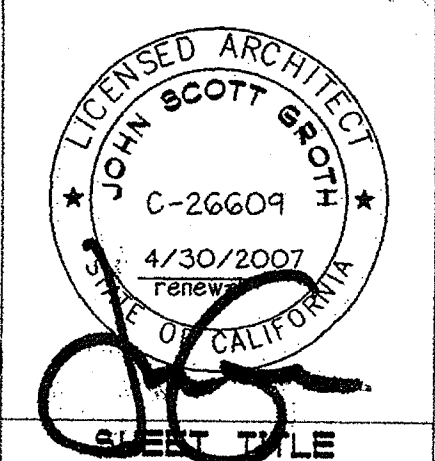
OUSD NO. 758-000
PROJECT NOS. 025
P. T. N. 73569-9
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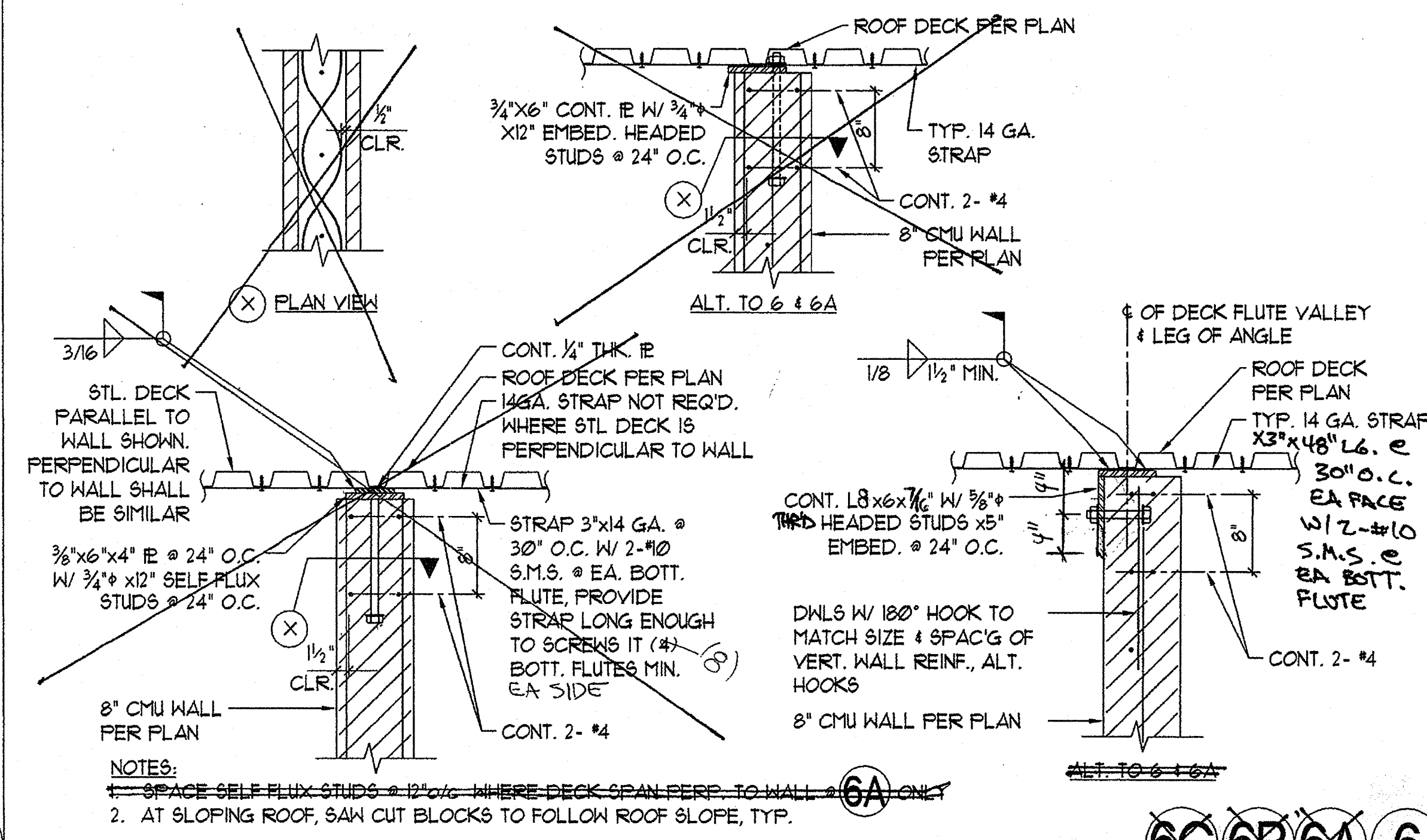
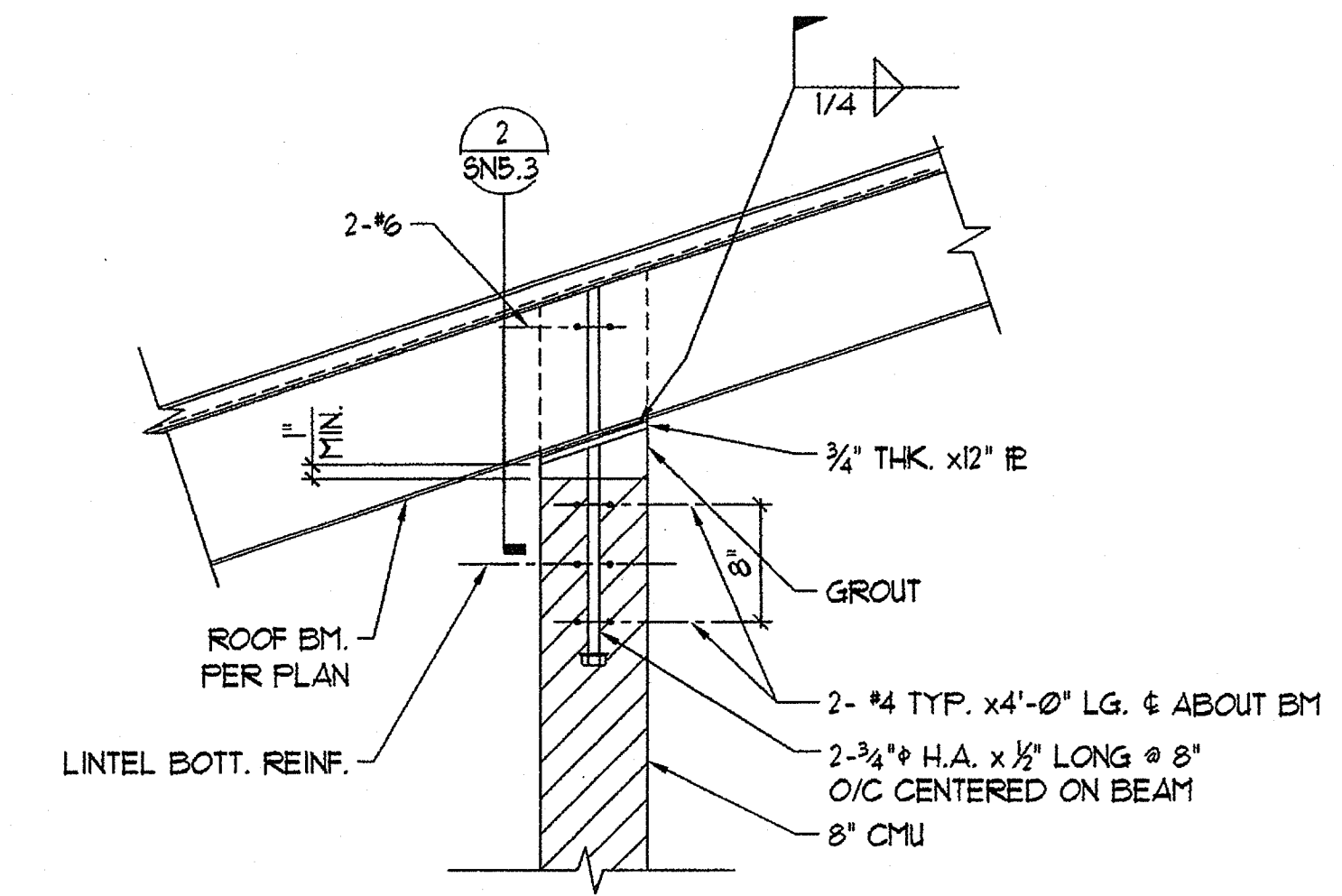
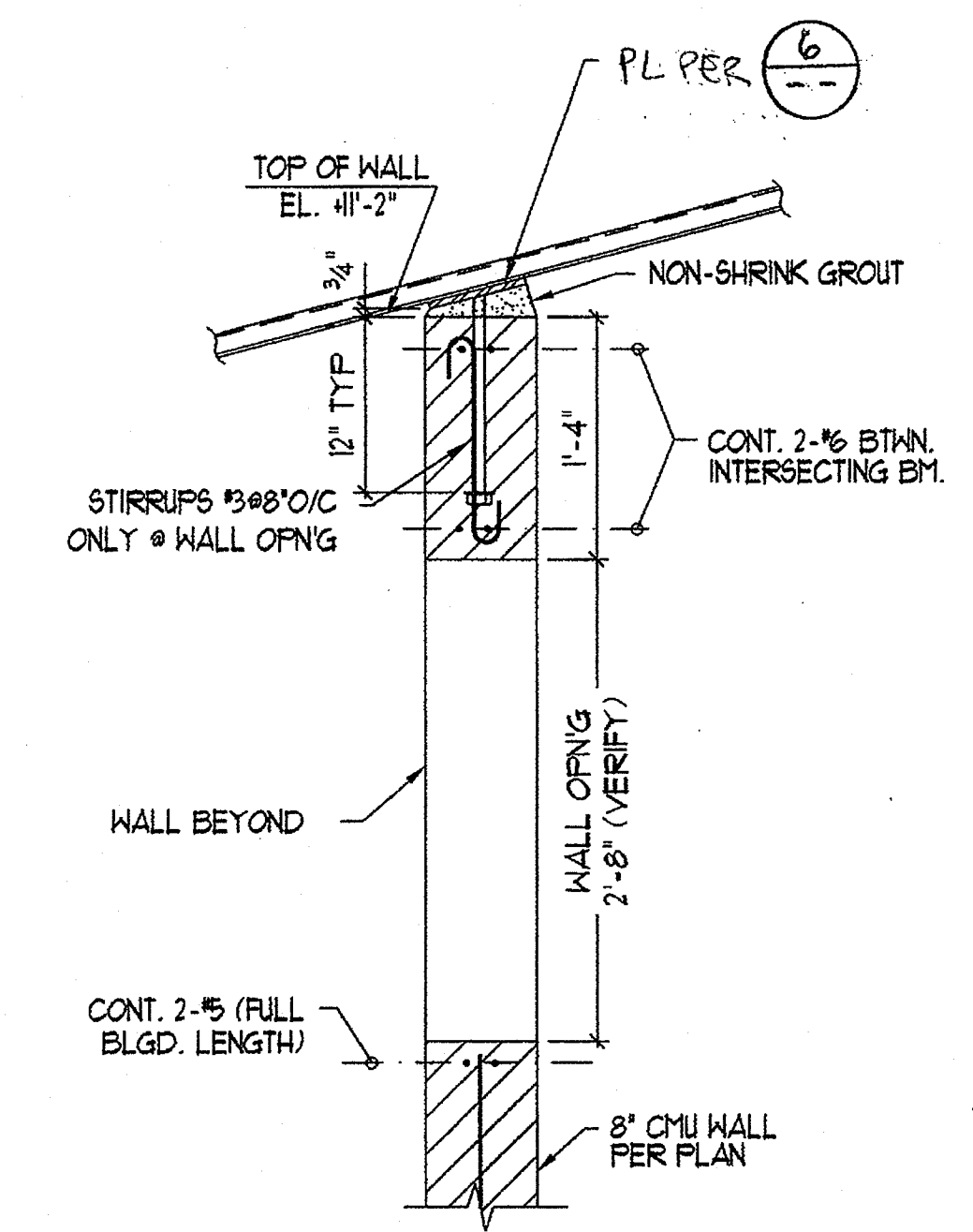
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FAX 760-754-8291

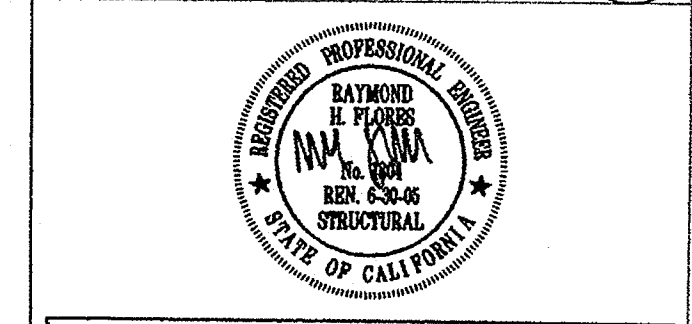
SUITE 234
92054

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OCEANSIDE, CALIFORNIA

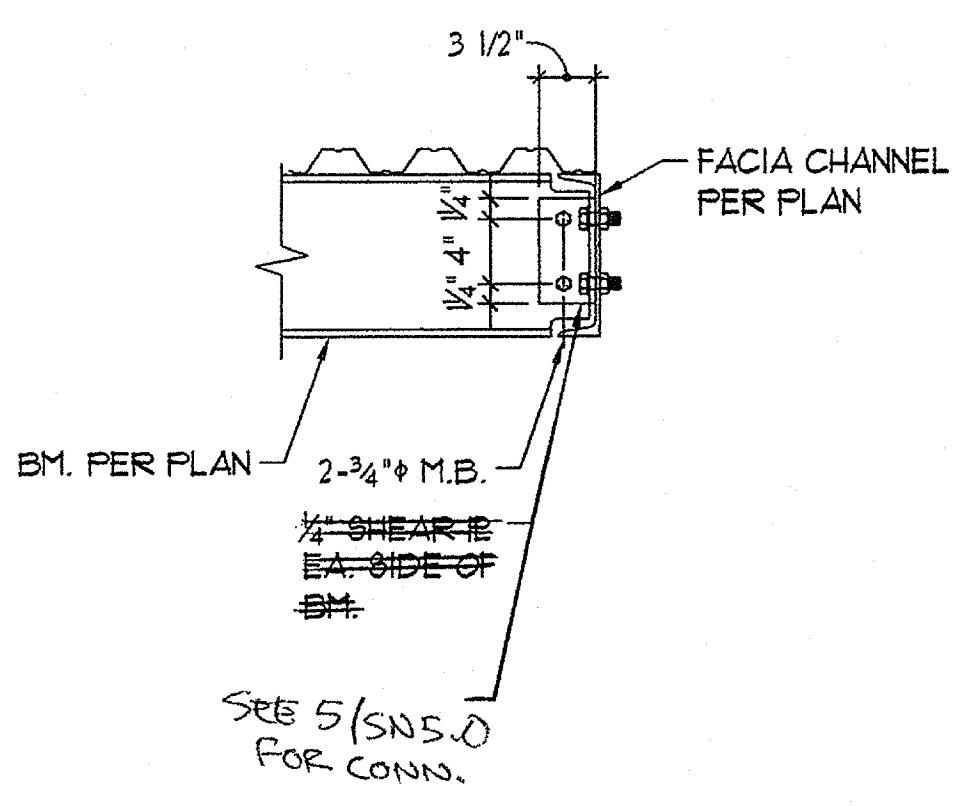


NOTES:
1. SPACE SELF-FLUX STUDS @ 12" O.C. WHERE DECK SPAN PERP. TO WALL @ 6A ONLY
2. AT SLOPING ROOF, SAW CUT BLOCKS TO FOLLOW ROOF SLOPE, TYP.

6C 6B 6A 6



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PROFESSIONAL ENGINEERS
7250 TRADE STREET, SUITE 500, SAN DIEGO, CALIFORNIA 92121
TEL: 619-594-0032 FAX: 619-594-0027

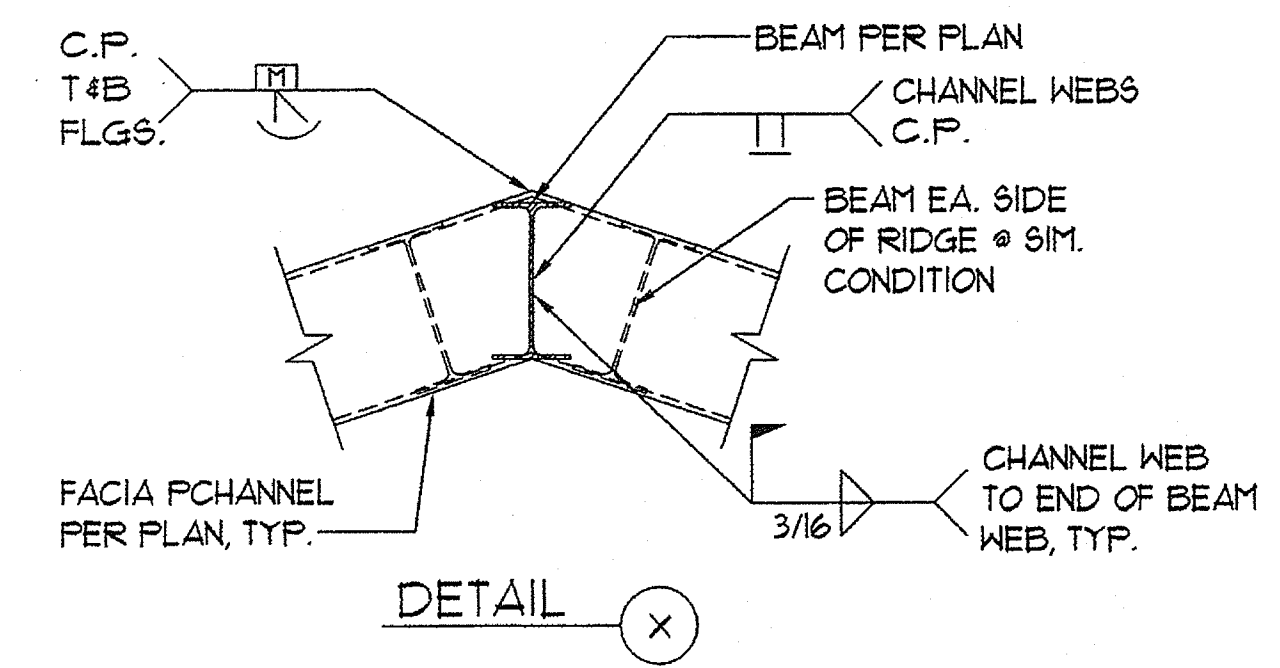


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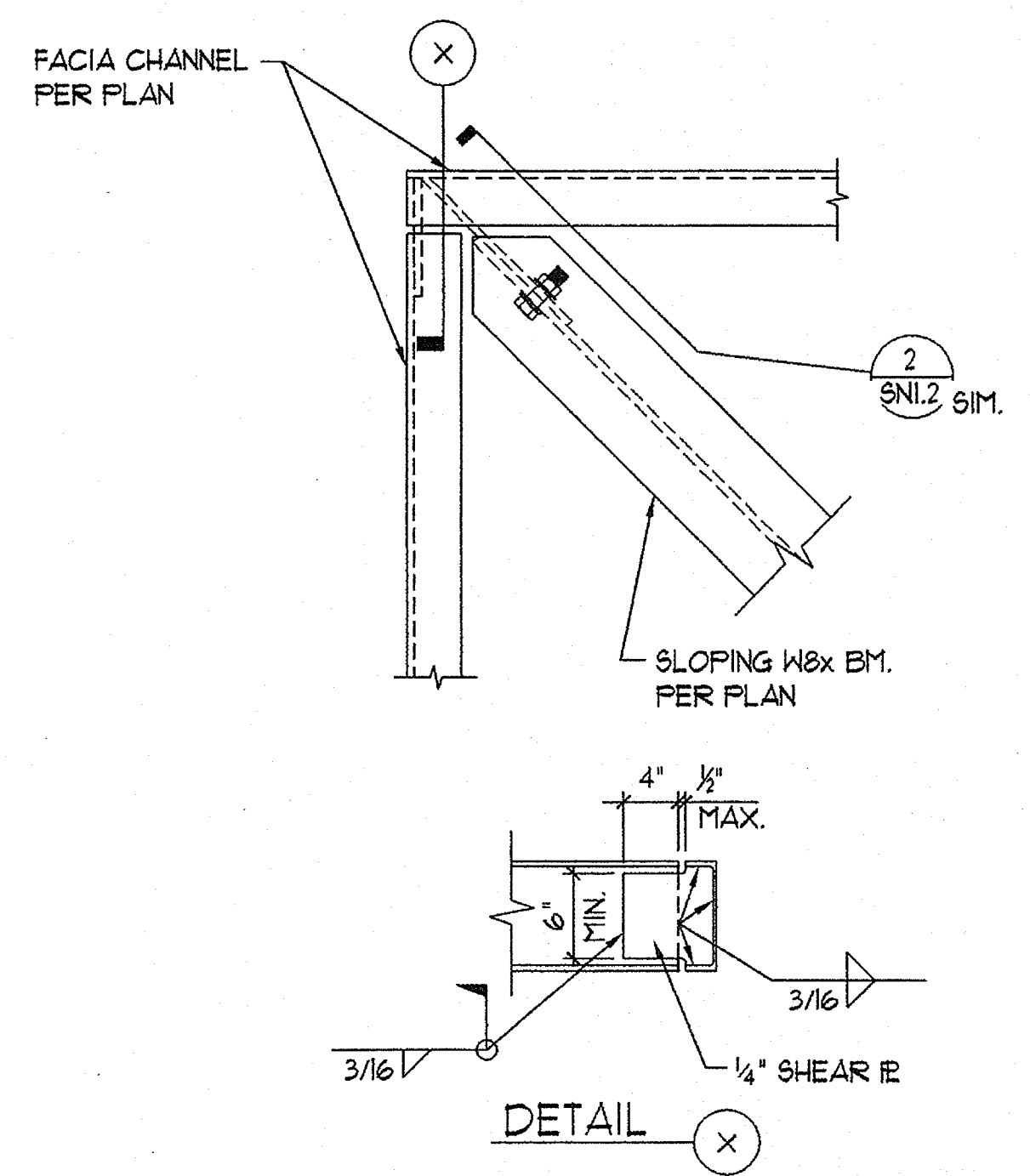


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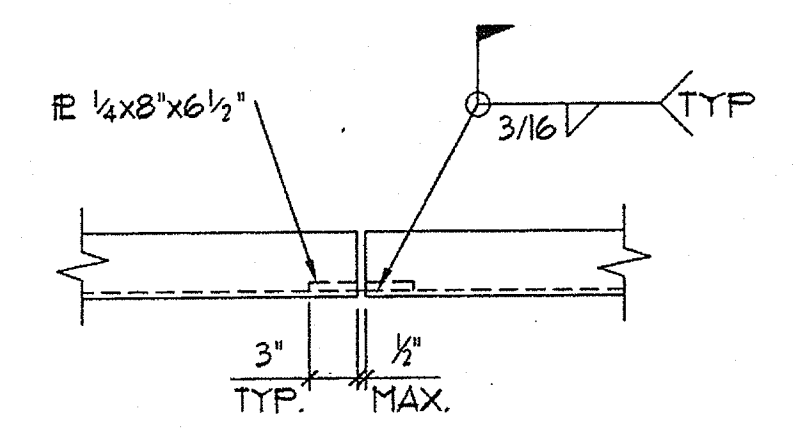
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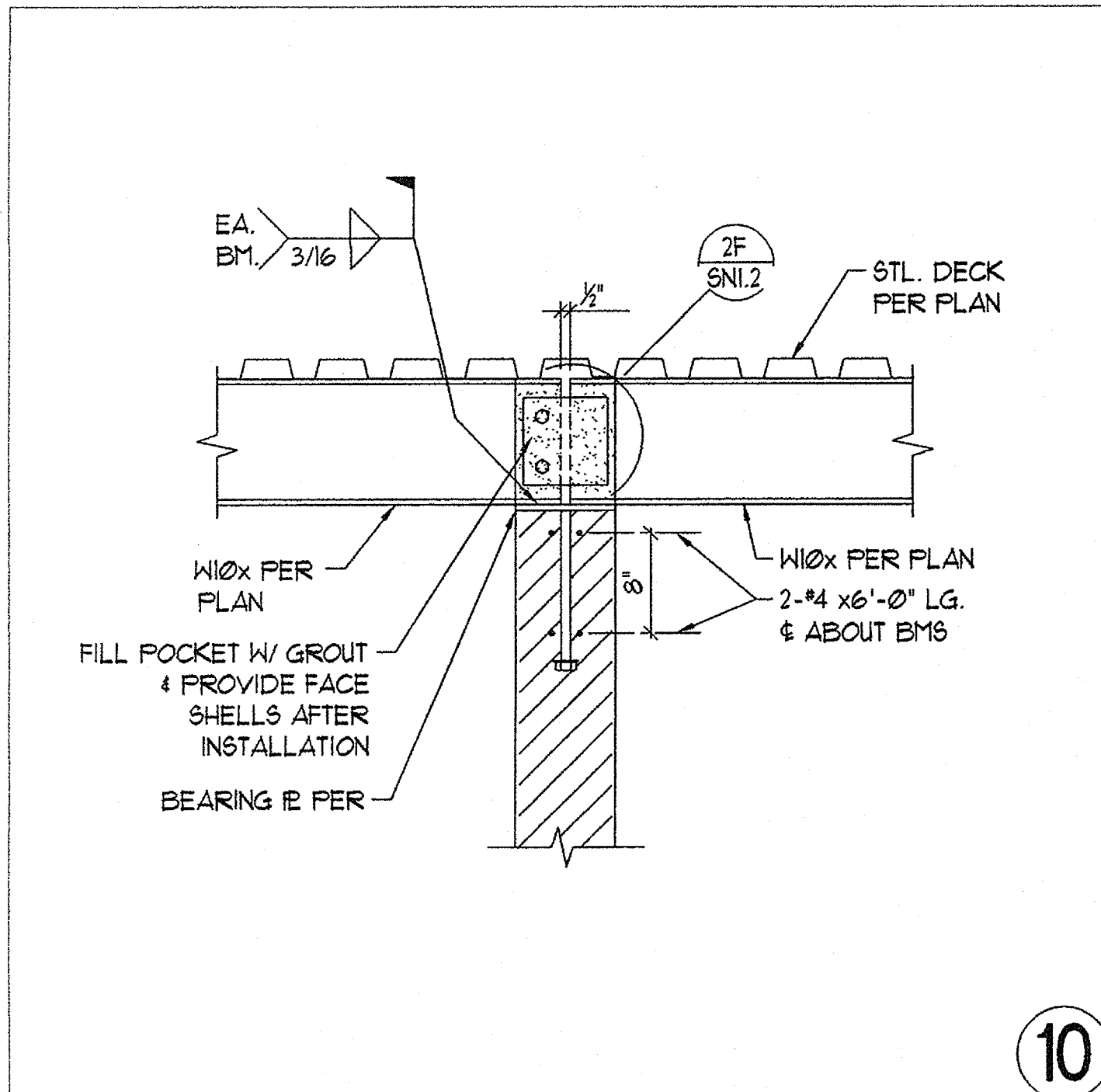
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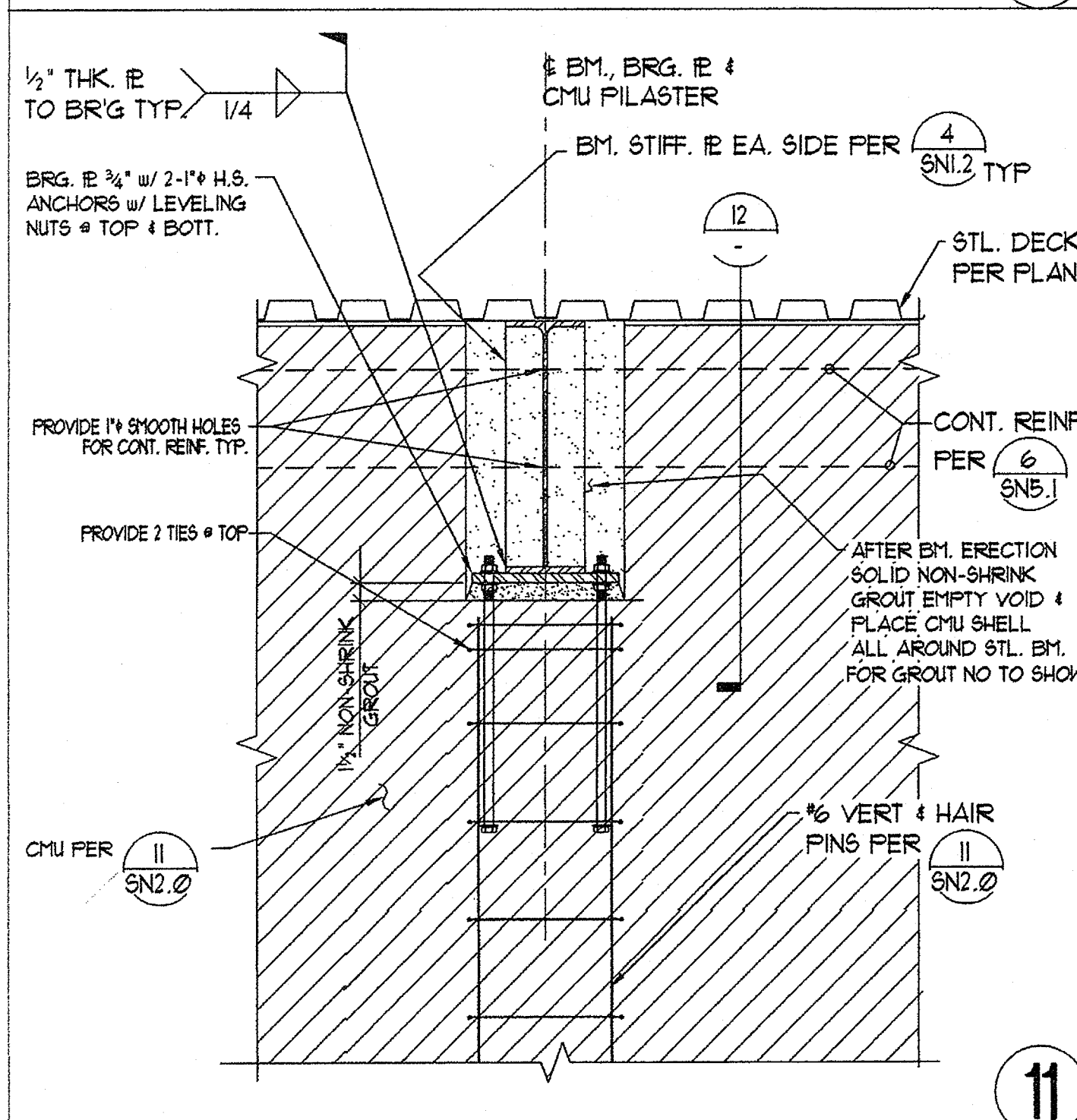


TYPICAL C10x FASCIA SPLICE

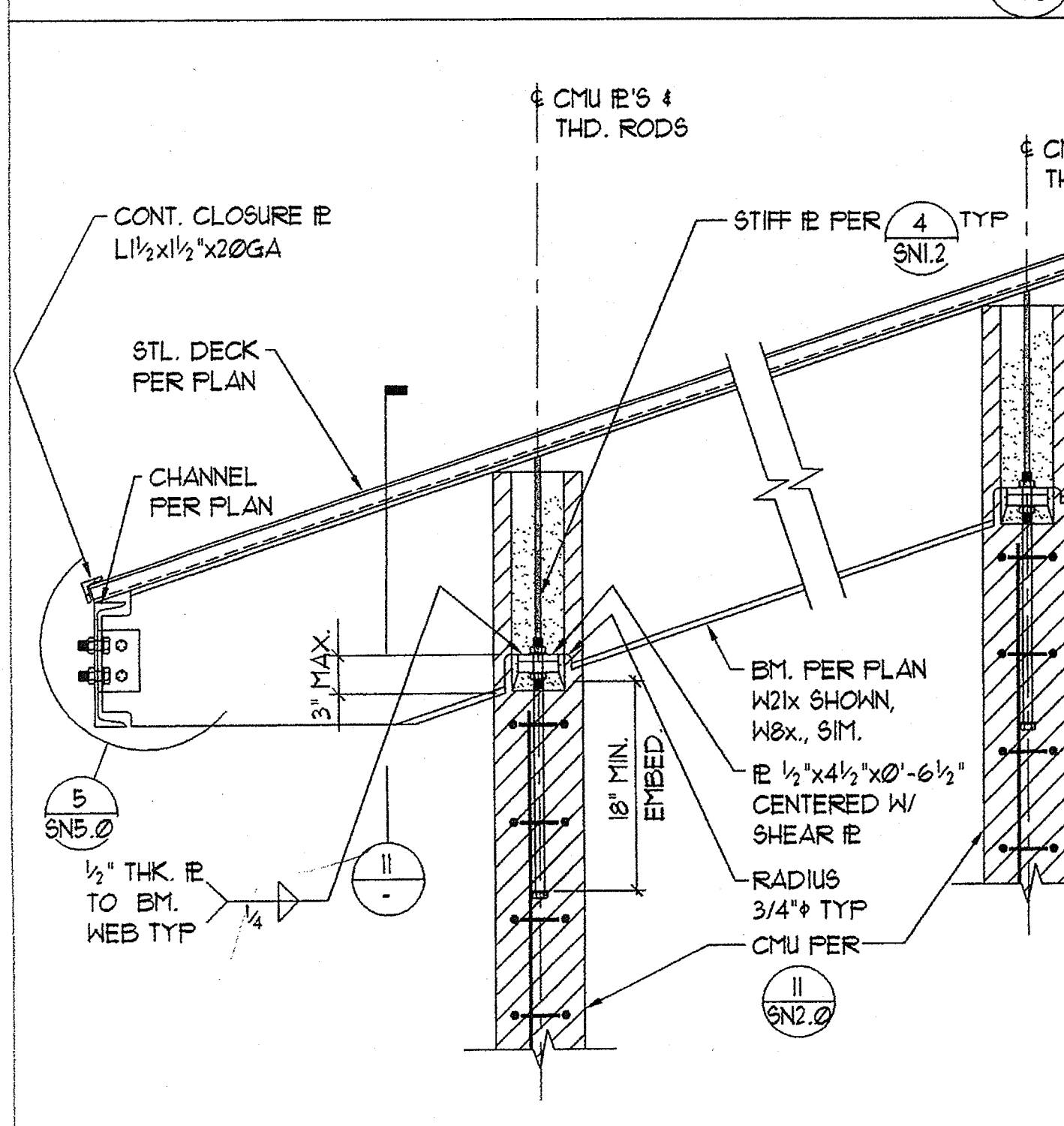
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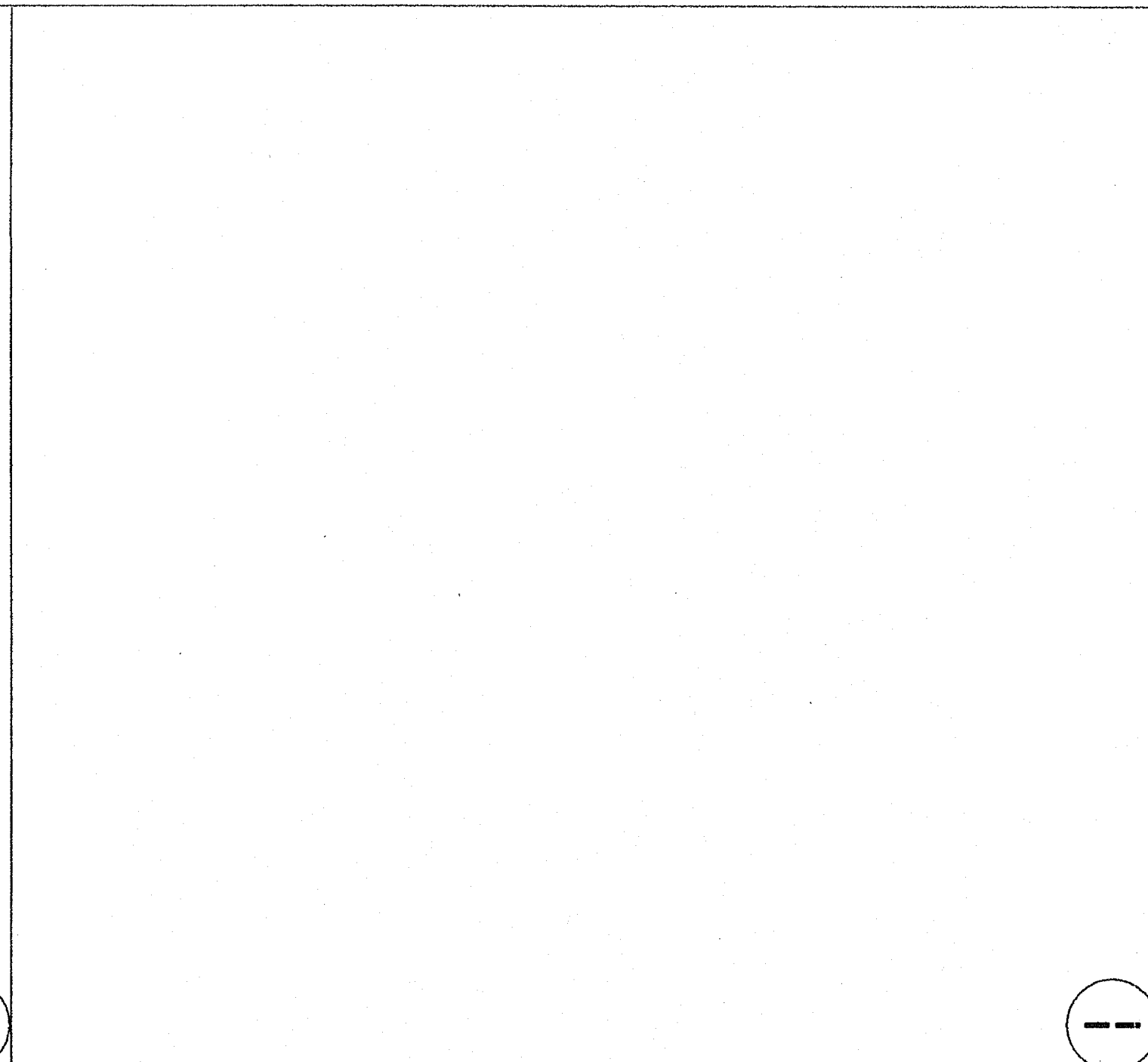
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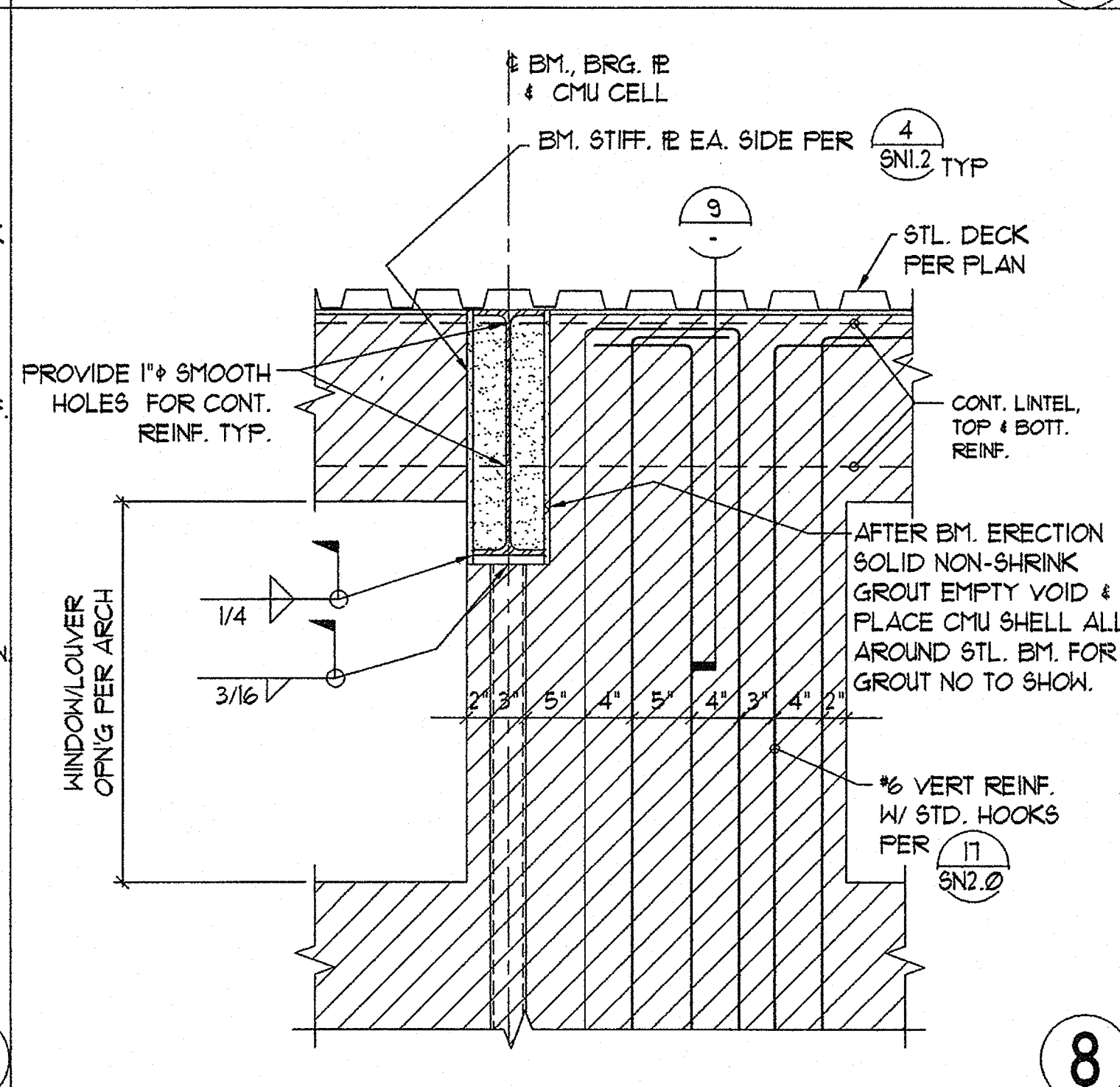
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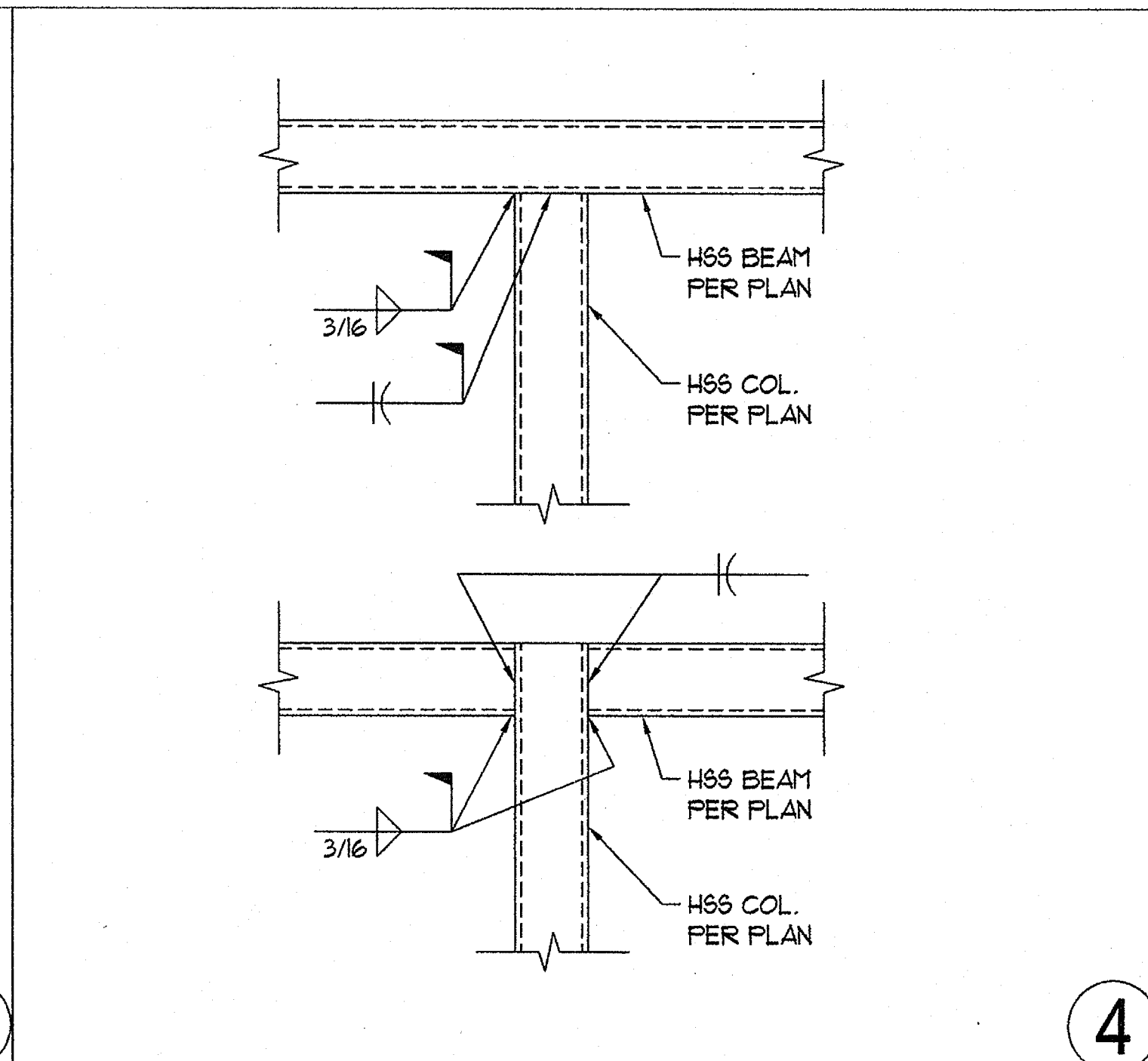
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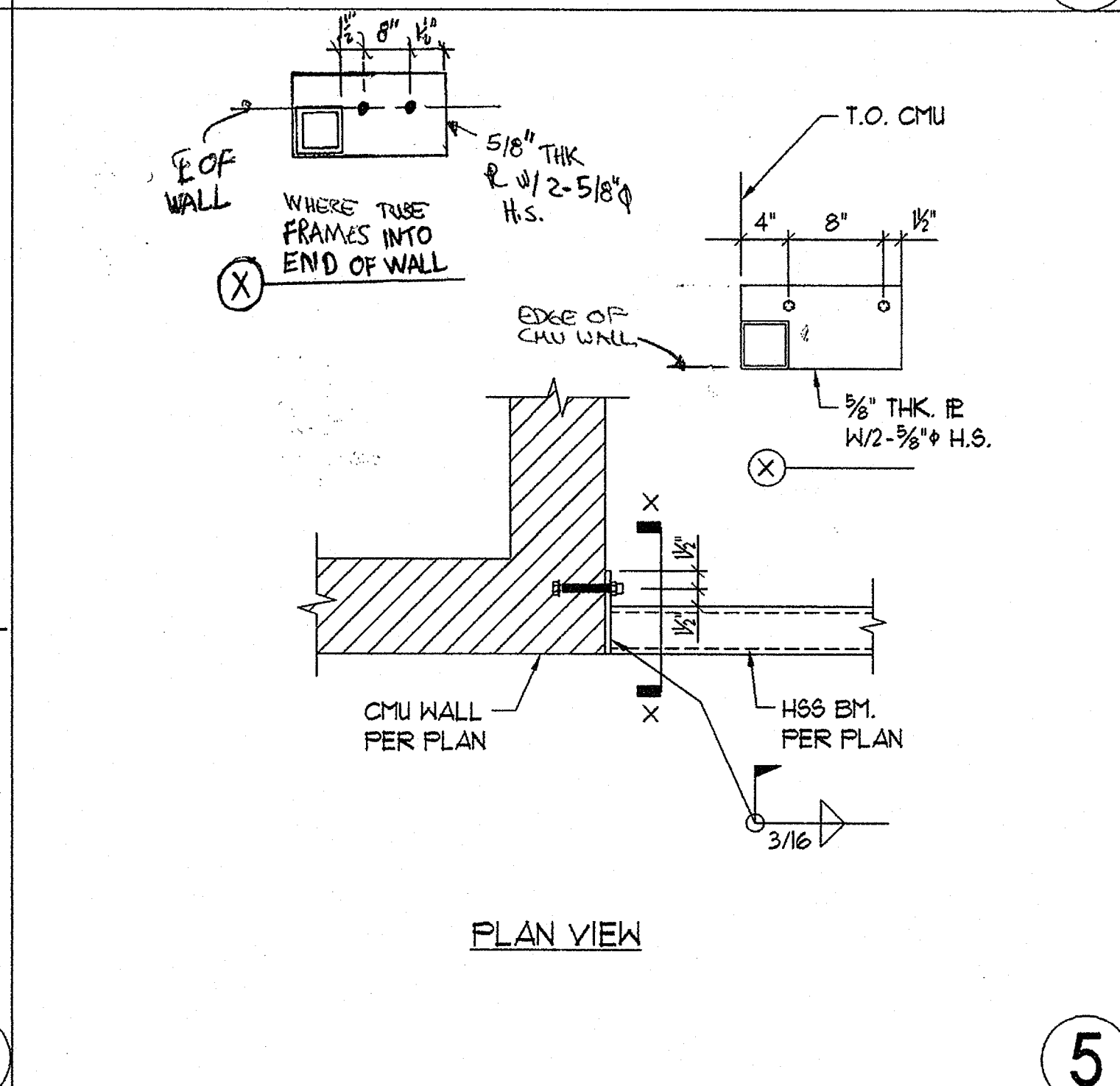
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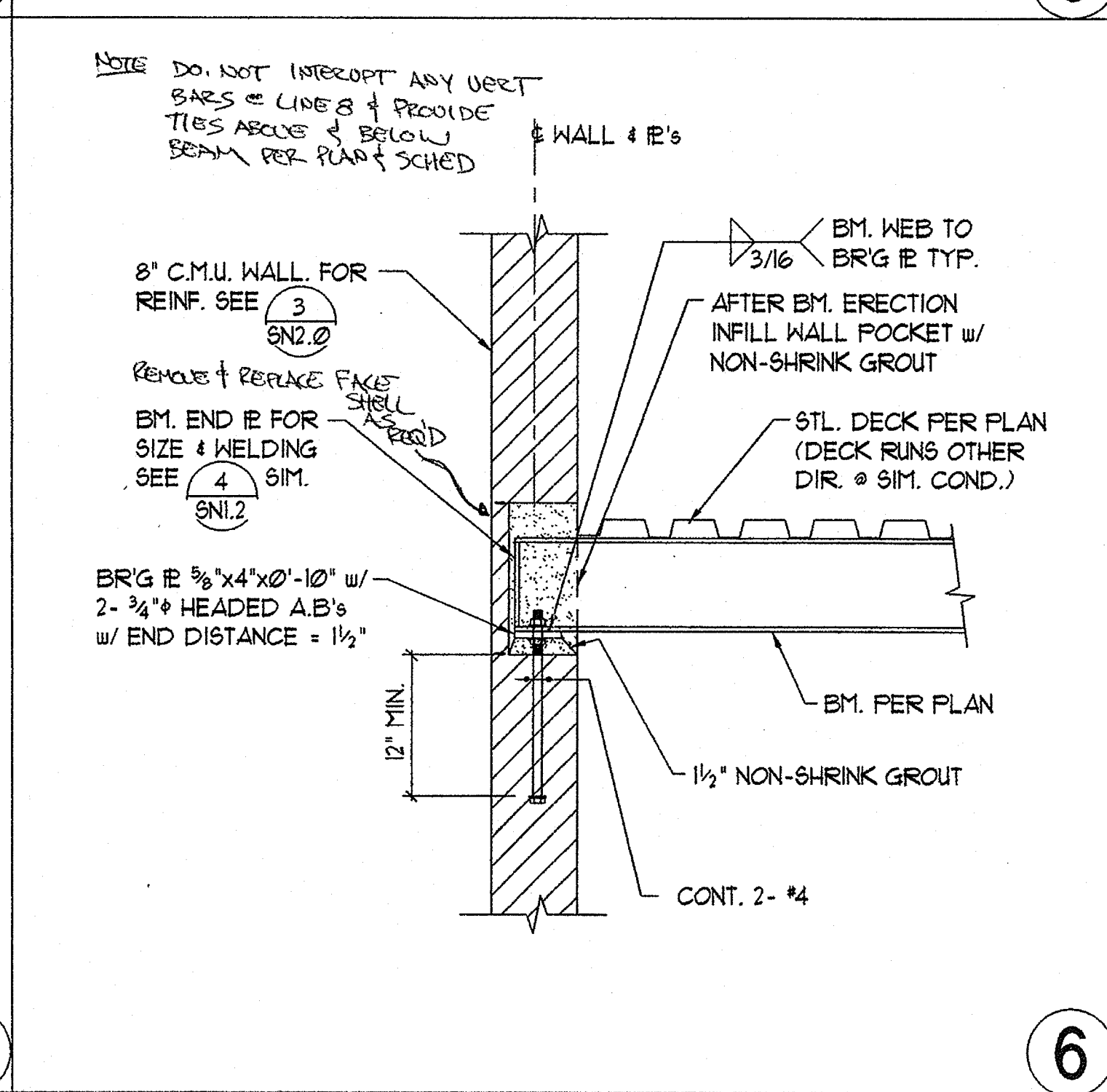
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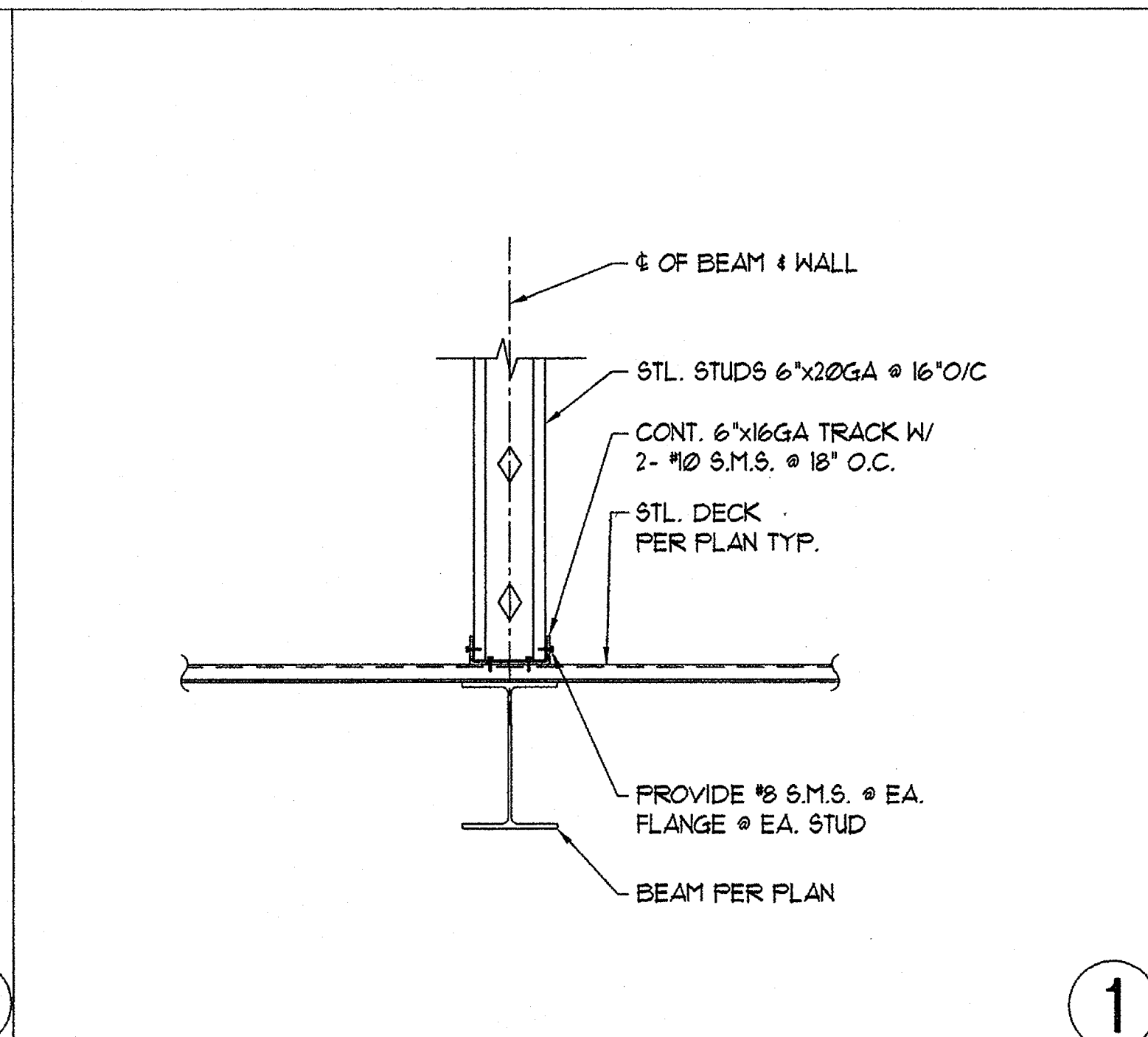
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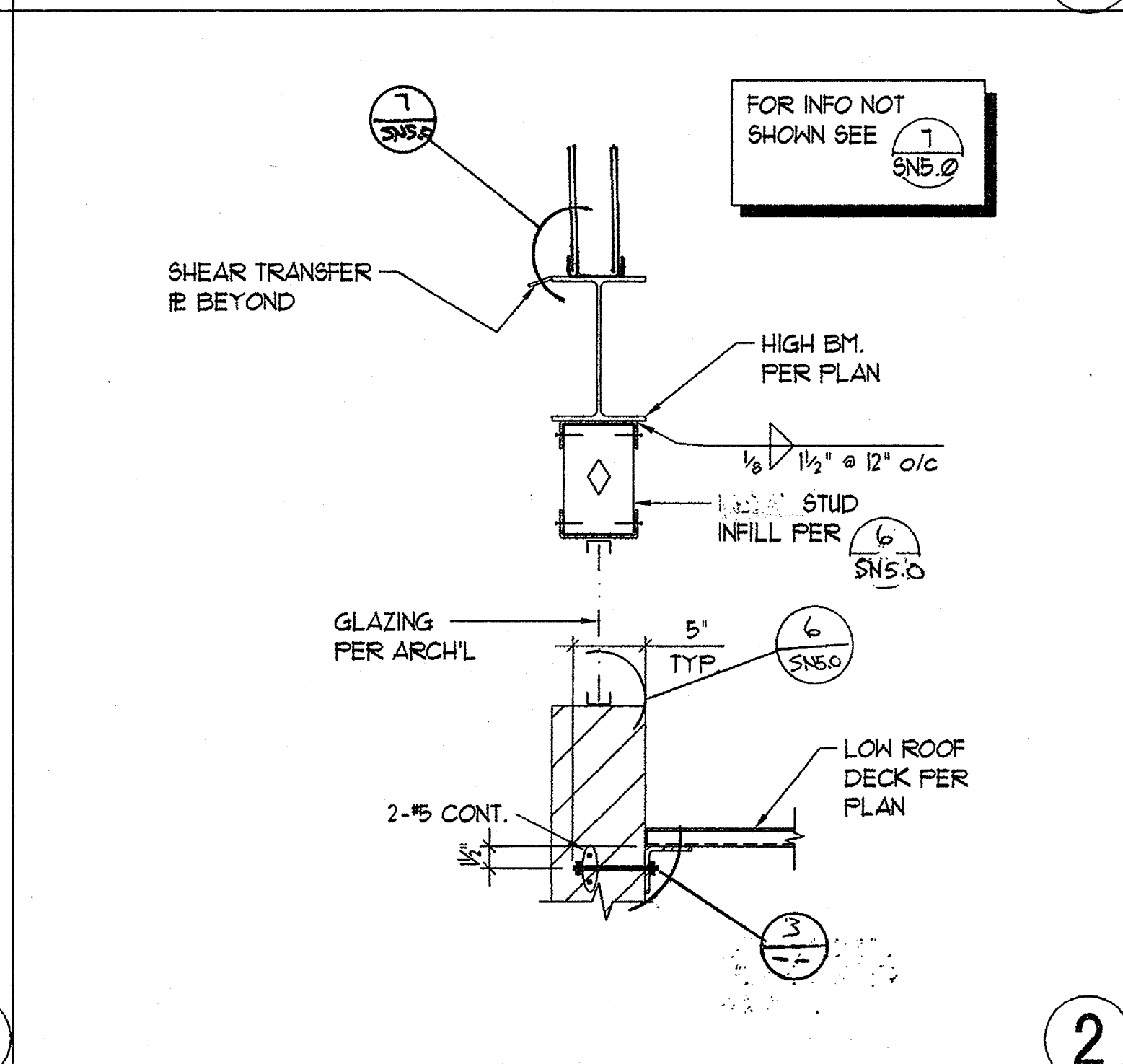
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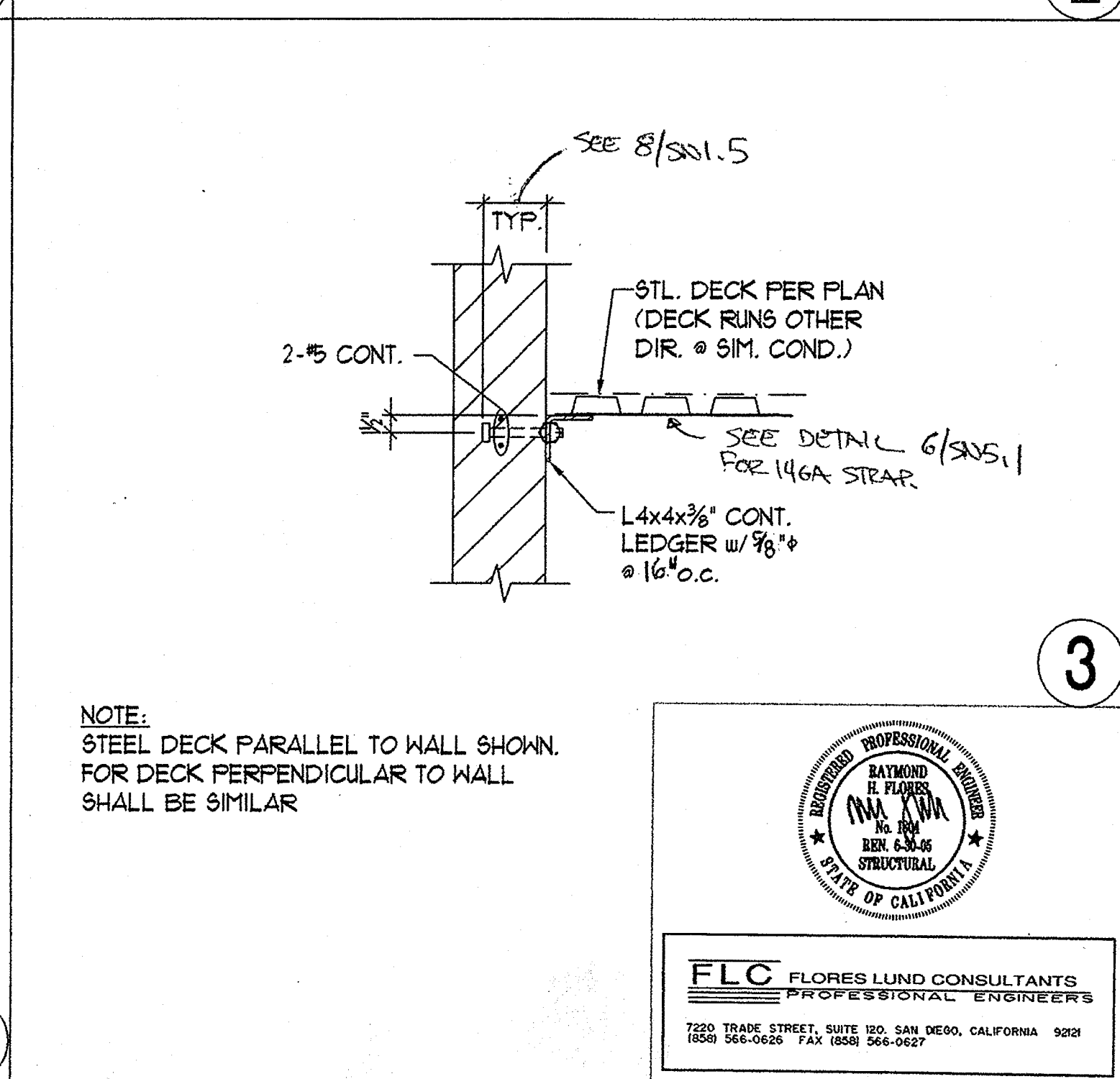
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PROJECT NO. 758-000

DATE 025

REVISIONS

JEFFERSON MS NEW CONSTRUCTION

823 ACACIA STREET OCEANSIDE, CA 92054

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GROTH ARCHITECTS, INC.

3355 MISSION AVE. SUITE 234 OCEANSIDE, CALIFORNIA 92054

PHONE 760-754-8191

FAX 760-754-8291

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4/30/2007

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SHEET TITLE

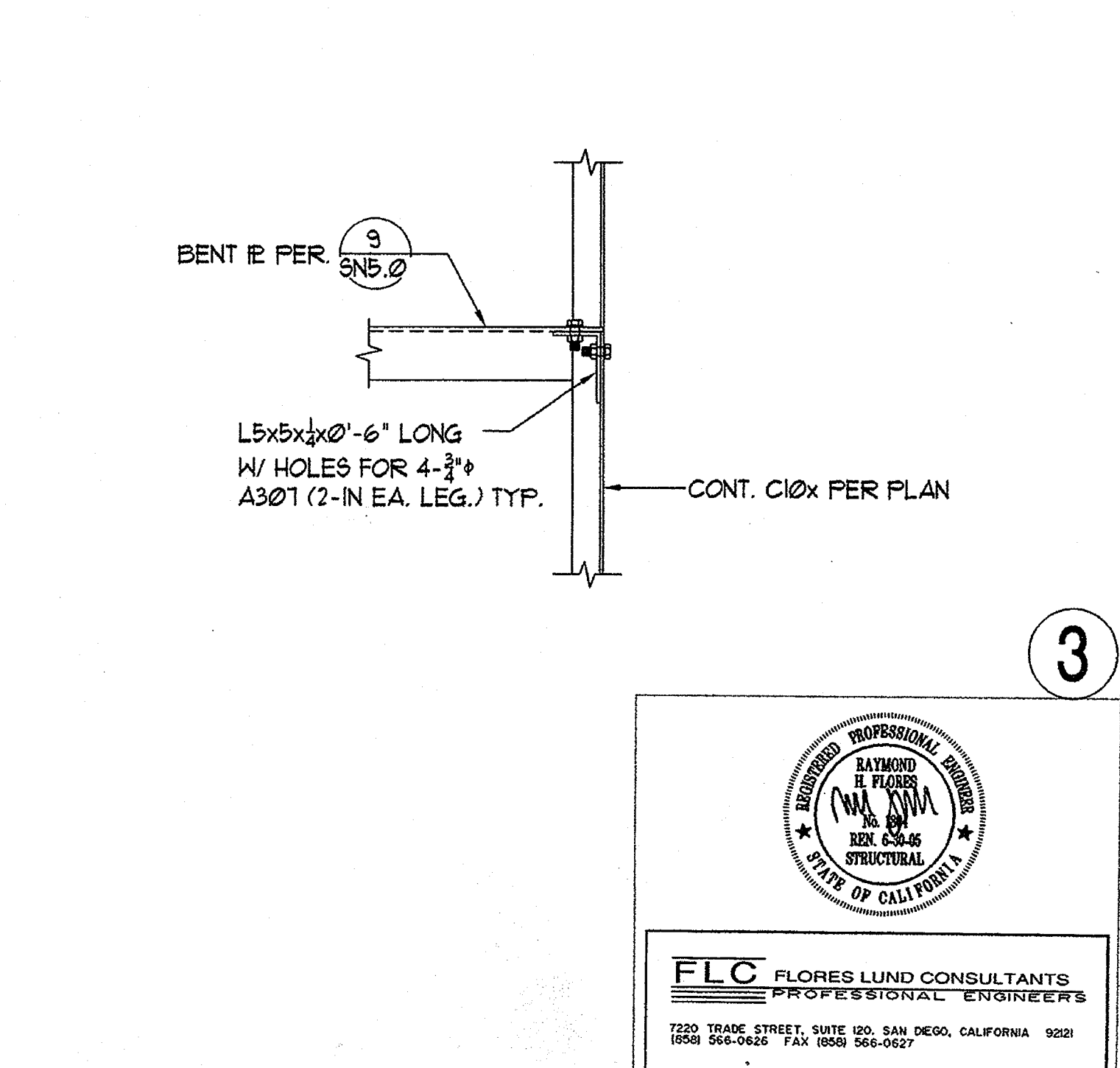
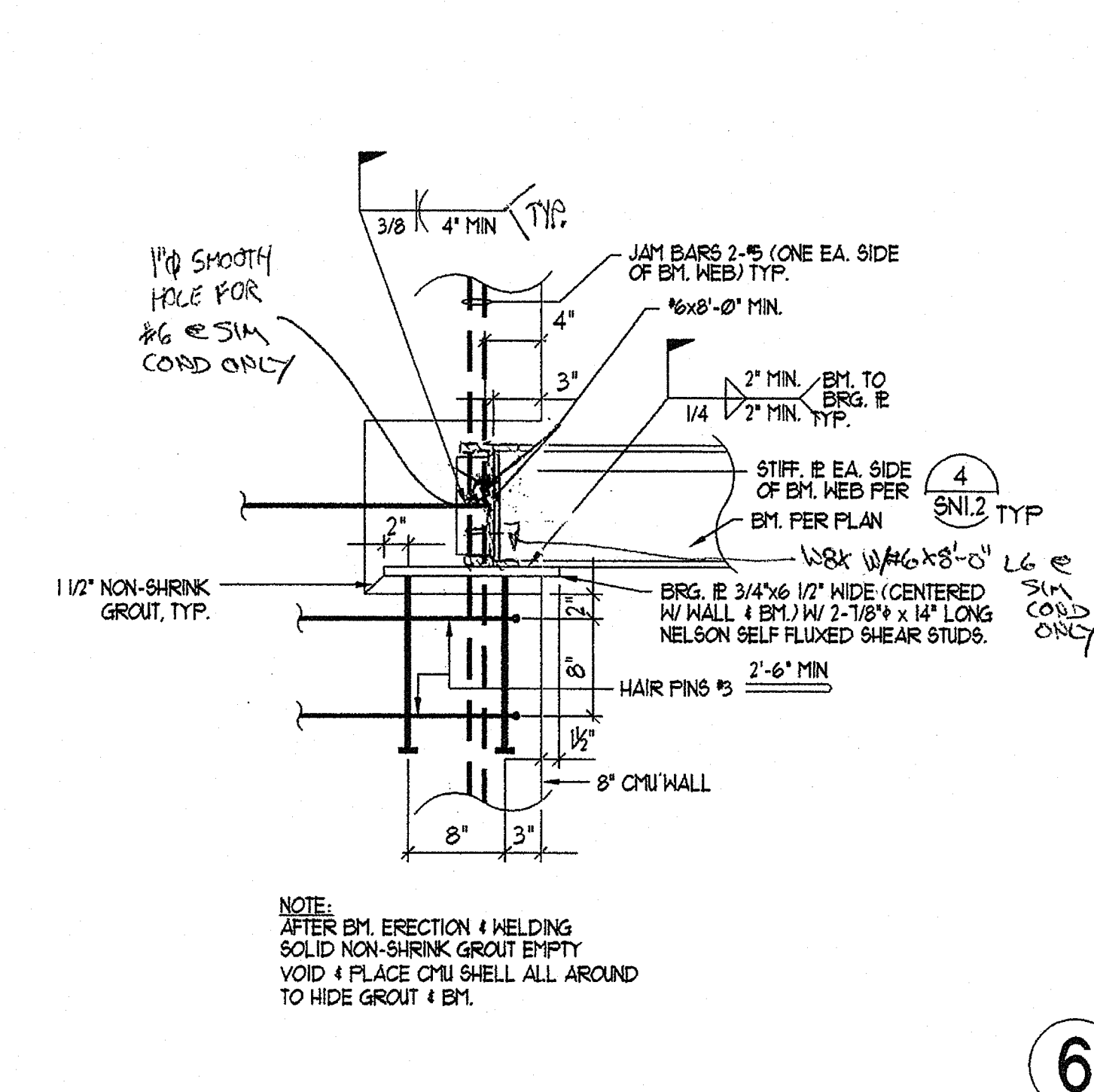
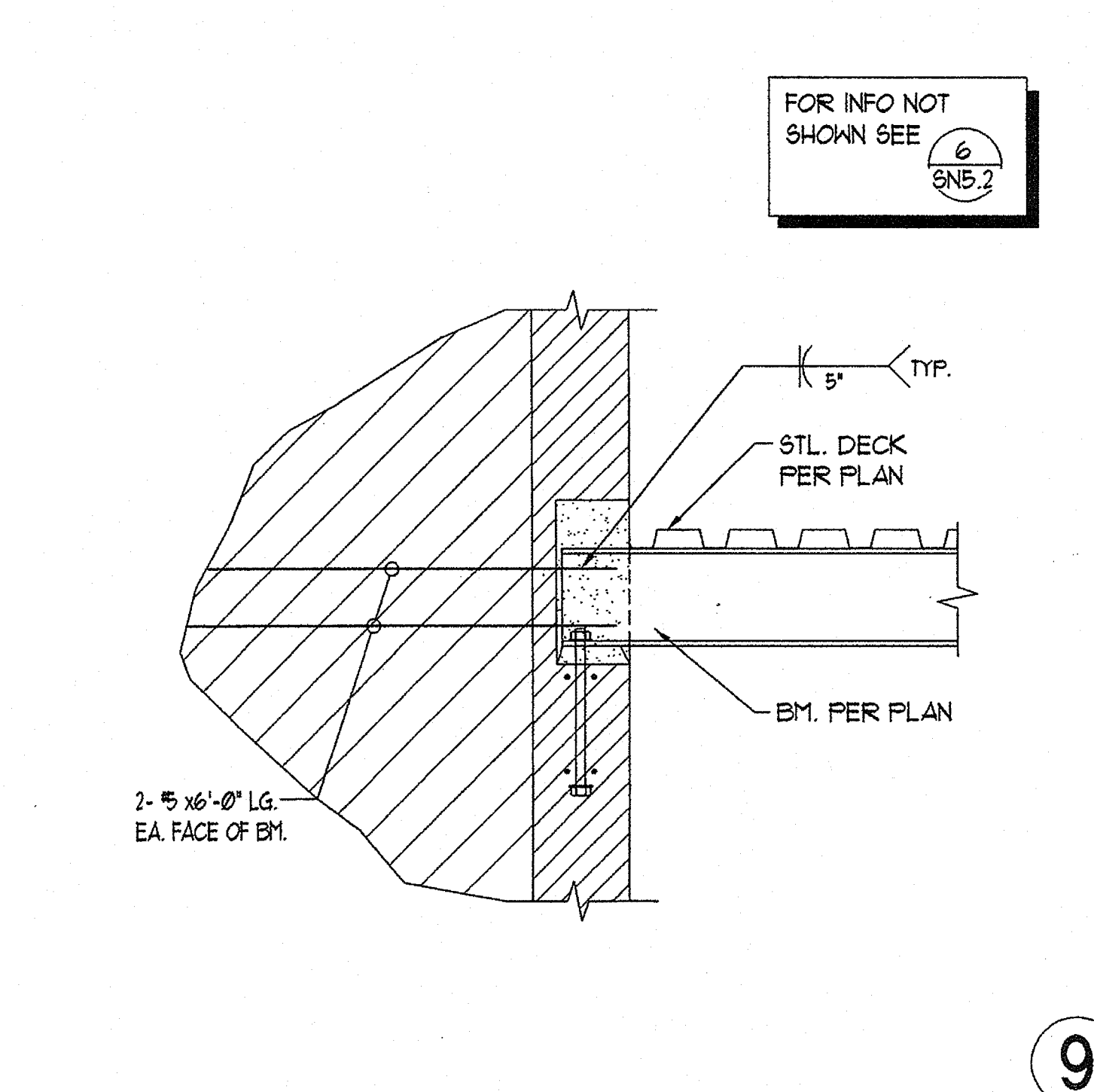
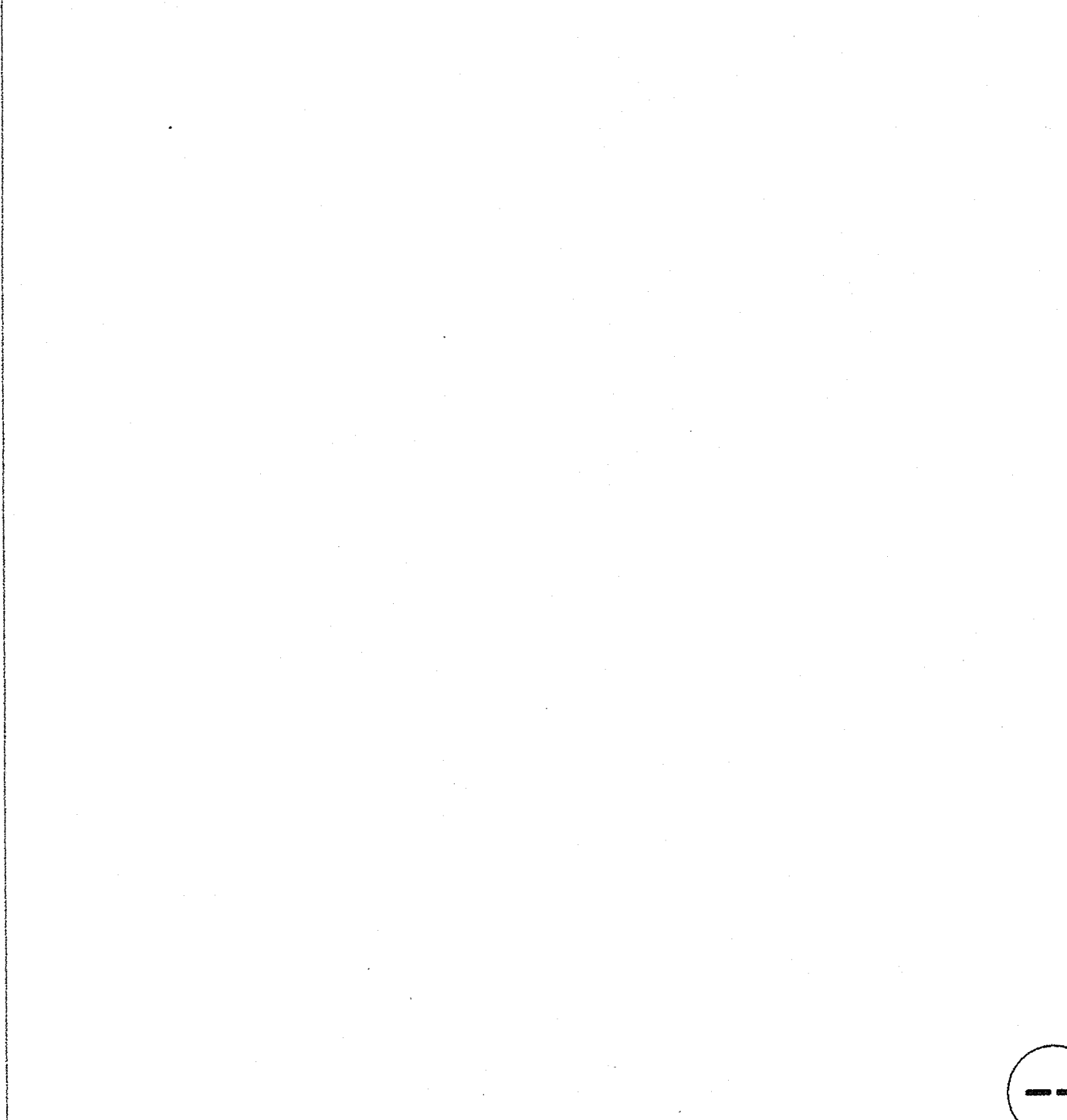
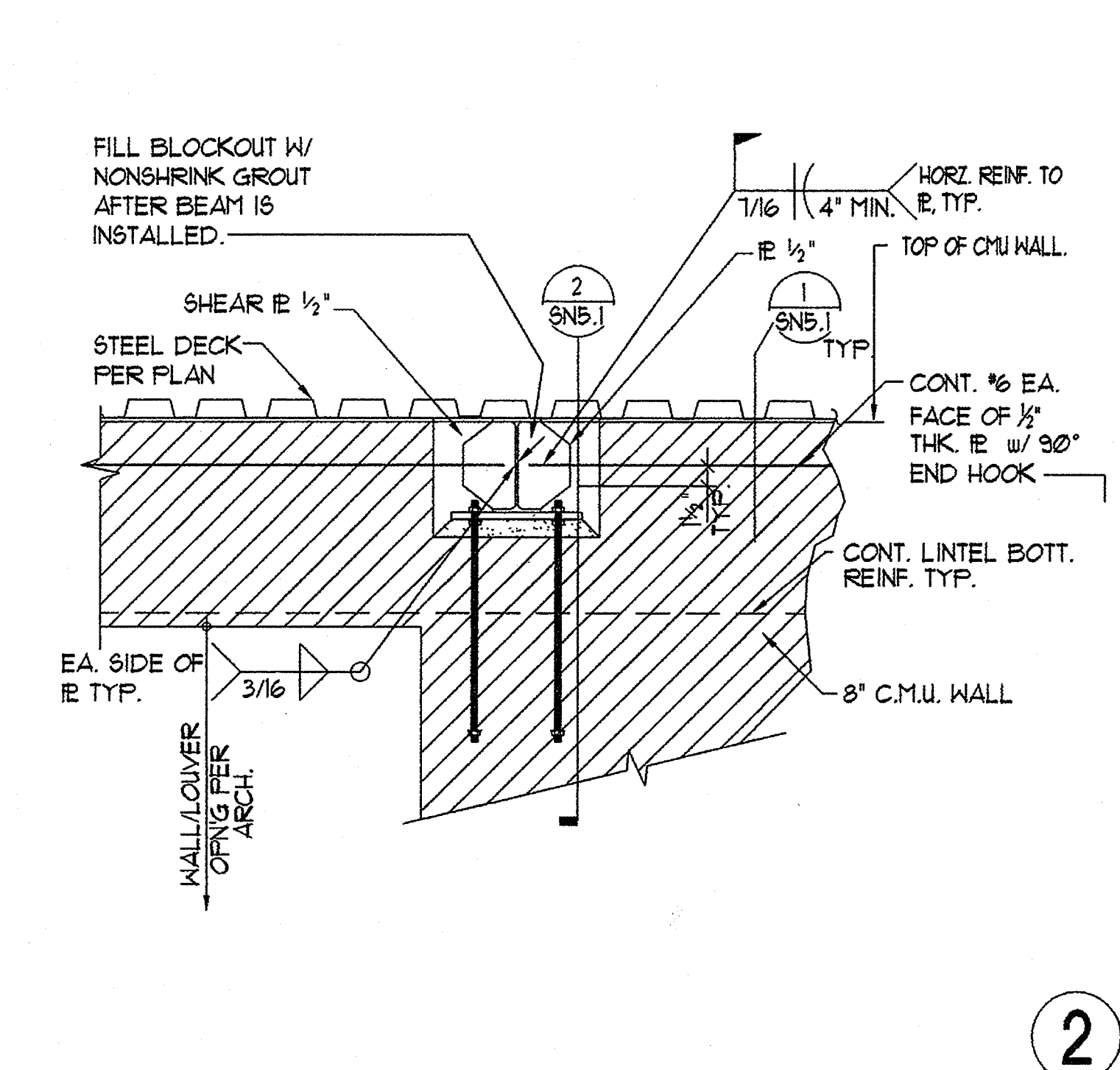
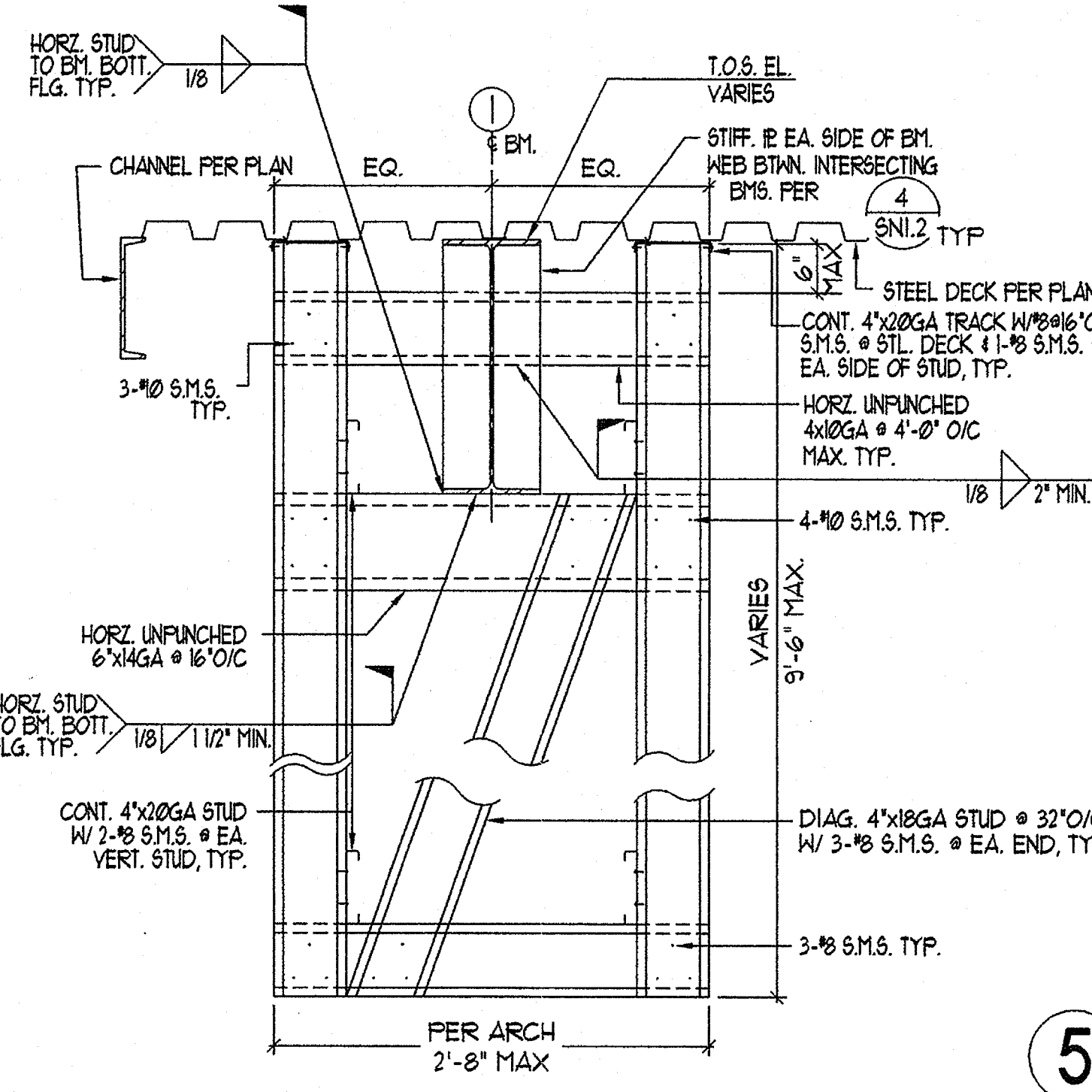
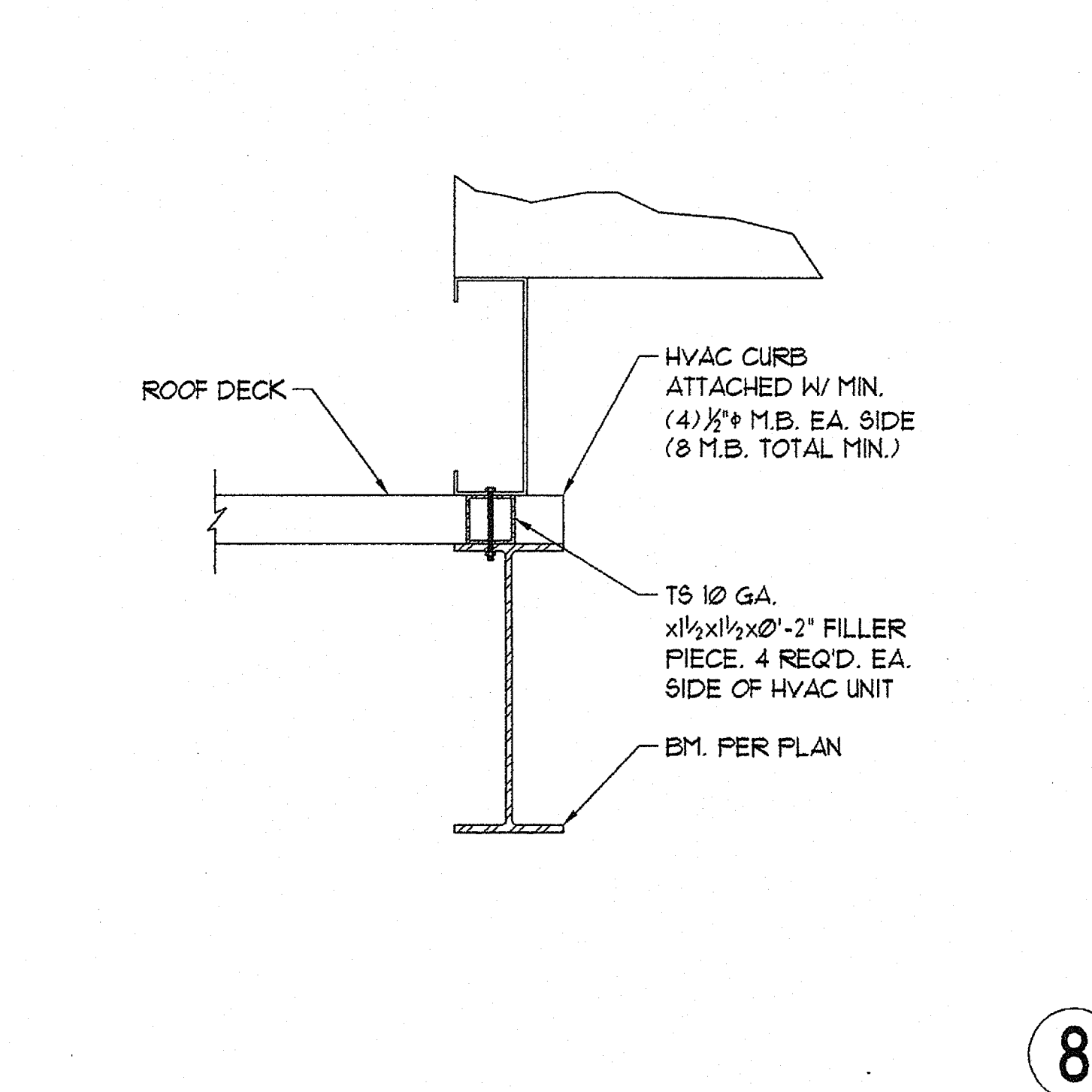
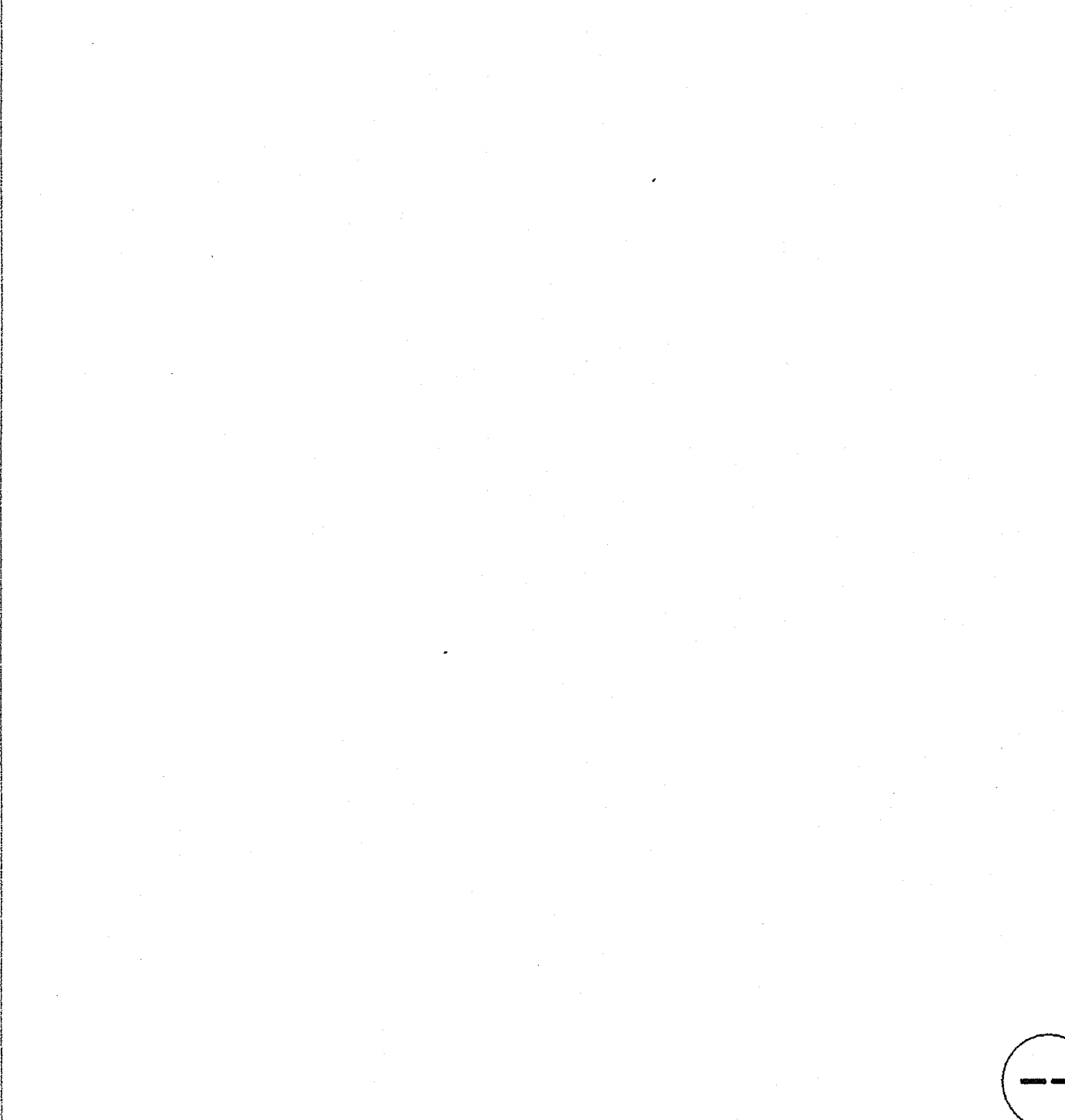
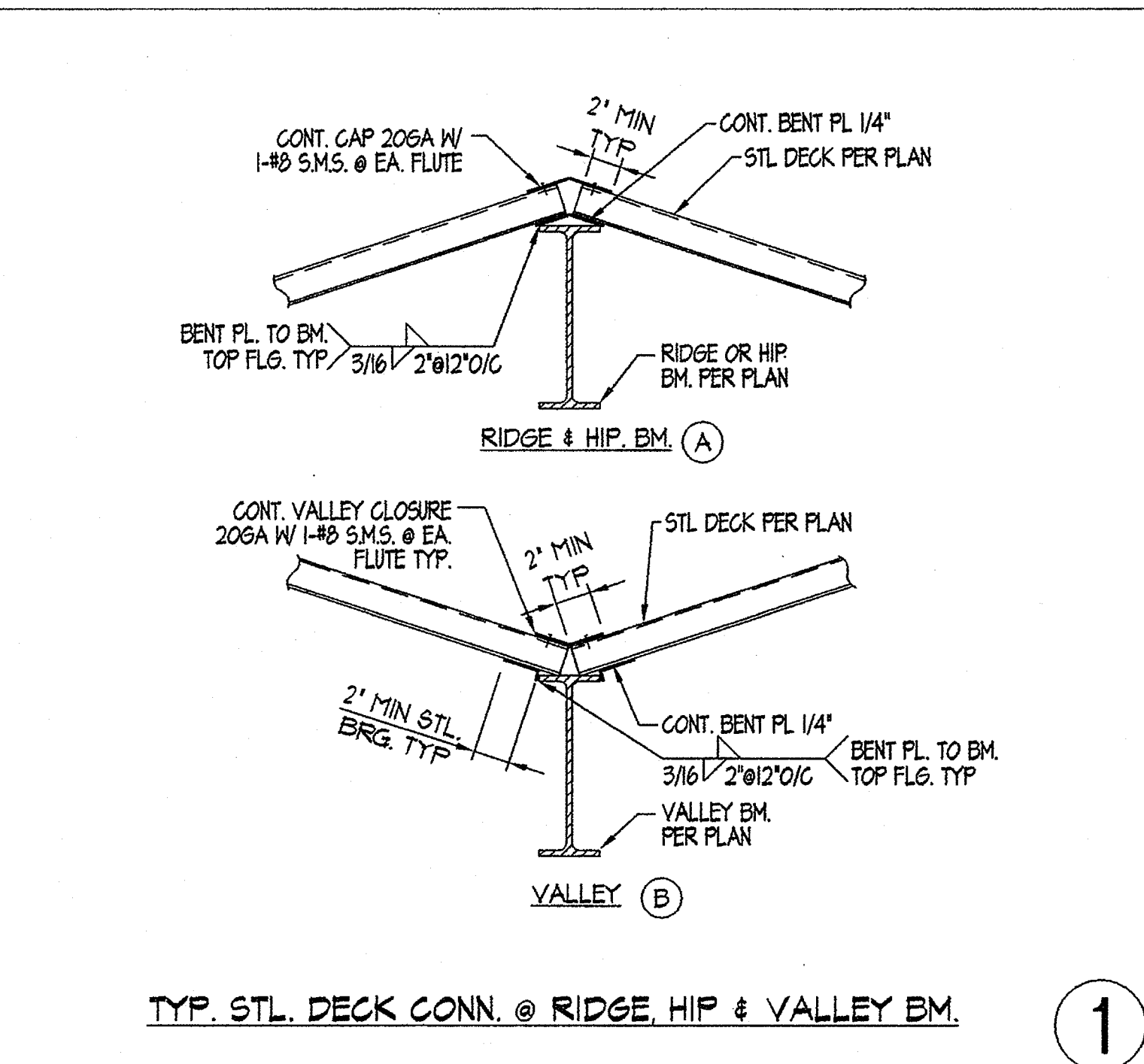
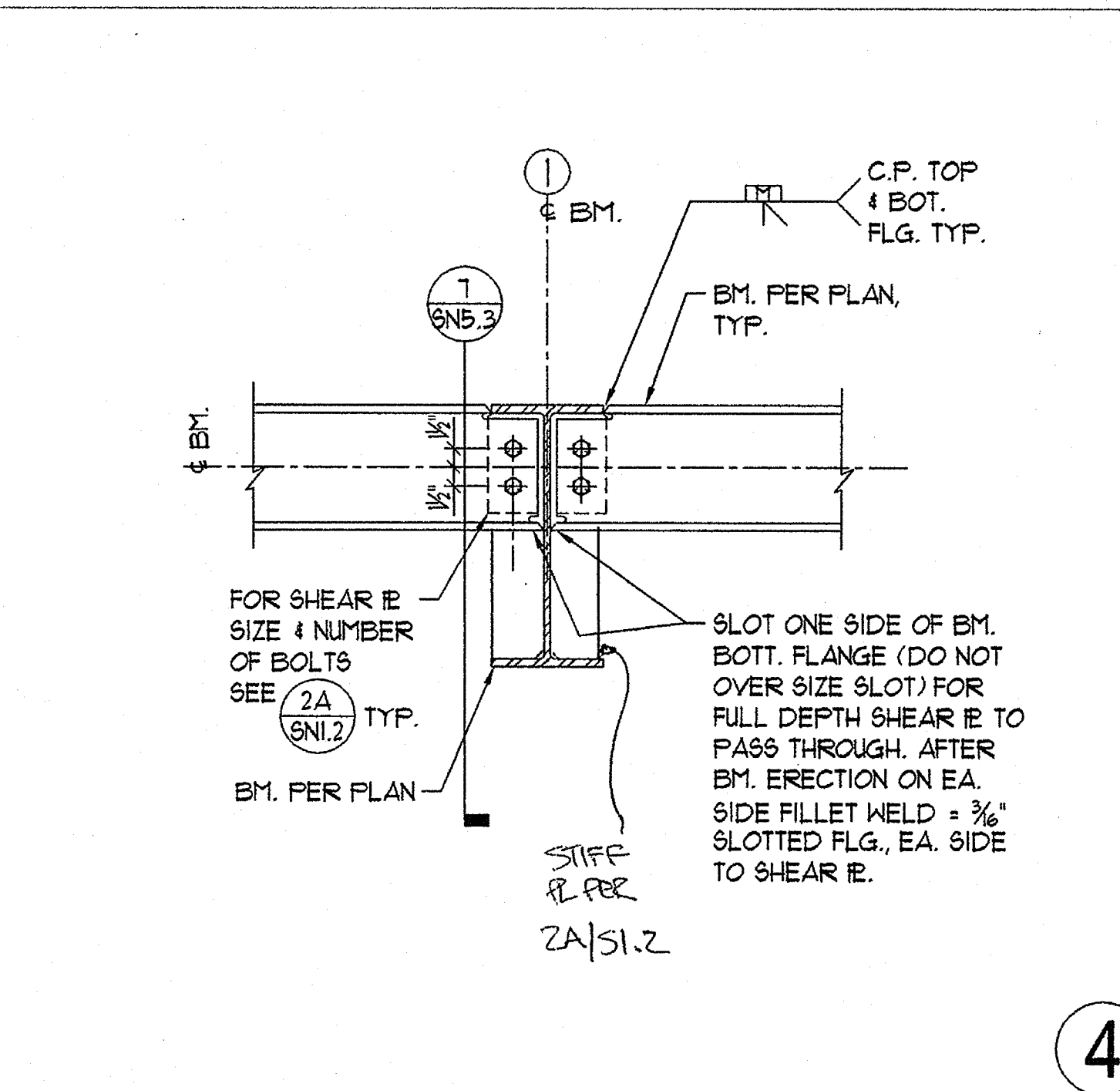
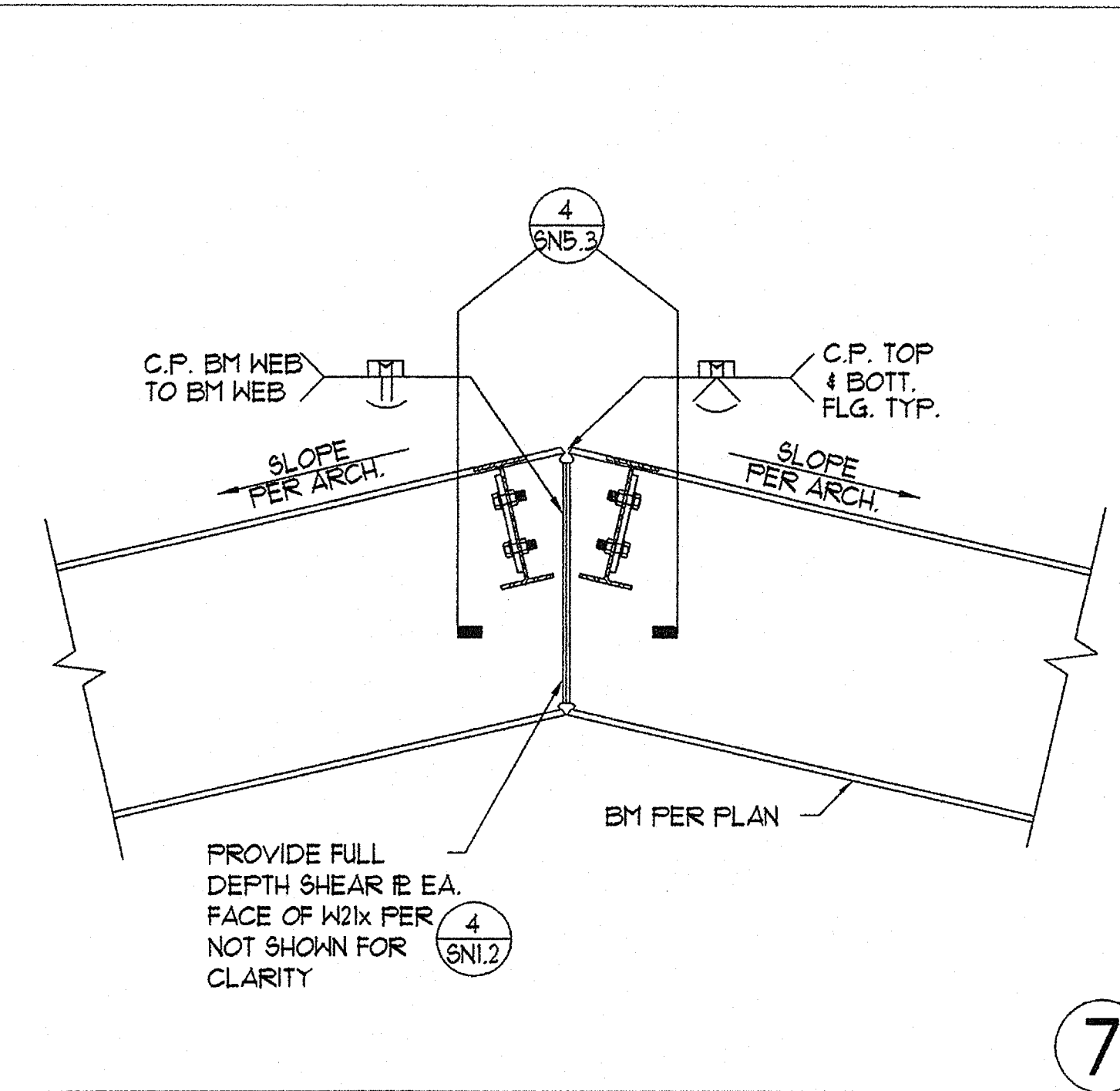
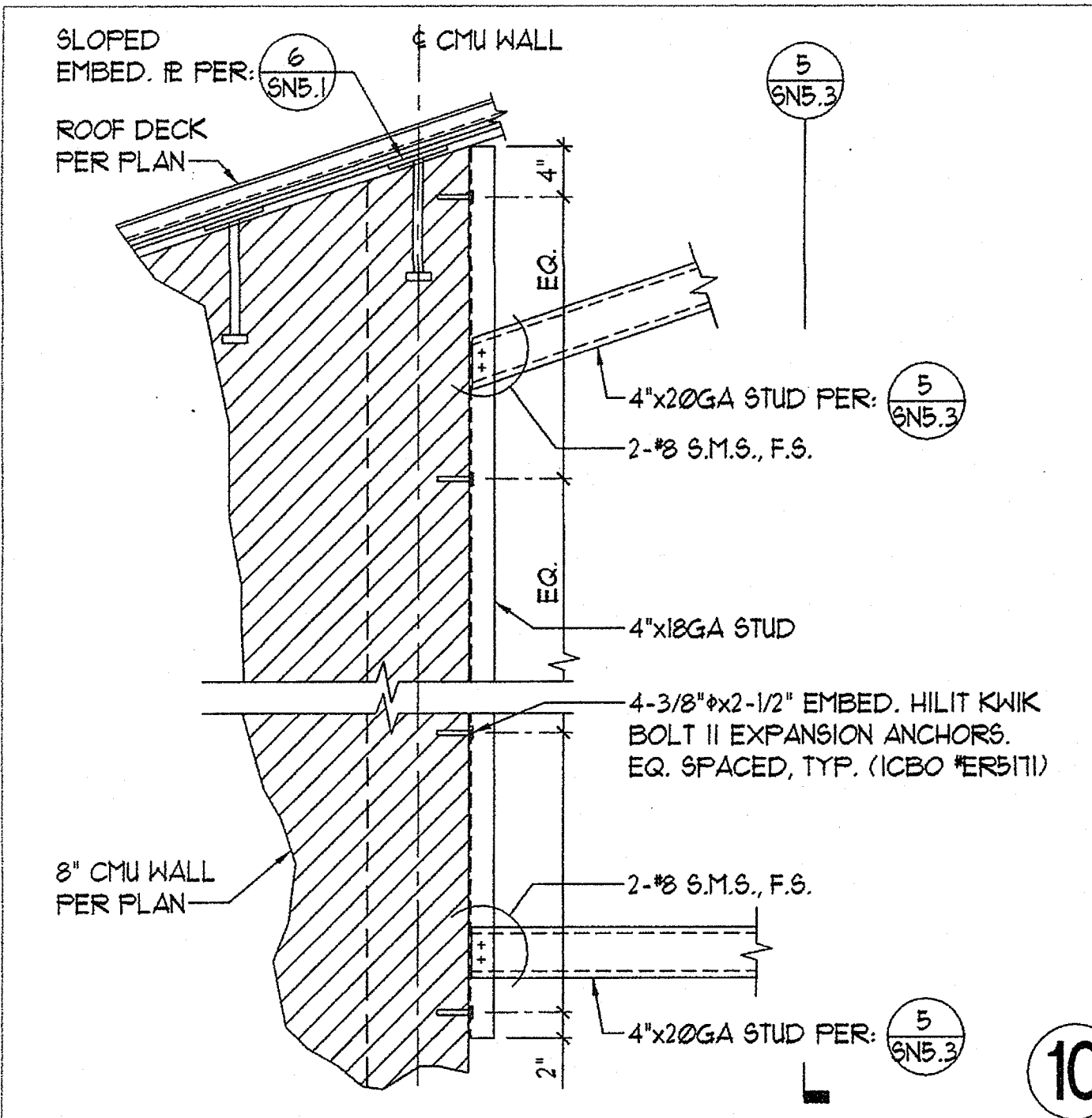
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FLC FLORES LUND CONSULTANTS PROFESSIONAL ENGINEERS

7220 TRADE STREET, SUITE 100, SAN DIEGO, CALIFORNIA 92121

TEL: 619-594-0629 FAX: 619-594-0627



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CUSSD NO. 758-000

PROJECT NOS. O25

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STATE OF CALIFORNIA

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FLC FLORES LUND CONSULTANTS

PROFESSIONAL ENGINEERS

7250 TRADE STREET, SUITE 200, SAN DIEGO, CALIFORNIA 92121

PHONE 619-594-0442 FAX 619-594-0627

PHONE 760-754-8191

FAX 760-754-8291

SPECIAL INSPECTION

SPECIAL INSPECTION BY SPECIAL INSPECTORS SATISFACTORY TO THE BUILDING OFFICIAL IS REQUIRED FOR THE FOLLOWING TYPES OF WORK IN CONFORMANCE WITH THE "CALIFORNIA BUILDING CODE (1997 UNIFORM BUILDING CODE AND REVISIONS INCLUDED IN THE TITLE 24) CHAPTER 17A.

- 1. CONCRETE - SPECIAL INSPECTION IS REQUIRED FOR THE FOLLOWING:
A. CONCRETE PLACEMENT: DURING THE TAKING OF TEST SPECIMENS AND PLACING OF REINFORCED CONCRETE AND PNEUMATICALLY PLACED CONCRETE.
B. BOLTS INSTALLED IN CONCRETE: SPECIAL INSPECTION IS REQUIRED PRIOR AND DURING THE INSTALLATION OF THE BOLTS AND PLACING OF THE CONCRETE AROUND SUCH BOLTS.
C. REINFORCING STEEL: DURING THE PLACING OF REINFORCING STEEL FOR ALL CONCRETE.
2. WELDING - SPECIAL INSPECTION IS REQUIRED FOR THE FOLLOWING:
A. DURING ALL FIELD WELDING.
B. DURING ALL STRUCTURAL WELDING INSPECTION OF ALL SHOP AND FIELD WELDING OPERATIONS, INCLUDING THE INSTALLATION OF AUTOMATIC END-WELDED STUD SHEAR CONNECTORS SHALL BE MADE BY A QUALIFIED WELDING INSPECTOR APPROVED BY DSA. SUCH INSPECTOR SHALL BE A PERSON TRAINED AND THOROUGHLY EXPERIENCED IN INSPECTING WELDING OPERATIONS. THE INSPECTOR'S ABILITY TO DISTINGUISH BETWEEN SOUND AND UNSOUND WELDING SHALL BE RELIABLY ESTABLISHED. THE MINIMUM REQUIREMENTS FOR A QUALIFIED WELDING INSPECTOR SHALL BE AS THOSE FOR AN AWS CERTIFIED WELDING INSPECTOR (CWI), AS DEFINED IN THE PROVISIONS OF THE 1992 EDITION OF AWS QCI, STANDARD AND GUIDE FOR QUALIFICATION AND CERTIFICATION OF WELDING INSPECTORS PUBLISHED BY THE AMERICAN WELDING SOCIETY. ALL WELDING INSPECTORS SHALL BE AS APPROVED BY DSA.
C. DURING ALL STRUCTURAL WELDING OF REINFORCING STEEL.
3. GRADING EXCAVATIONS AND FILLING PER SEC. 1701A.4.13

STRUCTURAL OBSERVATION (AS REQUIRED BY SECTION 1702A OF THE C.B.C.)

- 1. THE DESIGN PROFESSIONAL RESPONSIBLE FOR THE STRUCTURAL DESIGN SHALL SUBMIT A STATEMENT IN WRITING TO DSA, STATING THAT DURING THE CONSTRUCTION OF THIS STRUCTURE, SITE VISITS HAVE BEEN PERFORMED TO OBSERVE GENERAL COMPLIANCE WITH THE APPROVED STRUCTURAL PLANS, SPECIFICATIONS AND CHANGE ORDERS. FURTHER, THE STATEMENT SHALL, IN DETAIL, NOTE HOW ANY DEFICIENCIES HAVE BEEN CORRECTED.
2. AT A MINIMUM, THIS STRUCTURE REQUIRES SITE OBSERVATIONS BEFORE THE FOLLOWING HAS BEEN STARTED:
A. POURING THE INITIAL FOOTINGS.
B. DURING THE START OF ERECTION OF THE ROUGH STRUCTURE.
C. AFTER COMPLETION OF ALL THE ROUGH FRAMING.

POWDER DRIVEN SHOT PINS (LOW VELOCITY)

- 1. QUALIFICATION FOR USE OF ALL POWER ACTUATED TOOLS MUST MEET ANSI A10.3 STANDARD AS REQUIRED BY THE MANUFACTURER AND ALL OSHA REQUIREMENTS.
2. TESTING THE OPERATOR, TOOL, AND FASTENER SHALL BE PRE-QUALIFIED BY THE PROJECT INSPECTOR. HE SHALL OBSERVE THE TESTING OF THE FIRST 10 FASTENER INSTALLATIONS. A TEST "PULL-OUT" LOAD OF NOT LESS THAN TWICE THE DESIGN LOAD SHALL BE APPLIED TO THE PIN IN SUCH A MANNER AS NOT TO RESIST THE SPALLING TENDENCY OF THE CONCRETE SURROUNDING THE PIN. THEREAFTER, RANDOM TESTS UNDER THE PROJECT INSPECTOR'S SUPERVISION SHALL BE MADE OF APPROXIMATELY 1 IN 10 PINS. IF ANY PIN FAILS TESTING, TEST ALL PINS OF THE SAME CATEGORY NOT PREVIOUSLY TESTED UNTIL TWENTY (20) CONSECUTIVE PASS, THEN RESUME THE INITIAL TESTING FREQUENCY.

- 12. WALLS AND COLUMNS SHALL BE DOWELED FROM SUPPORTS WITH BARS OF THE SAME SIZE AND SPACING.
13. SPLICE CONTINUOUS REINFORCING IN ACCORDANCE WITH THE TYPICAL DETAIL 5 ON SHEET SP.1.1. STAGGER SPLICE ALL REINFORCING.
14. PROVIDE MINIMUM EMBEDMENT OF REINFORCING IN CONFORMANCE WITH THE TYPICAL DETAIL 1 ON SHEET SP.1.1
15. REFER TO THE ARCHITECTURAL, MECHANICAL AND ELECTRICAL DRAWINGS FOR LOCATIONS OF PIPES, DUCTS, VENTS AND SIMILAR OPENINGS.
16. REINFORCING, ANCHOR BOLTS AND ALL OTHER EMBEDDED ITEMS SHALL BE SECURELY HELD IN POSITION AND SHALL BE INSPECTED PRIOR TO PLACING CONCRETE.
17. CHAMFER: 3/4" ON ALL EXPOSED CORNERS.
18. ALL ANCHOR BOLTS SHALL BE TIED IN PLACE PRIOR TO PLACING CONCRETE.
19. NO FIELD BENDING OF REINFORCEMENT WILL BE PERMITTED UNLESS APPROVED BY DSA AND THE ENGINEER.

REINFORCING STEEL

- 1. DETAILS OF REINFORCEMENT SHALL BE IN ACCORDANCE WITH CHAPTER 7 OF "AMERICAN CONCRETE INSTITUTE" 318 UNLESS OTHERWISE NOTED. REINFORCING STEEL DETAILING, BENDING AND PLACING SHALL BE IN ACCORDANCE WITH THE CONCRETE REINFORCING STEEL INSTITUTE "MANUAL OF STANDARD PRACTICE", LATEST EDITION.
2. WELDING OF REINFORCING STEEL, IF PERMITTED BY THE ARCHITECT, SHALL BE IN ACCORDANCE WITH THE "STRUCTURAL WELDING CODE - REINFORCING STEEL" OF THE AMERICAN WELDING SOCIETY, AWS D1-4, AND SHALL BE PERFORMED BY WELDERS QUALIFIED UNDER THE PROCEDURES CONTAINED THEREIN.
3. ALL REINFORCING STEEL SHOP DRAWINGS SHALL BE REVIEWED BY THE STRUCTURAL ENGINEER AND THE ARCHITECT PRIOR TO FABRICATION.
4. ALL REINFORCING STEEL SHALL CONFORM TO ASTM A-615, GRADE 60 FOR NO.4 AND LARGER, OTHERWISE GRADE 40. WELDED REINFORCING WHERE PERMITTED SHALL CONFORM TO ASTM 706, OR A VERIFIED AND APPROVED EQUIVALENT.
5. WALLS AND COLUMNS SHALL BE DOWELED FROM SUPPORTS WITH BARS OF THE SAME SIZE AND SPACING.
6. SPACER TIES: PROVIDE A MINIMUM OF #3 TIES AT 24 INCHES IN ALL BEAMS AND FOOTINGS.
7. SPLICE MINIMUM REINFORCING IN ACCORDANCE WITH THE TYPICAL DETAIL 5 ON SHEET SP.1.1
8. PROVIDE MINIMUM EMBEDMENT OF REINFORCING IN CONFORMANCE WITH THE TYPICAL DETAIL 1 ON SHEET SP.1.1
9. BAR SUPPORTS SHALL BE PROVIDED IN ACCORDANCE WITH THE PROVISIONS OF "BAR SUPPORT SPECIFICATION" BY THE CONCRETE REINFORCING STEEL INSTITUTE (CRSI).
10. ALL REINFORCING STEEL AND OTHER EMBEDDED ITEMS SHALL BE SECURELY HELD IN POSITION AND SHALL BE INSPECTED PRIOR TO PLACING CONCRETE AND OR GROUT.
11. NO FIELD BENDING OF REINFORCEMENT WILL BE PERMITTED UNLESS APPROVED BY DSA AND THE ENGINEER.

METAL DECKS

- 1. METAL DECK SHALL BE STEEL AND SHALL BE "VERCO" PER PLAN. STEEL DECK OF OTHER MANUFACTURERS WILL BE ACCEPTABLE, PROVIDED THE DECK HAS PHYSICAL DIMENSIONS AND PHYSICAL PROPERTIES EQUAL TO OR GREATER THAN THE VERCO STEEL DECK, INCLUDING I.C.B.O. APPROVED DIAPHRAGM SHEAR VALUES.
2. ALL METAL SHALL BE GALVANIZED.
3. THE METAL DECK IS USED AS A DIAPHRAGM.
4. METAL DECK SHALL BE PLACED IN THREE SPAN LENGTHS WHEREVER POSSIBLE.
5. METAL DECK SHALL BE ERECTED SUCH THAT RIBS ARE NORMAL TO THE SUPPORTING MEMBERS.
6. HOLES OR OTHER OPENINGS LARGER THAN 6" IN DIAMETER, IF NOT SHOWN ON STRUCTURAL DRAWINGS, SHALL BE APPROVED BY ENGINEER PRIOR TO CUTTING.

COLD-FORMED METAL

- 1. PROVIDE METAL STUDS AND ACCESSORIES AS INDICATED ON THE DRAWINGS, AS SPECIFIED HEREIN, AND AS NEEDED FOR A COMPLETE AND PROPER INSTALLATION.
2. CONTRACTOR SHALL PROVIDE EFFECTIVE, FULL TIME QUALITY CONTROL OVER ALL FABRICATION AND ERECTION COMPLYING WITH THE PERTINENT CODES AND REGULATIONS OF GOVERNMENT AGENCIES (DSA) HAVING JURISDICTION.
3. ALL PRODUCTS TO BE MANUFACTURED BY THE CURRENT MEMBERS OF THE STEEL STUD MANUFACTURERS ASSOCIATION AND THE ICBO REPORT ER-4943P, INCLUDING THE SUPPLEMENTAL ICBO SUBMITTAL AND THE PRODUCT TECHNICAL INFORMATION.
4. ALL GALVANIZED STUDS AND JOISTS SHALL BE FORMED FROM STEEL THAT CORRESPONDS TO THE MINIMUM REQUIREMENTS OF 1996 A.I.S.I. STANDARDS.
5. ALL STRUCTURAL MEMBERS SHALL BE DESIGNED IN ACCORDANCE WITH THE AMERICAN IRON AND STEEL INSTITUTE (AISI) SPECIFICATION FOR THE DESIGN OF COLD-FORMED STEEL STRUCTURAL MEMBERS" 1996 EDITION.
6. ALL LIGHT GAGE STEEL MEMBERS: STUDS AND TRACKS SHALL BE FORMED FROM STEEL HAVING A MINIMUM 33,000 PSI YIELD POINT (ASTM A653 SS GRADE 33 OR ASTM A1011 SS GRADE 33) FOR THICKNESSES OF 0.0179 INCH THROUGH 0.0451 INCH, AND A MINIMUM 50,000 PSI YIELD POINT (ASTM A653 SS GRADE 50, CLASS 1 OR 3, OR ASTM A1011 SS GRADE 50) FOR THICKNESSES OF 0.0538 INCH THROUGH 0.1180 INCH.
7. PROVIDE ALL ACCESSORIES INCLUDING, BUT NOT LIMITED TO, TRACKS, CLIPS, WEB STIFFENERS, ANCHORS, FASTENING DEVICES, RESILIENT CLIPS, AND OTHER ACCESSORIES REQUIRED FOR A COMPLETE AND PROPER INSTALLATION, AND AS RECOMMENDED BY THE MANUFACTURER AND APPROVED BY DSA FOR THE STEEL MEMBERS USED.
8. FASTENING OF COMPONENTS SHALL BE WITH SELF-DRILLING SCREWS OR WELDING. SCREWS OR WELDS SHALL BE OF SUFFICIENT SIZE TO INSURE THE STRENGTH OF THE CONNECTION. ALL WELDS OF GALVANIZED STEEL SHALL BE TOUCHED UP WITH A ZINC-RICH PAINT. ALL WELDS OF CARBON SHEET STEEL SHALL BE TOUCHED UP WITH PAINT. WIRE TYING OF COMPONENTS SHALL NOT BE PERMITTED.

DESIGN CRITERIA

- 1. "CALIFORNIA CODE OF REGULATIONS" (C.C.R.), TITLE 24, 2001 EDITION AND STANDARDS REFERENCED THEREIN.
2. "RECOMMENDED LATERAL FORCE REQUIREMENTS AND COMMENTARY" BY THE STRUCTURAL ENGINEERS ASSOCIATION OF CALIFORNIA, 1999 EDITION.
3. DESIGN LOADS:
A. LIVE LOADS
ROOF.....20 PSF (REDUCIBLE)
B. WIND (PER UBC)
LESS THAN 40 FEET ABOVE GROUND..... 17.4 PSF (HRZ.)
9.4 PSF (UPWARD)
BASIC WIND SPEED..... 70 MPH
EXPOSURE CATEGORY = C
C. SEISMIC -
SEISMIC ZONE 4, Z=0.4 I = 1.0
SEISMIC SOURCE TYPE "B" R = 2.8
Na = 1.0 Vmax = 2.5 Ca
R = 1.0
SOIL PROFILE TYPE; Sd = 0.392 W (ULTIMATE STRENGTH)
Cv = 0.64(Nv) = 0.64 = 0.28 W (WORKING STRESS)
Ca = 0.44(Na) = 0.44

SLAB-ON-GRADE

- 1. THE PURPOSE OF THESE NOTES IS TO ACHIEVE THE BEST POSSIBLE FLOOR FINISH UTILIZING THE EXPERIENCE OF THE CONTRACTOR SINCE THE CONTRACTOR'S MEANS AND METHODS SIGNIFICANTLY AFFECT THE QUALITY AND THEREFORE THE SUCCESS OR FAILURE OF THE DESIGN.
2. THE CONTRACTOR SHALL SUBMIT A POUR SEQUENCE SCHEDULE FOR REVIEW BY THE ARCHITECT AND STRUCTURAL ENGINEER PRIOR TO CASTING ANY SLAB-ON-GRADES. THE SUBMITTAL MUST CONTAIN THE FOLLOWING: AMOUNT OF CEMENT, STRENGTH OF CONCRETE, AGGREGATE SIZE, SLUMP AMOUNT AND THE CONTRACTOR'S ENDORSEMENT THAT HE CAN PRODUCE A SUCCESSFUL SLAB-ON-GRADE.
3. IF A PUMP MIX IS PROPOSED, IT SHOULD BE PROPORTIONED TO MINIMIZE SHRINKAGE IN ADDITION TO CONFORMING TO ALL OTHER REQUIREMENTS.
4. AS A GUIDELINE TO THE CONTRACTOR, THE SLAB-ON-GRADE SHALL BE CAST IN SQUARE OR RECTANGULAR SECTIONS APPROXIMATELY 400 SQUARE FEET MAXIMUM IN AREA WITH A MAXIMUM DISTANCE OF 20 FEET BETWEEN CONSTRUCTION OR WEAKENED JOINTS.
5. AS A FURTHER GUIDELINE TO THE CONTRACTOR, THE DRAWINGS MAY CONTAIN SUGGESTED LOCATIONS FOR CONSTRUCTION JOINTS (C.J.) AND WEAKENED JOINTS (W.J.).
6. REFER TO DETAIL 7 ON SHEET SP.1.1 FOR WEAKENED JOINT (W.J.) AND CONSTRUCTION JOINT (C.J.) DETAIL.

REINFORCED CONCRETE

- 1. ALL CONCRETE WORK SHALL CONFORM TO THE "REQUIREMENTS FOR REINFORCED CONCRETE" (ACI 318) AND THE "SPECIFICATIONS FOR STRUCTURAL CONCRETE FOR BUILDINGS" (ACI 301), LATEST APPROVED EDITIONS, WITH MODIFICATIONS AS NOTED IN THE DRAWINGS, SPECIFICATIONS, AND TITLE 24.
2. ALL REINFORCING DETAILING SHALL CONFORM TO THE CONCRETE REINFORCING STEEL INSTITUTE "MANUAL OF STANDARD PRACTICE" LATEST EDITION.
3. CONTINUOUS INSPECTION BY AN D.S.A. APPROVED INSPECTOR IS REQUIRED FOR ALL STRUCTURAL REINFORCED CONCRETE WORK.
4. WELDING OF REINFORCING STEEL, IF PERMITTED BY THE D.S.A. AND THE STRUCTURAL ENGINEER, SHALL BE IN ACCORDANCE WITH THE "STRUCTURAL WELDING CODE - REINFORCING STEEL" OF THE AMERICAN WELDING SOCIETY, AWS D1.4, AND SHALL BE PERFORMED BY WELDERS QUALIFIED UNDER THE PROCEDURES CONTAINED THEREIN.
5. ALL REINFORCING STEEL SHOP DRAWINGS SHALL BE REVIEWED BY THE ARCHITECT AND STRUCTURAL ENGINEER PRIOR TO FABRICATION.
6. CONCRETE STRENGTHS: THE CONCRETE STRENGTHS SHOWN IN THE FOLLOWING TABLE ARE MINIMUM COMPRESSIVE STRENGTH AT 28 DAYS, THE AGGREGATES SHOWN ARE THE MAXIMUM SIZE (INCHES) AND THE SLUMP SHOWN IS THE MAXIMUM (INCHES). THE COMPRESSIVE STRENGTH OF CONCRETE SHALL BE PROPORTIONAL AS REQUIRED IN C.C.R. TITLE 24, SECTION 1905A.2 FOR EITHER METHOD A, B, OR C.
ITEM OF CONSTRUCTION (145 P.C.F. AVE.) STRENGTH AGGREGATE SLUMP
A. FOUNDATION SYSTEM 3,000 PSI 1 1/2" 4"
B. SLAB ON GRADE 3,000 PSI 1" 3"
C. MISC. PATCHING 3,000 PSI 3/8" 3"
7. BATCH PLANT INSPECTION MAY BE WAIVED PROVIDED THE CONCRETE PLANT COMPLES FULLY WITH THE REQUIREMENTS OF U.B.C. STANDARD NO. 26-13 AND HAS BEEN CERTIFIED BY D.S.A. TO COMPLY WITH THE REQUIREMENTS OF THE "NATIONAL READY MIXED CONCRETE ASSOCIATION". THE PLANT MUST BE EQUIPPED WITH AN AUTOMATIC BATCHER IN WHICH THE TOTAL BATCHING CYCLE, EXCEPT FOR THE MEASURING AND INTRODUCTION OF AN ADMIXTURE, IS COMPLETED BY ACTIVATING A SINGLE STARTER DEVICE.
WHEN BATCH PLANT INSPECTION IS WAIVED, THE FOLLOWING REQUIREMENTS SHALL APPLY AND SHALL BE DESCRIBED IN THE CONTRACT SPECIFICATIONS:
APPROVED INSPECTOR OF THE TESTING LABORATORY SHALL CHECK THE FIRST BATCHING AT THE START OF WORK AND FURNISH MIX PROPORTIONS TO THE LICENSED WEIGHMASTER.
LICENSED WEIGHMASTER TO POSITIVELY IDENTIFY MATERIALS AS TO QUANTITY AND CERTIFY TO EACH LOAD BY A TICKET.
TICKETS SHALL BE TRANSMITTED TO THE PROJECT INSPECTOR BY A TRUCK DRIVER WITH LOAD IDENTIFIED THEREON. INSPECTOR WILL NOT ACCEPT THE LOAD WITHOUT A LOAD TICKET IDENTIFYING THE MIX AND WILL KEEP A DAILY RECORD OF PLACEMENTS, IDENTIFYING EACH TRUCK, ITS LOAD AND TIME OF RECEIPT, AND APPROXIMATE LOCATION OF DEPOSIT IN THE STRUCTURE WILL TRANSMIT A COPY OF THE DAILY RECORD TO THE ENFORCEMENT AGENCY.
AT THE END OF THE PROJECT, THE WEIGHMASTER SHALL FURNISH AN AFFIDAVIT TO THE ENFORCEMENT AGENCY ON FORM SSS 411-8 CERTIFYING THAT ALL CONCRETE FURNISHED CONFORMS IN EVERY PARTICULAR TO PROPORTIONS ESTABLISHED BY MIX DESIGNS.
8. THE EVALUATION AND ACCEPTANCE OF CONCRETE SHALL CONFORM TO THE REQUIREMENTS OF C.B.C. TITLE 24 SECTION 1905A.6.
9. ALL REINFORCING STEEL SHALL CONFORM TO ASTM A615, GRADE 60 FOR NO. 4 AND LARGER, OTHERWISE GRADE 40.
10. MINIMUM PROTECTIVE CONCRETE COVERAGE OF REINFORCING:
A. ON EARTH SIDE WHEN PLACED AGAINST EARTH.....3 IN.
B. ON EARTH SIDE WHEN FORMED.....2 IN.
C. EXTERIOR WALL STEEL ABOVE GRADE.....1 1/2 IN.
D. INTERIOR WALL STEEL AND SUPPORTED SLABS.....1 IN.
E. TIED COLUMNS (TO TIES) ABOVE GRADE.....1 1/2 IN.
F. BEAMS (TO STIRRUPS) ABOVE GRADE.....1 1/2 IN.
G. WELDED WIRE FABRIC.....CENTER LINE OF SLAB
11. NO PIPES OR DUCTS SHALL BE PLACED IN CONCRETE COLUMNS, WALLS OR SLABS UNLESS SPECIFICALLY DETAILED OR UNLESS SLEEVES ARE PROVIDED IN ACCORDANCE WITH THE TYPICAL DETAIL 13 ON SHEET SP.1.1.

GENERAL NOTES

THE FOLLOWING GENERAL NOTES ARE A SUMMARY OF THE SPECIFICATIONS FOR THE CONVENIENCE OF THE CONTRACTOR. REFER TO THE SPECIFICATIONS AND ALL OTHER CONTRACT DOCUMENTS FALL ALL REQUIREMENTS.

GENERAL

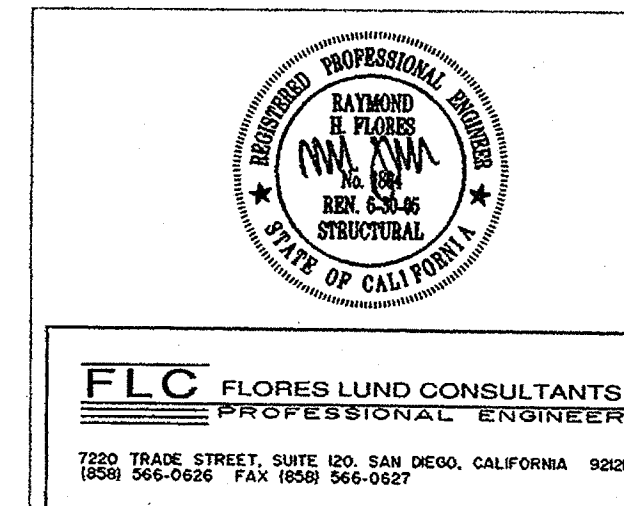
- 1. CONTRACTOR SHALL VERIFY ALL DIMENSIONS BEFORE STARTING WORK AND NOTIFY THE ARCHITECT IMMEDIATELY OF ANY DISCREPANCIES FOUND.
2. SPECIFIC CODES AND DETAILS SHALL TAKE PRECEDENCE OVER THESE GENERAL NOTES AND THE TYPICAL DETAILS ON SP.1.1 THROUGH SP.1.3 IN CASE OF CONFLICT. NOTIFY ENGINEER FOR CLARIFICATION.
3. WHERE NO CONSTRUCTION DETAILS ARE SHOWN OR NOTED FOR ANY PART OF THE WORK, THE DETAILS USED SHALL BE THE SAME AS FOR OTHER SIMILAR WORK, PROVIDED THAT PRIOR APPROVAL IS OBTAINED FROM THE ARCHITECT OR ENGINEER.
4. THE DESIGN IS BASED ON TITLE 24, CODE OF REGULATIONS (C.C.R.) 2001 EDITION.
5. NEITHER THE OWNER NOR THE ARCHITECT WILL ENFORCE SAFETY MEASURES OR REGULATIONS. THE CONTRACTOR SHALL DESIGN, CONSTRUCT AND MAINTAIN ALL SAFETY DEVICES, INCLUDING SHORING AND BRACING, AND SHALL BE SOLELY RESPONSIBLE FOR CONFORMING TO ALL LOCAL, STATE AND FEDERAL SAFETY AND HEALTH STANDARDS, LAWS AND REGULATIONS.

FOUNDATIONS:

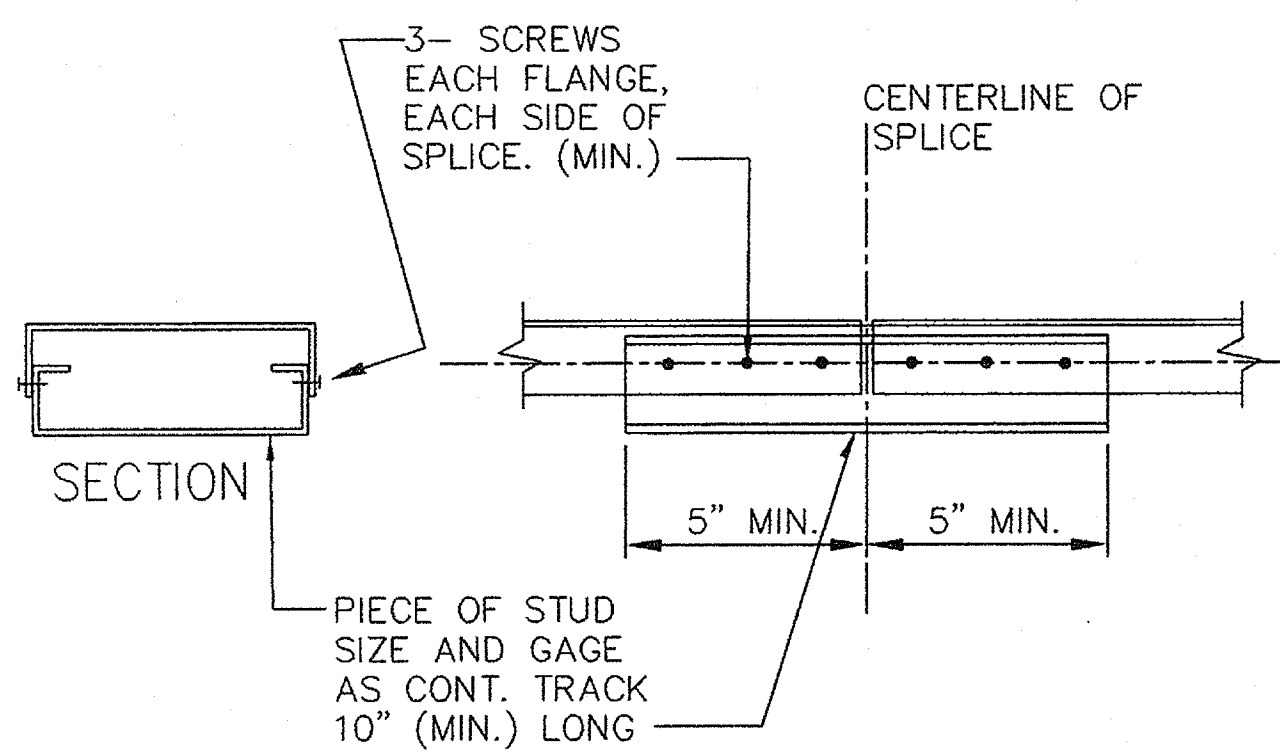
A PRELIMINARY EXPLORATION OF THE SOILS UNDERLYING THIS SITE WAS MADE BY CONSTRUCTION TESTING & ENGINEERING INC. AND IS DESCRIBED IN A REPORT DATED JUNE 10, 2004.

THE FOLLOWING ARE RECOMMENDATIONS AND INTERPRETATIONS FROM THIS REPORT. FLORES LUND CONSULTANTS (FLC) CAN NOT BE HELD LIABLE FOR RECOMMENDATIONS PREPARED BY A DIFFERENT FIRM. (FLC'S EXPERIENCE IN THIS FIELD IS NIL).

- 1. SITE PREPARATION
BEFORE GRADING, THE SITE SHOULD BE CLEARED OF ANY EXISTING DEBRIS AND OTHER DELETERIOUS MATERIALS. IN AREAS TO RECEIVE STRUCTURES OR DISTRESS-SENSITIVE IMPROVEMENTS, EXPANSIVE, SURFICIAL ERODED, DESICCATED, BURROWED, OR OTHERWISE LOOSE OR DISTURBED SOILS ON GRADE SHOULD BE SCARIFIED, MOISTURE CONDITIONED TO ABOVE OPTIMUM MOISTURE CONTENT, AND PROPERLY COMPACTED. MATERIALS A DETERMINATION OF THE SUITABILITY OF THE EXPOSED SUBGRADES SHOULD BE MADE IN THE FIELD BY AN ENGINEER OR GEOLOGIST FROM THIS FIRM. ORGANIC AND OTHER DELETERIOUS MATERIALS NOT SUITABLE FOR STRUCTURAL BACKFILL SHOULD BE DISPOSED OF OFFSITE AT A LEGAL DISPOSAL SITE.
2. SITE EXCAVATION
BASED ON THE PRESENCE OF SHALLOW UNDOCUMENTED FILL OR TOPSOIL MATERIALS, AREAS BENEATH PROPOSED NEW STRUCTURES ARE TO BE EXCAVATED TO COMPETENT NATIVE MATERIALS AND TO A MINIMUM DEPTH OF 18 INCHES BELOW ALL PROPOSED FOUNDATIONS TO MINIMIZE EFFECTS OF DIFFERENTIAL SETTLEMENTS. THESE EXCAVATIONS CAN GENERALLY BE ACCOMPLISHED USING HEAVY-DUTY CONSTRUCTION EQUIPMENT. HOWEVER, LOCALIZED CEMENTED OR VERY HARD ZONES MAY BE ENCOUNTERED DURING THESE OPERATIONS. GRADING ACTIVITIES SHOULD BE CONTINUOUSLY MONITORED BY CTE. SUCH OBSERVATIONS ARE ESSENTIAL TO IDENTIFY FIELD CONDITIONS THAT DIFFER FROM THOSE IDENTIFIED DURING OUR SUBSURFACE INVESTIGATION AND ADJUST DESIGNS TO ACTUAL FIELD CONDITIONS ENCOUNTERED.
3. FILL PLACEMENT AND COMPACTION
AS STATED, AN ENGINEER OR GEOLOGIST FROM CTE SHOULD BE CALLED UPON TO VERIFY THAT THE PROPER SITE PREPARATION HAS OCCURRED BEFORE FILL PLACEMENT BEGINS. FOLLOWING THE REMOVAL OF LOOSE OR DISTURBED SOILS, AREAS TO RECEIVE FILLS OR CONCRETE OR SLABS ON GRADE SHOULD BE SCARIFIED, MOISTURE CONDITIONED TO ABOVE OPTIMUM MOISTURE CONTENT, AND PROPERLY COMPACTED. FILL AND BACKFILL SHOULD BE COMPACTED TO A MINIMUM RELATIVE COMPACTION OF 90 PERCENT AS EVALUATED BY ASTM D-1557 AT MOISTURE CONTENTS BETWEEN OPTIMUM AND TWO PERCENT ABOVE OPTIMUM. THE OPTIMUM LIFT THICKNESS FOR BACKFILL SOIL WILL BE DEPENDENT ON THE TYPE OF COMPACTION EQUIPMENT USED. GENERALLY BACKFILL SHOULD BE PLACED IN UNIFORM LIFTS NOT EXCEEDING EIGHT INCHES IN LOOSE THICKNESS. BACKFILL PLACEMENT AND COMPACTION SHOULD BE DONE IN OVERALL CONFORMANCE WITH GEOTECHNICAL RECOMMENDATIONS AND LOCAL ORDINANCES.
4. FILL MATERIALS
SOILS DERIVED FROM ON-SITE MATERIALS ARE CONSIDERED SUITABLE FOR REUSE ON THE SITE AREA AS FILL, PROVIDED THEY ARE SCREENED OF ORGANIC MATERIALS AND MATERIALS GREATER THAN THREE INCHES IN MAXIMUM DIMENSION. IMPORTED FILL BENEATH STRUCTURES, PAVEMENTS AND WALKS SHOULD HAVE AN EXPANSION INDEX LESS THAN OR EQUAL TO 50 (PER UBC 18-1-8) WITH LESS THAN 35 PERCENT PASSING THE NO. 200 SIEVE. IMPORTED FILL SOILS FOR USE IN STRUCTURAL OR SLOPE AREAS SHOULD BE EVALUATED BY THE SOILS ENGINEER TO DETERMINE STRENGTH CHARACTERISTICS BEFORE PLACEMENT ON THE SITE.
5. THE ALLOWABLE SOIL BEARING PRESSURE IS 2,000 PSF FOR CONTINUOUS AND ISOLATED SPREAD FOOTINGS. FOOTINGS SHALL EXTEND A MINIMUM DEPTH OF 18 INCHES BELOW LOWEST ADJACENT SUBGRADE.
6. FOOTING ELEVATIONS SHOWN ARE FOR BIDDING PURPOSES ONLY AND ARE ASSUMED TO BE IN SUITABLE BEARING MATERIALS. THE ACTUAL ADEQUACY OF THE BEARING MATERIAL SHALL BE DETERMINED BY A PROFESSIONAL, LICENSED GEOTECHNICAL ENGINEER REGISTERED IN THE STATE OF CALIFORNIA PRIOR TO PLACING OF REINFORCING OR POURING OF CONCRETE, AND FOOTING ELEVATIONS SHALL BE ADJUSTED, OR OTHER REMEDIAL ACTION TAKEN, AS DIRECTED BY THIS PROFESSIONAL AND APPROVED BY THE ARCHITECT, AND DSA.
7. ALL ANCHORS SHALL BE TIED IN PLACE PRIOR TO CALLING FOR FOUNDATION INSPECTION.

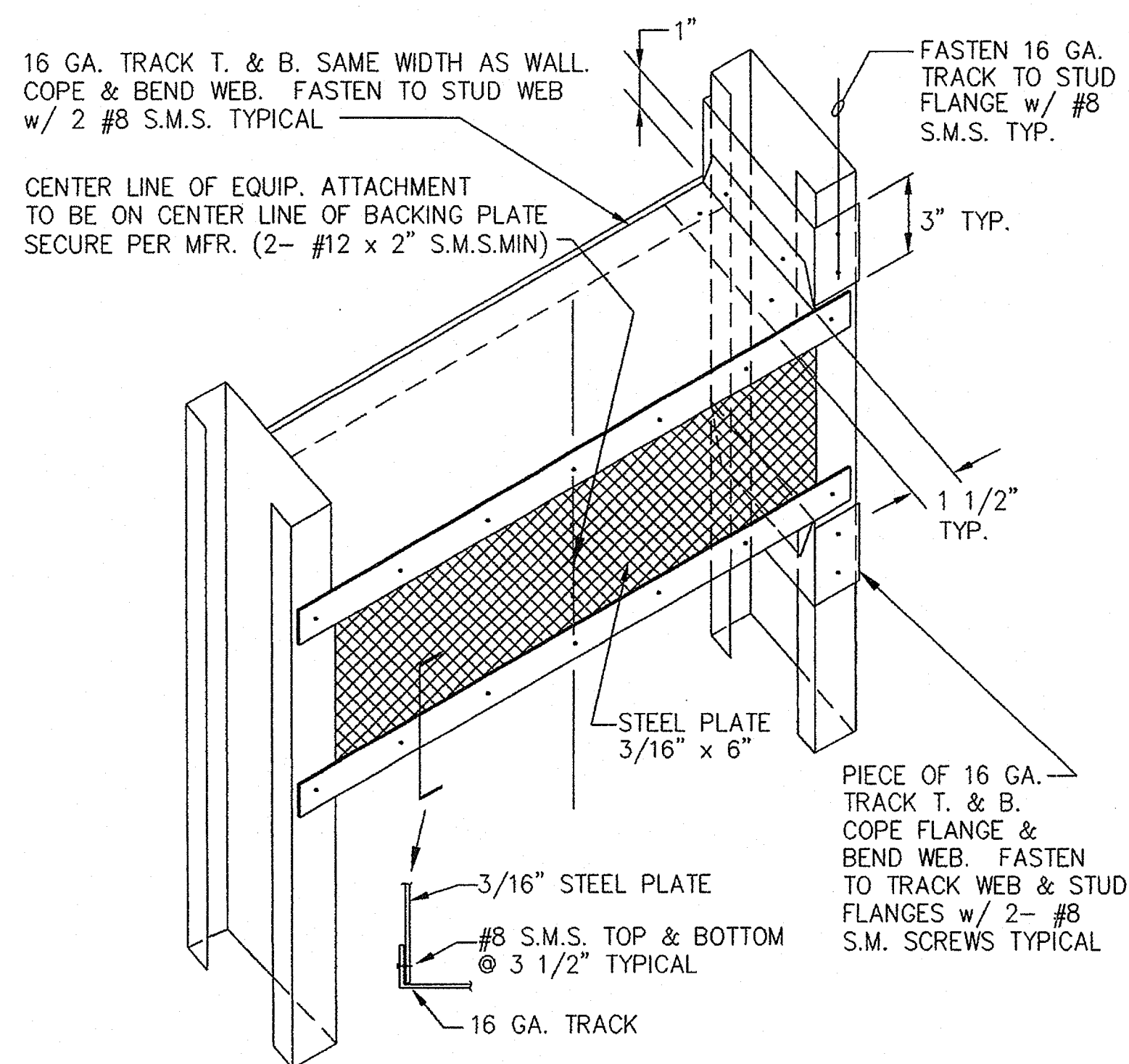


Vertical sidebar containing project information: PROJECT NO. 758-000, PROJECT NOS. 025, P. T. N. 73569-9, DATE, REVISIONS, JEFFERSON MS NEW CONSTRUCTION, 823 ACACIA STREET, OCEANSIDE, CA 92054, OCEANSIDE UNIFIED S.D., GROTH ARCHITECTS, INC., 3355 MISSION AVE., OCEANSIDE, CALIFORNIA 92054, PHONE 760-754-8191, FAX 760-754-8291, LICENSED ARCHITECT JOHN SCOTT BOLT, C-26609, 4/30/2007, SHEET TITLE GENERAL NOTES, SP.1.0



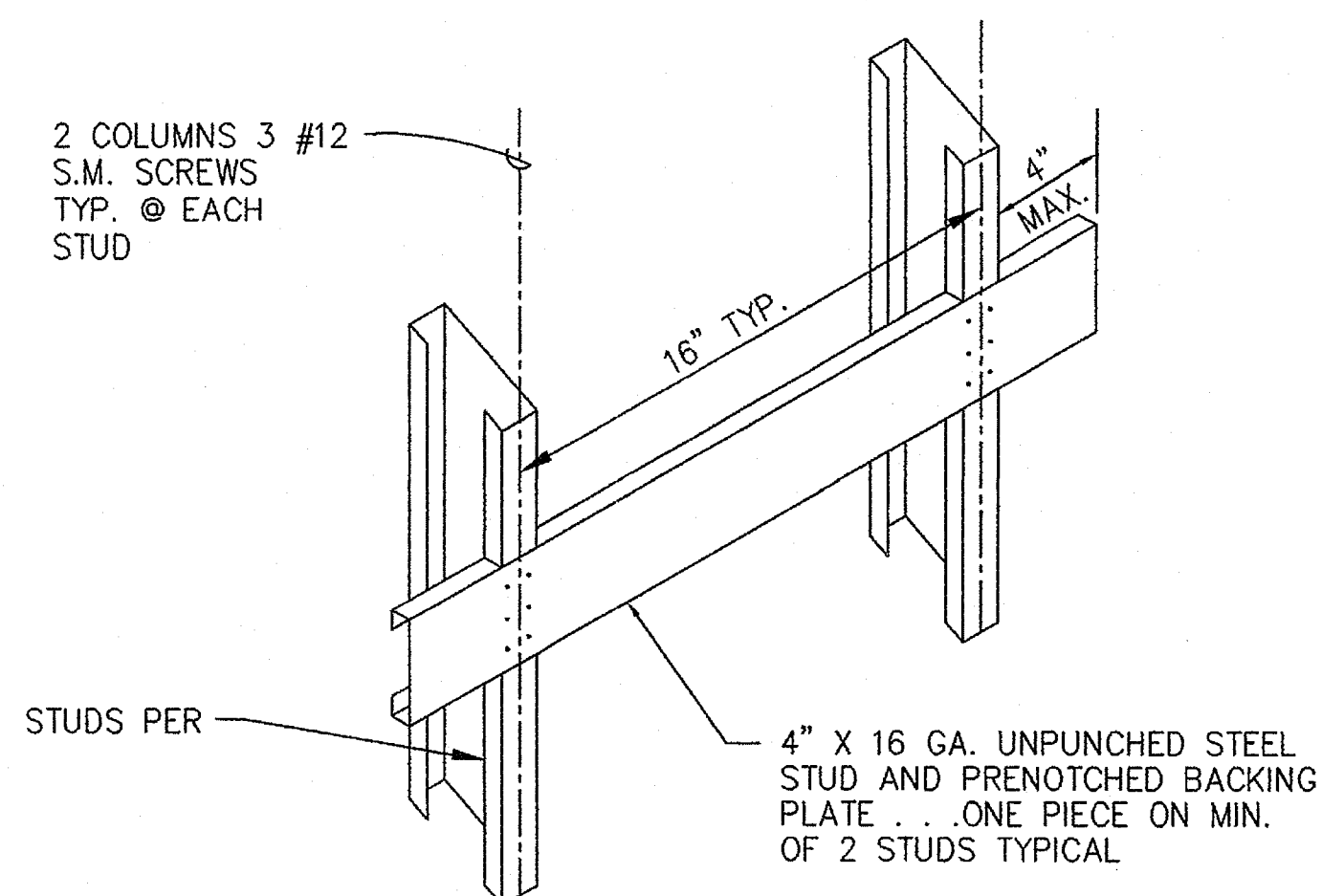
TYP. TOP STEEL TRACK SPLICE

14



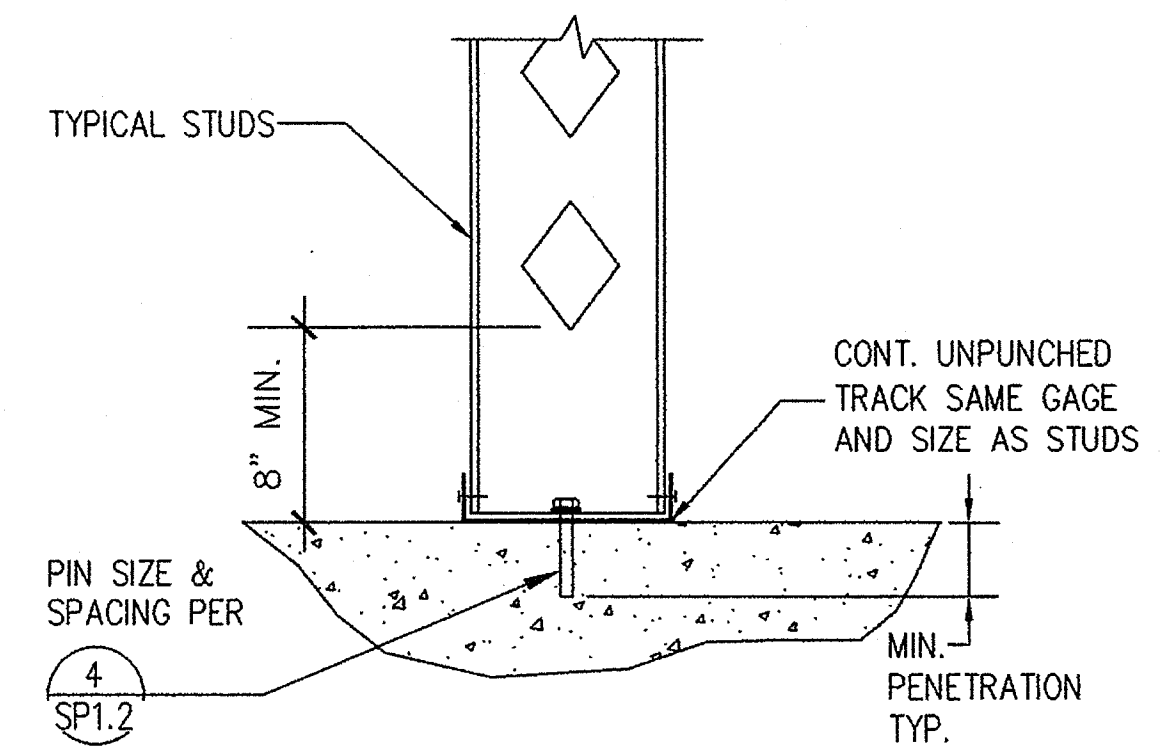
MECHANICAL, ELECTRICAL
EQUIPMENT BACKING PLATE

12



BACKING PLATE

8



BOTTOM OF WALL AT CONC.

1

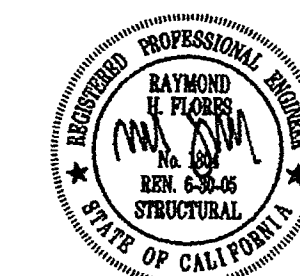
POWDER DRIVEN FASTENERS IN CONC.

MINIMUM SHANK DIAMETER	MINIMUM PENETRATION	TRACK DEPTH	TRACK GAGE	MAXIMUM SPACING
0.170"	1 1/4"	4" & 6"	20	32"
0.170"	1 1/2"	4" & 6"	18	24"
0.170"	1 1/2"	4" & 6"	14	16"

NOTES:

- FASTENERS SHALL BE PER ITW RAMSET/ RED HEAD ACTUATED FASTENERS. LATEST I.C.B.O. REPORT No. 1639, (TYP.)
- FOR TOOL QUALIFICATION AND FASTENER TESTING SEE "POWDER DRIVEN SHOT PINS" NOTES ON DWG. S1.0A

FASTENER SCHEDULE



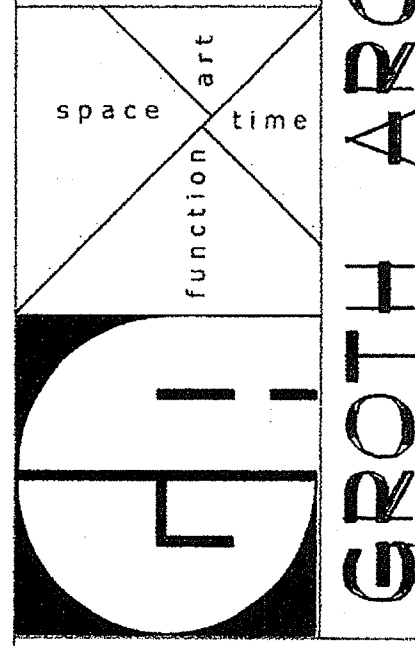
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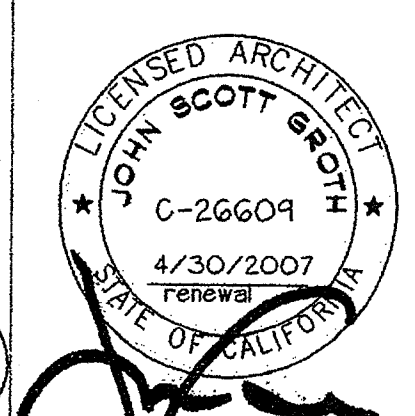
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P. T. N. 73569-9
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REVISIONS

JEFFERSON MS NEW CONSTRUCTION
823 ACACIA STREET
OCEANSIDE, CA 92054
OCEANSIDE UNIFIED S.D.
3355 MISSION AVE. SUITE 234
OCEANSIDE, CALIFORNIA 92054
PHONE 760-754-8191
FAX 760-754-8291

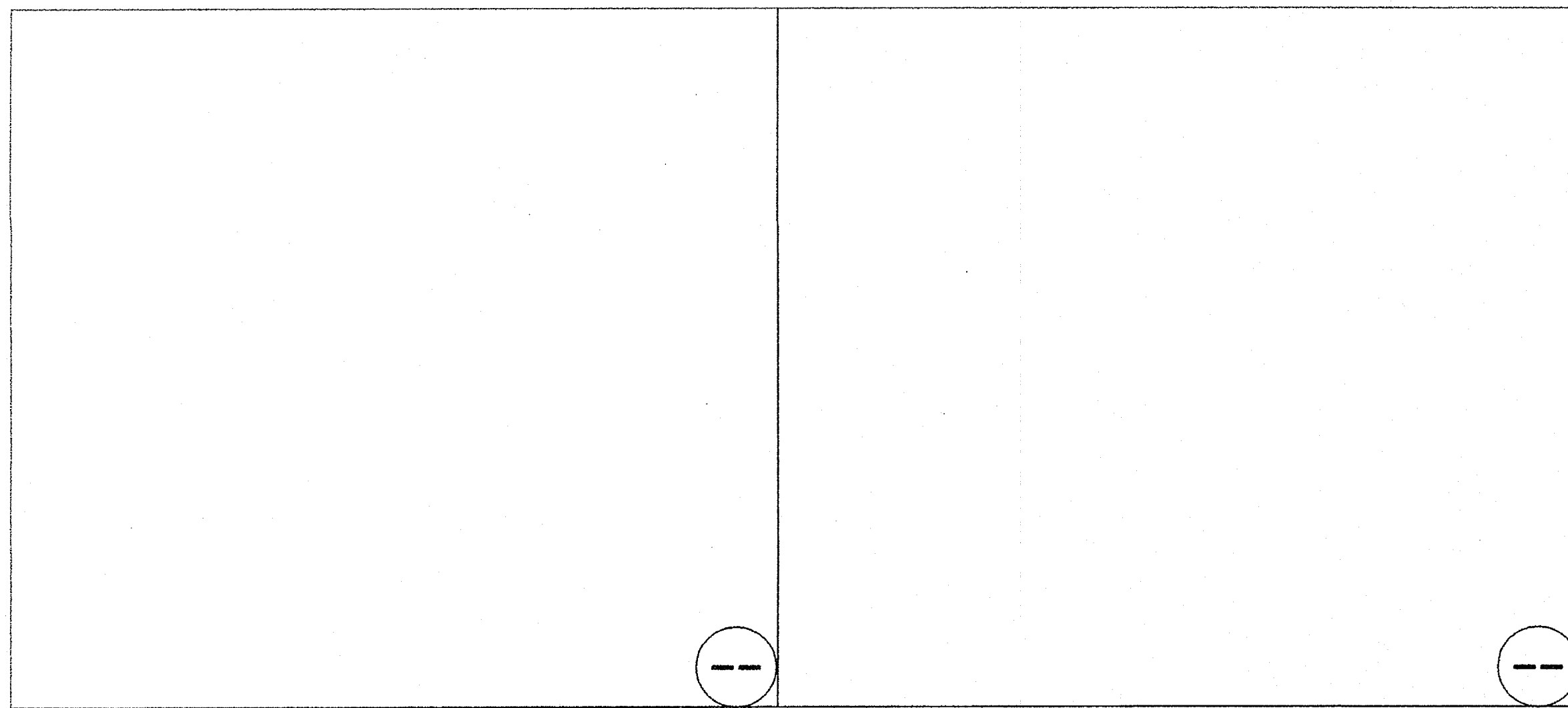


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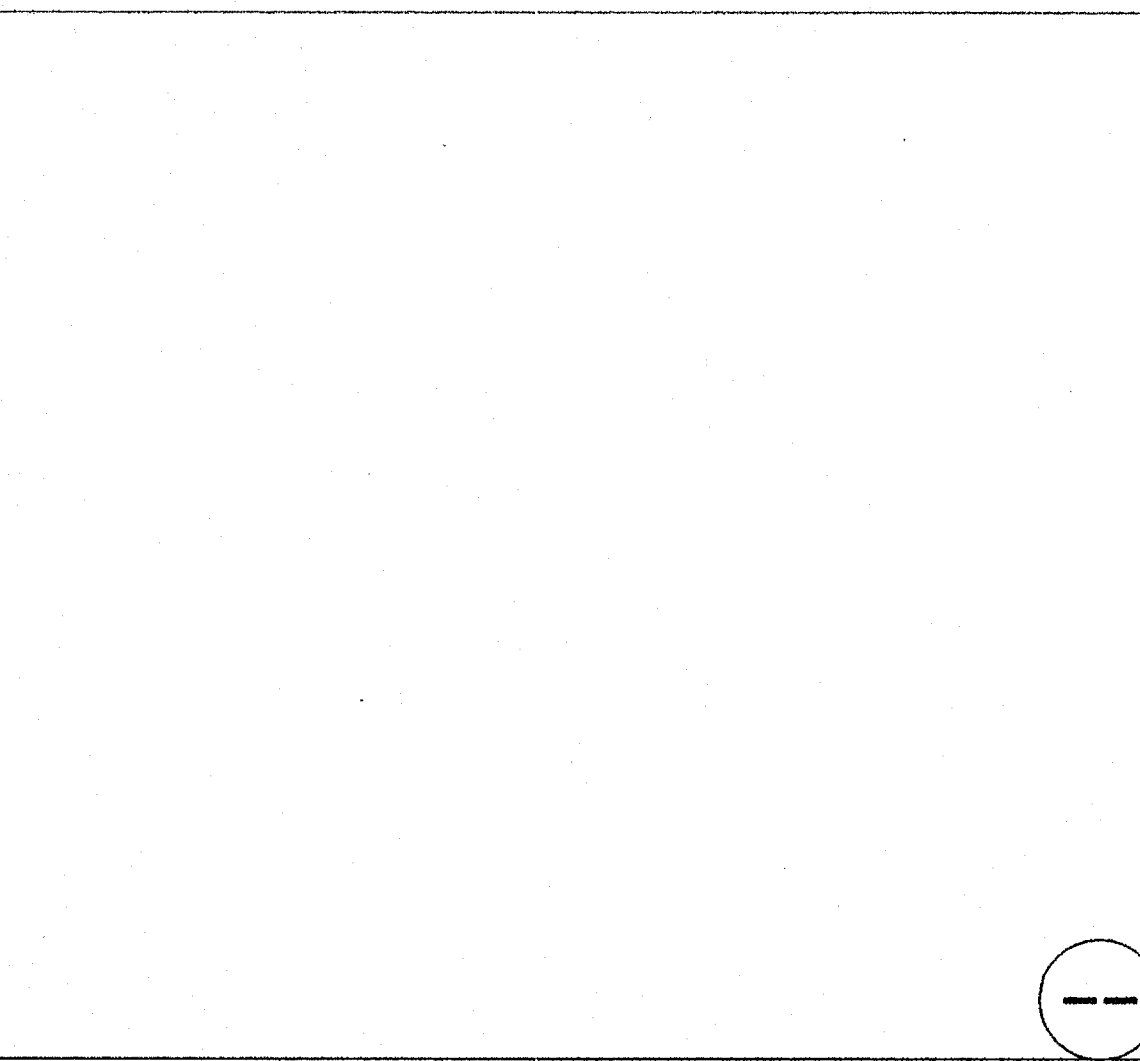
TYPICAL
DETAILS

SP1.2



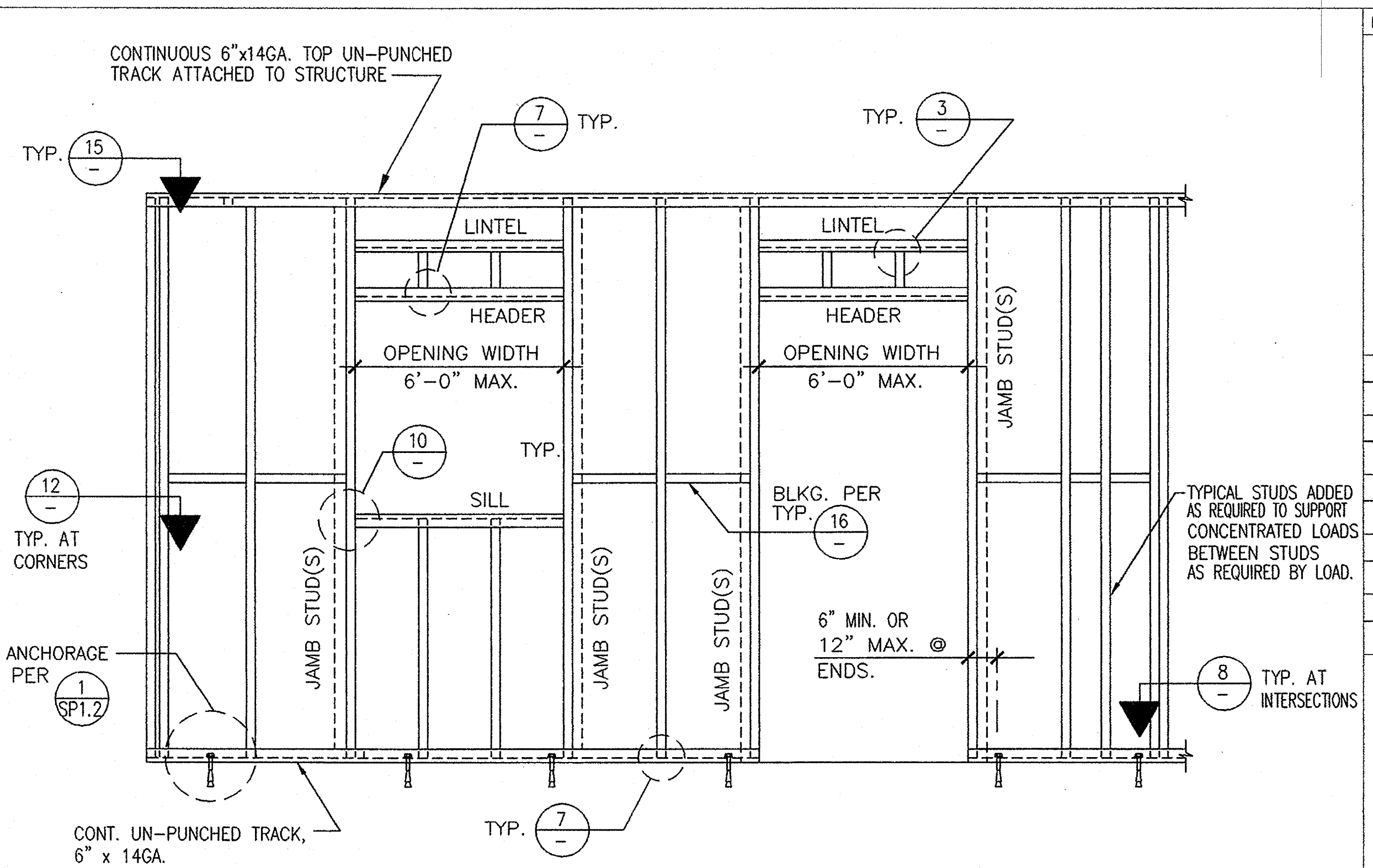
TYPICAL LINTEL SUPPORT CONNECTION

14



TYPICAL DETAIL

10



TYPICAL EXTERIOR AND INTERIOR WALL OPENING FRAMING

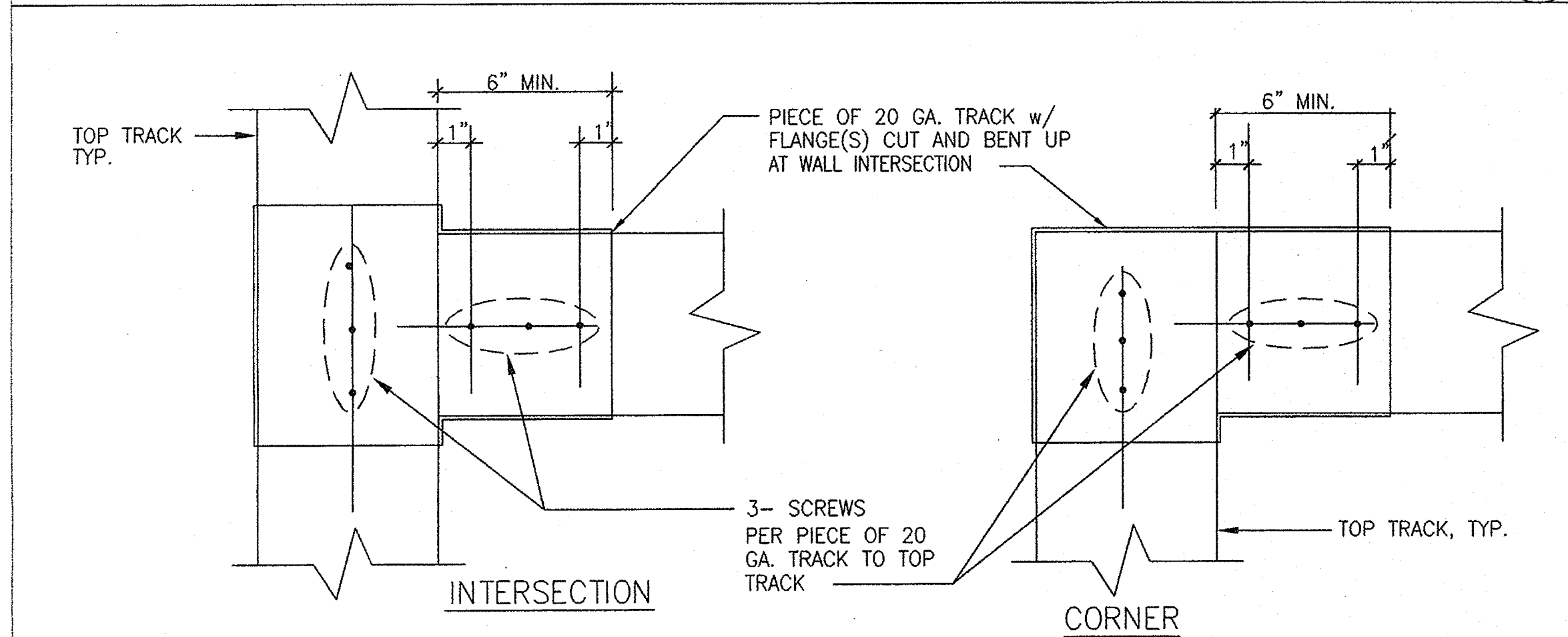
2

NOTES:

1. HEADER & SILL SAME WIDTH AS STUDS
2. ALL LIGHT GAGE FRAMING IN THIS PROJECT SHALL BE 20 GA. MIN. TYP. (U.N.O.)

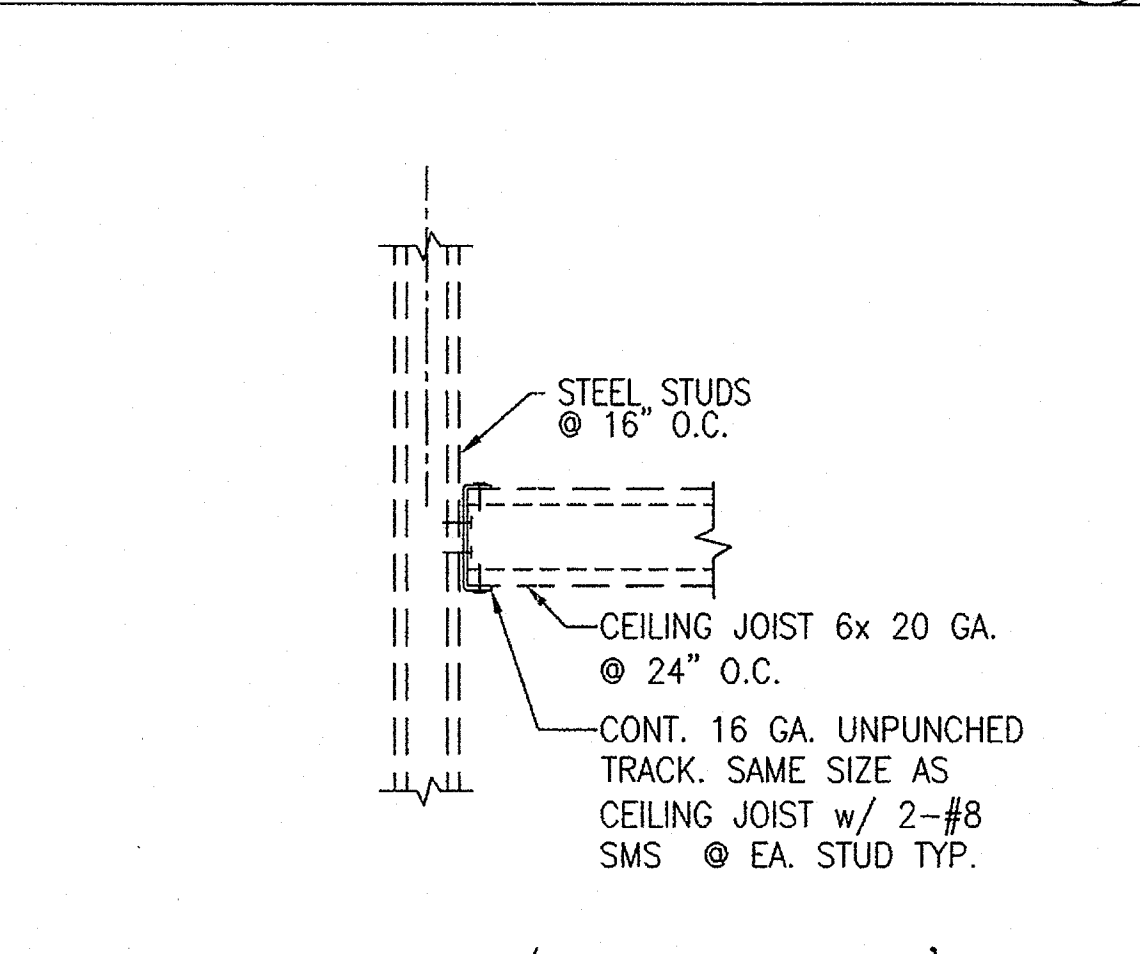
TYPICAL STEEL STUD SCREW SIZES (U.N.O.)

- 1 #8 SHEET METAL SCREWS FOR 20 GA. & 18 GA. COMPONENTS.
 - 2 #12 SHEET METAL SCREWS FOR 14 AND 16 GA. COMPONENTS.
- SCREW SIZE BASED ON THINNEST GAGE STEEL TO BE CONNECTED.



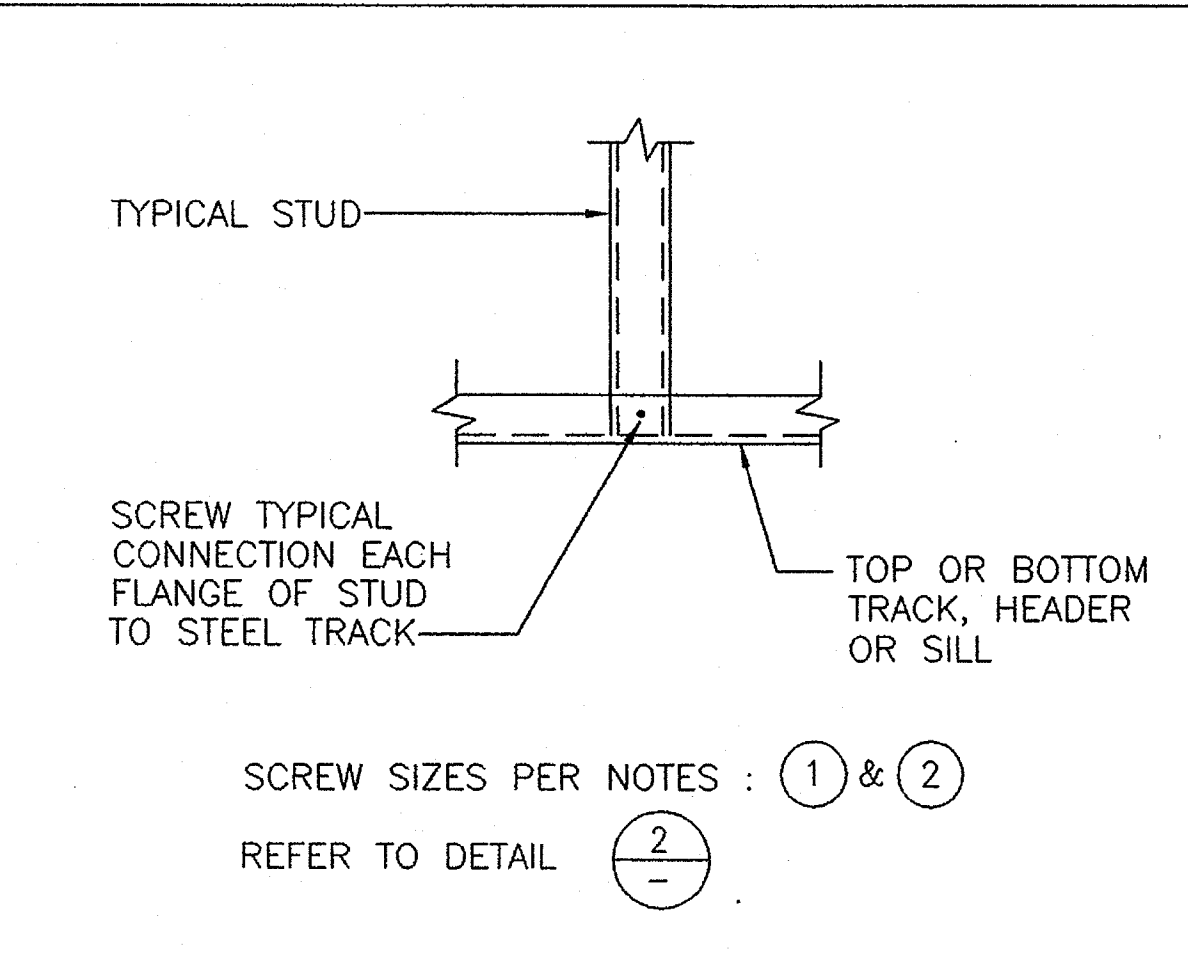
PLAN AT TOP TRACKS

15



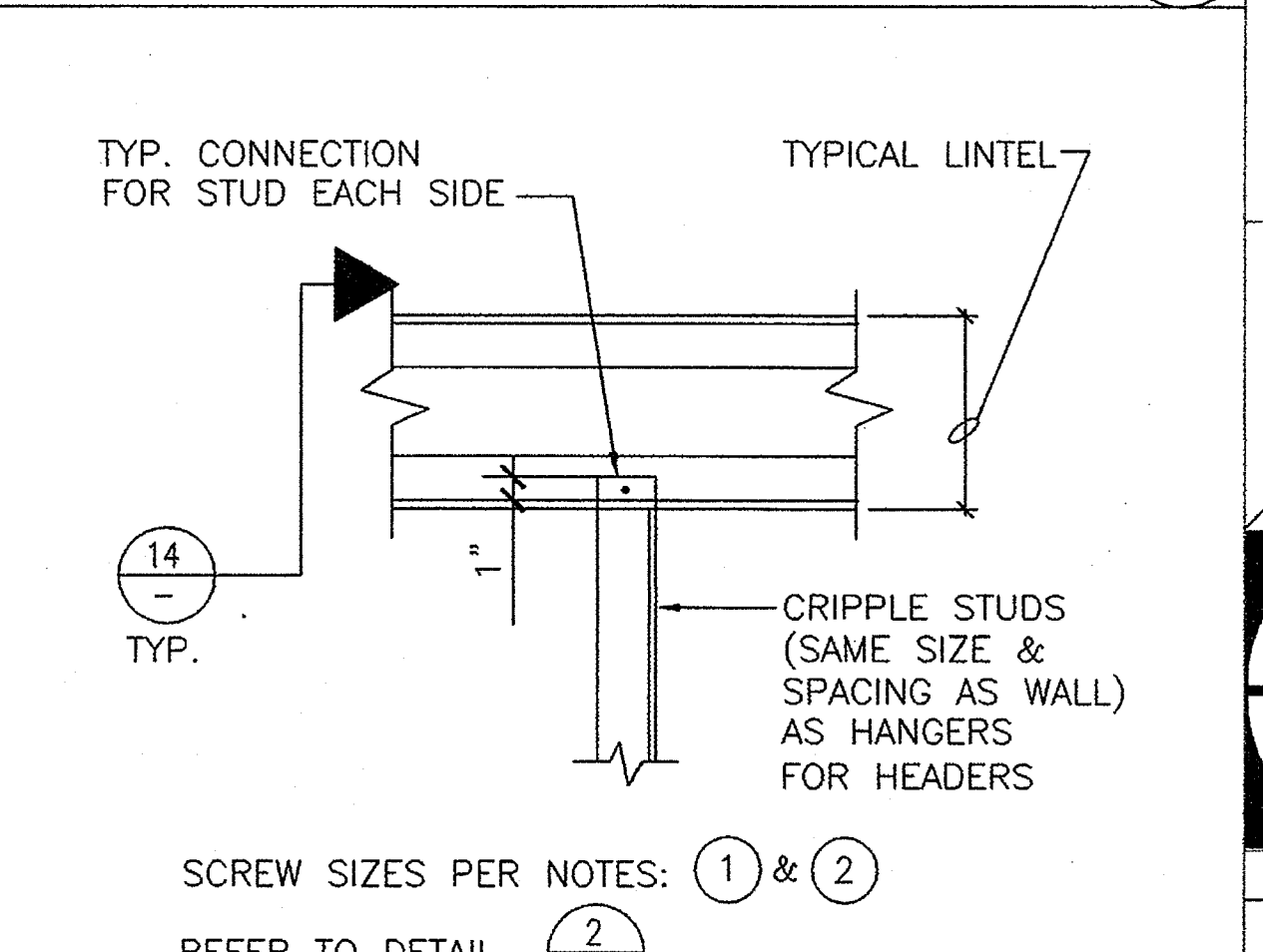
TYP. CEILING/SOFFIT FRM'G. @ STUD WALL

11



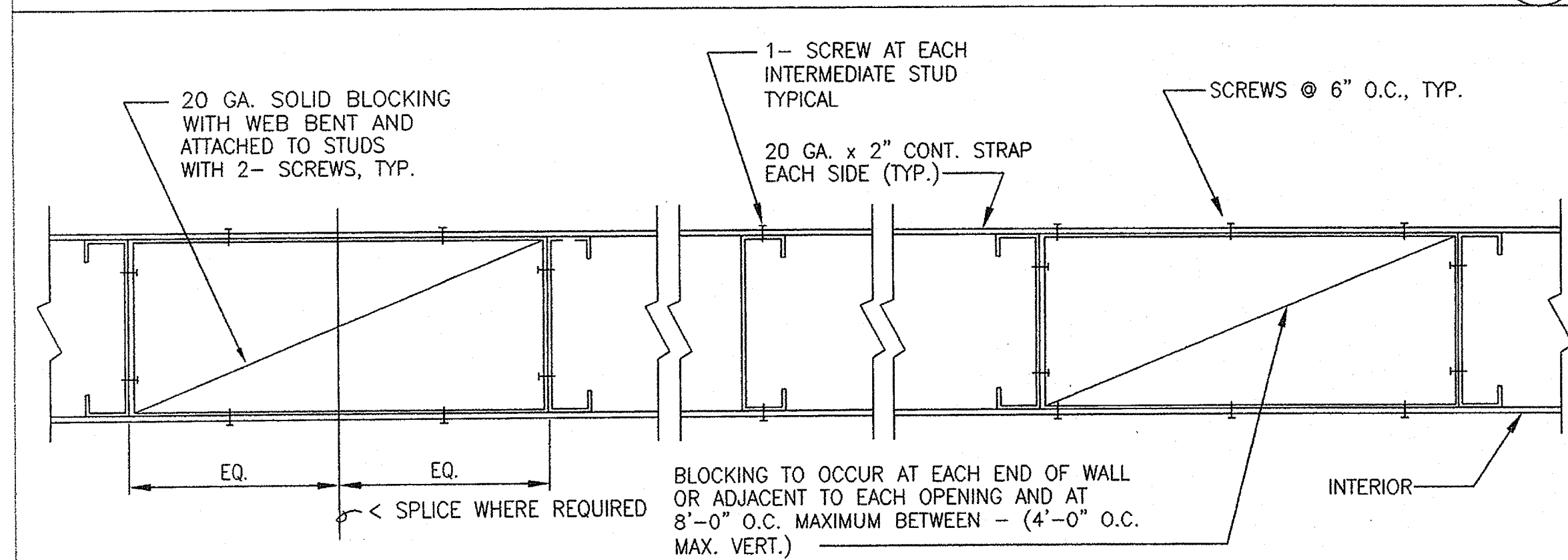
TYPICAL STUD CONNECTION

7



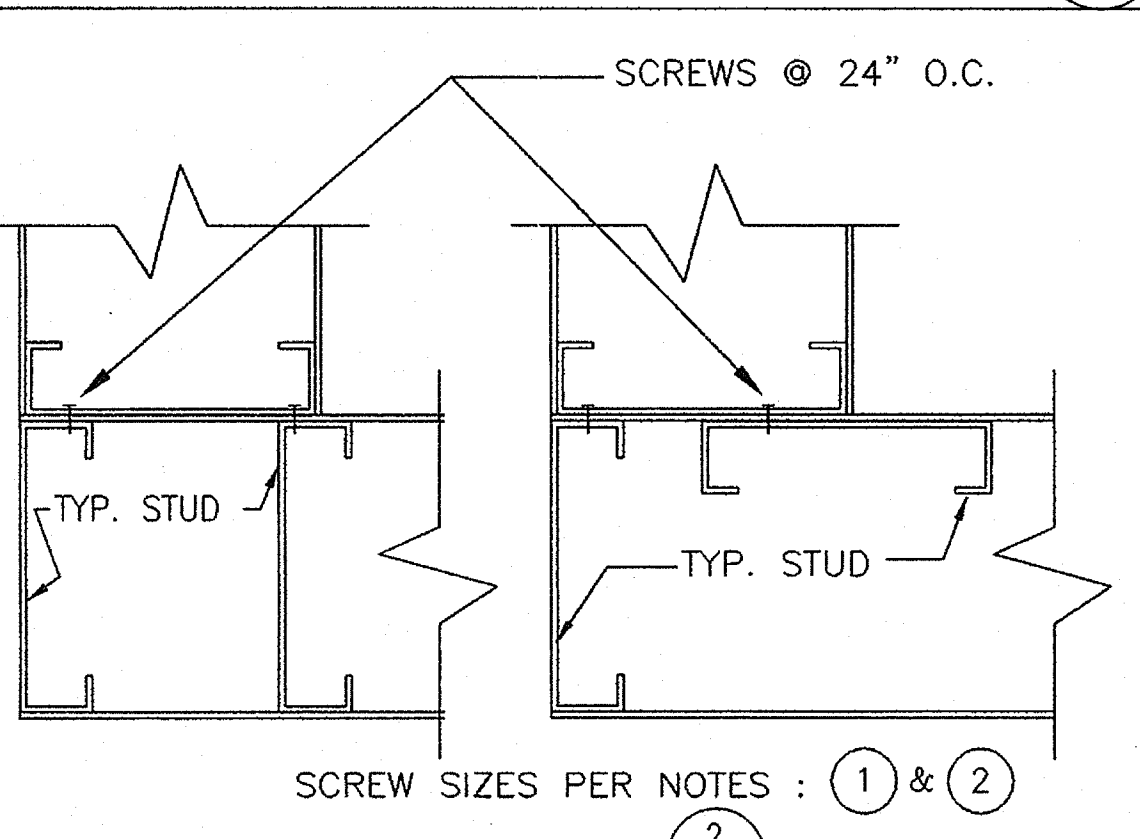
STUD CONNECTION TO LINTEL

3



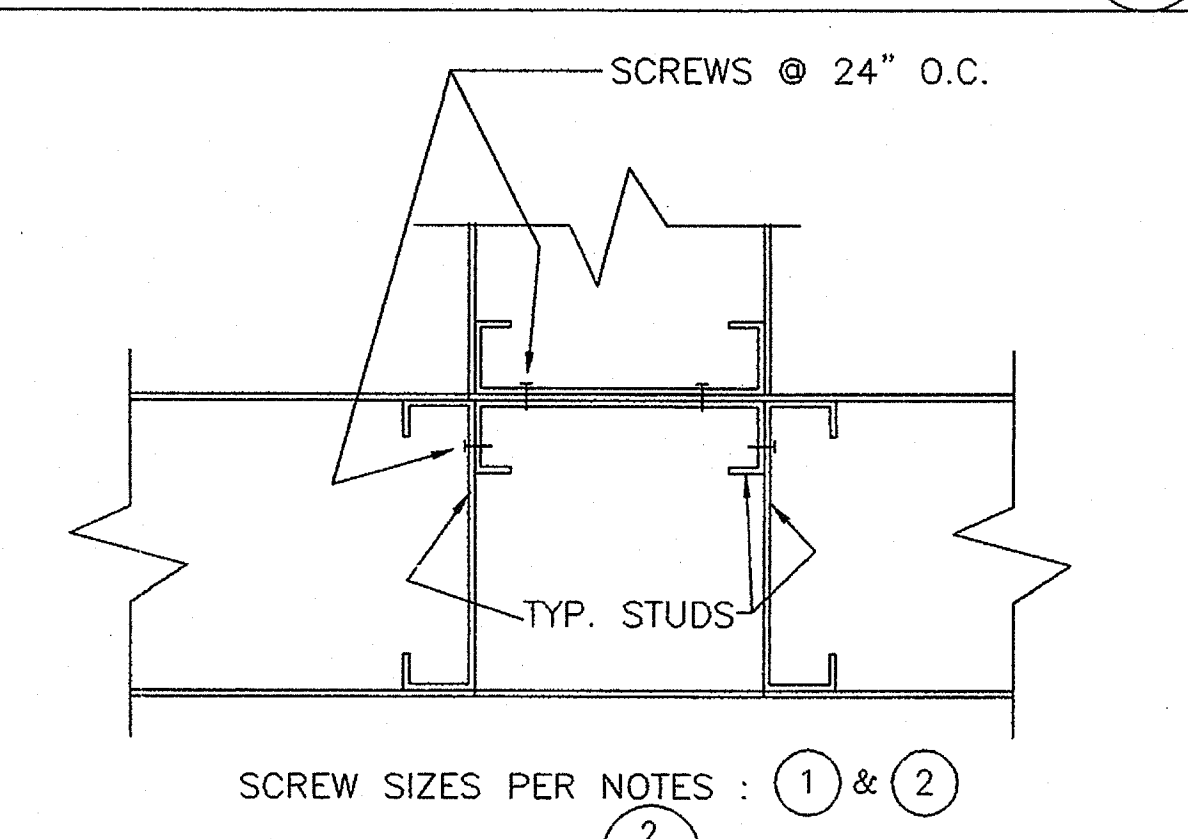
TYPICAL WALL BRIDGING OR BLOCKING

16



TYPICAL CORNER PLAN

12

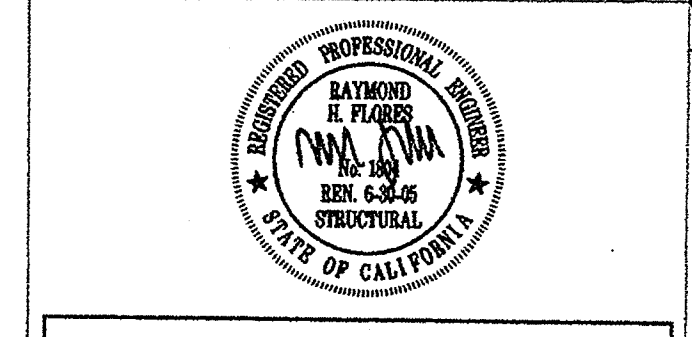


TYPICAL INTERSECTION PLAN

8

- NOTES:
1. ALL WEDGE TYPE ANCHORS SHALL BE ITW RAMSET/REDHEAD T&U BOLT PER LASTEST ICBO REPORT #ER-1372. FOR TESTING SEE GENERAL NOTES, DWG. SP1.0
 2. POWER DRIVEN PINS SHALL BE ITW RAMSET / REDHEAD PER LASTEST ICBO REPORT #1639

4



FLC FLORES LUND CONSULTANTS
PROFESSIONAL ENGINEERS
7225 TRADE STREET, SUITE 100, SAN DIEGO, CALIFORNIA 92121
TEL: 619-594-1000 FAX: 619-594-0927

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OCEANSIDE, CA 92054
OCEANSIDE UNIFIED S.D.

GROTH ARCHITECTS, INC.
3855 MISSION AVE. SUITE 234
OCEANSIDE CALIFORNIA 92054
PHONE 760-754-8191
FAX 760-754-8291

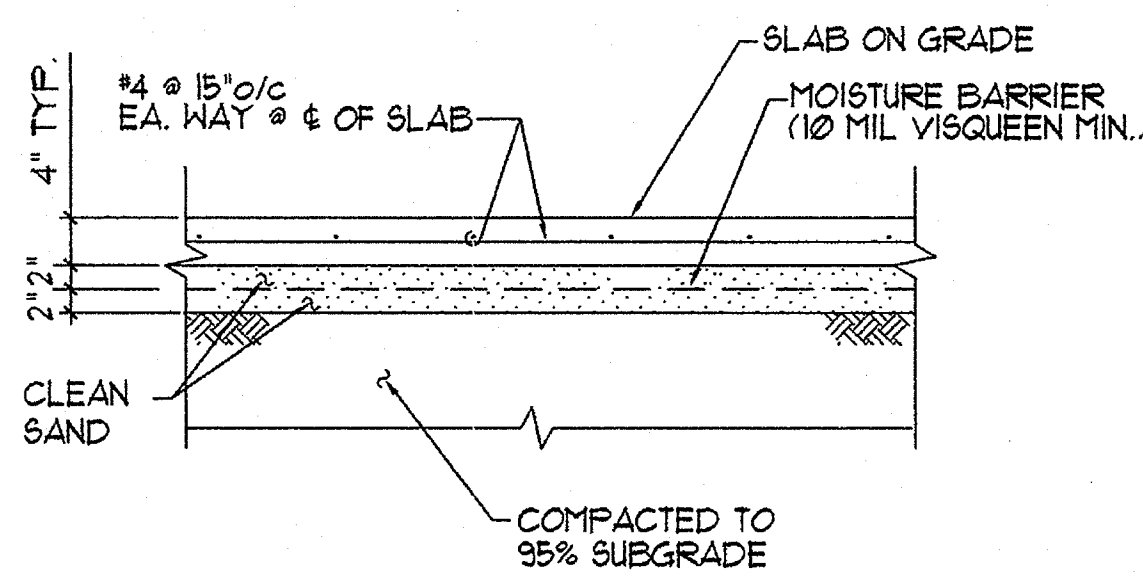
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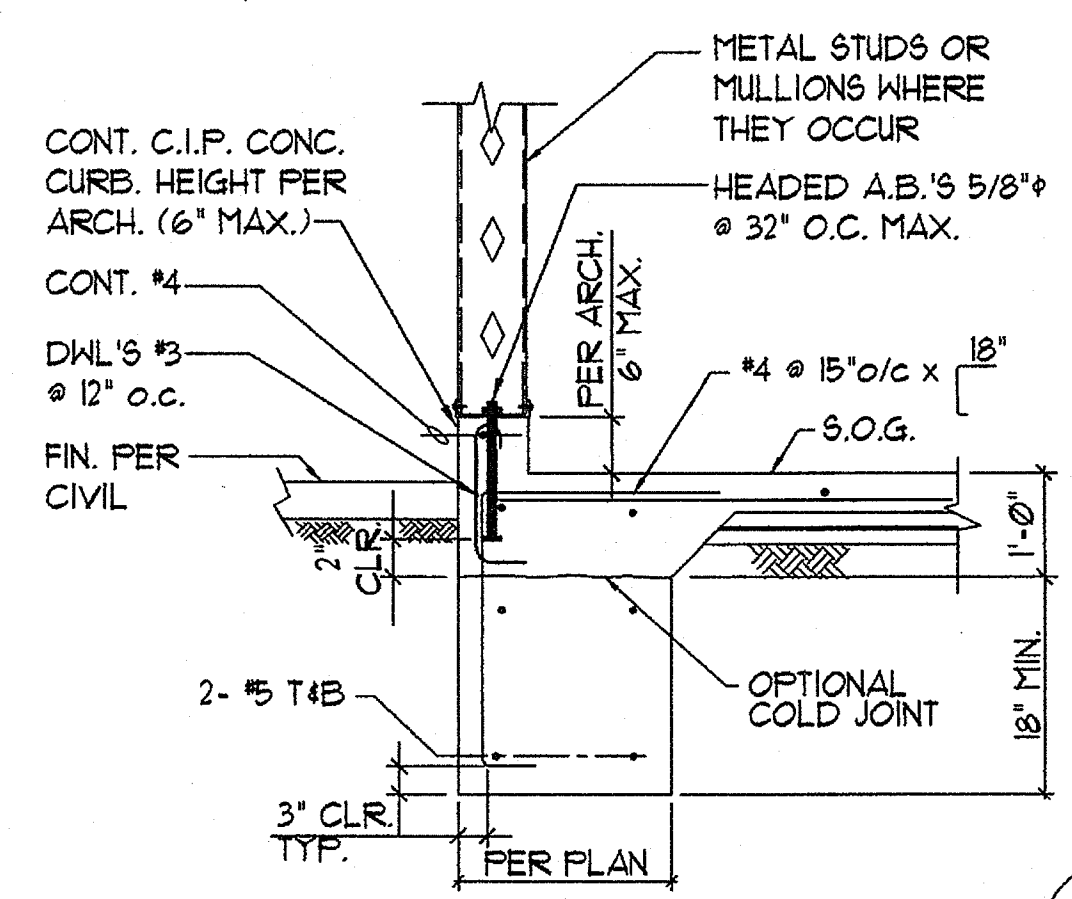
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C-26609
4/30/2007
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SHEET TITLE
TYPICAL DETAILS
SP1.3

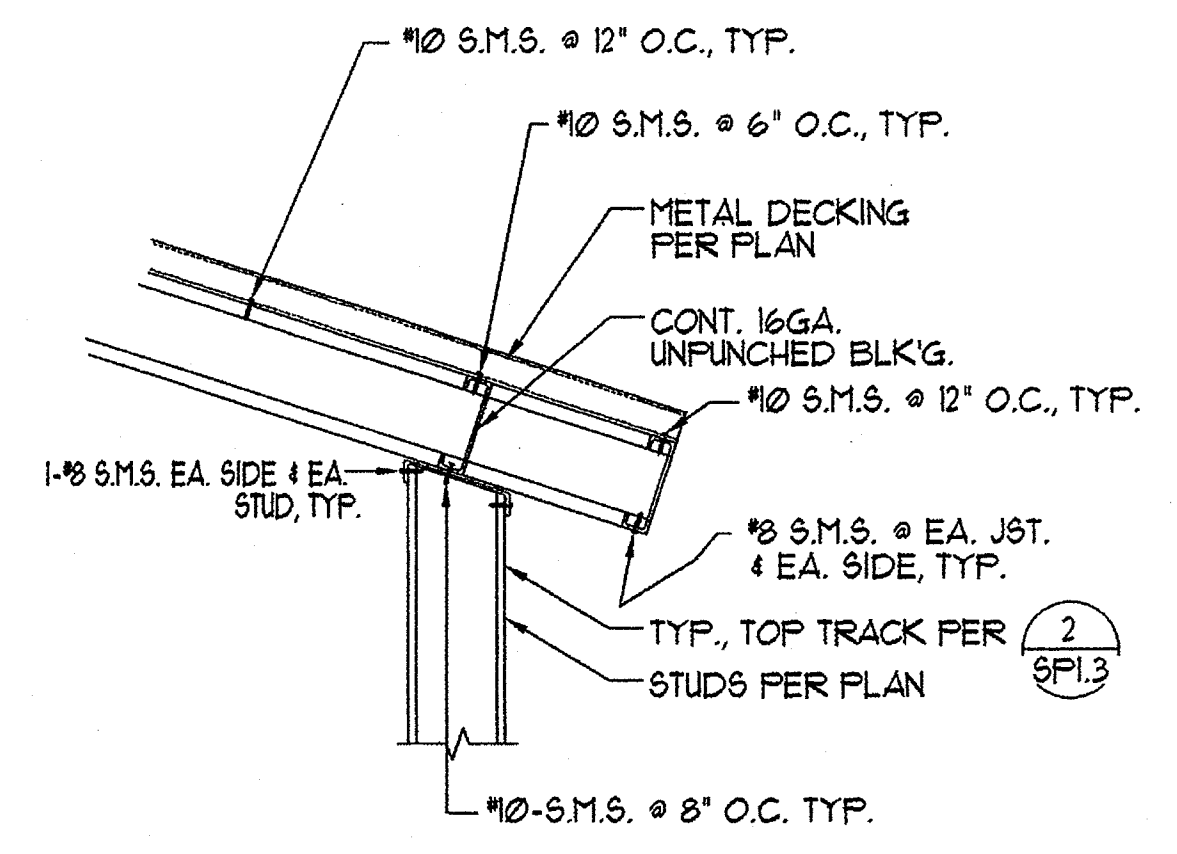


TYP. SLAB ON GRADE

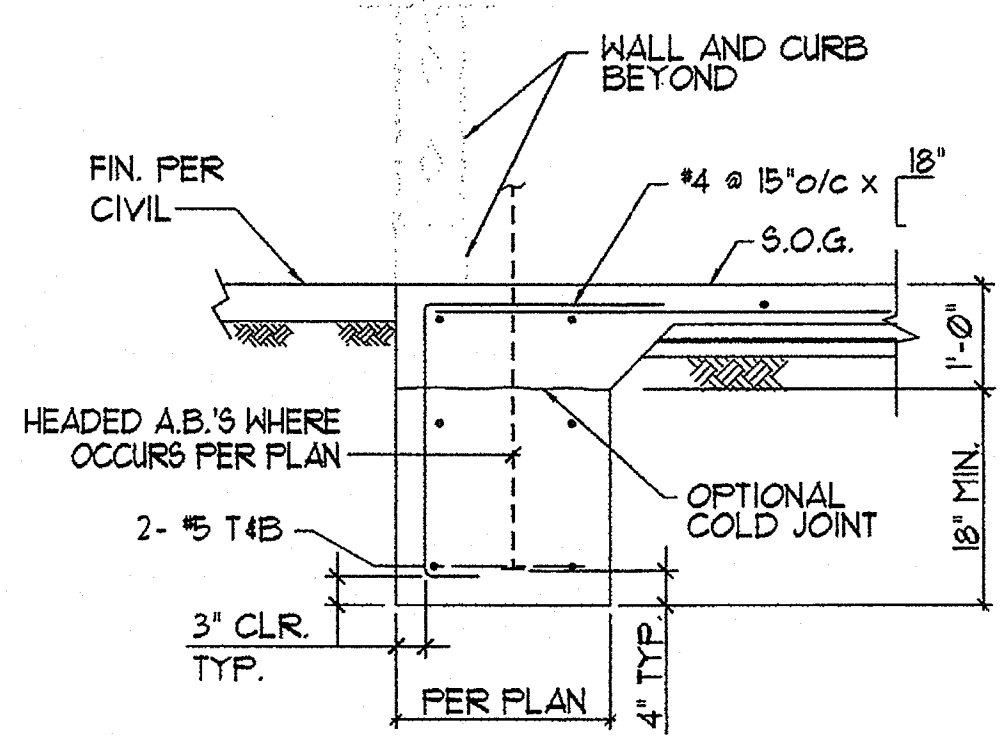
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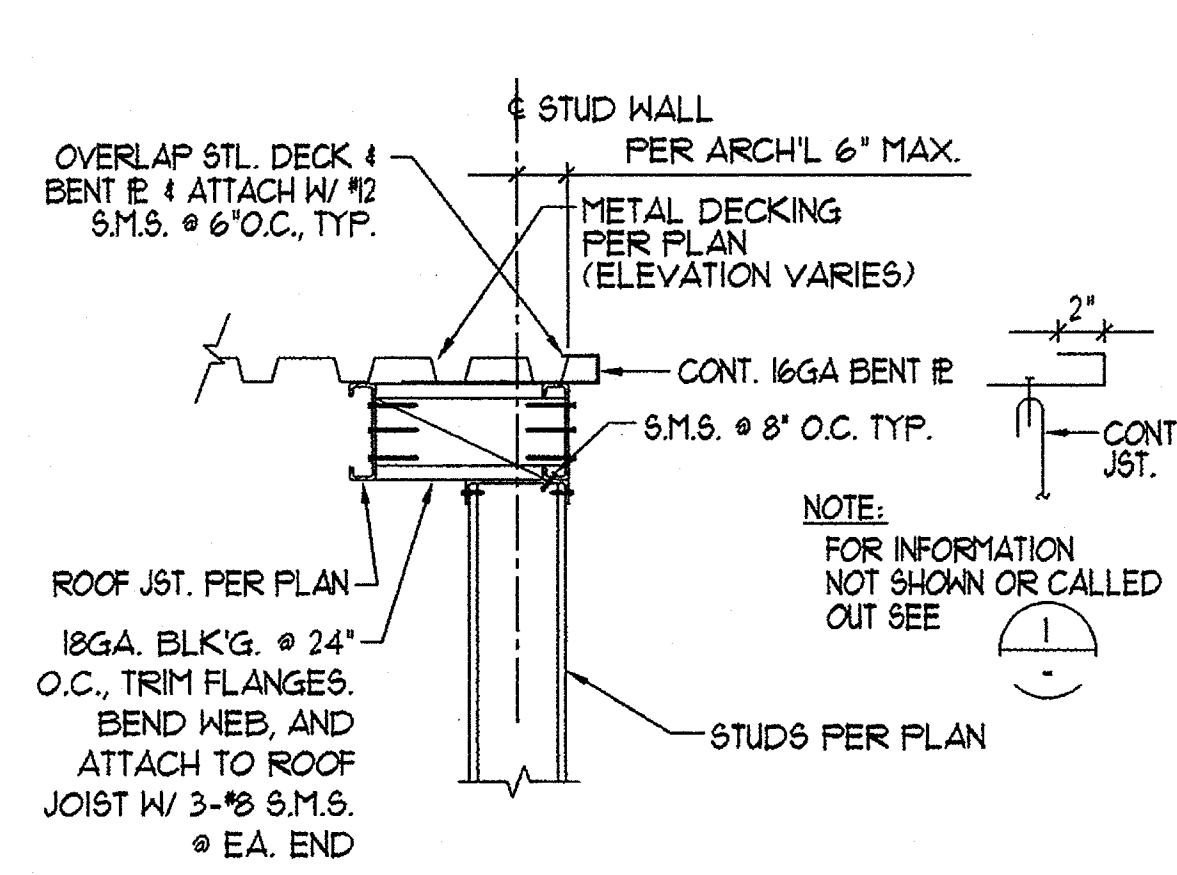
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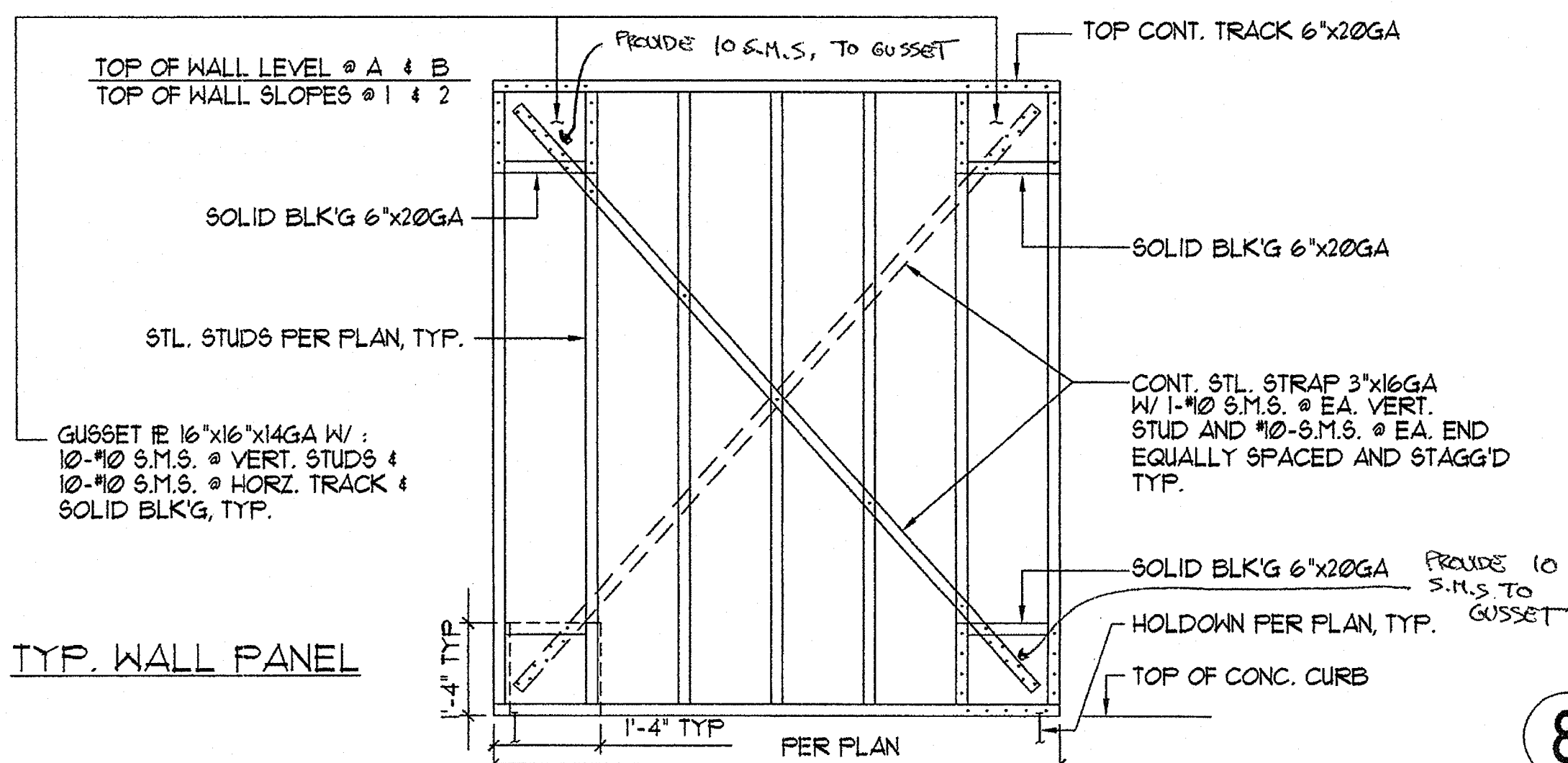
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6



2



TYP. WALL PANEL

8

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 OCEANSIDE, CA 92054
 OCEANSIDE UNIFIED S.D.

3355 MISSION AVE. SUITE 234 OCEANSIDE, CALIFORNIA 92054
 PHONE 760-754-8191
 FAX 760-754-8291

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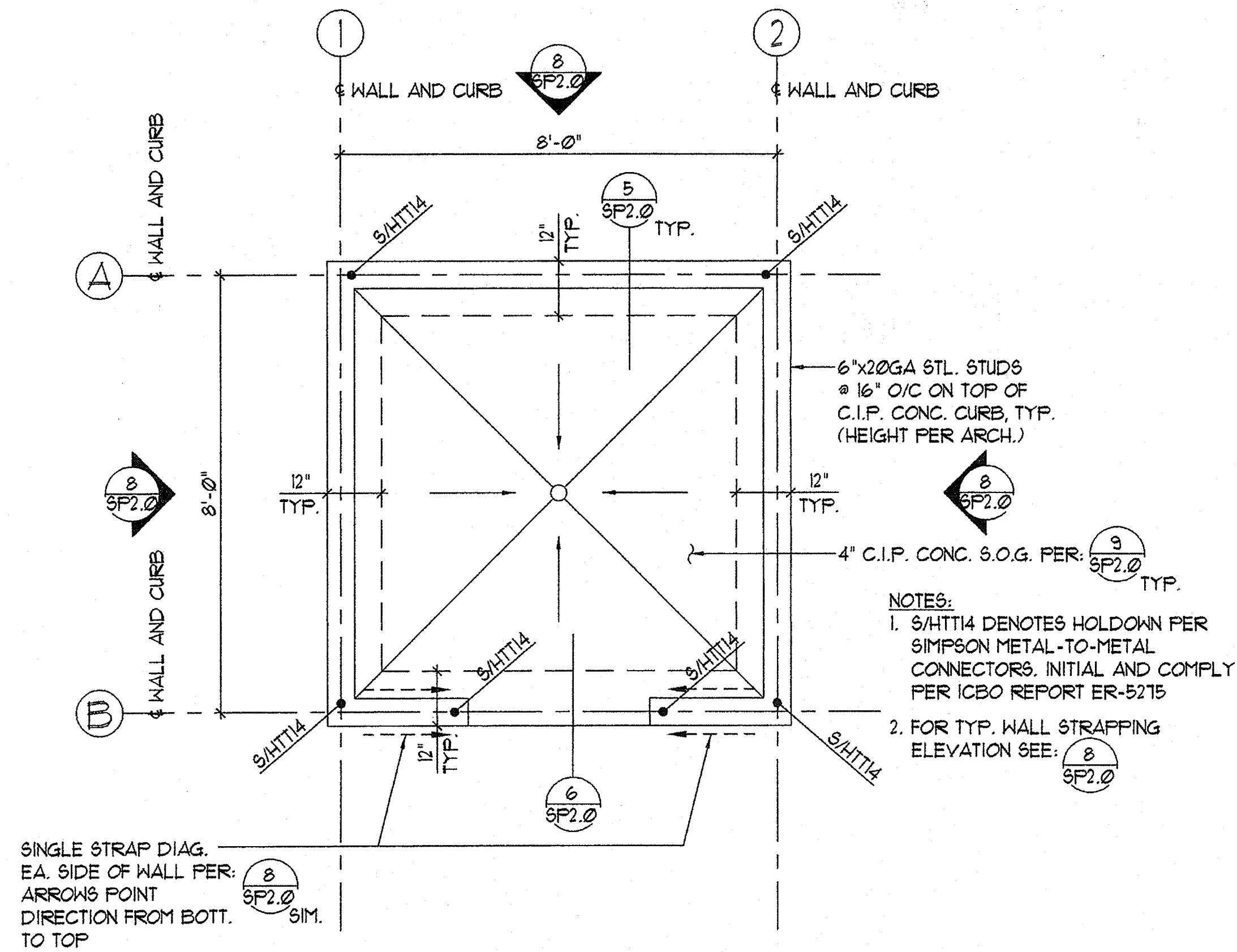
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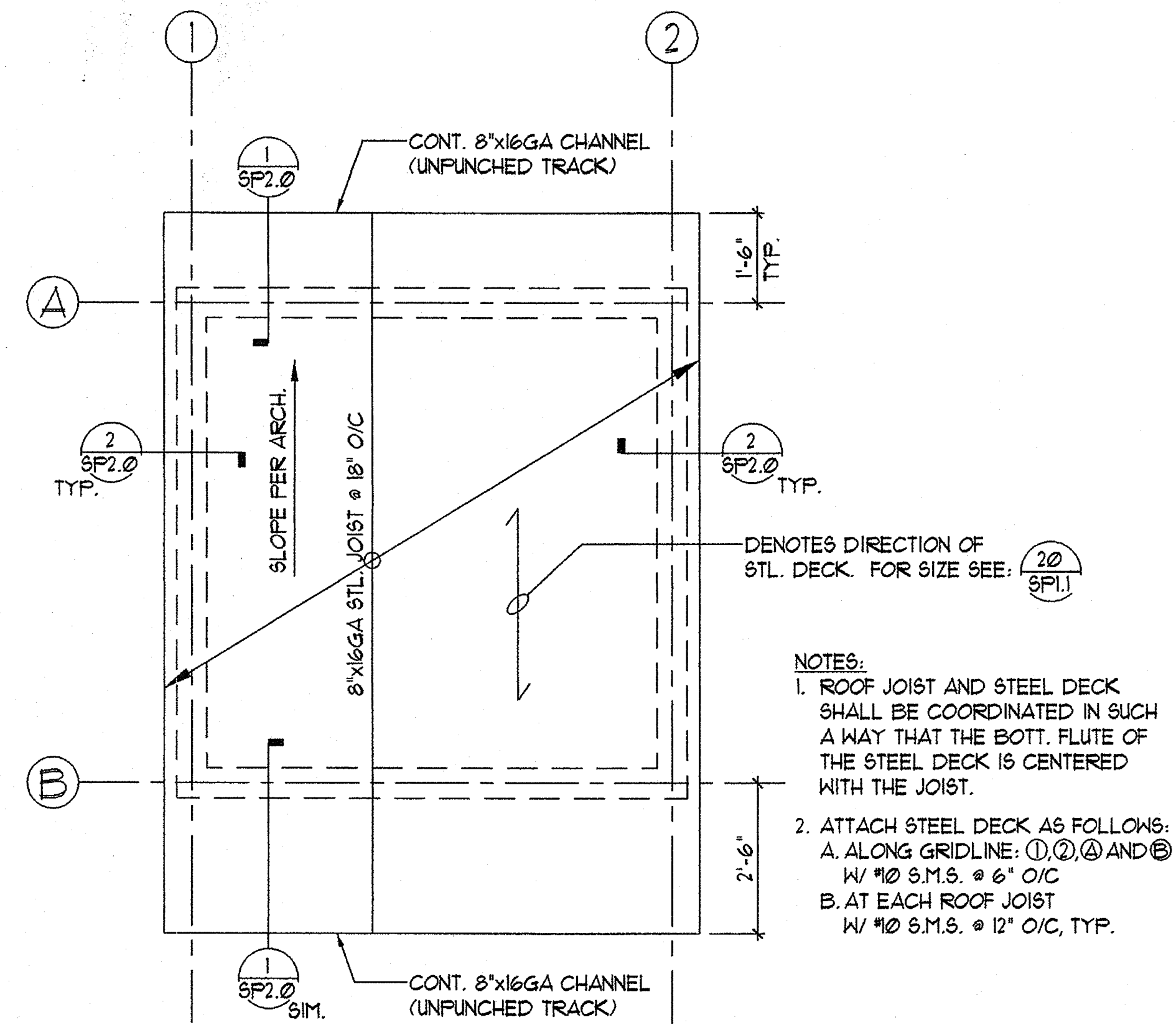
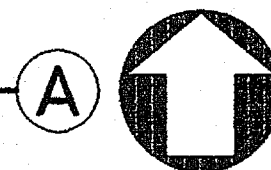
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 FLORES LUND CONSULTANTS
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 (619) 584-0626 FAX (619) 584-0627

SP2.0



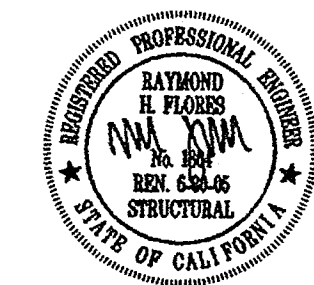
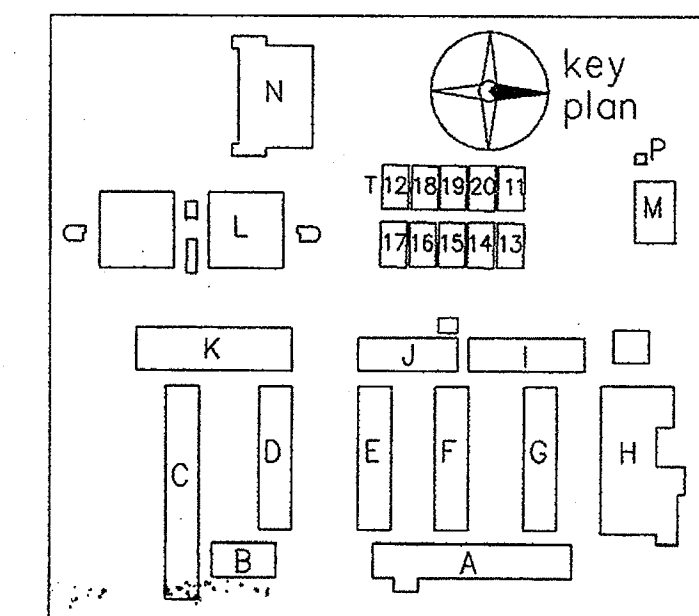
BUILDING P - FOUNDATION PLAN

1/2" = 1'-0"



BUILDING P - ROOF FRAMING PLAN

1/2" = 1'-0"



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 DATE

REVISIONS

JEFFERSON MS NEW CONSTRUCTION
 823 ACACIA STREET
 OCEANSIDE, CA 92054
 OCEANSIDE UNIFIED S.D.

space art
 function time

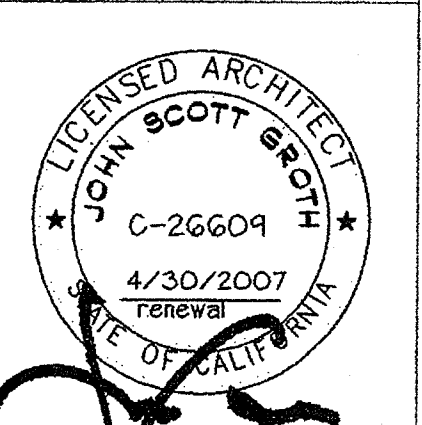


DSA

IDENTIFICATION STAMP
 DIV. OF THE STATE ARCHITECT
 OFFICE OF REGULATION SERVICES

4-106494

AC: [Signature] SS: [Signature]
 DATE: MAR 28 2005



SHEET TITLE

**BUILDING - P
 FOUNDATION
 AND ROOF
 PLANS**

SP3.0

PHONE 760-754-8191
 FAX 760-754-8291
 SUITE 234 92054
 3355 MISSION AVE. OCEANSIDE, CALIFORNIA